DATE <u>07/11/2008</u>	Columbia County Bu This Permit Must Be Prominently Posted o	Illaing Permit n Premises During Con		PERMIT 000027161
ADDITION TO LA COL		PHONE	344-4817	00002/101
APPLICANT JACOE ADDRESS 484	NW TURNER AVENUE	LAKE CITY	FL	32055
-	HEAST DEVELOPERS	PHONE	755-2082	Commence of the commence of th
ADDRESS 117	SW POPPY GLEN	LAKE CITY	FL	32024
	ACOB KIRSCH	PHONE	344-4817	·
LOCATION OF PROP	ERTY 90W. TL ON 247S, TL CALLAHA	N,TL HOPE HENRY, T	R BUTTERCUP	
	PROPERTY ON LEFT CORNER (	OF BUTTERCUP AND	POPPY GLN.	
TYPE DEVELOPMEN	T SFD,UTILITY EST	IMATED COST OF CO	NSTRUCTION 1	18800.00
HEATED FLOOR ARE	EA 1700.00 TOTAL AREA	A 2376.00	HEIGHT	STORIES 1
FOUNDATION CO	NC WALLS FRAMED RO	OOF PITCH 8/12	FLOOR	SLAB
LAND USE & ZONING		MAX	HEIGHT 21	
Minimum Set Back Red	quirments: STREET-FRONT 25.00	REAR	15.00 SIDE	10.00
NO. EX.D.U. 0	FLOOD ZONE X PP	DEVELOPMENT PERM	MIT NO.	
PARCEL ID 15-4S-	16-03023-532 SUBDIVISION	ROLLING MEADO	OWS	
LOT 32 BLOCK	K PHASE UNIT	TOTA	L ACRES 0.51	
000001634	CBC1253775	6	1//	
Culvert Permit No.	Culvert Waiver Contractor's License Num	ber	Applicant/Owner/Contra	actor
CULVERT	080058 BK	<u>J</u>	H	<u>Y</u>
Driveway Connection	Septic Tank Number LU & Zoning	g checked by App	roved for Issuance	New Resident
COMMENTS: PLAT	REQUIRES MFE AT 107.5, ELEVATION CONF	IRMATION LETTER		
COMMENTO.	TEQUITED THE ETTE TOTAL, ELLE TITLETT COTT			
REQUIRED AT SLAB	ADQUILLE MAZITA 107.0, BBZ TATAON GOLA			
·	ABQUILLO III Z III 10/10, BBZ III III O II GOIN		Check # or Cash	12110
·	FOR BUILDING & ZONIN			12110 (footer/Slab)
·	FOR BUILDING & ZONIN  Foundation	G DEPARTMENT	ONLY	(footer/Slab)
REQUIRED AT SLAB  Temporary Power	FOR BUILDING & ZONIN  Foundation  date/app. by	G DEPARTMENT  date/app. by	ONLY  Monolithic	(footer/Slab) date/app. by
REQUIRED AT SLAB	FOR BUILDING & ZONIN  Foundation  date/app. by  mbing Slab	G DEPARTMENT  date/app. by	ONLY  Monolithic	(footer/Slab)  date/app. by
Temporary Power  Under slab rough-in plu	FOR BUILDING & ZONIN  Foundation  date/app. by  mbing Slab date/app. by	G DEPARTMENT  date/app. by  date/app. by	ONLY  Monolithic  Sheathing/Nailing	(footer/Slab) date/app. by
Temporary Power  Under slab rough-in plu	FOR BUILDING & ZONIN  Foundation  date/app. by  mbing Slab date/app. by	G DEPARTMENT  date/app. by	ONLY  Monolithic  Sheathing/Nailing	(footer/Slab)  date/app. by
Temporary Power  Under slab rough-in plu	FOR BUILDING & ZONIN  Foundation  date/app. by  mbing  date/app. by  Rough-in plumbing about the second control of the second contro	date/app. by  date/app. by  ove slab and below wood	ONLY  Monolithic  Sheathing/Nailing	(footer/Slab)  date/app. by  date/app. by  date/app. by
Temporary Power  Under slab rough-in plu  Framing  date  Electrical rough-in	FOR BUILDING & ZONIN  Foundation  date/app. by  mbing  date/app. by  Rough-in plumbing about the state of the	date/app. by  date/app. by  ove slab and below wood	ONLY  Monolithic  Sheathing/Nailing  floor  Peri. beam (Lintel)	(footer/Slab)  date/app. by  g
Temporary Power  Under slab rough-in plu  Framing	FOR BUILDING & ZONIN  Foundation  date/app. by  mbing Slab  date/app. by  Rough-in plumbing abe /app. by  Heat & Air Duct  date/app. by  C.O. Final	date/app. by  date/app. by  ove slab and below wood  date/app. by	ONLY  Monolithic  Sheathing/Nailing  floor  Peri. beam (Lintel)  Culvert	(footer/Slab)  date/app. by  date/app. by  date/app. by
Temporary Power  Under slab rough-in plu  Framing  date  Electrical rough-in  Permanent power	FOR BUILDING & ZONIN  Foundation  date/app. by  mbing  date/app. by  Rough-in plumbing above  /app. by  Heat & Air Duct  date/app. by  C.O. Final  date/app. by  g, electricity and plumbing	date/app. by  date/app. by  ove slab and below wood  date/app. by  ate/app. by	ONLY  Monolithic  Sheathing/Nailing  floor  Peri. beam (Lintel)  Culvert  Pool	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by
Temporary Power  Under slab rough-in plu  Framing  date  Electrical rough-in  Permanent power	FOR BUILDING & ZONIN  Foundation  date/app. by  mbing  date/app. by  Rough-in plumbing above  /app. by  Heat & Air Duct  date/app. by  C.O. Final  date/app. by  g, electricity and plumbing	date/app. by  date/app. by  ove slab and below wood  date/app. by  ate/app. by	ONLY  Monolithic  Sheathing/Nailing  floor  Peri. beam (Lintel)  Culvert  Pool  date	(footer/Slab)  date/app. by  g date/app. by  date/app. by  date/app. by
Temporary Power  Under slab rough-in plut  Framing  date  Electrical rough-in  Permanent power  M/H tie downs, blocking  Reconnection	FOR BUILDING & ZONIN  Foundation  date/app. by  mbing  date/app. by  Rough-in plumbing above  /app. by  Heat & Air Duct  date/app. by  C.O. Final  date/app. by  g, electricity and plumbing  Dump pole  date/app. by  date/app. by  date/app.	date/app. by  date/app. by  ove slab and below wood  date/app. by  ate/app. by	ONLY  Monolithic  Sheathing/Nailing  floor  Peri. beam (Lintel)  Culvert  Pool  date  date/app. by	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by
Temporary Power  Under slab rough-in plut  Framing  date  Electrical rough-in  Permanent power  M/H tie downs, blocking	FOR BUILDING & ZONIN  Foundation  date/app. by  mbing Slab  date/app. by  Rough-in plumbing above /app. by  Heat & Air Duct  date/app. by  C.O. Final  date/app. by  g, electricity and plumbing  Pump pole  date/app. by  Travel Trailer	date/app. by  date/app. by  ove slab and below wood  date/app. by  ate/app. by	ONLY  Monolithic  Sheathing/Nailing  floor  Peri. beam (Lintel)  Culvert  Pool  date  date/app. by  Re-roof	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  date/app. by
Temporary Power  Under slab rough-in plut  Framing  date  Electrical rough-in  Permanent power  M/H tie downs, blocking  Reconnection  M/H Pole	FOR BUILDING & ZONIN  Foundation  date/app. by  mbing  date/app. by  Rough-in plumbing above  /app. by  Heat & Air Duct  date/app. by  C.O. Final  date/app. by  g, electricity and plumbing  Pump pole  date/app. by  Travel Trailer  y	date/app. by  date/app. by  ove slab and below wood  date/app. by  ate/app. by  Utility Pol app. by  tte/app. by	ONLY  Monolithic  Sheathing/Nailing  floor  Peri. beam (Lintel)  Culvert  Pool  date  date/app. by  Re-roof	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  ate/app. by  te/app. by
Temporary Power  Under slab rough-in plut  Framing  date  Electrical rough-in  Permanent power  M/H tie downs, blocking  Reconnection  M/H Pole  date/app. b	FOR BUILDING & ZONIN  Foundation  date/app. by  mbing  date/app. by  Rough-in plumbing above  /app. by  Heat & Air Duct  date/app. by  C.O. Final  date/app. by  g, electricity and plumbing  Pump pole  date/app. by  Travel Trailer  y	date/app. by  date/app. by  ove slab and below wood  date/app. by  ate/app. by  Utility Polapp. by  tte/app. by	Monolithic Sheathing/Nailing  I floor   Peri. beam (Lintel)   Culvert date/app. by  Re-roof date  SURCHARGE FEE	(footer/Slab)  date/app. by  date/app. by  date/app. by  date/app. by  ate/app. by  te/app. by  11.88
Temporary Power  Under slab rough-in plut  Framing  date  Electrical rough-in  Permanent power  M/H tie downs, blocking  Reconnection  M/H Pole  date/app. b	FOR BUILDING & ZONIN  Foundation  date/app. by  mbing Slab  date/app. by  Rough-in plumbing above /app. by  Heat & Air Duct  date/app. by  C.O. Final  date/app. by  g, electricity and plumbing  Pump pole  date/app.  Travel Trailer  y  ZONING CERT. FEE \$ 50.00	date/app. by  date/app. by  ove slab and below wood  date/app. by  ate/app. by  Utility Polapp. by  tte/app. by  ste/app. by  tte/app. by	ONLY  Monolithic  Sheathing/Nailing  floor  Peri. beam (Lintel)  Culvert  d  Pool  date  date/app. by  Re-roof  date  SURCHARGE FEE  WASTE FEE	date/app. by  date/app. by  date/app. by  date/app. by  date/app. by  ate/app. by  te/app. by  11.88

**PERMIT** 

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

22

### **Columbia County Building Permit Application**

Totalible County Ball	any Fermit Application	
For Office Use Only Application # 0801-51 Date	e Received 1-11-08 By LH	Damie # 1/134/ 57/11
	ate/8.01.08 Plans Examiner	
Flood Zone Development Permit NA Zon	ning RSF-2 Land Use Plan Ma	an Catagor NEC Lan Day
6 31 1 Z	ration Confirmation Lette Re	ap Category 123. ~~ DEC
1 100	State Road Info	
>07-4232	Fax 386	759 -5047
Name Authorized Person Signing Permit 2006	Mirsch Phone 3	86-344-4817
Address 40 1 NW forner ave. #101,		32035-5-
Owners Name Southeast Developer	Phone 38	16-755-2082
911 Address 17 SW POPPY GIN.	, Lote City, Fl	3700 4
Contractors Name Sacob Kirsch - Compa	Ss Builders Phone 3	56-344-4817
Address 484 NW turner ave, #101,	Lake City FZ	32055
Fee Simple Owner Name & Address Southeast	Developers Group	o, Lake City. Fl
Bonding Co. Name & Address_N/H-	,	
Architect/Engineer Name & Address Nicholas Paul	Geisler 1758 NW	Brown rd. l.C. Fo
Mortgage Lenders Name & Address (Olombia Ba	nk 173 NW hillsbor	o SI Lake City
Circle the correct power company - FL Power & Light - C	Sigv Elec Suwannee Valley El	De Brancata B
Property ID Number 15-45-16-03023-532	Estimated Cost of Construction	ec <u>Progressive Energy</u>
Subdivision Name Polling Meadows	Lot <u>37_</u> Block _	
Driving Directions from Usgo/441 - go Usgo	West to (R-247	TL follow
to 1/1/1	g Meadows Sub.	+ RHOPE Helpicy
		cop and Poppy
Type of Construction $\bigcirc \mathcal{F}, \bigcirc$ .	Number of Eviding December	
Total Acreage Lot Size Do you need a -C	ulvert Permit or Culvert Walver	on Property
Action Distance of Shocking from Property Lines - Front	Side 30 Side 30	
Total Building Height $7/8^{\prime\prime}$ Number of Stories	_ Heated Floor Area 1700	01
	70701 2 2	Roof Pitch 8/17
Application is hereby made to obtain a permit to do work and installation has commenced prior to the issuance of a permit all laws regulating construction in this jurisdiction	installations as indicated. I cert	ify that no work or
o distribution.		
OWNERS AFFIDAVIT: I hereby certify that all the foregoing in compliance with all applicable laws and regulating construct	formation is accurate and all wo	rk will be done in
- approable laws and regulating construct	ion and zoning.	
WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE.		SULT IN YOU PAYING
LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE	E OF COMMENCEMENT.	CONSOLI WITH TOOK
	Q- 1. 2	(1)
Owner Builder or Authorized Remanphy Notarized iLetter	Contractor Signature	10
Michelle Fischer  COUNTY OF COLUMNIA	Contractors License Number Competency Card Number	Der CBC 1253775
Expires: SEP 24, 2010	NOTARY STAMP/SEAL	
Sworn to (or affirmed) and subscribed of the BANDING CO., INC.	1 20 -10 10 1	
his day of	Typenelle Asc	ner
Personally known or Produced Identification	Notary Signature	(Revised Sept. 2006)

To 1 100 NOTHER 1-18-08

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING. CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

### FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

#### NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Owners Signature

**CONTRACTORS AFFIDAVIT:** By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's Signature (Permitee)

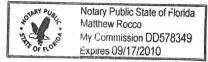
Contractor's License Number CBC 1753775 Columbia County Competency Card Number

Affirmed under penalty of perjury to by the <u>Contractor</u> and subscribed before me this  $\frac{31}{20}$  day of  $\frac{300}{200}$  20.68.

Personally known V or Produced Identification

SEAL:

State of Florida Notary Signature (For the Contractor)



# **Columbia County Building Department Culvert Permit**

### Culvert Permit No.

000001634

DATE $000$	11/2008 PARCEL ID #	15-45-16-03023-532	
APPLICANT	JACOB KIRSCH	PHONE 344-48	17
ADDRESS _	484 NW TURNER AVENUE	LAKE CITY	FL 32055
OWNER SO	OUTHEAST DEVELOPERS	PHONE 755-208	82
ADDRESS _1	17 SW POPPY GLEN	LAKE CITY	FL 32024
CONTRACTO	OR JACOB KIRSCH	PHONE 344-48	17
LOCATION O	OF PROPERTY 90W. TL ON 247S, TL CA	LLAHAN,TL HOPE HENRY, TR BU	ГТЕRCUР
PROPERTY ON I	LEFT CORNER OF BUTTERCUP AND POPPY	GLN.	
SUBDIVISION	V/LOT/BLOCK/PHASE/UNIT ROLLIN	G MEADOWS 3	2
	1/1		
SIGNATURE			
		_ ==	
	INSTACLATION REQUIREMEN		
Х	Culvert size will be 18 inches in diame driving surface. Both ends will be mite thick reinforced concrete slab.	ter with a total lenght of 32 feet, red 4 foot with a 4 : 1 slope and p	leaving 24 feet of poured with a 4 inch
	INSTALLATION NOTE: Turnouts will a) a majority of the current and exist b) the driveway to be served will be particular turnouts shall be concrete or paved concrete or paved driveway, which current and existing paved or conc	ing driveway turnouts are paved paved or formed with concrete. ed a minimum of 12 feet wide or ever is greater. The width shall o	the width of the
	Culvert installation shall conform to the	he approved site plan standards.	
	Department of Transportation Permit	installation approved standards.	
	Other		

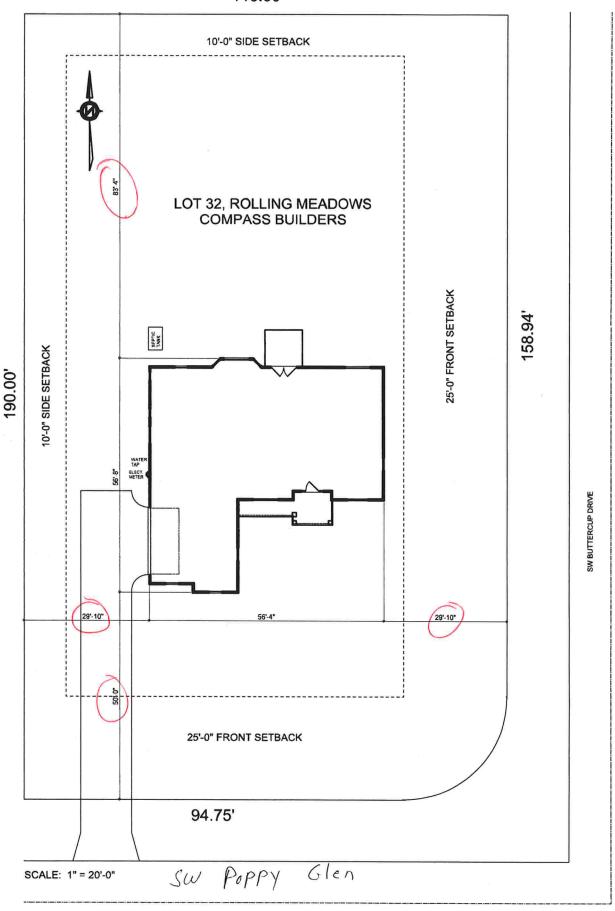
ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00





15-4S-16-03023-532 LOT 32 ROLLING MEADOWS S/D. WD 1063-1963. WD 1078-2776.

