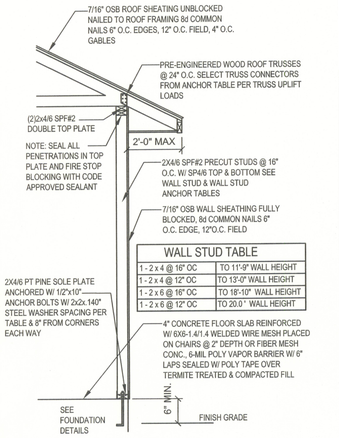
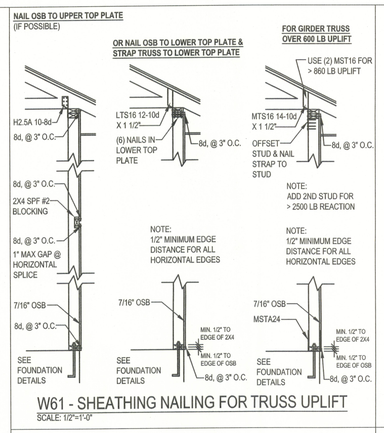


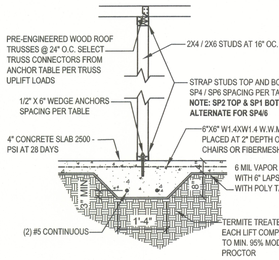
WALL STUD TABLE	
1 - 2 x 4 @ 16" OC	TO 11'-9" WALL HEIGHT
1 - 2 x 4 @ 12" OC	TO 13'-0" WALL HEIGHT
1 - 2 x 6 @ 16" OC	TO 18'-10" WALL HEIGHT
1 - 2 x 6 @ 12" OC	TO 20.0 ' WALL HEIGHT

STUD ANCHOR TABLE			
TYPICAL TRUSS UPLIFT & MAX 16'-0" WALL HEIGHT	ANCHOR BOLT SPACING	SP4 / SP6 SPACING	ALTERNATE SPH4/SPH6 SPACING
770 LB	48" O.C.	48" O.C.	N/A
950 LB	48" O.C.	32" O.C.	N/A
1270 LB	32" O.C.	16" O.C.	32" O.C.
1500 LB	24" O.C.	16" O.C.	16" O.C.
2200 LB	LTT31 W/ 5/8" X 7" WEDGE ANCHOR	N/A	(2) HTS20 NAILED TO STUD PACK
NOTE: SP2 TOP & SP1 BOTTOM ALTERNATE FOR SP4/6			
NOTE: MINIMUM ANCHOR BOLT SPACING FOR WALLS WITH A HEIGHT GREATER THAN 10'-0" AND LESS THAN 14'-0" SHALL BE 32" O.C.			

W1 - SINGLE STORY EXT. WALL SECTION
SCALE: 1/2"=1'-0" REV-22-AUG-03

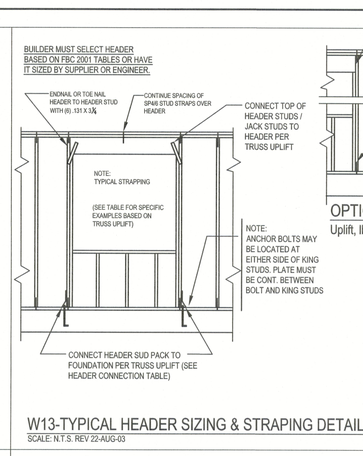


W61 - SHEATHING NAILING FOR TRUSS UPLIFT
SCALE: 1/2"=1'-0"



TYPICAL TRUSS UPLIFT	WEDGE ANCHOR SPACING	SP4 / SP6 SPACING	TRUSS CONNECTOR
400 LB	48" O.C.	48" O.C.	H2.5A
600 LB	48" O.C.	32" O.C.	H10
1000 LB	32" O.C.	16" O.C.	HTS20
2200 LB	LTT31 W/ 5/8" X 7" WEDGE ANCHOR	N/A	(2) HTS20 NAILED TO STUD PACK

F4 - INTERIOR BEARING FOOTING
SCALE: 1/2"=1'-0" REV-22-AUG-03



W13-TYPICAL HEADER SIZING & STRAPING DETAIL
SCALE: N.T.S. REV 22-AUG-03

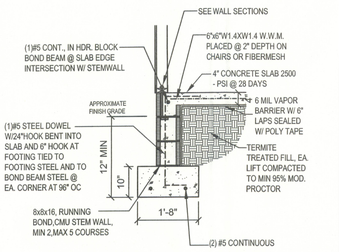
Load Bearing Header Sizing Methods (BY BUILDER)
1. Determine header size from FBC 2001, Tables 2308.3 A, B, & C, or 2308.5.
2. Use supplier published data or Southern pine span tables.
3. For engineered lumber beams have suppliers engineer size beam.

