

DTES:						
ALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS ANS, TEMPORARY AND PERMANENT BRACING DETAILS, ECTIONS, AND UPLIFT AND REACTION LOADS FOR						
S. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER E ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS DESCRIPTIONS						
URNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR TIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS ECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.						
E ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET MENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS						
R SOILS TEST PROVES OTHERWISE) DMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 2500 PSI.						
CED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE C (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE ED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS						
CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 BIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. HASTM C 1116						
PLIANCE WHEN REQUESTED BY BUILDING OFFICIAL. RE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL E WITH ACI 302, JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB						
TH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND ITS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. ION OF CONTROL JOINTS IS SUBJECT TO OWNER AND /AL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT						
D ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.) ADE 40, DEFORMED BARS, FY = 40 KSI. ALL LAP SPLICES 40 * DB ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN 215 08. UN O						
200FS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, ERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING EDGES STAGGERED						
ORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, RCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED						
D IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED JFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED DS.						
NCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN S THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR						
ONSIBILITY: INER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE RT OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.						
FIONS, FOUNDATION BEARING CAPACITY, GRADE AND ID SPEED AND DEBRIS ZONE, AND FLOOD ZONE. IND CONSTRUCTION TECHNIQUES, WHICH COMPLY IENTS FOR THE STATED WIND VELOCITY AND						
US LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU IITS A CONTINUOUS LOAD PATH CONNECTION, CALL NEER IMMEDIATELY.						
NUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, INECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL						
ESIGN: PLANS FOR COMPLIANCE WITH FBCR,						
INS, UPLIFTS, AND BEARING LOCATIONS IN SUBMITTED TO THE WIND LOAD ENGINEER. IT IS OF THE BUILDER TO CHECK ALL DETAILS OF THE TEM DESIGN SUBMITTED BY THE TRUSS HAVE IT SIGNED, AND SEALED BY A DESIGN						
INVETTIGNED, AND SCALED FOR DEQUIRED IAL LOADS. THE BUILDER IS RESPONSIBLE TO UAL TRUSS MEMBER AND THE TRUSS ROOF AND TO PROVIDE RESTRAINT FOR ANY LATERAL						
R SHOULD USE CARE CHECKING THE ROOF WIND LOAD ENGINEER IS SPECIFICALLY NOT IE TRUSS LAYOUT WHICH WAS CREATED BY THE ER AND THE TRUSS DESIGNER ALSO DENIES						
					_	
2X4 SPF #2 BLOCKING				tion	ldition	<i></i>
"#2 NG 1/4" @ 6" OC				Istruc	loff Ac	DDRESS nount Rd iy, FL
@ 6" OC				(e Co	e Der	DJECT A 03 Piner Lake Ci
7/16" OSB FULLY BLOCKED FROM SLAB TO ROOF DECK, 8d 6" OC EDGE, 12" OC FIELD				Blak	lichel	PR(
I6" OSB					2	
EARWALL ARE PERPENDICULAR TO SHEARWALL						
				DIMENSIONS Stated dimens	S: sions supercede sca	aled
				Mark Disoswa Do not procee	and the substantian of the subst	n. on. Y RIGHTS:
(NOTE: HAVE TRUSS DESIGNER LOAD TRUSS FOR 400 PLF DRAG LOAD)				Mark Disoswa its common la these instrume not to be repre-	ny, P.E. hereby expr w copyrights and pr ents of service. This oduced, altered or c	ressly reserves roperty right in s document is copied in any
INTERIOR SHEAR WALL ONE STORY WOOD FRAME W/ STRAPS & AB	DESIGN CRITERIA & BUILDING CODE	& LOADS: 6TH EDITION FLORIDA BUILDING CODE RESIDENT (2017)	TIAL .	form or manne permission an CERTIFICATI	er without first the e id consent of Mark I ON: I hereby certify	xpress written Disosway. v that I have
	CODE FOR DESIGN LOAD	ASCE 7-10		portions of the comply with the Building Code to the best of	 plan, relating to will plan, relating to will e 6th Edition Florid Residential (2017) mv knowledge. 	applicable ind engineering la
	(ASCE 7-10, 3S GUST) WIND EXPOSURE (BUILDER MUST FIELD VE	ERIFY)		LIMITATION: building, at sp	This design is valid ecified location.	for one
	TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VE RISK CATEGORY			MAR THIS PDI	K DISOSWAY P.E.	53915 GNATURE
	INTERNAL PRESSURE COEFFICIENT ROOF ANGLE	0.18 7-45 DEGREES		AND EL <u>COPIE</u> <u>SIGNEI</u> VERIFY	ECTRONIC SEAL. <u>1</u> <u>S ARE NOT CONS</u> <u>D OR SEALED. </u> YO SIGNATURE ON T	<u>PRINTED</u> I <u>DERED</u> U MUST HIS PDF.
	MEAN ROOF HEIGHT C&C DESIGN PRES FLOOR LOADING	30 FT SURES SEE TABLE			DISOS	<u>IFY.</u>
	ROOMS OTHER THAN SLEEPING ROOM SLEEPING ROOMS	40 PSF LIVE LOAD 30 PSF LIVE LOAD		Within the PROFILE	STATE OF	
	ROOF LOADING FLAT OR < 4:12 4:12 TO < 12:12	20 PSF LIVE LOAD 16 PSF LIVE LOAD		Т	lesday, June 23, 20	20
	12:12 & GREATER SOIL BEARING CAP FLOOD ZONF	12 PSF LIVE LOAD ACITY 1500 PSF THIS BUILDING IS NOT IN THE FLOOD	D ZONE	Mar 163 S	k Disosway W Midtown	P.E.
		1		Lake (Suite 103 City, Florida	32025
	COMPONENT & CLADIN EFFECTIVE WIND AREA (FT2)	G DESIGN PRESSURES 130 MPH (E) ZONE 4 ZONE 5 INTERIOR END 4' FROM ALL	(P C) (Vult)	disoswa	ydesign@g	mail.com
	0-20 GARAGE DOOR DESIGN	+42.6 -46.2 +42.6 PRESSURES 130 MPH (EXP C) (ASE	-57))	JO	B NUMBE 200593	ER:
	9x7 GARAGE DOOR 16x7 GARAGE DOOR	+22.6 -25.5 +21.7 -24.1			S-1	