117

This Instrument Prepared By: Michael H. Harrell Abstract & Title Services, Inc. 283 NW Cole Terrace Lake City, Florida 32055

#### WARRANTY DEED

THIS WARRANTY DEED made this 28th day of March, 2006, Milestone HS, LLC, a Florida Limited Liability Company, A Florida Limited Liability Company, hereinafter called the grantor, to Southeast Developers Group, Inc, A Florida Corporation, whose mailing address is: 197 SW Waterford Court, Ste 105, Lake City, FL 32025 hereinafter called the grantee:

(Wherever used herein the terms "grantor" and "grantee" include all the parties to this instrument and the heirs, legal representatives and assigns of individuals, and the successors and assigns of corporation)

WITNESSETH that the Grantor, for and in consideration of the sum of \$10.00 and other valuable considerations, receipt whereof is hereby acknowledged, hereby grants, bargains, sells, aliens, remises, releases, conveys, and confirms unto the Grantee, all that certain land situate in COLUMBIA County, Florida, viz: Parcel ID# P/O R03023-005

LOT 32 OF ROLLING MEADOWS, A SUBDIVISION ACCORDING TO THE PLAT THEREOF AS RECORDED IN PLAT BOOK 8, PAGES 45 AND 46, OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

TOGETHER with all tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining.

TO HAVE AND TO HOLD, the same in fee simple forever.

AND the Grantor hereby covenants with said Grantee that the Grantor is lawfully seized of said land in fee simple; that the Grantor has good right and lawful authority to sell and convey said land; that the Grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December

IN WITNESS WHEREOF, the said grantor has signed and sealed these presents the day and year first

Signed, sealed and delivered in our presence: Milestone HS. PRINTED NAME Robert S. Stewart, Managing Member STATE OF FLORIDA COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 28th day of March, 2006 by Robert S. Stewart as Managing Member of Milestone HS, LLC, a Florida Limited Liability Company personally known to me or, if not personally known to me, who produced identification and who did not take an oath.

(SEAL)

MICHAEL H. HARRELL MY COMMISSION # DD 406480

EXPIRES: April 8, 2009

NOTARY PUBLIC

My Commission Expires:

Inst:2006007817 Date:03/29/2006 Time:15:18

Doc Stamp-Deed : 455.00

DC,P.Dewitt Cason,Columbia County B:1078 P:2776

# COLUMBIA COUNTY 9-1-1 ADDRESSING / GIS DEPARTMENT

P. O. Box 1787, Lake City, FL 32056-1787 Telephone: (386) 758-1125 \* Fax: (386) 758-1365 \* E-mail: ron\_croft@columbiacountyfla.com

### **ADDRESS ASSIGNMENT DATA**

The Columbia County Board of County Commissioners has passed Ordinance 2001-9, which provides for a uniform numbering system. A copy of this ordinance is available in the Clerk of Court records, located in the courthouse. This new numbering system will increase the efficiency of POLICE, FIRE AND EMERGENCY MEDICAL vehicles responding to calls within Columbia County by immediately identifying the location of the caller.

Residential or Other Structure on Parcel Number: 15-4S-16-03023-532

Address Assignment: 117 SW POPPY GLN, LAKE CITY, FL, 32024

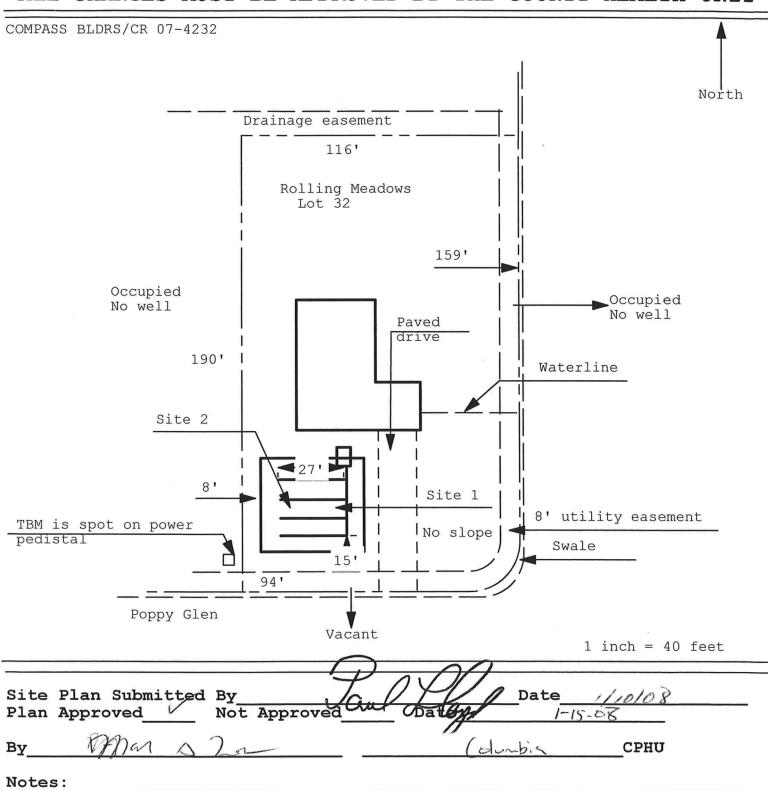
Note: LOT 32 ROLLING MEADOWS S/D

Any questions concerning this information should be referred to the Columbia County 9-1-1 Addressing / GIS Department at the address or telephone number above.

08-0054

Application for Onsite Sewage Disposal System Construction Permit. Part II Site Plan Permit Application Number: 080|-5|

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH UNIT



Project Name:

Address:

Owner:

City, State:

Compass Builders - Kailee

**Spec House** 

Lot: Sub: Rolling Meadows, Plat:

Lake City, FL 32025- 6+32

**Compass Builders** 

Columbia 27097

# FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Builder:

Permitting Office:

Jurisdiction Number: 22100

Permit Number:

Climate Zone: North	20100
1. New construction or existing 2. Single family or multi-family 3. Number of units, if multi-family 4. Number of Bedrooms 5. Is this a worst case? 6. Conditioned floor area (ft²) 7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default) a. U-factor:	12. Cooling systems a. Central Unit  b. N/A  c. N/A  13. Heating systems a. Electric Heat Pump  b. N/A  c. N/A  14. Hot water systems a. Electric Resistance  b. N/A  c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump)  15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)
Glass/Floor Area: 0.15  Total as-built p  Total base p	oints: 21957 oints: 23500 PASS
I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.  PREPARED BY:  DATE:  I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.  OWNER/AGENT:  DATE:	Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.  BUILDING OFFICIAL:  DATE:

EnergyGauge® (Version: FLRCPB v4.5.2)

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

# **SUMMER CALCULATIONS**

# Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub: Rolling Meadows, Plat:, Lake City, FL, 32025-

PERMIT #:

BASE		AS-E	BUILT			
GLASS TYPES .18 X Conditioned X BSPM = Points Floor Area	O <sup>o</sup> Type/SC Orn	verhang it Len F	Hgt Area X	SPM X	SOF	= Points
.18 1700.0 18.59 5689.0	1.Double, Clear	W 1.5	10.0 75.0	38.52	0.98	2828.0
	2.Double, Clear		10.0 20.0	38.52	0.98	754.0
	3.Double, Clear		10.0 40.0	38.52	0.98	1508.0
	4.Double, Clear	N 1.5	8.0 15.0	19.20	0.97	278.0
	5.Double, Clear	E 1.5	8.0 30.0	42.06	0.96	1208.0
	6.Double, Clear	E 9.5	10.0 13.3	42.06	0.53	297.0
	7.Double, Clear	E 5.5	10.0 30.0	42.06	0.69	875.0
,	8.Double, Clear	S 1.5	8.0 16.0	35.87	0.92	529.0
	9.Double, Clear	S 1.5	8.0 20.0	35.87	0.92	662.0
	As-Built Total:		259.3	,		8939.0
WALL TYPES Area X BSPM = Points	Туре	R-Va	alue Area	X SPN	<b>/</b> =	Points
Adjacent 180.0 0.70 126.0	1. Frame, Wood, Exterior	13	3.0 1146.7	1.50		1720.0
Exterior 1146.7 1.70 1949.4	2. Frame, Wood, Adjacent	13		0.60		108.0
Base Total: 1326.7 2075.4	As-Built Total:		1326.7			1828.0
DOOR TYPES Area X BSPM = Points	Туре		Area	X SPM	1 =	Points
Adjacent 20.0 2.40 48.0	1.Exterior Insulated		20.0	4.10		82.0
Exterior 20.0 6.10 122.0	2.Adjacent Insulated		20.0	1.60		32.0
Base Total: 40.0 170.0	As-Built Total:		40.0			114.0
CEILING TYPES Area X BSPM = Points	Туре	R-Value	Area X S	SPM X SC	M =	Points
Under Attic 1700.0 1.73 2941.0	1. Under Attic	30	.0 1850.0 1	1.73 X 1.00		3200.5
Base Total: 1700.0 2941.0	As-Built Total:		1850.0			3200.5
FLOOR TYPES Area X BSPM = Points	Туре	R-Va	alue Area	X SPM	1 =	Points
	1. Slab-On-Grade Edge Insulation	5	.0 190.0(p	-36.20		-6878.0
Raised 0.0 0.00 0.0						
Base Total: -7030.0	As-Built Total:		190.0			-6878.0
INFILTRATION Area X BSPM = Points			Area	X SPM	=	Points
1700.0 10.21 17357.0			1700.0	) 10.21		17357.0

# **SUMMER CALCULATIONS**

# Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

	BASE		AS-BUILT					
Summer Ba	se Points: 21	1202.4	Summer As-Built Points: 24560.5					
Total Summer Points	X System = Multiplier	Cooling Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)					
21202.4	0.3250	6890.8	(sys 1: Central Unit 36000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Unc(R),Int(AH),R6.0(INS)  24561					

### WINTER CALCULATIONS

### Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025-

PERMIT #:

BASE	AS-BUILT					
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area	Overhang Type/SC Ornt Len Hgt Area X WPM X WOF	= Points				
.18 1700.0 20.17 6172.0	1.Double, Clear W 1.5 10.0 75.0 20.73 1.01	1563.0				
- "	2.Double, Clear W 1.5 10.0 20.0 20.73 1.01	416.0				
*	3.Double, Clear W 1.5 10.0 40.0 20.73 1.01	833.0				
	4.Double, Clear N 1.5 8.0 15.0 24.58 1.00	368.0				
	5.Double, Clear E 1.5 8.0 30.0 18.79 1.02 6.Double, Clear E 9.5 10.0 13.3 18.79 1.27	574.0				
	6.Double, Clear E 9.5 10.0 13.3 18.79 1.27 7.Double, Clear E 5.5 10.0 30.0 18.79 1.14	318.0 641.0				
	8.Double, Clear S 1.5 8.0 16.0 13.30 1.04	221.0				
	9.Double, Clear S 1.5 8.0 20.0 13.30 1.04	276.0				
	As-Built Total: 259.3	5210.0				
WALL TYPES Area X BWPM = Points	Type R-Value Area X WPM =	Points				
Adjacent 180.0 3.60 648.0	1. Frame, Wood, Exterior 13.0 1146.7 3.40	3898.8				
Exterior 1146.7 3.70 4242.8	2. Frame, Wood, Adjacent 13.0 180.0 3.30	594.0				
Base Total: 1326.7 4890.8	As-Built Total: 1326.7	4492.8				
DOOR TYPES Area X BWPM = Points	Type Area X WPM =	Points				
Adjacent 20.0 11.50 230.0	1.Exterior Insulated 20.0 8.40	168.0				
Exterior 20.0 12.30 246.0	2.Adjacent Insulated 20.0 8.00	160.0				
Base Total: 40.0 476.0	As-Built Total: 40.0	328.0				
CEILING TYPES Area X BWPM = Points	Type R-Value Area X WPM X WCM =	Points				
Under Attic 1700.0 2.05 3485.0	1. Under Attic 30.0 1850.0 2.05 X 1.00	3792.5				
Base Total: 1700.0 3485.0	As-Built Total: 1850.0	3792.5				
FLOOR TYPES Area X BWPM = Points	Type R-Value Area X WPM =	Points				
Slab 190.0(p) 8.9 1691.0 Raised 0.0 0.00 0.0	1. Slab-On-Grade Edge Insulation 5.0 190.0(p 7.60	1444.0				
Base Total: 1691.0	As-Built Total: 190.0	1444.0				
INFILTRATION Area X BWPM = Points	Area X WPM =	Points				
1700.0 -0.59 -1003.0	1700.0 -0.59	-1003.0				

# **WINTER CALCULATIONS**

### Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

	BASE		AS-BUILT					
Winter Base	Points:	15711.8	Winter As-Built Points: 14264.3					
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)					
15711.8	0.5540	8704.3	(sys 1: Electric Heat Pump 36000 btuh ,EFF(7.7) Ducts:Unc(S),Unc(R),Int(AH),R6.0 14264.3 1.000 (1.069 x 1.169 x 0.93) 0.443 0.950 6974.5 14264.3 1.00 1.162 0.443 0.950 6974.5					

# **WATER HEATING & CODE COMPLIANCE STATUS**

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025- PERMIT #:

BASE					×		Α	Α	S-BUII	LT		
WATER HEA Number of Bedrooms	TING X	Multiplier		Total	Tank Volume	EF	Number of Bedrooms	х	Tank X Ratio	Multiplier	X Credit Multiplie	Total
3		2635.00		7905.0	50.0	0.90	3		1.00	2693.56	1.00	 8080.7
					As-Built Total:					8080.7		

	CODE COMPLIANCE STATUS												
	BASE						AS-BUILT						
Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
6891		8704		7905		23500	6902		6974		8081		21957

**PASS** 



# **Code Compliance Checklist**

### Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub: Rolling Meadows, Plat:, Lake City, FL, 32025-

PERMIT #:

#### **6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST**

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members.  EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

### 6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

# ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

### ESTIMATED ENERGY PERFORMANCE SCORE\* = 85.6

The higher the score, the more efficient the home.

Spec House, Lot: , Sub: Rolling Meadows, Plat: , Lake City, FL, 32025-

1. 2. 3. 4.	New construction or existing Single family or multi-family Number of units, if multi-family Number of Bedrooms	New Single family 1 3		Cooling systems a. Central Unit b. N/A	Cap: 36.0 kBtu/hr SEER: 13.00	_
5. 6. 7.	ls this a worst case? Conditioned floor area (ft²) Glass type¹ and area: (Label reqd.	No 1700 ft <sup>2</sup> by 13-104.4.5 if not default)		s. N/A		_
	U-factor: (or Single or Double DEFAULT) SHGC: (or Clear or Tint DEFAULT)	Description Area 7a. (Dble Default) 259.3 ft <sup>2</sup>	_ *	Heating systems a. Electric Heat Pump	Cap: 36.0 kBtu/hr HSPF: 7.70	
a.	Floor types Slab-On-Grade Edge Insulation N/A	7b. (Clear) 259.3 ft <sup>2</sup> R=5.0, 190.0(p) ft		o. N/A		_
9. a. b. c. d. e. 10.	N/A Wall types Frame, Wood, Exterior Frame, Wood, Adjacent N/A N/A N/A Ceiling types Under Attic N/A	R=13.0, 1146.7 ft <sup>2</sup> R=13.0, 180.0 ft <sup>2</sup> R=30.0, 1850.0 ft <sup>2</sup>		Hot water systems  Electric Resistance  N/A  Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) HVAC credits (CF-Ceiling fan, CV-Cross ventilation,	Cap: 50.0 gallons EF: 0.90 PT,	_ _ _
11. a.	N/A Ducts Sup: Unc. Ret: Unc. AH: Interior N/A	Sup. R=6.0, 50.0 ft		HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)		
Consin the	tify that this home has complie struction through the above end is home before final inspection d on installed Code compliant der Signature:	ergy saving features which a. Otherwise, a new EPL I features.	n will be in Display Ca	stalled (or exceeded)	THE STATE OF THE S	ALORIDA
	ress of New Home:			Cip;	CODWETRUST	
****	TE. The leavest of			II I I I DE LEDO	The same of the sa	

\*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

# **Residential System Sizing Calculation**

Summary Project Title:

Spec House

Lake City, FL 32025-

Compass Builders - Kailee

(ot 32

Code Only Professional Version

Climate: North

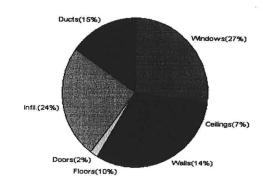
1/10/2008

				1/10/200	8
Location for weather data: Gaine	sville - Def	aults: Latitu	ude(29) Altitude(152 ft.) Temp Rang	re(M)	
Humidity data: Interior RH (50%	) Outdoor	wet bulb (7	7F) Humidity difference(54gr.)	,-()	
Winter design temperature	33		Summer design temperature	92	F
Winter setpoint	70	F	Summer setpoint	75	
Winter temperature difference	37	F	Summer temperature difference	17	-
Total heating load calculation	30539	Btuh	Total cooling load calculation	46532	·
Submitted heating capacity	% of calc	Btuh	Submitted cooling capacity	% of calc	
Total (Electric Heat Pump)	117.9	36000	Sensible (SHR = 0.75)		27000
Heat Pump + Auxiliary(0.0kW)	117.9	36000	Latent	110.0	
			Total (Electric Heat Pump)		36000