Jack Studs and King Studs (BY BUILDER)
4. Lookup jack studs from FBC 2001, Tables 2308.3 A, B, & C, or 2308.5.
5. Use one jack stud for every 3000 lb vertical load.
6. Total king plus jack studs = studs needed to be there if no opening was there.

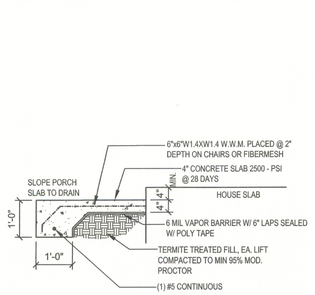
Header Uplift Connections (BY BUILDER)
7. Calculate the uplift at each end of the header by summing the moments of all truss uplifts and dividing by the length of the header.
8. Select header connections from table below or mfg. catalog to connect header to stud (top connection) and stud to foundation (bottom connection).

Option #	Uplift, lb.	Top Connector		Bottom Connector	
#1	< 800	End nail or toe nail w/6-.131"x3.25"		SP4, 6-10dx1½"	690
#2	< 1500	LSTA12, 10-10d	755	(2) SP4, 6-10dx1½", ½" AB	1380
#3	< 1760	LSTA18, 14-10d	1055	LTT20B, 10-16d, ½" AB	1750
#4	< 2500	(2) LSTA18, 14-10d	2110	LTT31, 18-10dx1½"x10" AB	2185
#5	< 3885	(3) LSTA18, 14-10d	3480	HTT16, 18-16d, ½"x10" AB	4175
Uplift greater than 3885 lb requires engineering design					

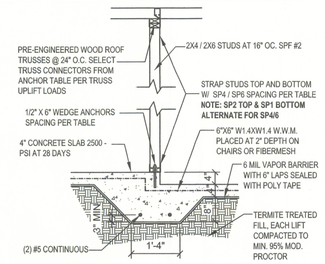
FBC2001, TABLE 2308.3A Header Spans For Exterior Bearing Walls Supporting Roof+Ceiling (20psf+20psf)		Header Spans (ft-in)	Building Width / Truss Span (ft)					
			20		28		36	
			Span	NJ	Span	NJ	Span	NJ
NOTES: NJ = Number of jack studs required to support each end. Building width is measured perpendicular to the ridge. For widths between those shown, spans may be interpolated. Spans are based on uniform loads on header.	Header Sizes	2-2x4	3-6	1	3-2	1	2-10	1
		2-2x6	5-5	1	4-8	1	4-2	1
		2-2x8	6-10	1	5-11	1	5-4	1
		2-2x10	8-5	2	7-3	2	6-6	1
		2-2x12	9-9	2	8-5	2	7-6	1
		3-2x8	8-4	1	7-5	1	6-8	1
		3-2x10	10-6	1	9-1	2	8-2	1
		3-2x12	12-2	2	10-7	2	9-5	1
		4-2x8	9-2	1	8-4	1	7-6	1
		4-2x10	11-8	1	10-6	1	9-5	1
		4-2x12	14-1	1	12-2	2	10-11	1



F1 - STEM WALL FOUNDATION
SCALE: 1/2"=1'-0" REV-27-MAY-03

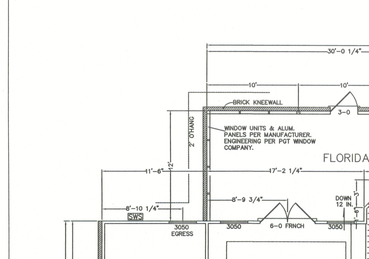


F2 - PORCH SLAB
SCALE: 1/2"=1'-0" REV-22-AUG-03



TYPICAL TRUSS UPLIFT	WEDGE ANCHOR SPACING	SP4 / SP6 SPACING	TRUSS CONNECTOR
400 LB	48" O.C.	48" O.C.	H2.5A
600 LB	48" O.C.	32" O.C.	H10
1000 LB	32" O.C.	16" O.C.	HTS20
2200 LB	LTT31 W/ 5/8" X 7" WEDGE ANCHOR	N/A	(2) HTS20 NAILED TO STUD PACK