OF 3 SHEETS

STRUCTURAL PLAN NOTES

SN-1	ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)	
SN-2	ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)	
SN-3	USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD	
SN-4	DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS	
SN-5	PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCSI1-03, BCSI-B1, BCSI-B2, & BCSI-B3. BCSI-B1, BCSI-B2, & BCSI-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE	ć
(12) 2X10	ER LEGEND HEADER/BEAM CALL-OUT (U.N.O.)	
	NUMBER OF KING STUDS (FULL LENGTH) NUMBER OF JACK STUDS (UNDER HEADER) SPAN OF HEADER SIZE OF HEADER MATERIAL	
L		

THREADED ROD LEGEND



LUMBER SIZE & GRADE MINUMUM REQUIREMENTS

FRAMING 2x SYP @ 24" O.C.

RIDGE BOARD

2X6 SYP #2

- INDICATES LOCATION OF: 3/8" A307 ALL THREADED ROD





(WHERE NO SHEATHING IS APPLIED)

SECTION CUT PARALLEL TO VALLEY RAFTER

ROOF OVER FRAMING & BRACING DETAIL SCALE: N.T.S

1/2" SHEATHING

(NOT REQUIRED IF SLEEPERS ARE USED)

2X4 BLOCKING-



STRUCTURAL PLAN SCALE: 1/4" = 1'-0"

	TO ANON (EDGE	
ACTUAL V	s REQUIRED	SHEARWALL

	TRANSVERSE	LONGITUDUNAL
ACTUAL	6398 LBF	11179 LBF
REQUIRED	4752 LBF	3178 LBF

EXISTING

0" SLAB ELEVATION

(F1) S-2/ SCALE: 1/4" = 1'-0"

VALLEY ROOF PLAN MEMBER LEGEND

TRUSS

= = = TRUSS UNDER VALLEY FRAMING = = = = = = VALLEY RAFTER OR RIDGE

CRIPPLE

2'-0" O.C. (TYP.)