### WINTER CALCULATIONS

Winter Heating Load (for 1700 sqft)

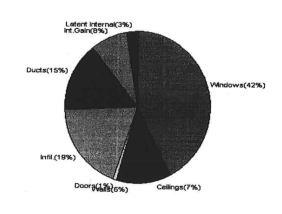
Load component			Load	
Window total	259	sqft	8348	Btuh
Wall total	1327	sqft	4357	Btuh
Door total	40	sqft	518	Btuh
Ceiling total	1850	sqft	2180	Btuh
Floor total	190	sqft	3107	Btuh
Infiltration	181	cfm	7345	Btuh
Duct loss			4684	Btuh
Subtotal			30539	Btuh
Ventilation	0	cfm	0	Btuh
TOTAL HEAT LOSS			30539	Btuh



### **SUMMER CALCULATIONS**

Summer Cooling Load (for 1700 sqft)

Load component			Load	
Window total	259	sqft	19662	Btuh
Wall total	1327	sqft	2663	Btuh
Door total	40	sqft	392	Btuh
Ceiling total	1850	sqft	3064	Btuh
Floor total			0	Btuh
Infiltration	159	cfm	2953	Btuh
Internal gain			3780	Btuh
Duct gain			5838	Btuh
Sens. Ventilation	0	cfm	0	Btuh
Total sensible gain			38352	Btuh
Latent gain(ducts)			1181	Btuh
Latent gain(infiltration)			5799	Btuh
Latent gain(ventilation)			0	Btuh
Latent gain(internal/occup	r)	1200	Btuh	
Total latent gain			8180	Btuh
TOTAL HEAT GAIN			46532	Btuh



Version 8 For Florida residences only

EnergyGauge® System PREPARED BY: DATE:

# **System Sizing Calculations - Winter**

# Residential Load - Whole House Component Details

Spec House

Lake City, FL 32025-

Compass Builders - Kailee

Code Only Professional Version

Climate: North

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/10/2008

Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	75.0	32.2	2414 Btu
2	2, Clear, Metal, 0.87	W	20.0	32.2	644 Btu
3	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btu
4	2, Clear, Metal, 0.87	N	15.0	32.2	483 Btu
5	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btu
6	2, Clear, Metal, 0.87	E	13.3	32.2	429 Btu
7	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btu
8	2, Clear, Metal, 0.87	S	16.0	32.2	515 Btu
9	2, Clear, Metal, 0.87	S	20.0	32.2	644 Btu
	Window Total		259(sqft)		8348 Btu
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1147	3.3	3766 Btu
2	Frame - Wood - Adj(0.09)	13.0	180	3.3	591 Btu
	Wall Total		1327		4357 Btu
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btu
2	Insulated - Adjacent		20	12.9	259 Btu
	Door Total		40		518Btu
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1850	1.2	2180 Btu
	Ceiling Total		1850		2180Btu
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	190.0 ft(p)	16.4	3107 Btu
·	Floor Total		190		3107 Btu
			Envelope Su	btotal:	18510 Btu
Infiltration	Туре	ACH X Volu	ıme(cuft) walls(sqf	) CFM=	
	Natural	0.80	13600 1327	181.3	7345 Btu
Ductload			(D	LM of 0.181)	4684 Btu
All Zones		Sens	ible Subtotal Al	Zones	30539 Btu

# **Manual J Winter Calculations**

Residential Load - Component Details (continued)

Spec House

Project Title: Compass Builders - Kailee

Code Only Professional Version

Climate: North

1/10/2008

Lake City, FL 32025-

/HOLE HOUSE TOTAL	S	
	Subtotal Sensible Ventilation Sensible Total Btuh Loss	30539 Btuł 0 Btuł 30539 Btuł

PARTICIPATION	(2) (2) (2) (3) (3) (4) (4) (4) (4) (4) (4) (4) (4) (4) (4
EQUIPMENT	
the state of the s	

1. Electric Heat Pump # 36000 Btuh

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



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# **System Sizing Calculations - Winter**

# Residential Load - Room by Room Component Details Project Title: Code C

Spec House

Compass Builders - Kailee

**Professional Version** 

Climate: North

Lake City, FL 32025-

Reference City: Gainesville (Defaults) Winter Temperature Difference: 37.0 F

1/10/2008

### Component Loads for Zone #1: Main

Mindon	TR				The state of the s
Window	Panes/SHGC/Frame/U	Orientation	Area(sqft) X	HTM=	Load
1	2, Clear, Metal, 0.87	W	75.0	32.2	2414 Btuh
2	2, Clear, Metal, 0.87	W	20.0	32.2	644 Btuh
3	2, Clear, Metal, 0.87	W	40.0	32.2	1288 Btuh
4	2, Clear, Metal, 0.87	N	15.0	32.2	483 Btuh
5	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btuh
6	2, Clear, Metal, 0.87	E	13.3	32.2	429 Btuh
7	2, Clear, Metal, 0.87	E	30.0	32.2	966 Btuh
8	2, Clear, Metal, 0.87	S	16.0	32.2	515 Btuh
9	2, Clear, Metal, 0.87	S	20.0	32.2	644 Btuh
	Window Total		259(sqft)		8348 Btuh
Walls	Туре	R-Value	Area X	HTM=	Load
1	Frame - Wood - Ext(0.09)	13.0	1147	3.3	3766 Btuh
2	Frame - Wood - Adj(0.09)	13.0	180	3.3	591 Btuh
	Wall Total		1327		4357 Btuh
Doors	Туре		Area X	HTM=	Load
1	Insulated - Exterior		20	12.9	259 Btuh
2	Insulated - Adjacent		20	12.9	259 Btuh
	Door Total		40		518Btuh
Ceilings	Type/Color/Surface	R-Value	Area X	HTM=	Load
1	Vented Attic/D/Shin	30.0	1850	1.2	2180 Btuh
	Ceiling Total	9.	1850		2180Btuh
Floors	Туре	R-Value	Size X	HTM=	Load
1	Slab On Grade	5	190.0 ft(p)	16.4	3107 Btuh
	Floor Total		190		3107 Btuh
		z	one Envelope Su	ibtotal:	18510 Btuh
Infiltration	Туре	ACH X Volu	ıme(cuft) walls(sqf	) CFM=	
	Natural	0.80	13600 1327	181.3	7345 Btuh
Ductload	Average sealed, Supply(R6.	0-Attic), Returi	n(R6.0-Attic) (D	LM of 0.181)	4684 Btuh
Zone #1		Sens	sible Zone Subto	otal	30539 Btuh

# **Manual J Winter Calculations**

Residential Load - Component Details (continued)

Spec House

Project Title: Compass Builders - Kailee

Code Only Professional Version

Climate: North

Lake City, FL 32025-

1/10/2008

	Subtotal Sensible Ventilation Sensible Total Btuh Loss	30539 Btuh 0 Btuh 30539 Btuh
QUIPMENT		Mawamazasia

Key: Window types (SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(Frame types - metal, wood or insulated metal)
(U - Window U-Factor or 'DEF' for default)
(HTM - ManualJ Heat Transfer Multiplier)
Key: Floor size (perimeter(p) for slab-on-grade or area for all other floor types)



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# **System Sizing Calculations - Summer**

# Residential Load - Whole House Component Details

Spec House

Project Title: Compass Builders - Kailee

Lake City, FL 32025-

Summer Temperature Difference: 17.0 F

Code Only Professional Version Climate: North

1/10/2008

### **Component Loads for Whole House**

Reference City: Gainesville (Defaults)

	Type*		Over	hang	Wind	dow Area	(sqft)	H	HTM	Load	
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross		Jnshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	75.0	0.0	75.0	29	80	5964	Btuh
2	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	20.0	0.0	20.0	29	80	1590	Btuh
3	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	40.0	0.0	40.0	29	80	3181	Btuh
4	2, Clear, 0.87, None,N,N	N	1.5ft	8ft.	15.0	0.0	15.0	29	29	434	
5	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	30.0	0.0	30.0	29	80	2385	
6	2, Clear, 0.87, None,N,N	E	9.5ft	10ft.	13.3	9.1	4.2	29	80	600	
7 8	2, Clear, 0.87, None,N,N	E	5.5ft	10ft.	30.0	0.0	30.0	29	80	2385	Btuh
9	2, Clear, 0.87, None,N,N 2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	16.0	16.0	0.0	29	34	463	
9	Excursion	5	1.5ft	8ft.	20.0	20.0	0.0	29	34	200	Btuh
	Window Total				250 /					2079	
Walls	Type		DV	1	259 (					19662	Btuh
	*		K-Va		I-Value	Area(			HTM	Load	
1 2	Frame - Wood - Ext Frame - Wood - Adj			13.0/		1146			2.1	2392	
2				13.0/	0.09	180			1.5		Btuh
	Wall Total						7 (sqft)			2663	Btuh
Doors	Туре				Area (sqft)			HTM	Load		
1	Insulated - Exterior					20.	_		9.8	196	Btuh
2	Insulated - Adjacent					20.	0		9.8	196	Btuh
184	Door Total					40	(sqft)			392	Btuh
Ceilings	Type/Color/Surface		R-Va	alue	Area(sqft)			HTM	Load		
1	Vented Attic/DarkShingle			30.0		1850	0.0		1.7	3064	Btuh
	Ceiling Total					1850	(sqft)		3.83	3064	
Floors	Туре		R-Va	lue		Siz			НТМ	Load	D.c.i.
1	Slab On Grade			5.0		190	) (ft(p))		0.0		Btuh
	Floor Total			0.0			(sqft)		0.0		Btuh
*****	Total					130.0	(sqit)				Diun
					1	En	velope §	Subtotal	:	25781	Btuh
nfiltration	Туре		Α	СН	Volume	e(cuft) w	all area	(saft)	CFM=	Load	-
	SensibleNatural			0.70		13600	1327	(-4.4)	181.3	2953	Btuh
Internal		(	Occup			Btuh/occ		Δ	Appliance	Load	
gain				6		( 230		,	2400	3780	Btul
						Se	nsible E	nvelope	Load:	32514	Btuh
uct load							(DGN	/l of 0.18	80)	5838	Btuł
						Sen	sible Lo	ad All 2	Zones	38352 E	3tuh

### **Manual J Summer Calculations**

Residential Load - Component Details (continued)

Spec House

Project Title: Compass Builders - Kailee

Code Only Professional Version

Climate: North

1/10/2008

# WHOLE HOUSE TOTALS

Lake City, FL 32025-

	Sensible Envelope Load All Zones	32514	Btuh
	Sensible Duct Load	5838	Btuh
	Total Sensible Zone Loads	38352	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	38352	Btuh
<b>Totals for Cooling</b>	Latent infiltration gain (for 54 gr. humidity difference)	5799	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	1181	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	o	Btuh
	Latent total gain	8180	Btuh
1	TOTAL GAIN	46532	Btuh

1. Central Unit	#	36000 Btuh
EQUIPMENT		

\*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

# **System Sizing Calculations - Summer**

# Residential Load - Room by Room Component Details Project Title: Code C

Spec House

Compass Builders - Kailee

Code Only Professional Version

Lake City, FL 32025-

Climate: North

Reference City: Gainesville (Defaults)

Summer Temperature Difference: 17.0 F

1/10/2008

### Component Loads for Zone #1: Main

	Type*		Overhang Window Area(sqft) HTM		HTM	Load					
Window	Pn/SHGC/U/InSh/ExSh/IS	Ornt	Len	Hgt	Gross	Shaded	Unshaded	Shaded	Unshaded		
1	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	75.0	0.0	75.0	29	80	5964	Btuh
2	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	20.0	0.0	20.0	29	80	1590	Btuh
3	2, Clear, 0.87, None,N,N	W	1.5ft	10ft.	40.0	0.0	40.0	29	80	3181	
4	2, Clear, 0.87, None,N,N	N	1.5ft	8ft.	15.0	0.0	15.0	29	29	434	
5	2, Clear, 0.87, None,N,N	E	1.5ft	8ft.	30.0	0.0	30.0	29	80	2385	
6	2, Clear, 0.87, None,N,N	E	9.5ft	10ft.	13.3	9.1	4.2	29	80	600	
7	2, Clear, 0.87, None,N,N	E	5.5ft	10ft.	30.0	0.0	30.0	29	80	2385	
8	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	16.0	16.0	0.0	29	34	463	
9	2, Clear, 0.87, None,N,N	S	1.5ft	8ft.	20.0	20.0	0.0	29	34		Btuh
	Window Total				259 (					17583	Btuh
Walls	Туре		R-Va	alue/U	-Value	Area	(sqft)		HTM	Load	
1	Frame - Wood - Ext			13.0/	0.09	114	6.7		2.1	2392	Btuh
2	Frame - Wood - Adj			13.0/	0.09	180	0.0		1.5		Btuh
	Wall Total					132	7 (sqft)			2663	Btuh
Doors	Туре					Area	(sqft)		HTM	Load	
1	Insulated - Exterior					20	.0		9.8	196	Btuh
2	Insulated - Adjacent					20	.0		9.8	196	Btuh
	Door Total					4	0 (sqft)		1947 - 61	392	Btuh
Ceilings	Type/Color/Surface		R-Va	alue		Area			нтм	Load	
1	Vented Attic/DarkShingle			30.0		185			1.7	3064	Btuh
	Ceiling Total			77.7			0 (sqft)			3064	
Floors	Type		R-Va	alue		Siz			нтм	Load	Dian
1	Slab On Grade		., .,	5.0					0.0	0	Btuh
1	PROTECT AND ENGINEERS			5.0			90 (ft(p))		0.0	_	
	Floor Total					190.	0 (sqft)			Ü	Btuh
						Z	one Enve	elope Su	ubtotal:	23702	Btuh
nfiltration	Туре		Δ	CH	Volum	e(cuft) v	wall area	(saft)	CFM=	Load	
	SensibleNatural		,	0.70	Join	13600	1327	(Jqit)	158.7	2953	Btuh
Internal			Occup			Btuh/oc			Appliance	Load	- Juli
		•	Cocup	6		X 23		,	2400	3780	Btuh
gain				O		^ 23	0 +		2400	3/60	Diun
	-					Se	ensible E	Envelope	e Load:	30435	Btuh
Duct load	Average sealed, Supply	(R6.0-/	Attic),	Retur	n(R6.0	-Attic)		(DGM c	of 0.180)	5465	Btuh
							Sensib	le Zone	Load	35900	Btuh

### The following window Excursion will be assigned to the system loads.

Windows	July excursion for System 1	Excursion Subtotal:	2079 Btuh 2079 Btuh
		Excursion Subtotal.	2079 Dtuli

# **Manual J Summer Calculations**

Residential Load - Component Details (continued)

Project Title:
Compass Builders - Kailee

Continued

Spec House

Lake City, FL 32025-

Code Only Professional Version

Climate: North

1/10/2008

	373 Bt
Sensible Excursion Load	2452 Btu
	Sensible Excursion Load

### **Manual J Summer Calculations**

Residential Load - Component Details (continued)

Spec House

Lake City, FL 32025-

Project Title: Compass Builders - Kailee

Code Only Professional Version Climate: North

1/10/2008

### WHOLE HOUSE TOTALS

	Sensible Envelope Load All Zones	32514	Btuh
-	Sensible Duct Load	5838	Btuh
	Total Sensible Zone Loads	38352	Btuh
	Sensible ventilation	0	Btuh
	Blower	0	Btuh
Whole House	Total sensible gain	38352	Btuh
<b>Totals for Cooling</b>	Latent infiltration gain (for 54 gr. humidity difference)	5799	Btuh
	Latent ventilation gain	0	Btuh
	Latent duct gain	1181	Btuh
	Latent occupant gain (6 people @ 200 Btuh per person)	1200	Btuh
	Latent other gain	0	Btuh
	Latent total gain	8180	Btuh
	TOTAL GAIN	46532	Btuh

1. Central Unit	#	36000 Btuh
EQUIPMENT		

\*Key: Window types (Pn - Number of panes of glass)

(SHGC - Shading coefficient of glass as SHGC numerical value or as clear or tint)
(U - Window U-Factor or 'DEF' for default)
(InSh - Interior shading device: none(N), Blinds(B), Draperies(D) or Roller Shades(R))
(ExSh - Exterior shading device: none(N) or numerical value)
(BS - Insect screen: none(N), Full(F) or Half(H))

(Ornt - compass orientation)



Version 8 For Florida residences only

# **Residential Window Diversity**

### MidSummer

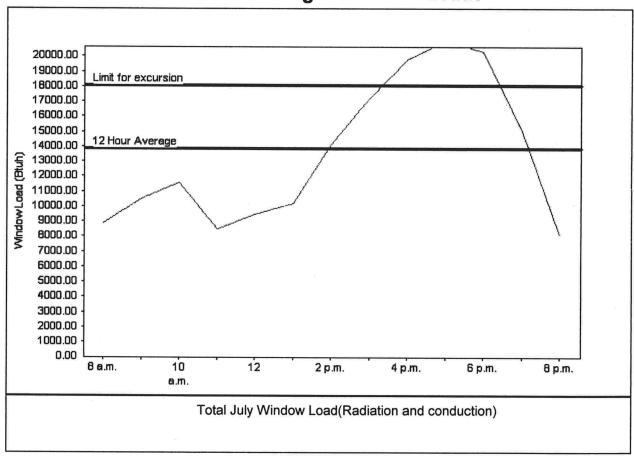
Spec House Lake City, FL 32025Project Title: Compass Builders - Kailee

Code Only Professional Version Climate: North

1/10/2008

Weather data for: Gainesville - Defa	aults		
Summer design temperature	92 F	Average window load for July	13873 Btu
Summer setpoint	75 F	Peak window load for July	20990 Btu
Summer temperature difference	17 F	Excusion limit(130% of Ave.)	18035 Btu
Latitude	29 North	Window excursion (July)	2955 Btuh

### **WINDOW Average and Peak Loads**



This application has glass areas that produce large heat gains for part of the day. Variable air volume devices are required to overcome spikes in solar gain for one or more rooms. Install a zoned system or provide zone control for problem rooms. Single speed equipment may not be suitable for the application.