F5 - INTERIOR BEARING STEP FOOTING
SCALE: 1/2"=1'-0" REV-22-AUG-03



AREA SUMMARY

CONDITIONED	1934 SF
FL. ROOM	307 SF
GARAGE	546 SF
ENTRY	86 SF
TOTAL ROOF	2873 SF

N2-GENERAL NOTES:

FOUNDATION: FOR POINT LOADS GRATER THAN 5000 lb OR REPETITIVE TRUSS LOADS GRATER THAN 2000 lb PER TRUSS PROVIDE A THICKENED SLAB OR PAD FOOTING 1'-0"D X 1 sq ft. FOR EVERY 1000 lb OF BEARING REINFORCE WITH #5 @ 8" O.C. EACH WAY

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS SHALL BE F'c = 3000 PSI. WHERE EXCESS WATER IS ADDED TO THE CONCRETE SO THAT ITS SERVICABILITY IS DEGRADED, THE ATTAINMENT OF REQUIRED STRENGTH SHALL NOT RELEASE THE CONTRACTOR FROM PROVIDING SUCH MODIFICATIONS AS MAY BE REQUIRED BY THE ENGINEER TO PROVIDE A SERVICEABLE MEMBER OR SURFACE. ALL CONCRETE SHALL BE VIBRATED. NO REPAIR OR RUBBING OF CONCRETE SURFACES SHALL BE MADE PRIOR TO INSPECTION BY AND APPROVAL OF THE ENGINEER, OWNER OR HIS REPRESENTATIVE.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTHS SHALL BE 1/2 INCH TO 2 INCHES IN LENGTH. DOSAGE AMOUNTS SHALL BE FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. SYNTHETIC FIBERS SHALL COMPLY WITH ASTM C 1116. THE MANUFACTURER OR SUPPLIER SHALL PROVIDE CERTIFICATION OF COMPLIANCE WITH ASTM C 1116 WHEN REQUESTED BY THE BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

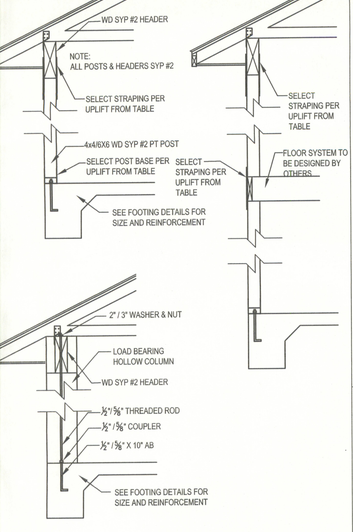
REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAPS SPLICES 48 * db (30" FOR #5 BARS) UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-95 WITH ACI 315-96 UNLESS NOTED OTHERWISE. ALL TENSION DEVELOPMENT LENGTHS SHALL BE 30 INCHES.

R FOR CONNECTORS, ANCHORS, AN EQUIVALENT DEVICE OF THE ICES LISTED IN THE EXAMPLE IFACTURER'S INSTALLATION

SPECIFIED IN DRAWINGS BUT NO OUTED CMU.

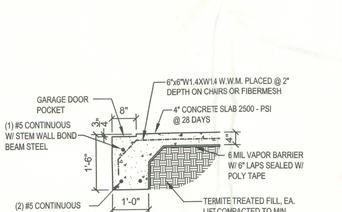
5/8" BOLTS TO BE 3" x 3" x 9/64"; 6"; NO.

OR ACCEPTED BY FBC TEST

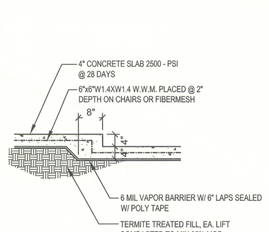


SYP #2 PT WD POSTS			
TYPICAL POST UPLIFT	POST BASE ANCHOR	BETWEEN FLOOR STRAPING	HEADER STRAPING
555 LB	ABA44 W/ (6)-10d & ½" AB	(2) LSTA21 W/ (6)-10d EA.	(2) LSTA21 W/ (6)-10d EA.
720 LB	ABA66 W/ (8)-16d & ⅝" AB	(2) LSTA21 W/ (8)-10d EA.	(2) LSTA21 W/ (8)-10d EA.
2200 LB	ABU44 W/ (12)-16d, (2)½ BOLTS & ⅝"AB	(2) LSTA21 W/ (16)-10d EA.	(2) LSTA21 W/ (16)-10d EA.
2300 LB	ABU66 W/ (12)-16d, (2)½ BOLTS & ⅝"AB	(2) LSTA21 W/ (16)-10d EA.	(2) LSTA21 W/ (16)-10d EA.
HOLLOW COLUMN			
1500 LB	½" X 10" AB ATTACHED TO ½" THREADED ROD WITH ½" COUPLER THRU COLUMN & HEADER WITH 2" WASHER & NUT TOP		
2300 LB	⅝" X 10" AB ATTACHED TO ⅝" THREADED ROD WITH ⅝" COUPLER THRU COLUMN & HEADER WITH 3" WASHER & NUT TOP		