CRIPPLES 4'-0" O.C. FOR 20 psf (TL) AND 10 psf (TD) (TYP. SHINGLE ROOF) MAX

CONNECTION REQUIREMENT NOTES

1	2X4 RAFTERS TO RIDGE	3 -16d OR 6131 x 3" TOE NAILS
2	CRIPPLE TO RIDGE	3 - 16d OR 6131 x 3" FACE NAILS
3	CRIPPLE TO RAFTERS	3 - 16d OR 6131 x 3" FACE NAILS
4	RAFTER TO SLEEPER OR BLOCKING	6 -16d OR 12131 x 3" TOE NAILS
5	SLEEPER TO TRUSS	4 - 16d OR 8131 x 3" FACE NAILS EACH TRUSS
6	RIDGE BOARD TO ROOF BLOCK	3 -16d OR 6131 x 3" TOE NAILS
7	RIDGE BOARD TO TRUSS	3 -16d OR 6131 x 3" TOE NAILS
8	PURLIN TO TRUSS (TYP.)	3 -16d OR 6131 x 3" NAILS
8	PURLIN TO TRUSS (IF CRIPPLE IS ATTACHED TO PURLIN)	4 -16d OR 8131 x 3" NAILS
9	TRUSS TO BLOCKING	3 -16d OR 6131 x 3" END NAILS
10	CRIPPLE TO TRUSS	3 -16d OR 6131 x 3" FACE NAILS
11	CRIPPLE TO PURLIN	3 -16d OR 6131 x 3" FACE NAILS
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GENERAL NOTES

MAXIMUM RAFTER SPANS 6'-0" FOR 2X4, 9'-0" FOR 2X6 SPF #2 OR SYP #2. MAXIMUM ROOF AREA PER SUPPORT 16ft2 IN ZONES 2 & 3 , 24ft2 IN ZONE 1. (EXAMPLE: 4'-0" O.C. X 4'-0" SPAN = 16ft2 OR 2'-0" X 8'-0" SPAN = 16ft2) PURLINS REQUIRED 2'-0" O.C. IF EXISTING SHEATHING IS REMOVED. PURLINS SHOULD OVERLAP SHEATHING ONE TRUSS SPACING MINIMUM.

IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING A MINIMUM OF 6", AND NAIL UPWARDS THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8 - 8d COMMON WIRE NAILS.

THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS:

-SPANS (DISTANCS BETWEEN HEELS) 40'-0" OR LESS - MAXIMUM VALLEY HEIGHT: 14'-0" OR LESS

-MAXIMUM WIND SPEED: 130 MPH - MAXIMUM MEAN ROOF HEIGHT: 30 FEET

- MAXIMUM TOTAL LOADING: 40 psf - MEETS FBC 2014/ASCE 7-10 WIND REQUIREMENTS

- EXPOSURE CATEGORY "C", I = 1.0, Kzt = 1.0 - ENCLOSED BUILDING

CRIPPLE, BRACING, & BLOCKING NOTES

-2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 5'-0" TO 10'-0" LONG NAILED w/ 2 - 10d NAILS OR 2X4 "T" OR SCAB BRACE NAILD TO FLAT EDGE OF CRIPPLE WITH 8d NAILS @ 8" O.C. "T" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQURE TWO CLB'S OR BOTH FACES w/ "T" OR SCAB. USE STRESS GRADED LUMBER & BOX OR COMMON NAILS.

- NARROW EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER, AS LONG AS THE PROPER NUMBER OF NAILS ARE INSTALLED INTO RIDGE BOARD - INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED.

- INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES FALL BETWEEN LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED, - APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.

TALL STEM WALL TABLE: The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Durowall ladder reinforcement at 16"OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTIC FOR	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.) (II		L REINFORCE " CMU STEMW NCHES O.C.)		
		#5	#7	#8	#5	#7	Ι
3.3	3.0	96	96	96	96	96	T
4.0	3.7	96	96	96	96	96	T
4.7	4.3	88	96	96	96	96	T
5.3	5.0	56	96	96	96	96	T
6.0	5.7	40	80	96	80	96	T
6.7	6.3	32	56	80	56	96	Τ
7.3	7.0	24	40	56	40	80	T
8.0	7.7	16	32	48	32	64	Ι
8.7	8.3	8	24	32	24	48	Γ
9.3	9.0	8	16	24	16	40	Ι

MASONRY NOTE:

MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

	ACI530.1-02 Section	Specific Requirements
1.4A	Compressive strength	8" block bearing walls F'm = 1500 psi
2.1	Mortar	ASTM C 270, Type N, UNO
2.2	Grout	ASTM C 476, admixtures require approval
2.3	CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3	Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4	Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (25" for #5)
2.4F	Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class G60, 0.60 oz/ft2 or 304SS
2.4F	Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft2 or 304SS
3.3.E.2	Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7	Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