EnergyGauge® System Sizing for Florida residences only PREPARED BY:

DATE:

EnergyGauge® FLRCPB v4.5.2



# PRODUCT APPROVAL SPECIFICATION SHEET

As required by Florida Statute 553.842 and Florida Administrative Code 9B-72, please provide the information and approval numbers on the building components listed below if they will be utilized on the construction project for which you are applying for a building permit. We recommend you contact your local product supplier should you not know the product approval number for any of the applicable listed products. Statewide approved products are listed online @ www.floridabuilding.org

Category/Subcategory	Manufacturer	Product Description	Approval Number(s)
1. EXTERIOR DOORS			
A. SWINGING	Jeld-Wen	Exterior Swinning door	FL- 49X-R1
B. SLIDING			1
C. SECTIONAL/ROLL UP	ROYNOR	Raynor	FL-4867
D. OTHER			
2. WINDOWS			
A. SINGLE/DOUBLE HUNG	MI- Products	Single Hung Window	FL-5108
B. HORIZONTAL SLIDER		3	1 3 1 3
C. CASEMENT		- v	
D. FIXED			
E. MULLION			
F. SKYLIGHTS			
G. OTHER			
3. PANEL WALL			
A. SIDING	James Hurdie	Hardi Plank Siding	FL-889-RI
B. SOFFITS	Kaycan	Aluminum Soffit	FL- 4957
C. STOREFRONTS			- 113/
D. GLASS BLOCK			
E. OTHER			
4. ROOFING PRODUCTS	<del> </del>		
A. ASPHALT SHINGLES	Elk Roofing	Asphalt Shingles	FL-586-RZ
B. NON-STRUCT METAL		TOPHETT SHITTES	PZ-386-KZ
C. ROOFING TILES			
D. SINGLE PLY ROOF			
E. OTHER			
5. STRUCT COMPONENTS			
A. WOOD CONNECTORS	SIMPSON Strong to	Truss STIAPS	FL- 474-RI
B. WOOD ANCHORS	1	3/14/35	-L- 7/7-XI
C. TRUSS PLATES			
D. INSULATION FORMS			
E. LINTELS			
F. OTHERS			
6. NEW EXTERIOR			
ENVELOPE PRODUCTS			
A			<del></del>

The products listed below did not demonstrate product approval at plan review. I understand that at the time of inspection of these products, the following information must be available to the inspector on the jobsite; 1) copy of the product approval, 2) performance characteristics which the product was tested and certified to comply with, 3) copy of the applicable manufacturers installation requirements. Further, I understand these products may have to be removed if approval cannot be demonstrated during inspection.

APPLICANT SIGNATURE DATE

RESIDENTIAL MINIMUM PLAN REQUIREMENTS AND CHECKLIST FOR FLORIDA BUILDING CODE 2004 and FLORIDA RESIDENTIAL CODE 2004 WITH AMENDMENTS ONE (1) AND TWO (2) FAMILY DWELLINGS

### ALL REQUIREMENTS ARE SUBJECT TO CHANGE **EFFECTIVE OCTOBER 1, 2005**

ALL BUILDING PLANS MUST INDICATE THE FOLLOWING ITEMS AND INDICATE COMPLIANCE WITH CHAPTER 16 OF THE FLORIDA BUILDING CODE 2004 BY PROVIDING CALCULATIONS AND DETAILS THAT HAVE THE SEAL AND SIGNATURE OF A CERTIFIED ARCHITECT OR ENGINEER REGISTERED IN THE STATE OF FLORIDA, OR ALTERNATE METHODOLOGIES, APPROVED BY THE STATE OF FLORIDA BUILDING COMMISSION FOR ONE-AND-TWO FAMILY DWELLINGS. FOR DESIGN PURPOSES THE FOLLOWING BASIC WIND SPEED AS PER FIGURE 1609 SHALL BE USED.

WIND SPEED LINE SHALL BE DEFINED AS FOLLOWS: THE CENTERLINE OF INTERSTATE 75.

- 1. ALL BUILDINGS CONSTRUCTED EAST OF SAID LINE SHALL BE ——— 100 MPH
- 3. NO AREA IN COLUMBIA COUNTY IS IN A WIND BORNE DEBRIS REGION

# APPLICANT – PLEASE CHECK ALL APPLICABLE BOXES BEFORE SUBMITTAL

GENERAL	L REOUIRE	MENTS: Two (2) complete
Applicant	Plans Exam	MENTS: Two (2) complete sets of plans containing the following:
<b>.</b> /	0	All drawings must be clear, concise and drawn to scale ("Optional"
<b>123</b> /	0	Designers name and signoture on decomposition of the same and signoture on decomposition of the same and signoture on decomposition of the same and signoture on the same and signoture of the same and same
B	0	architect or engineer, official seal shall be affixed.  Site Plan including:  a) Dimensions of lot  b) Dimensions of building set backs
M	0	C) Location of all other buildings on lot, well and septic tank if applicable, and all utility easements.  d) Provide a full legal description of property.
	u	Plans or specifications must state compliance with FBC Section 1609.  The following information must be shown as per section 1603.1.4 FBC  a. Basic wind speed (3-second gust), miles per hour (km/hr).  b. Wind importance factor, Iw, and building classification from Table 1604.5 or Table 6-1, ASCE 7 and building classification in Table 1-1, ASCE 7.  c. Wind exposure, if more than one wind exposure is utilized, the wind exposure and applicable wind direction shall be indicated.  d. The applicable enclosure classifications and, if designed with ASCE 7, internal pressure coefficient.  e. Components and Cladding. The design wind appears and applicable and applications.
D D D	0	psf (kN/m²) to be used for the design of exterior component and cladding materials not specifally designed by the registered design professional.  Elevations including:  a) All sides b) Roof pitch c) Overhang dimensions and detail with attic ventilation

0	0 0 0	d) Location, size and height above roof of chimneys. e) Location and size of skylights f) Building height e) Number of stories Floor Plan including:
	. 🗖	a) Rooms labeled and dimensioned.
	0	b) Shear walls identified.
		c) Show product approval specification as required by Fla. Statute 553.842 and Fla. Administrative Code 9B-72 (see attach forms).
4	0	d) Show safety glazing of glass, where required by code.
	0	e) Identify egress windows in bedrooms, and size.  f) Fireplace (gas vented), (gas non-vented) or wood burning with
0	0	hearth, (Please circle applicable type).  g) Stairs with dimensions (width, tread and riser) and details of guardrails and handrails.
þ	•	h) Must show and identify accessibility requirements (accessible bathroom)
ф		Foundation Plan including:  a) Location of all load-bearing wall with required footings indicated as standard
П	0	or monolithic and dimensions and reinforcing.  b) All posts and/or column footing including size and reinforcing
0	Ö	c) Any special support required by soil analysis such as piling
ф		d) Location of any vertical steel.
0		Roof System;
	٥	<ol> <li>a) Truss package including:         <ol> <li>Truss layout and truss details signed and sealed by Fl. Pro. Eng.</li> <li>Roof assembly (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)</li> </ol> </li> </ol>
0		b) Conventional Framing Layout including:  1. Rafter size, species and spacing 2. Attachment to wall and uplift 3. Ridge beam sized and valley framing and support details 4. Roof assembly (FBC 106.1.1.2)Roofing systems, materials, manufacturer, fastening requirements and product evaluation with wind resistance rating)
0	0	Wall Sections including: a) Masonry wall
J	<u>u</u>	1. All materials making up wall
		<ol> <li>Block size and mortar type with size and spacing of reinforcement</li> <li>Lintel, tie-beam sizes and reinforcement</li> </ol>
		<ol> <li>Gable ends with rake beams showing reinforcement or gable truss and wall bracing details</li> </ol>
		<ol> <li>All required connectors with uplift rating and required number and size of fasteners for continuous tie from roof to foundation shall be designed by a Windload engineer using the engineered roof truss plans.</li> </ol>
		<ol> <li>Roof assembly shown here or on roof system detail (FBC 106.1.1.2) Roofing system, materials, manufacturer, fastening</li> </ol>
		requirements and product evaluation with resistance rating)
		<ul> <li>7. Fire resistant construction (if required)</li> <li>8. Fireproofing requirements</li> </ul>
		9. Shoe type of termite treatment (termiticide or alternative method)
		10. Slab on grade
œ.		<ul> <li>a. Vapor retarder (6mil. Polyethylene with joints lapped 6 inches and sealed)</li> <li>b. Must show control joints, synthetic fiber reinforcement or</li> </ul>
		Welded fire fabric reinforcement and supports  11. Indicate where pressure treated wood will be placed  12. Provide insulation R value for the following:

		b) Wood frame wall
		1. All materials making up wall
		2. Size and species of studs
		<ol><li>Sheathing size, type and nailing schedule</li></ol>
		4. Headers sized
		<ol><li>Gable end showing balloon framing detail or gable truss and wall</li></ol>
		ninge bracing detail
		6. All required fasteners for continuous tie from roof to foundation
		(truss anchors, straps, anchor bolts and washers) shall be designed
		by a Windload engineer using the engineered roof truss plans.
		7. Roof assembly shown here or on roof system detail (FBC
		106.1.1.2) Rooting system, materials, manufacturer fastening
		requirements and product evaluation with wind resistance rating)
		8. Fire resistant construction (if applicable)
		9. Fireproofing requirements
		10. Show type of termite treatment (termiticide or alternative method)
		11. Slab on grade
		<ul> <li>Vapor retarder (6Mil. Polyethylene with joints lapped 6 inches and sealed</li> </ul>
		b. Must show control joints, synthetic fiber reinforcement or
		welded wire fabric reinforcement and supports
		<ul><li>12. Indicate where pressure treated wood will be placed</li><li>13. Provide insulation R value for the following:</li></ul>
		a. Attic space
		b. Exterior wall cavity
1000		c. Crawl space (if applicable)
	0	c) Metal frame wall and roof (designed, signed and sealed by Florida Prof
		Engineer of Architect)
0	0	Floor Framing System:
Ц	u	<ul> <li>a) Floor truss package including layout and details, signed and sealed by Florida Registered Professional Engineer</li> </ul>
	0	b) Floor joist size and spacing
	ā	c) Girder size and spacing
		d) Attachment of joist to girder
		e) Wind load requirements where applicable
		Plumbing Fixture layout
		Electrical layout including:
	0	a) Switches, outlets/receptacles, lighting and all required GFCI outlets identified
0	0	b) Ceiling fans
		c) Smoke detectors
	0	d) Service panel and sub-panel size and location(s)
		e) Meter location with type of service entrance (overhead or underground)
0	0	i) Appliances and HVAC equipment
0 0	0	g) Arc Fault Circuits (AFCI) in bedrooms
П	Ц	h) Exhaust fans in bathroom
0		HVAC information
	٥	a) Energy Calculations (dimensions shall match plans)
0	0	b) Manual J sizing equipment or equivalent computation
	Ö	c) Gas System Type (LP or Natural) Location and BTU demand of equipment  Disclosure Statement for Owner Builders
0	Ö	*** Notice Of Commencement Pegnined Poly
Ö	Ö	*** Notice Of Commencement Required Before Any Inspections Will Be Done Private Potable Water
V-12-0-1	-	

a. Attic space
b. Exterior wall cavity
c. Crawl space (if applicable)



**Project Information for:** 

L265563

Builder:

Compass Builders

Lot:

32-1

Subdivision:

Rolling Meadows

County: Truss Count: Baker

Design Program: MiTek 20/20 6.3

**Building Code:** 

FBC2004/TPI2002

**Truss Design Load Information:** Gravity:

Wind:

Roof (psf): 42.0

Wind Standard: ASCE 7-02

Wind Exposure: B

Floor (psf): N/A

T19G

J1925175

28

Wind Speed (mph): 110

Note: See the individual truss drawings for special loading conditions.

Contractor of Record, responsible for the Structural Engineering:

Jacob C. Kirsch Florida License No. CBC1253775

Address: 196 Southwest Huntsview Way Lake City, Florida 32024

Truss Design Engineer: Julius Lee, PE Florida P.E. License No. 34869

Address: 1109 Coastal Bay Blvd. Boynton Beach, FL 33435

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1-2002 Section 2.2

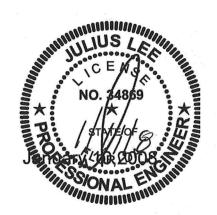
2. The seal date shown on the individual truss component drawings must match the seal date on this index sheet.

3. The Truss Design Engineer's responsibility relative to this structure consists solely of the design of the individual truss components and does not include the design of any additional structural elements including but not limited to continuous lateral bracing elelments in the web and chord planes. See Florida Administrative Code 61G15-31.003 sections 3 c) & 5 and Chapter 2 of the National Design Standard for Metal Plate Connected Wood Truss Construction ANSI/TPI 1-2002 for additional information on the responsibilities of the delegated "Truss Design Engineer". Builders FirstSource and Julius Lee, PE do not accept any additional delegations beyond the scope of work described in the referenced documents above.

> Date 1/10/08 1/10/08 1/10/08 1/10/08 1/10/08 1/10/08 1/10/08 1/10/08 1/10/08 1/10/08 1/10/08 1/10/08 1/10/08 1/10/08 1/10/08 1/10/08 1/10/08 1/10/08 1/10/08

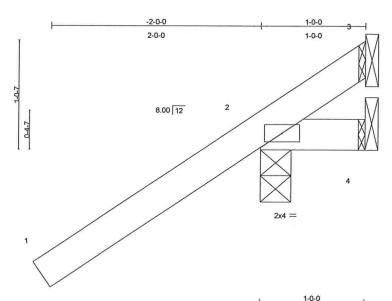
No.	Drwg. #	Truss ID	Date	No.	Drwg. #	Truss ID	
1	J1925148	CJ01	1/10/08	29	J1925176	T20	
2	J1925149	CJ02	1/10/08	30	J1925177	T21	
3	J1925150	CJ03	1/10/08	31	J1925178	T22G	
4	J1925151	EJ01	1/10/08	32	J1925179	T23G	T
5	J1925152	EJ02	1/10/08	33	J1925180	T24	
6	J1925153	HJ01	1/10/08	34	J1925181	T25	
7	J1925154	P01	1/10/08	35	J1925182	T26	
8	J1925155	P02	1/10/08	36	J1925183	T27	
9	J1925156	P03	1/10/08	37	J1925184	T28	Ì
10	J1925157	T01G	1/10/08	38	J1925185	T30	
11	J1925158	T02G	1/10/08	39	J1925186	T31	Ī
12	J1925159	T03	1/10/08	40	J1925187	T32	Ī
13	J1925160	T04	1/10/08	41	J1925188	T33	
14	J1925161	T05	1/10/08	42	J1925189	T34	Ī
15	J1925162	T06	1/10/08	43	J1925190	T35	
16	J1925163	T07	1/10/08	44	J1925191	T36	
17	J1925164	T08G	1/10/08	45	J1925192	T37	
18	J1925165	T09	1/10/08	46	J1925193	T38	
19	J1925166	T10	1/10/08	47	J1925194	T39	
20	J1925167	T11	1/10/08				
21	J1925168	T12	1/10/08				
22	J1925169	T13	1/10/08				
23	J1925170	T14	1/10/08				
24	J1925171	T15	1/10/08				
25	J1925172	T16	1/10/08				
26	J1925173	T17	1/10/08				
27	J1925174	T18	1/10/08				

1/10/08



Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:29:59 2008 Page 1



1-0-0 LOADING (psf) **SPACING** 2-0-0 CSI **DEFL** in (loc) I/defl L/d **PLATES** GRIP 20.0 TCLL 1.25 Plates Increase TC 0.31 Vert(LL) -0.00 >999 360 MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.01 -0.002 >999 Vert(TL) 240 **BCLL** 10.0 Rep Stress Incr WB 0.00 3 YES Horz(TL) 0.00 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 7 lb

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=270/0-3-8, 4=5/Mechanical, 3=-100/Mechanical

Max Horz 2=119(load case 6)

Max Uplift 2=-312(load case 6), 3=-100(load case 1)

Max Grav 2=270(load case 1), 4=14(load case 2), 3=161(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-90/111

BOT CHORD 2-4=0/0

#### JOINT STRESS INDEX

2 = 0.32

#### **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 312 lb uplift at joint 2 and 100 lb uplift at joint 3. Continued on page 2

Julius Lee Trues Design Engineer Florida PE No. 24868 1 109 Coastal Bay Blyd Boynton Beach, Ft. 25436

January 10,2008

Scale = 1:10.4

Marming - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265563	CJ01	JACK	2	1	J	11925148
	1	COO VINDO 8			Job Reference (optional)	