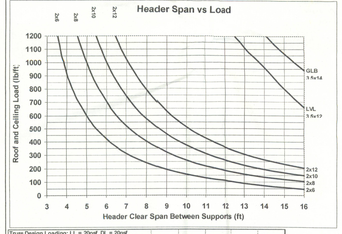
W12 - PORCH HEADER ANCHORS
SCALE: N.T.S. REV-18-JUL-03



F3 - GARAGE DOOR POCKET
SCALE: 1/2"=1'-0" 22-AUG-03

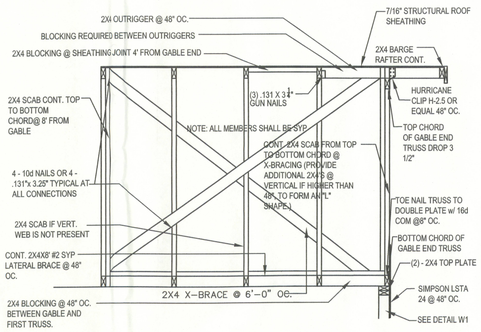


F12 - NON - BEARING STEP FOOTING
SCALE: 1/2"=1'-0" REV-08-JAN-05



True Design Loading: LL = 20psf, DL = 20psf			
7" Jack Truss		220	Half of 7' truss span + 24" overhang
Porch width, 8'		240	Half of 8' truss span + 24" overhang
Garage width, 20'		480	Half of 20' truss span + 24" overhang
Garage width, 24'		500	Half of 24' truss span + 24" overhang
Main house width, 32'		720	Half of 32' truss span + 24" overhang
Main house width, 40'		880	Half of 40' truss span + 24" overhang
Main house width, 60'		1280	Half of 60' truss span + 24" overhang
Bonus Room, above garage		1025	Half of 24' truss span and 14' bonus room
Notes:			
1. Deflection limits: TL=L/240, LL=L/180.			
2. Duration factor, Cd = 1.25, applied to Fb and Fv (No increase to E or Fc, for duration of load.)			
3. 2x headers are SYP#2 with OSB 8inch spacer and (2) 12x8, 16" OC, strength increased 8% for OSB)			
4. 3.5" x 14" GLB is Anthony Power Header, 2600 Fb, 1.9 E, or GP (2) 1.75x14 LVL, 2850 Fb, 2.0 E,			
5. 3.5" x 12" LVL is Blue Line - GP Lam, (2) 1.75" x 11 7/8" LVL, 2900 Fb, 2.0 E, 2882 Fv			
6. Minimum bearing for SYP header, Fc = 565psi, 2500 lb per jack stud. (For SPF plate, Fc = 425psi, 2200 lb per			
7. Shear strength is increased 50% for minimal splits and checks headers cut to size on site.			
8. Deflection has not been adjusted for creep due to long term loading, Kd = 1.0.			
9. The chart is based on NDS2001 bending, horizontal shear, and deflection requirements.			
10. Chart is for uniform loads only.			
11. (For non-uniform loads sum the loads on the header, divide by header span, and multiply by 2.)			

W71 - HEADER SPANS FOR ROOF/CEILING LOAD



W10 - TYPICAL GABLE END (X-BRACING)
SCALE: 1/2"=1'-0"

N5 - TRUSS UPLIFT CONNECTOR TABLE
REV-18-NOV-04

All connectors are Simpson Strongtie, uno. Select top and bottom connections from this table or SST catalog to meet truss uplift. Use fasteners as specified.