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:29:59 2008 Page 2

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 34866 †100 Coesial Bay Blyd (Boynton Beach, Ft. 03436

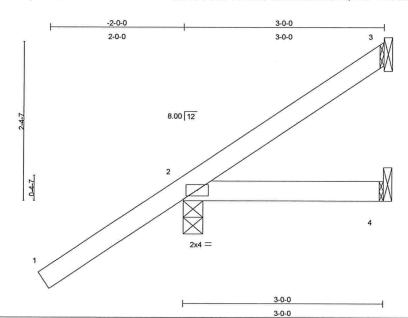
January 10,2008



	Job	Truss	Truss Type	Qty	Ply	00
-	L265563	CJ02	JACK	2	1	J1925149
						Job Reference (optional)

Builders First Source, Jacksonville ,Florida 32244

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LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/Ti	2-0-0 1.25 1.25 YES PI2002	CSI TC BC WB (Mate	0.33 0.06 0.00 rix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.00 -0.01 -0.00	(loc) 2-4 2-4 3	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 14 lb	<b>GRIP</b> 244/190
---	---	--	--------------------------------	------------------------------	--	-------------------------------	--------------------------	-------------------------------	--------------------------	---------------------------------	---------------------

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

3-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=27/Mechanical, 2=258/0-3-8, 4=14/Mechanical

Max Horz 2=179(load case 6)

Max Uplift 3=-33(load case 7), 2=-207(load case 6)

Max Grav 3=34(load case 4), 2=258(load case 1), 4=42(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-72/19

2-4=0/0 **BOT CHORD** 

#### JOINT STRESS INDEX

2 = 0.27

#### NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 3 and 207 lb uplift at joint 2. Continued on page 2

January 10,2008

Scale = 1:16.2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



	Job	Truss	Truss Type	Qty	Ply	0 0	٦
2 %	L265563	CJ02	JACK	2	1	J1925149	)
	,					Job Reference (optional)	

Builders First Source, Jacksonville ,Florida 32244

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:29:59 2008 Page 2

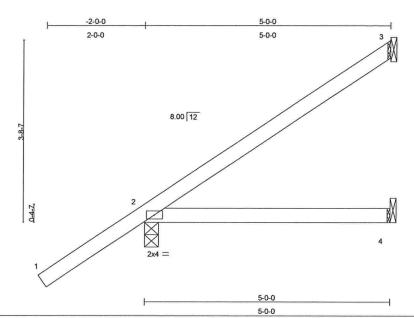
LOAD CASE(S) Standard

January 10,2008





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LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/TI	2-0-0 1.25 1.25 YES PI2002	CSI TC BC WB (Mate	0.33 0.16 0.00 rix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.03 -0.05 -0.00	(loc) 2-4 2-4 3	I/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 20 lb	<b>GRIP</b> 244/190
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LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=101/Mechanical, 2=302/0-3-8, 4=24/Mechanical

Max Horz 2=240(load case 6)

Max Uplift 3=-103(load case 6), 2=-188(load case 6)

Max Grav 3=101(load case 1), 2=302(load case 1), 4=72(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-90/43

BOT CHORD 2-4=0/0

# **JOINT STRESS INDEX**

2 = 0.30

## **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 103 lb uplift at joint 3 and 188 lb uplift at joint 2. Continued on page 2

Truss Design Engineer Florida PE No. 34869 1 100 Coastal Bay Blvd Boynton Besch, FL 33435

January 10,2008

Scale = 1:22.1

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TP1 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	00	
L265563	CJ03	JACK	2	1	5	J1925150
					Job Reference (optional)	

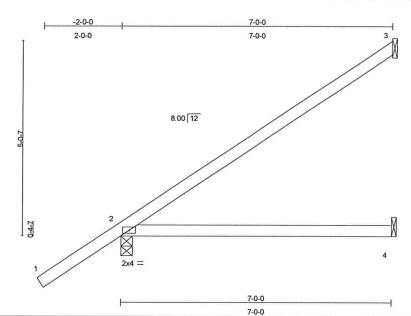
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:00 2008 Page 2

LOAD CASE(S) Standard



	Job	Truss	Truss Type	Qty	Ply	0 0	
į.	L265563	EJ01	MONO TRUSS	3	1	J19251	51
						Job Reference (optional)	

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.45	Vert(LL)	-0.11	2-4	>730	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.33	Vert(TL)	-0.20	2-4	>417	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 27 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS

(lb/size) 3=164/Mechanical, 2=357/0-3-8, 4=34/Mechanical

Max Horz 2=218(load case 6)

Max Uplift 3=-110(load case 6), 2=-124(load case 6)

Max Grav 3=164(load case 1), 2=357(load case 1), 4=102(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-138/72

BOT CHORD 2-4=0/0

## JOINT STRESS INDEX

2 = 0.34

# NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 3 and 124 lb uplift at joint 2.

Julius Lee Truse Design Engineer Florida PE No. 34888 1109 Coestel Bay Blvd Boynton Beach, FL 30435

January 10,2008

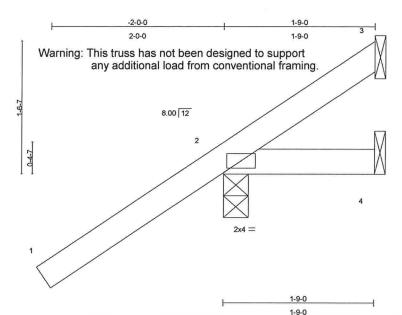
Scale = 1:28.0

# LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00
L265563	EJ02	MONO TRUSS	4	1	J1925152
					Job Reference (optional)

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LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.00	2	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.02	Vert(TL)	-0.00	2	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	3	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 10 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 BRACING

TOP CHORD

Structural wood sheathing directly applied or

1-9-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=239/0-3-8, 4=9/Mechanical, 3=-24/Mechanical

Max Horz 2=142(load case 6)

Max Uplift 2=-233(load case 6), 3=-24(load case 1)

Max Grav 2=239(load case 1), 4=26(load case 2), 3=55(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-67/37

BOT CHORD 2-4=0/0

## JOINT STRESS INDEX

2 = 0.27

#### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 2 and 24 lb uplift at joint 3. Continued on page 2

Truse Design Engineer Florida FE No. 34888 1109 Coestal Bey Blvd Boynton Beach, FL 33435

January 10,2008

Scale = 1:12.6

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Ì	Job	Truss	Truss Type	Qty	Ply	00	
A.	L265563	EJ02	MONO TRUSS	4	1	J	11925152
						Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:01 2008 Page 2

LOAD CASE(S) Standard

Julius Les Truse Design Endineer Flonds FE No. 34888 1109 Coastal Bay Blvd Boynton Beach, FL 33435



	Job	Truss	Truss Type	Qty	Ply	00	
4	L265563	HJ01	MONO TRUSS	1	1		J1925153
						Job Reference (optional)	

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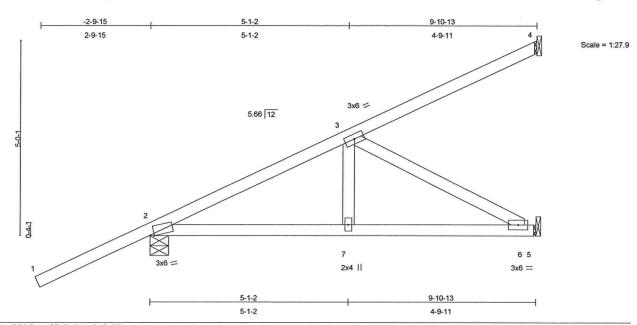


Plate Of	fsets (X,Y	): [2:0-1-4,0-0-7]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.56	Vert(LL)	-0.04	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.06	6-7	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.22	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 46 lb	

LUMBER		BRACING	
TOP CHORD 2 X 4 SYP	No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD 2 X 4 SYP	No.2		6-0-0 oc purlins.
WEBS 2 X 4 SYP	No.3	<b>BOT CHORD</b>	Rigid ceiling directly applied or 10-0-0 oc
			bracing.

**REACTIONS** (lb/size) 4=234/Mechanical, 2=460/0-5-11, 5=251/Mechanical

Max Horz 2=367(load case 5)

Max Uplift 4=-238(load case 5), 2=-242(load case 5), 5=-109(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD BOT CHORD 1-2=0/66, 2-3=-466/0, 3-4=-134/73 2-7=-243/394, 6-7=-243/394, 5-6=0/0

WEBS

3-6=-447/276, 3-7=0/186

## **JOINT STRESS INDEX**

2 = 0.85, 3 = 0.13, 6 = 0.12 and 7 = 0.13

## NOTES

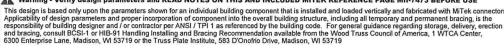
- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; Lumber DOL=1.60 plate grip DOL=1.60.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 238 lb uplift at joint 4, 242 lb uplift at joint 2 and 109 lb uplift at joint 5.

Continued on page 2

Truss Cresion Endineer Florida PE No. 34868 1109 Coastal Boy Blyd Boynton Seach, Ft. 33435

January 10,2008

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Job	Truss	Truss Type	Qty	Ply	00	
, L265563	HJ01	MONO TRUSS	1	1	J1925153	3
L200000	11001	MONO TROSS			Job Reference (optional)	

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## **NOTES**

5) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

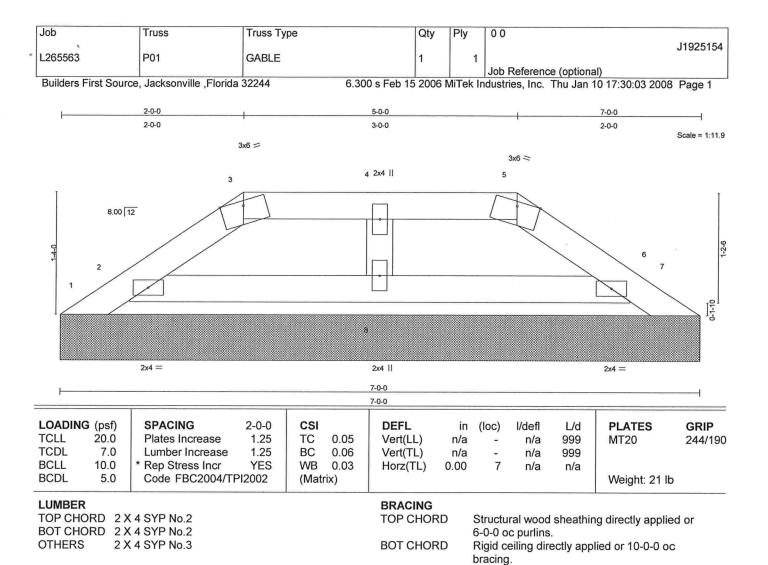
Uniform Loads (plf)

Vert: 1-2=-54 Trapezoidal Loads (plf)

Vert: 2=-3(F=25, B=25)-to-4=-134(F=-40, B=-40), 2=0(F=5, B=5)-to-5=-25(F=-7, B=-7)

Julius Les Truss Cosion Engineer Planda PE No. 34868 1100 Coastal Bay Blvd





REACTIONS (lb/size) 1=-9/7-0-0, 7=-9/7-0-0, 2=146/7-0-0, 6=146/7-0-0, 8=142/7-0-0

Max Horz 1=-33(load case 4)

Max Uplift 1=-42(load case 4), 7=-32(load case 11), 2=-90(load case 5), 6=-71(load

case 4), 8=-37(load case 5)

Max Grav 1=53(load case 5), 7=33(load case 4), 2=170(load case 10), 6=170(load case 11), 8=142(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-54/53, 2-3=-111/91, 3-4=-69/89, 4-5=-69/89, 5-6=-111/91, 6-7=-16/32

**BOT CHORD** 2-8=-29/69, 6-8=-29/69

**WEBS** 

4-8=-108/124

## JOINT STRESS INDEX

2 = 0.18, 3 = 0.03, 4 = 0.07, 5 = 0.03, 6 = 0.18 and 8 = 0.07

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail" Continued on page 2

January 10,2008

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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- [-	Job	Truss	Truss Type	Qty	Ply	00
*	_265563	P01	GABLE	1	1	J1925154
				**		Job Reference (optional)

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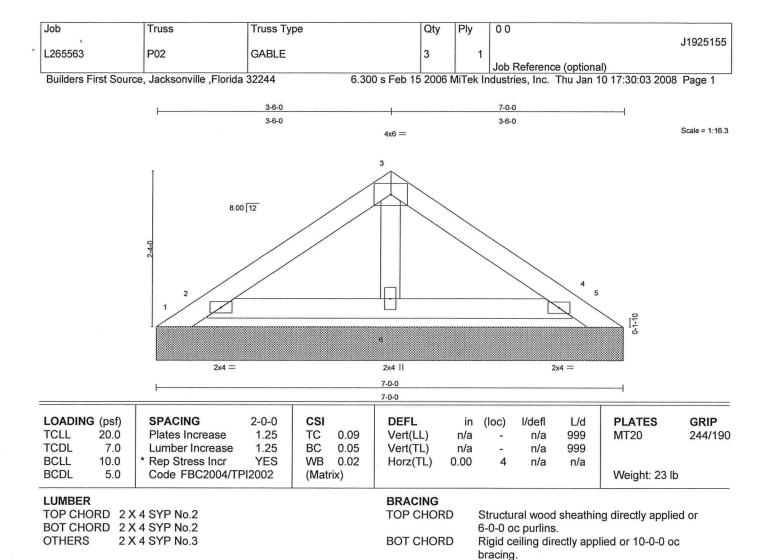
### **NOTES**

- 4) Provide adequate drainage to prevent water ponding.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 4-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 1, 32 lb uplift at joint 7, 90 lb uplift at joint 2, 71 lb uplift at joint 6 and 37 lb uplift at joint 8.
- 10) SEE MITER STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Les Truse Design Engineer Flonda PE No. 24969 1100 Caastal Bay Blvd Bovnion Beach, Ft. 28435





REACTIONS (lb/size) 1=-72/7-0-0, 5=-72/7-0-0, 2=214/7-0-0, 4=214/7-0-0, 6=131/7-0-0

Max Horz 1=60(load case 5)

Max Uplift 1=-72(load case 1), 5=-72(load case 1), 2=-118(load case 6), 4=-108(load case 7)

Max Grav 1=78(load case 5), 5=56(load case 7), 2=214(load case 1), 4=214(load case 1), 6=131(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-90/79, 2-3=-68/46, 3-4=-68/46, 4-5=-46/51

BOT CHORD 2-6=-11/37, 4-6=-11/37

WEBS 3-6=-97/43

## **JOINT STRESS INDEX**

2 = 0.22, 3 = 0.02, 4 = 0.22 and 6 = 0.03

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail" Continued on page 2

Truse Design Engineer Florida FE No. 3-1869 1100 Gestal Bay Blvd Boynton Besch, FL 33-435

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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ĺ	Job	Truss	Truss Type	Qty	Ply	00
ě	L265563	P02	GABLE	3	1	J1925155
						Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:03 2008 Page 2

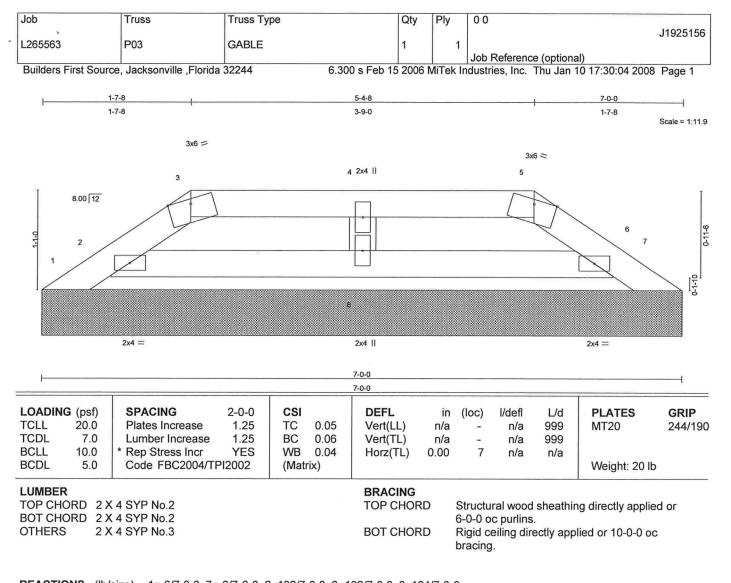
### **NOTES**

- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 4-0-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 72 lb uplift at joint 1, 72 lb uplift at joint 5, 118 lb uplift at joint 2 and 108 lb uplift at joint 4.
- 9) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Les Truse Design Engineer Florida PE No. 34888 1 109 Crastel Bay Blvd Bovnton Beach, Ft. 33435





**REACTIONS** (lb/size) 1=-6/7-0-0, 7=-6/7-0-0, 2=132/7-0-0, 6=132/7-0-0, 8=164/7-0-0

Max Horz 1=-26(load case 4)

Max Uplift 1=-22(load case 4), 7=-16(load case 11), 2=-69(load case 5), 6=-52(load

case 4), 8=-49(load case 5)

Max Grav 1=34(load case 5), 7=17(load case 4), 2=143(load case 10), 6=143(load

case 11), 8=164(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-39/37, 2-3=-100/90, 3-4=-62/81, 4-5=-62/81, 5-6=-100/90, 6-7=-8/24

BOT CHORD

2-8=-34/62, 6-8=-34/62

**WEBS** 

4-8=-129/151

## JOINT STRESS INDEX

2 = 0.15, 3 = 0.03, 4 = 0.08, 5 = 0.03, 6 = 0.15 and 8 = 0.08

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

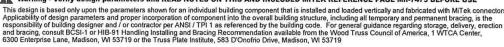
2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Truss designed for wind loads in the plane of the truss only. For stude exposed to wind (normal to the face), see MiTek "Standard Gable End Detail" Continued on page 2

Julius Les Truse Cesign Engineer Florida PE No. 34888 1199 Ceastal Bay Blvd Boynton Beach, FL 33435

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





Job	Truss	Truss Type	Qty	Ply	00
L265563	P03	GABLE	î	1	J1925156
			i, •.	,,•,	Job Reference (optional)

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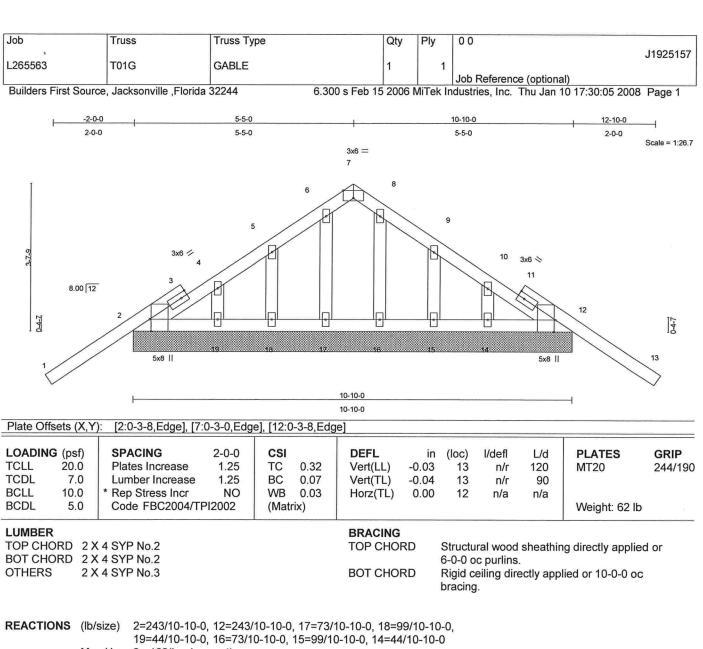
### **NOTES**

- 4) Provide adequate drainage to prevent water ponding.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 4-0-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 1, 16 lb uplift at joint 7, 69 lb uplift at joint 2, 52 lb uplift at joint 6 and 49 lb uplift at joint 8.
- 10) SEE MITEK STANDARD PIGGYBACK TRUSS CONNECTION DETAIL FOR CONNECTION TO BASE TRUSS

LOAD CASE(S) Standard

Julius Lee Trues Cesign Engineer Flonda PE No. 34858 1100 Ceastel Bay Blvd





Max Horz 2=-122(load case 4)

Max Uplift 2=-221(load case 6), 12=-235(load case 7), 17=-18(load case 5),

18=-111(load case 6), 19=-16(load case 7), 15=-114(load case 7),

14=-12(load case 6)

Max Grav 2=243(load case 1), 12=243(load case 1), 17=73(load case 1), 18=101(load

case 10), 19=68(load case 2), 16=73(load case 1), 15=101(load case 11),

14=68(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-74/74, 3-4=-67/84, 4-5=-51/70, 5-6=-37/106, 6-7=-31/100.

7-8=-31/100, 8-9=-37/106, 9-10=-29/39, 10-11=-29/45, 11-12=-56/35, 12-13=0/61

BOT CHORD 2-19=-6/132, 18-19=-6/132, 17-18=-6/132, 16-17=-6/132, 15-16=-6/132,

14-15=-6/132, 12-14=-6/132

WEBS 6-17=-61/27, 5-18=-82/110, 4-19=-43/36, 8-16=-61/1, 9-15=-82/110, 10-14=-43/31

# JOINT STRESS INDEX

2 = 0.46, 3 = 0.00, 3 = 0.16, 4 = 0.02, 5 = 0.06, 6 = 0.03, 7 = 0.12, 8 = 0.03, 9 = 0.06, 10 = 0.02, 11 = 0.00, 11 = 0.16, 12 = 0.46, 14 = 0.02, 15 = 0.06, 16 = 0.02, 17 = 0.02, 18 = 0.06 and 19 = 0.02

Continued on page 2



	Job	Truss	Truss Type	Qty	Ply	0.0
ų.	L265563	T01G	GABLE	1	1	J1925157
						Job Reference (optional)

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#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 221 lb uplift at joint 2, 235 lb uplift at joint 12, 18 lb uplift at joint 17, 111 lb uplift at joint 18, 16 lb uplift at joint 19, 114 lb uplift at joint 15 and 12 lb uplift at joint 14.

LOAD CASE(S) Standard

Julius Les Truse Design Engineer Florida PE No. 3-1869 1 100 Coastal Bay Blvd



Job Truss Truss Type Qty Ply 00 J1925158 L265563 T02G **GABLE** Job Reference (optional)

Builders First Source, Jacksonville ,Florida 32244

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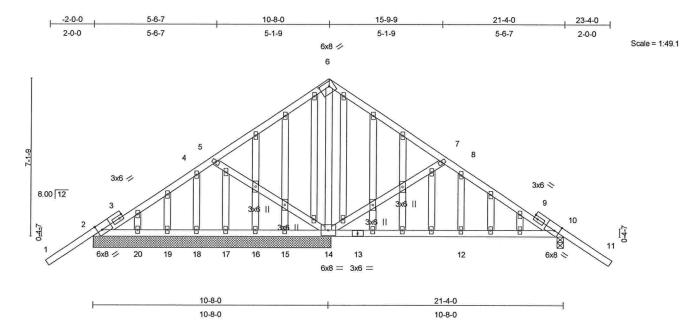


Plate Offsets (X,Y):	[2:0-2-9,Edge], [6:0	-5-4,0-2-8]	, [10:0-2-	9,Edge]						
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 10.0 BCDL 5.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/Tl	2-0-0 1.25 1.25 NO PI2002	CSI TC BC WB (Mate	0.64 0.22 0.74 rix)	DEFL Vert(LL) Vert(TL) Horz(TL)	 (loc) 12-14 12-14 10	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 175 lb	<b>GRIP</b> 244/190

LUMBER TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3 2 X 4 SYP No.3 **OTHERS** 

BRACING TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.

**BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=567/10-9-8, 14=1392/10-9-8, 15=-4/10-9-8, 16=21/10-9-8, 17=9/10-9-8, 18=293/10-9-8, 19=-16/10-9-8, 20=86/10-9-8, 10=462/0-3-8

Max Horz 2=-243(load case 4)

Max Uplift 2=-387(load case 6), 14=-1071(load case 7), 15=-17(load case 2), 16=-16(load case 5), 18=-204(load case 7), 19=-17(load case 10), 20=-52(load case 7), 10=-445(load case 7)

Max Grav 2=599(load case 10), 14=1392(load case 1), 15=69(load case 5), 16=64(load case 2), 17=32(load case 2), 18=293(load case 1), 19=32(load case 7), 20=90(load case 10), 10=485(load case 11)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-79/128, 2-3=-323/172, 3-4=-208/106, 4-5=-220/162, 5-6=-237/347, 6-7=-275/378,

7-8=-319/353, 8-9=-378/503, 9-10=-476/474, 10-11=0/62

**BOT CHORD** 2-20=-177/188, 19-20=-177/188, 18-19=-177/188, 17-18=-177/188, 16-17=-177/188,

15-16=-177/188, 14-15=-177/188, 13-14=-258/356, 12-13=-258/356, 10-12=-258/356

**WEBS** 5-14=-312/401, 6-14=-900/860, 7-14=-547/783, 4-18=-272/235, 8-12=-275/173 Engineer 2. 34868 Bay Blvd -h. ft. 00406

### JOINT STRESS INDEX

2 = 0.70, 3 = 0.00, 3 = 0.45, 4 = 0.34, 5 = 0.42, 6 = 0.94, 7 = 0.42, 8 = 0.34, 9 = 0.00, 9 = 0.45, 10 = 0.70, 12 = 0.34, 13 = 0.15, 14 = 0.150.25, 15 = 0.34, 16 = 0.34, 17 = 0.34, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.61, 24 = 0.34, 25 = 0.61, 26 = 0.34, 26 = 0.34, 27 = 0.34, 28 = 0.34, 29 = 0.34, 29 = 0.34, 20 = 0.327 = 0.34, 28 = 0.34, 29 = 0.34, 30 = 0.34, 31 = 0.34, 32 = 0.34, 33 = 0.34, 34 = 0.61, 35 = 0.34, 36 = 0.34, 37 = 0.61, 38 = 0.34Continued on page 241 = 0.34, 42 = 0.34 and 43 = 0.34

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connec Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occ. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



	Job	Truss	Truss Type	Qty	Ply	0 0	
40	L265563	T02G	GABLE	1	1	J1925	5158
						Job Reference (optional)	

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#### NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 387 lb uplift at joint 2, 1071 lb uplift at joint 14, 17 lb uplift at joint 15, 16 lb uplift at joint 16, 204 lb uplift at joint 18, 17 lb uplift at joint 19, 52 lb uplift at joint 20 and 445 lb uplift at joint 10.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

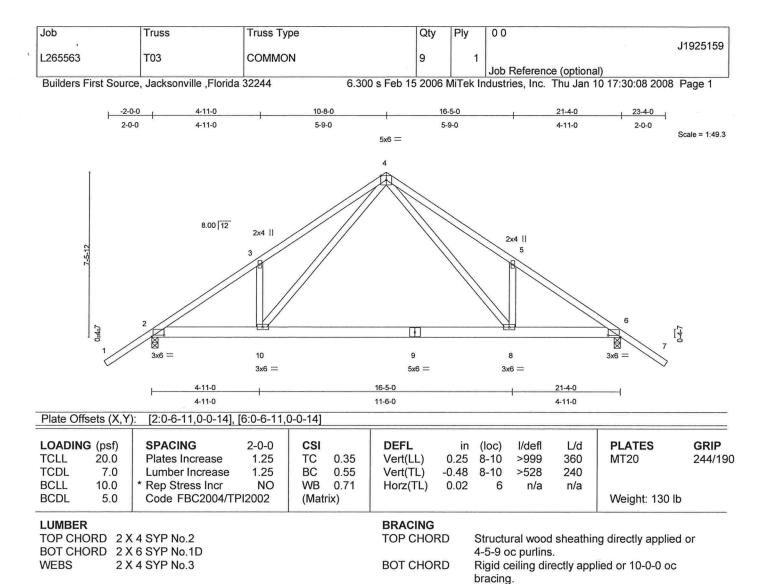
## LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-6=-114(F=-60), 6-7=-141(F=-87), 7-11=-54, 2-10=-10

Julius Lær Truss Design Enginser Florida PE No. 34969 1109 Coastal Bay Blyd





**REACTIONS** (lb/size) 2=1139/0-3-8, 6=1139/0-3-8

Max Horz 2=-194(load case 4)

Max Uplift 2=-336(load case 6), 6=-336(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/65, 2-3=-1810/731, 3-4=-1793/903, 4-5=-1793/903, 5-6=-1810/731, 6-7=0/65

**BOT CHORD** 2-10=-427/1429, 9-10=-116/797, 8-9=-116/797, 6-8=-427/1429 **WEBS** 3-10=-252/259, 4-10=-493/1023, 4-8=-493/1023, 5-8=-252/259

## JOINT STRESS INDEX

2 = 0.78, 3 = 0.33, 4 = 0.50, 5 = 0.33, 6 = 0.78, 8 = 0.69, 9 = 0.86 and 10 = 0.69

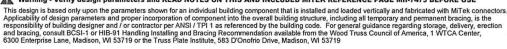
#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE





Job	Truss	Truss Type	Qty	Ply	00
L265563	T03	COMMON	a	1	J1925159
220000	100	COMMON	,		Job Reference (optional)

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### **NOTES**

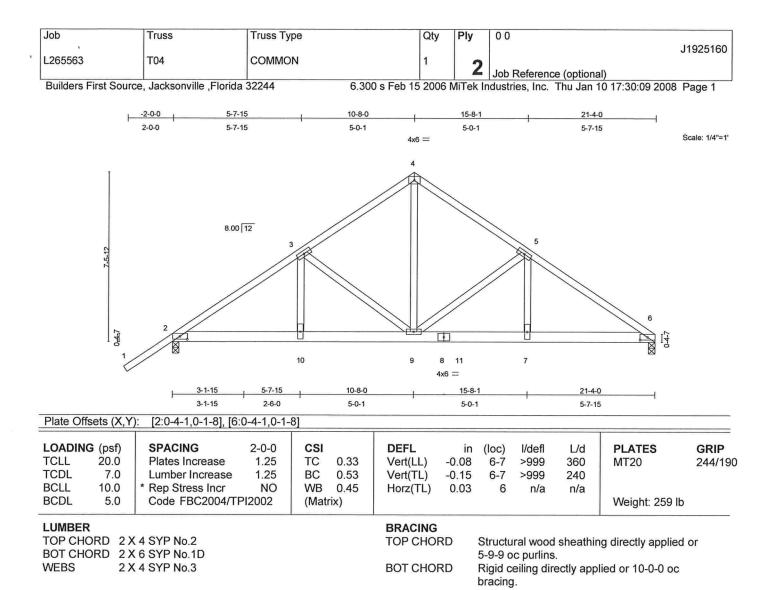
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 336 lb uplift at joint 2 and 336 lb uplift at joint 6.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-7=-54, 2-10=-10, 8-10=-70(F=-60), 6-8=-10





**REACTIONS** (lb/size) 6=3933/0-3-8, 2=1629/0-3-8

Max Horz 2=223(load case 4)

Max Uplift 6=-1103(load case 6), 2=-487(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/65, 2-3=-2424/606, 3-4=-2197/641, 4-5=-2202/633, 5-6=-4803/1329

BOT CHORD 2-10=-496/1935, 9-10=-496/1935, 8-9=-1036/3941, 8-11=-1036/3941,

7-11=-1036/3941, 6-7=-1036/3941

WEBS 3-10=-18/143, 3-9=-223/109, 4-9=-603/2090, 5-9=-2729/878, 5-7=-784/2784

# JOINT STRESS INDEX

2 = 0.73, 3 = 0.81, 4 = 0.54, 5 = 0.81, 6 = 0.73, 7 = 0.44, 8 = 0.54, 9 = 0.64 and 10 = 0.44

#### **NOTES**

 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads Connections have been provided to distribute only loads.

Truss Design Engineer Florida PE No. 34869 1 100 Coastal Bay Blvd Boynton Beach, Ft. 35436

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors, Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	٦
L265563	T04	COMMON	1	2	J1925160	0
		9			Job Reference (optional)	- 1

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### **NOTES**

- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are 3x8 MT20 unless otherwise indicated.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1103 lb uplift at joint 6 and 487 lb uplift at joint 2.

## LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-4=-54, 4-6=-54, 2-11=-10, 6-11=-491(F=-481)

Julius Les Truss Design Engineer Florida ME No. 34866 1103 Coastal Bay Blvd



Job	Truss	Truss Type	Qty	Ply	00
L265563	T05	COMMON	1	1	J1925161
				725	Job Reference (optional)

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Scale = 1:47.4

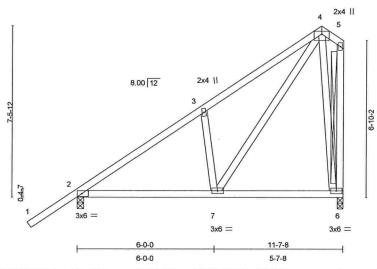


Plate Of	fsets (X, Y	<u>/): [2:0-3-9,0-1-8]</u>										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.02	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.05	2-7	>999	240	100000000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.35	Horz(TL)	0.00	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 79 lb	

# LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

**WEBS** 2 X 4 SYP No.3 \*Except\*

5-6 2 X 4 SYP No.2

## BRACING

**BOT CHORD** 

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing. **WEBS** 

T-Brace:

2 X 4 SYP No.3 - 5-6

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance. Brace must cover 90% of web length.