Uplift SPF	Uplift SYP	Truss Connector	To Plate	To Truss / Rafter
320	455	H3	4-8d	4-8d
245	350	H5A	3-8d	3-8d
535	600	H2.5A	5-8d	5-8d
620	720	H10	6-10dx1½"	6-10dx 1½"
850	990	LTS12	8-8dx1½"	8-8dx 1½"
1245	1450	HTS20	10-10d or 12-10dx1½"	10-10d or 12-10dx 1½"
1265	1470	H16, H16-2	10-10dx1½"	2-10dx 1½"
1785	2050	LGT2	14-10d Sinker	16-16d Sinker
3655	4200	MGT	⅝" Thd. Rod	22-10d
SPF	SYP	Strap Connector	To One Member	To Other Member
760	885	SP4	6-10dx1½"	N/A
865	1005	CS20	9-8d or 7-10d	9-8d or 7-10d
1085	1265	LSTA18-24	7-10d	7-10d
1170	1360	SPH4	12-10dx1½"	N/A
420	455	SSP	4-10d	3-10d to double plate or 1-10d to single
600	825	DSP	8-10d	6-10d to double plate or 2-10d to single
1420	1650	CS16	14-8d or 11-10d	14-8d or 11-10d
SPF	SYP	Column Anchor	To Foundation	To Column / Truss
1160	1350	LTT19	⅝"x 16" AB	8-16d Sinkers
1985	2310	LTTI31	⅝"x 16" AB	18-10dx 1½"
2385	2775	HD2A	⅝"x 16" AB	2-⅝" Bolts
3590	4175	HTT16	⅝"x 16" AB	18-16d
1975	2300	ABU66	⅝"x 16" AB	12-16d

Studs Supporting Trusses: The builder is responsible for gravity loads, but you should put an extra 2x4 stud under truss bearing location for each 3000 lb of reaction. Check the minimum bearing requirements of the truss and top plate (SPF, Fc=425psi=2230lb/siy).

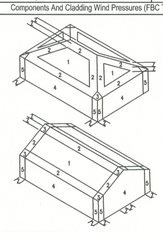
Manufacturer and product number are listed for example not endorsement. An equivalent device of the same or other manufacturer can be substituted for any devices listed in the example tables as long as it meets the required load capacities. Manufacturer's installation instructions must be followed to achieve rated loads. All connections exposed directly to the weather shall be hot dipped galvanized after fabrication. Loads are increased for wind duration. Strap uplift may be reduced proportionally to number of nails. See spec sheet for alternate nail sizes (10d=.84"/66, 10dx1½"=.80"/104, 10d+12d=16d sinker), SPF=.86"SYP

N4-WIND LOAD DESIGN DATA

(Wind loads are per FBC 2001, Section 1606.2 for enclosed simple diaphragm buildings with mean roof height less than 60' or the least horizontal dimension; not sited on the upper half of an unobstructed 60' high hill with >10% slope.)

Basic Wind Speed	110 MPH
Wind Exposure	B
Wind Importance Factor	1.0
Building Category	II
Internal pressure Coefficient	N/A (Enclosed)
Building not in the high velocity hurricane zone	
Building not in the wind-borne debris region	
Mean Roof Height	< 30 ft
Roof Angle	10-45 degrees

Components And Cladding Wind Pressures (FBC Table1606.2 B&C)



Zone	Effective Wind Area (ft2)			
	10		100	
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
Total Shear Wall Segments				
2'-4"min for 8'-0"H wall 2'-10"min for 10'-0"H wall				
	Transverse		Longitudinal	
Required	35.4'		29.8'	
Actual	92.7'		54.0'	

All exterior walls are type II shear walls ACTUAL SHEAR WALL length is the tota of all wall segments with full height sheathing and width to height ratio greate than 1: 3.5 (plus special shear wall segments if noted.) REQUIRED SHEAR WALL length is from WFCM-2001, table 3.17A & 3.17B with table 3.17E adjustment for type II shear wall (or equivalent calculation) REV-27-Jun-03

Mark Disosway, P.E.
POB 868, Lake City, FL 32056, Ph (386) 754-5419, Fax (386) 269-4871

October 5, 2005

Building Department

Re: Permit # , Perry Jonathan , Baker Residence, CR 242 - Section 28 TWP. Columbia County, FL

Dear Building Official:

Please accept this letter as addendum to the plans for the above referenced house to change all references to FBC 2001 to FBC 2004.

- The plan was drawn prior to the effective date for FBC 2004, 01 October 2005.
- Since the wind load requirements of FBC 2004 remain basically unchanged from FBC 2001 there are no structural changes required to this plan.

REV-27-Jul-04

Mark Disosway, PE
Florida Registered Professional Engineer

Cc Builder
Perry Jonathan

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