**REACTIONS** (lb/size) 2=495/0-3-8, 6=351/0-3-0

Max Horz 2=286(load case 6)

Max Uplift 2=-141(load case 6), 6=-144(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-439/27, 3-4=-392/176, 4-5=-47/54, 5-6=-88/79

**BOT CHORD** 2-7=-226/294, 6-7=-41/49

**WEBS** 3-7=-265/281, 4-7=-270/384, 4-6=-389/350

# JOINT STRESS INDEX

2 = 0.41, 3 = 0.15, 4 = 0.27, 5 = 0.25, 6 = 0.23 and 7 = 0.27

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	٦
L265563	T05	COMMON	1	1	J1925161	
2200000	100	COMMON			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:10 2008 Page 2

#### NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 2 and 144 lb uplift at joint 6.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Horida PE No. 24869 1109 Ceastel Bay Blvri Boynton Beach, Ft. 38436



Job	Truss	Truss Type	Qty	Ply	00
1 265563	T06	MONO HIP	1	1	J1925162
L203303	100	Micros Till	' I		Job Reference (optional)

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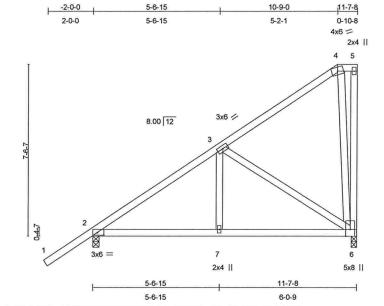


Plate Of	fsets (X, Y	<u>(): [2:0-3-9,0-1-8]</u>										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.03	6-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.05	6-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 77 lb	

# LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

# BRACING

TOP CHORD

BOT CHORD F

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 6=351/0-3-0, 2=495/0-3-8

Max Horz 2=300(load case 6)

Max Uplift 6=-153(load case 6), 2=-133(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-443/0, 3-4=-116/39, 4-5=-8/3, 5-6=-113/87

BOT CHORD 2-7=-226/297, 6-7=-226/297

WEBS 3-7=0/193, 3-6=-319/235, 4-6=-240/275

## JOINT STRESS INDEX

2 = 0.32, 3 = 0.14, 4 = 0.44, 5 = 0.21, 6 = 0.11 and 7 = 0.14

#### **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Les Truss Design Engineer Florida PE No. 34869 1109 Ceastel Bay Blvd. Boymon Beach, Ft. 22435

January 10,2008

Scale: 1/4"=1"

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00
L265563	T06	MONO HIP	1	1	J1925162
220000	100	Morto IIII		'	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:10 2008 Page 2

## **NOTES**

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 153 lb uplift at joint 6 and 133 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Truse Design Engineer Florida PE No. 24969 1109 Coasial Bay Slvd



Job	Truss	Truss Type	Qty	Ply	00
L265563	T07	MONO TRUSS	8	4	J1925163
L200000	107	Michael Michael	ľ	L . '	Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:11 2008 Page 1

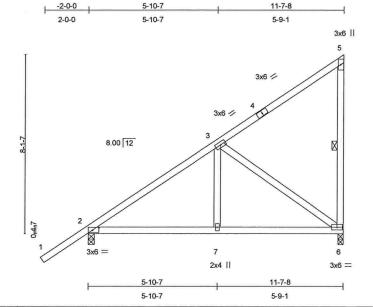


Plate Of	fsets (X,Y	<u>(): [2:0-3-9,0-1-8]</u>		1							1	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.02	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.17	Vert(TL)	-0.04	2-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.28	Horz(TL)	0.01	6	n/a	n/a		
BCDL	CDL 5.0 Code FBC2004/TPI2002		(Mat	rix)				ध्या सर		Weight: 68 lb		

LUMBE	0

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

# BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc

BOT CHORD

**WEBS** 

bracing.

1 Row at midpt

5-6

REACTIONS (lb/size) 6=351/0-3-0, 2=495/0-3-8

Max Horz 2=315(load case 6)

Max Uplift 6=-171(load case 6), 2=-123(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-437/0, 3-4=-109/0, 4-5=-78/59, 5-6=-125/137

BOT CHORD 2-7=-225/291, 6-7=-225/291 WEBS 3-7=0/193, 3-6=-341/262

# **JOINT STRESS INDEX**

2 = 0.36, 3 = 0.14, 4 = 0.24, 5 = 0.23, 6 = 0.18 and 7 = 0.14

## **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Lee Truss Design Engineer Florids FIE No. 24869 1169 Coestel Bay Blvd Boynton Beach, FL 22435

January 10,2008

Scale = 1:49 4

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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ĺ	Job	Truss	Truss Type	Qty	Ply	00	
	L265563	T07	MONO TRUSS	g.	1	J1925	5163
	L200000	107	MONO TROSS	0		Job Reference (optional)	

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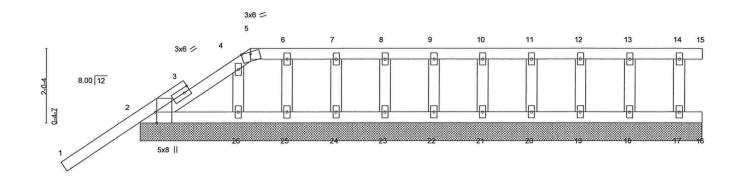
## **NOTES**

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 171 lb uplift at joint 6 and 123 lb uplift at joint 2.

LOAD CASE(S) Standard



Job		Truss	Truss Type	Qty	Ply	00	
L26556	63	T08G	GABLE	1	1	J	1925164
		Mary programme 2.				Job Reference (optional)	
Builde	rs First Source,	Jacksonville ,Florida	32244 6.300 s Feb 1	5 2006 N	∕liTek Ir	dustries, Inc. Thu Jan 10 17:30:12 2008 F	age 1
É	-2-0-0	3-0-0			15-3-8	3	—
	2-0-0	3-0-0			12-3-8		
Builde F	-2-0-0	3-0-0	32244 6.300 s Feb 1	5 2006 N	15-3-8	dustries, Inc. Thu Jan 10 17:30:12 2008 F	Page



15-3-8 15-3-8

Plate Of	fsets (X,Y	): [2:0-3-8,Edge]		r	_	,					·	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.00	1	n/r	120	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.05	Vert(TL)	-0.01	1	n/r	90		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.02	Horz(TL)	-0.00	15	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 73 lb	

LUMBER	BRACING	
TOP CHORD 2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD 2 X 4 SYP No.2		6-0-0 oc purlins.
OTHERS 2 X 4 SYP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc
		bracing.

**REACTIONS** (lb/size) 15=10/15-3-8, 2=239/15-3-8, 16=1/15-3-8, 26=85/15-3-8, 25=88/15-3-8, 24=85/15-3-8, 23=85/15-3-8, 22=85/15-3-8, 21=85/15-3-8, 20=85/15-3-8, 19=85/15-3-8, 18=89/15-3-8, 17=68/15-3-8

Max Horz 2=166(load case 6)

Max Uplift 15=-8(load case 4), 2=-209(load case 6), 26=-29(load case 7), 25=-74(load case 4), 24=-49(load case 4), 23=-54(load case 4), 22=-53(load case 4), 21=-53(load case 4), 20=-53(load case 4), 19=-53(load case 4), 18=-56(load case 4), 17=-41(load case 4)

Max Grav 15=10(load case 11), 2=239(load case 1), 16=4(load case 2), 26=94(load case 2), 25=88(load case 1), 24=86(load case 11), 23=85(load case 1), 22=85(load case 11), 21=85(load case 1), 20=85(load case 11), 19=85(load case 1), 18=89(load case 11), 17=68(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-44/7, 3-4=-29/21, 4-5=-21/7, 5-6=0/0, 6-7=0/0, 7-8=0/0, 8-9=0/0,

9-10=0/0, 10-11=0/0, 11-12=0/0, 12-13=0/0, 13-14=0/0, 14-15=0/0

BOT CHORD 2-26=0/0, 25-26=0/0, 24-25=0/0, 23-24=0/0, 22-23=0/0, 21-22=0/0, 20-21=0/0,

19-20=0/0, 18-19=0/0, 17-18=0/0, 16-17=0/0

4-26=-74/49, 6-25=-73/74, 7-24=-72/59, 8-23=-72/61, 9-22=-72/61, 10-21=-72/61,

11-20=-72/61, 12-19=-71/61, 13-18=-76/64, 14-17=-57/48

Julius Less Trues Design Engineer Florida PE No. 24898 1109 Coastal Bay Blvd Boynton Beach, FL 23435

January 10,2008

Continued on page 2

**WEBS** 

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T08G	GABLE	1	1		J1925164
2200000	1000	GABLE	'		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:12 2008 Page 2

## JOINT STRESS INDEX

2 = 0.44, 3 = 0.00, 3 = 0.15, 4 = 0.04, 5 = 0.04, 6 = 0.04, 7 = 0.03, 8 = 0.03, 9 = 0.03, 10 = 0.03, 11 = 0.03, 12 = 0.03, 13 = 0.04, 14 = 0.03, 17 = 0.03, 18 = 0.04, 19 = 0.03, 20 = 0.03, 21 = 0.03, 22 = 0.03, 23 = 0.03, 24 = 0.03, 25 = 0.04 and 26 = 0.03

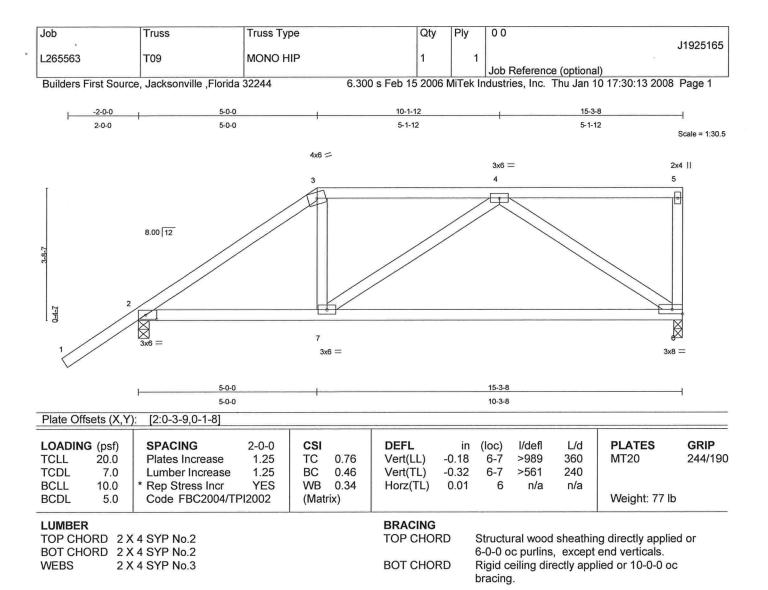
### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Provide adequate drainage to prevent water ponding.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 1-4-0 oc.
- 9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 15, 209 lb uplift at joint 2, 29 lb uplift at joint 26, 74 lb uplift at joint 25, 49 lb uplift at joint 24, 54 lb uplift at joint 23, 53 lb uplift at joint 22, 53 lb uplift at joint 21, 53 lb uplift at joint 20, 53 lb uplift at joint 19, 56 lb uplift at joint 18 and 41 lb uplift at joint 17.

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Flonds PE No. 3-Jees 1189 Coesiel Bey Blyd





REACTIONS (lb/size) 6=471/0-3-0, 2=609/0-3-8

Max Horz 2=177(load case 6)

Max Uplift 6=-141(load case 4), 2=-184(load case 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-667/251, 3-4=-487/263, 4-5=-79/4, 5-6=-127/93

BOT CHORD 2-7=-266/484, 6-7=-298/475

WEBS 3-7=0/193, 4-7=0/132, 4-6=-518/355

## JOINT STRESS INDEX

2 = 0.43, 3 = 0.38, 4 = 0.16, 5 = 0.79, 6 = 0.62 and 7 = 0.12

#### **NOTES**

 Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

 \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Truss Design Engineer Florida FE No. 34969 1100 Casstal Bay Blyd Doynton Geach, FL 23425

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	0 0
L265563	T09	MONO HIP	4	1	J1925165
L200000	109	IMONO HIP	1		Job Reference (optional)

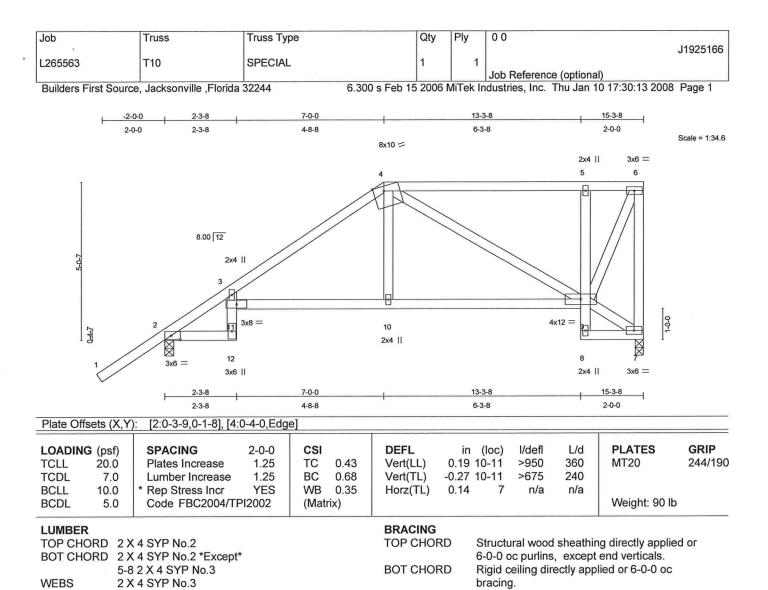
6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:13 2008 Page 2

### NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 141 lb uplift at joint 6 and 184 lb uplift at joint 2.

LOAD CASE(S) Standard





**REACTIONS** (lb/size) 7=471/0-3-0, 2=609/0-3-8

Max Horz 2=220(load case 6)

Max Uplift 7=-132(load case 5), 2=-186(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-661/162, 3-4=-672/313, 4-5=-226/130, 5-6=-203/118, 6-7=-437/251

BOT CHORD 2-12=-247/437, 11-12=-12/84, 3-11=-2/91, 10-11=-348/548, 9-10=-348/554,

8-9=0/19, 5-9=-254/198, 7-8=-59/44

WEBS 4-10=-25/252, 4-9=-380/253, 6-9=-283/484, 7-9=-48/68

## JOINT STRESS INDEX

2 = 0.53, 3 = 0.72, 4 = 0.68, 5 = 0.53, 6 = 0.42, 7 = 0.14, 8 = 0.09, 9 = 0.56, 10 = 0.18, 11 = 0.83 and 12 = 0.36

## NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.
3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other Coliva ueads page 2

Trues Design Endinser Flonda FE No. 34868 1100 Coasial Bay Blvd Boynton Besch, FL 93435

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00
1 265563	T10	SPECIAL	1	1	J1925166
L203303	110	SPECIAL	•		Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:14 2008 Page 2

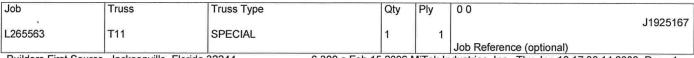
## **NOTES**

4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 132 lb uplift at joint 7 and 186 lb uplift at joint 2.

LOAD CASE(S) Standard





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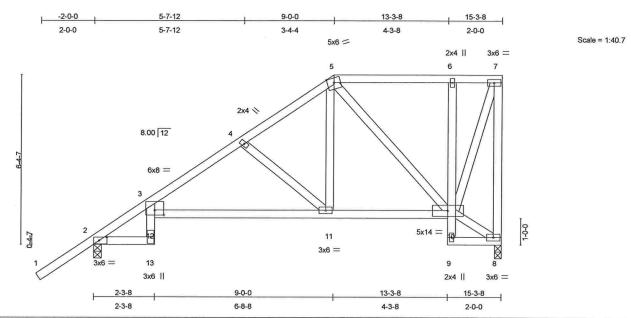


Plate Offsets (X,Y): [2:0-3-9.0-1-8], [3:0-4-0.0-2-3]

TCDL	(psf) 20.0 7.0 10.0 5.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/TF	2-0-0 1.25 1.25 YES PI2002	CSI TC BC WB (Mat	0.50 0.63 0.26 rix)	DEFL Vert(LL) Vert(TL) Horz(TL)	000.000.000	(loc) 11-12 11-12 8	I/defl >999 >647 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 104 lb	<b>GRIP</b> 244/190
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 IRA		
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TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 \*Except\*

6-9 2 X 4 SYP No.3

**WEBS** 

2 X 4 SYP No.3

# BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

**BOT CHORD** 

Rigid ceiling directly applied or 9-1-8 oc

bracing.

REACTIONS (lb/size) 8=471/0-3-0, 2=609/0-3-8

Max Horz 2=263(load case 6)

Max Uplift 8=-133(load case 5), 2=-182(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-648/113, 3-4=-699/298, 4-5=-514/245, 5-6=-152/95, 6-7=-142/89,

7-8=-433/276

**BOT CHORD** 2-13=-258/417, 12-13=-14/90, 3-12=0/156, 11-12=-427/595, 10-11=-237/362,

9-10=0/24, 6-10=-200/150, 8-9=-38/50

**WEBS** 4-11=-291/239, 5-11=-152/347, 5-10=-319/216, 7-10=-276/437, 8-10=-55/42

## JOINT STRESS INDEX

2 = 0.47, 3 = 0.82, 4 = 0.13, 5 = 0.20, 6 = 0.30, 7 = 0.41, 8 = 0.14, 9 = 0.09, 10 = 0.35, 11 = 0.22, 12 = 0.00 and 13 = 0.37

#### NOTES

1) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp 🐇 B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) Provide adequate drainage to prevent water ponding.

Continued on page 2

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00
L265563	T11	SPECIAL	1	1	J1925167
220000		0, 20, 12	,		Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:14 2008 Page 2

### **NOTES**

- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 133 lb uplift at joint 8 and 182 lb uplift at joint 2.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T12	SPECIAL	1	1		J1925168
		0. 202	"	1	Job Reference (optional)	

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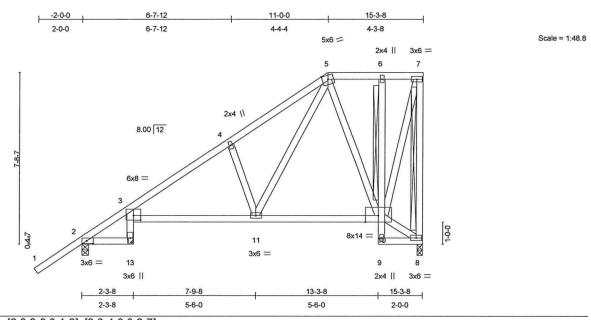


Plate Of	tsets (X,Y	): [2:0-3-9,0-1-8], [3:	0-4-0,0-2-	·/]								
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	0.21	11-12	>851	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.29	11-12	>616	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.29	Horz(TL)	0.14	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 113 lb	

LUMBER		BRACING		
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing	ng directly applied or
BOT CHORD	2 X 4 SYP No.2 *Except*		6-0-0 oc purlins, except	end verticals.
	6-9 2 X 4 SYP No.3	<b>BOT CHORD</b>	Rigid ceiling directly app	lied or 6-0-0 oc
WEBS	2 X 4 SYP No.3		bracing. Except:	
			T-Brace:	2 X 4 SYP No.3 -

6-10

**WEBS** T-Brace: 2 X 4 SYP No.3 - 7-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=471/0-3-0, 2=609/0-3-8

Max Horz 2=305(load case 6)

Max Uplift 8=-139(load case 6), 2=-171(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-657/68, 3-4=-661/225, 4-5=-619/317, 5-6=-110/69, 6-7=-107/69,

7-8=-438/289

**BOT CHORD** 2-13=-283/431, 12-13=-14/86, 3-12=0/115, 11-12=-403/545, 10-11=-165/225,

9-10=0/20, 6-10=-97/69, 8-9=-47/36

**WEBS** 4-11=-308/283, 5-11=-324/488, 5-10=-323/267, 7-10=-259/401, 8-10=-38/52

NT STRESS INDEX
2 = 0.48, 3 = 0.94, 4 = 0.15, 5 = 0.26, 6 = 0.12, 7 = 0.39, 8 = 0.15, 9 = 0.06, 10 = 0.12, 11 = 0.39, 12 = 0.00 and 13 = 0.36

January 10,2008 Continued on page 2

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
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Job	Truss	Truss Type	Qty	Ply	00
L265563	T12	SPECIAL	1	1	J1925168
	112	0. 202			Job Reference (optional)

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## **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 139 lb uplift at joint 8 and 171 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 34868 1166 Coastal Bay Blvd Boynton Beach, FL 33435



Job	Truss	Truss Type	Qty	Ply	0 0
L265563	T13	SPECIAL	1	1	J1925169
L203303	113	OF LOIAL			Job Reference (optional)

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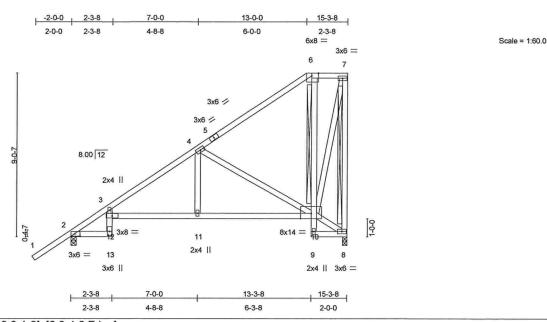


Plate Of	fsets (X,Y	): [2:0-3-9,0-1-8], [6:	0-4-8,Edg	e]			-					
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.41	Vert(LL)	0.20	11-12	>894	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.26	11-12	>685	240	Johnson - Charles	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.14	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Mat	rix)	, , , , ,					Weight: 110 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
<b>BOT CHORD</b>	2 X 4 SYP No.2
WEBS	2 X 4 SYP No.3

**BRACING** TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

**BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc

bracing. Except:

T-Brace:

2 X 4 SYP No.3 -

6-10

**WEBS** 

T-Brace:

2 X 4 SYP No.3 - 7-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 8=471/0-3-0, 2=609/0-3-8

Max Horz 2=349(load case 6)

Max Uplift 8=-185(load case 6), 2=-153(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-661/15, 3-4=-678/175, 4-5=-231/0, 5-6=-128/12, 6-7=-96/69,

7-8=-434/296

**BOT CHORD** 2-13=-299/438, 12-13=-14/83, 3-12=-7/85, 11-12=-414/552, 10-11=-414/552,

9-10=0/18, 6-10=-177/198, 8-9=-75/44

**WEBS** 4-11=-34/255, 4-10=-513/387, 7-10=-317/437, 8-10=-49/86

NT STRESS INDEX
2 = 0.47, 3 = 0.63, 4 = 0.19, 5 = 0.29, 6 = 0.77, 7 = 0.43, 8 = 0.15, 9 = 0.08, 10 = 0.14, 11 = 0.18, 12 = 0.85 and 13 = 0.36

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This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation availed from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T13	SPECIAL	1	4	J19251	169
220000	110	S1 250 12			Job Reference (optional)	

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## **NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 185 lb uplift at joint 8 and 153 lb uplift at joint 2.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00
L265563	T14	MONO HIP	1	1	J1925170
			7.		Job Reference (optional)

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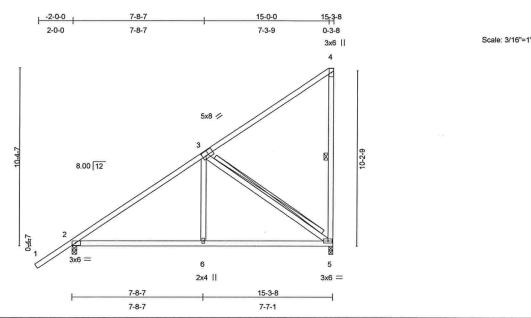


Plate Of	Plate Offsets (X,Y): [2:0-3-9,0-1-8], [3:0-2-8,0-3-0]													
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP		
TCLL	20.0	Plates Increase	1.25	TC	0.40	Vert(LL)	-0.06	2-6	>999	360	MT20	244/190		
TCDL	7.0	Lumber Increase	1.25	BC	0.30	Vert(TL)	-0.13	2-6	>999	240				
BCLL	10.0	* Rep Stress Incr	YES	WB	0.23	Horz(TL)	0.01	5	n/a	n/a				
BCDL			(Mat	rix)						Weight: 89 lb				

•	11	RA	B	_	D

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3

# BRACING

TOP CHORD

**BOT CHORD** 

6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc

Structural wood sheathing directly applied or

bracing.

**WEBS** 

1 Row at midpt

4-5

T-Brace:

2 X 4 SYP No.3 - 3-5

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

**REACTIONS** (lb/size) 5=471/0-3-0, 2=609/0-3-8

Max Horz 2=393(load case 6)

Max Uplift 5=-233(load case 6), 2=-126(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-592/0, 3-4=-141/75, 4-5=-157/165

BOT CHORD 2-6=-299/398, 5-6=-301/395

**WEBS** 3-6=0/261, 3-5=-472/359

# JOINT STRESS INDEX

2 = 0.45, 3 = 0.79, 4 = 0.32, 5 = 0.27 and 6 = 0.18

January 10,2008

Continued on page 2

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00
L265563	T14	MONO HIP	1	1	J1925170
L200000	1.17	WORO TIII			Job Reference (optional)

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## **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 233 lb uplift at joint 5 and 126 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Trues Design Engineer Florida FE No. 34888 1 100 Coastal Bay Blvri



Job	Truss	Truss Type	Qty	Ply	0 0	
L265563	T15	MONO HIP	1	1		J1925171
				- "	Job Reference (optional)	

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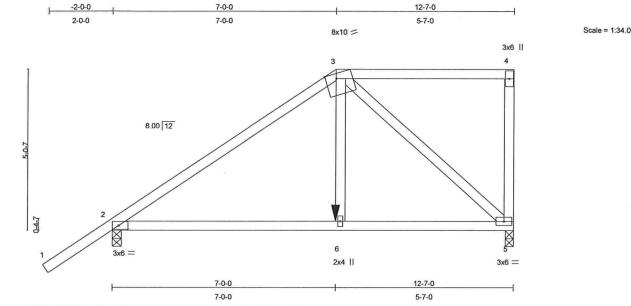


Plate Offsets (X,Y): [2:0-3-9,0-1-8], [3:0-4-0,Edge]

LOADIN		SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.64	Vert(LL)	-0.05	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.29	Vert(TL)	-0.10	2-6	>999	240		
BCLL	10.0	* Rep Stress Incr	NO	WB	0.76	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TPI2002		(Mat	rix)						Weight: 67 lb	

# LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

# BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 5=937/0-3-0, 2=792/0-3-8

Max Horz 2=221(load case 5)

Max Uplift 5=-458(load case 4), 2=-340(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-937/354, 3-4=-53/38, 4-5=-300/245

BOT CHORD 2-6=-340/694, 5-6=-346/710

WEBS 3-6=-189/504, 3-5=-872/431

# **JOINT STRESS INDEX**

2 = 0.55, 3 = 0.82, 4 = 0.48, 5 = 0.41 and 6 = 0.36

#### NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 458 Colhingid aniping and 340 lb uplift at joint 2.

Trues Design Engineer Florida FE No. 34864 1 199 Geastal Bay Blvd Boynton Beach, FL 33436

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 Dronfrie Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T15	MONO HIP	1	1	J19251	71
					Job Reference (optional)	- 1

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# **NOTES**

6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

 Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 1-3=-54, 3-4=-118(F=-64), 2-6=-10, 5-6=-22(F=-12)

Concentrated Loads (lb) Vert: 6=-411(F)

> Julius Les Truss Design Engineer Florida PE No. 34869 1100 Coastal Bay Blvd Boynton Beach, FL 33436



Job	Truss	Truss Type	Qty	Ply	00
L265563	T16	MONO HIP	2	1	J1925172
L200000	110	INICINO TIII	_		Job Reference (optional)

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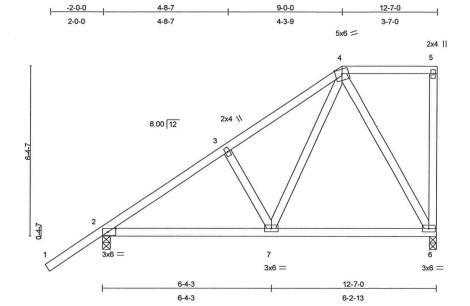


Plate Of	fsets (X,Y	): [2:0-3-9,0-1-8]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.03	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.19	Vert(TL)	-0.05	2-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.27	Horz(TL)	0.01	6	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	4					Weight: 77 lb	

1	1	IIV	NP.	E	D

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3

## BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc

bracing.

**REACTIONS** (lb/size) 6=382/0-3-0, 2=524/0-3-8

Max Horz 2=263(load case 6)

Max Uplift 6=-110(load case 6), 2=-161(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-503/102, 3-4=-383/142, 4-5=-11/1, 5-6=-76/57

BOT CHORD 2-7=-271/354, 6-7=-128/164

WEBS 3-7=-189/195, 4-7=-124/280, 4-6=-318/256

# JOINT STRESS INDEX

2 = 0.52, 3 = 0.11, 4 = 0.32, 5 = 0.22, 6 = 0.21 and 7 = 0.23

#### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

Julius Les Trues Design Engineer Honda PE No. 34888 1189 Gestel Bay Blyd Boynton Beach, Et 33488

January 10,2008

Scale = 1:40.7

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

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Job	Truss	Truss Type	Qty	Ply	00	
L265563	T16	MONO HIP	2	1	J19251	72
	170.00.00				Job Reference (optional)	

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# **NOTES**

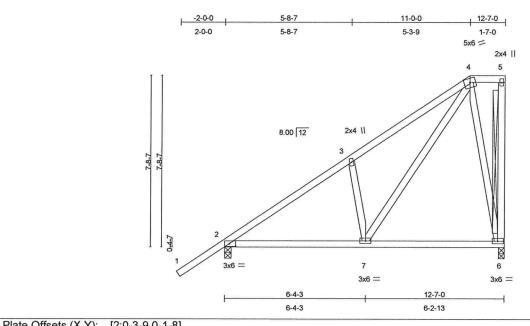
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 110 lb uplift at joint 6 and 161 lb uplift at joint 2.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00
L265563	T17	MONO HIP	2	1	J1925173
2203303	1.16	INICINO I III			Job Reference (optional)

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LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.03	2-7	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	ВС	0.20	Vert(TL)	-0.05	2-7	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.39	Horz(TL)	0.00	6	n/a	n/a		
<b>BCDL</b>	5.0	Code FBC2004/TF	2002	(Mat	rix)						Weight: 84 lb	

1 1	IR	R	D	_	R

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 WEBS

# **BRACING**

TOP CHORD

**BOT CHORD** 

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc

bracing.

**WEBS** 

T-Brace:

2 X 4 SYP No.3 - 5-6

Scale = 1:48.8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 6=382/0-3-0, 2=524/0-3-8

Max Horz 2=305(load case 6)

Max Uplift 6=-152(load case 6), 2=-142(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-487/27, 3-4=-428/167, 4-5=-8/0, 5-6=-44/28

**BOT CHORD** 2-7=-257/332, 6-7=-66/77

WEBS 3-7=-269/282, 4-7=-267/392, 4-6=-386/352

# JOINT STRESS INDEX

2 = 0.44, 3 = 0.15, 4 = 0.31, 5 = 0.18, 6 = 0.27 and 7 = 0.29

January 10,2008

Continued on page 2



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T17	MONO HIP	2	1	J	1925173
					Job Reference (optional)	

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## NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 152 lb uplift at joint 6 and 142 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lee Trust Design Engineer Florida HE No. 24866 1109 Coasiel Bay Alvd Bovaton Beach, Ft 93435



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T18	MONO TRUSS	6	1	J192517	4
			-		Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:19 2008 Page 1

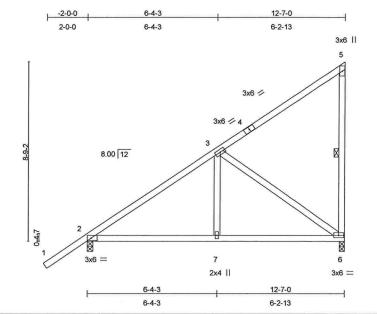


Plate Of	fsets (X,Y	(): [2:0-3-9,0-1-8]										
LOADIN TCLL TCDL BCLL BCDL	20.0 7.0 10.0 5.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/Ti	2-0-0 1.25 1.25 YES Pl2002	CSI TC BC WB (Mate	0.30 0.20 0.36 rix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.03 -0.06 0.01	(loc) 2-7 2-7 6	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 74 lb	<b>GRIP</b> 244/190
	ORD 2) ORD 2)	( 4 SYP No.2 ( 4 SYP No.2 ( 4 SYP No.3				BRACING TOP CHO BOT CHO	RD	6-0-0 c	c purlins eiling dir	, except	ng directly applie end verticals. lied or 10-0-0 od	

**WEBS** 

1 Row at midpt

REACTIONS (lb/size) 6=382/0-3-0, 2=524/0-3-8

Max Horz 2=335(load case 6)

Max Uplift 6=-187(load case 6), 2=-124(load case 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-479/0, 3-4=-118/0, 4-5=-84/64, 5-6=-135/146

**BOT CHORD** 2-7=-247/321, 6-7=-247/321

**WEBS** 3-7=0/210, 3-6=-376/288

## JOINT STRESS INDEX

2 = 0.38, 3 = 0.16, 4 = 0.09, 5 = 0.26, 6 = 0.19 and 7 = 0.15

### NOTES

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi Continued on page 2

5-6

January 10,2008



This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T18	MONO TRUSS	6	1	,	J1925174
L200000	110	MONO TROOS	o .	1	Job Reference (optional)	

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## NOTES

4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 187 lb uplift at joint 6 and 124 lb uplift at joint 2.

LOAD CASE(S) Standard



Job	Truss	Truss Type	C	Qty	Ply	00	
L265563	T19G	GABLE	i		1		J1925175
	1.166	07.1522	,			Job Reference (optional)	
Builders First S	Source, Jacksonville ,I	Florida 32244	6.300 s Feb 15 2	2006	MiTek In	dustries, Inc. Thu Jan 10 17:30:20 2008	Page 1
-2-	0-0	3-0-0				12-7-0	
2-0	0-0	3-0-0			3	9-7-0	'
							Scale = 1:25.

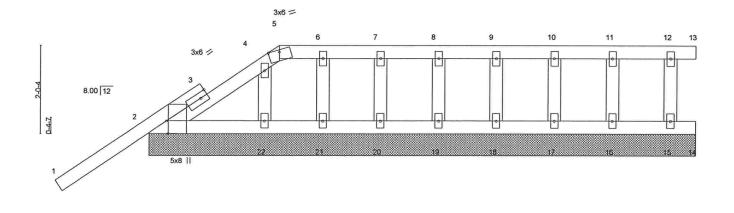


Plate Offsets (X,Y): [2:0-3-8, Edge] 2-0-0 LOADING (psf) SPACING CSI DEFL L/d **PLATES** GRIP I/defl in (loc) **TCLL** 20.0 1.25 TC 0.31 Vert(LL) 244/190 Plates Increase -0.00n/r 120 MT20 **TCDL** 7.0 Lumber Increase 1.25 BC 0.05 Vert(TL) -0.01 1 n/r 90 **BCLL** 10.0 Rep Stress Incr NO WB 0.02 -0.00Horz(TL) 13 n/a n/a **BCDL** 5.0 Code FBC2004/TPI2002 (Matrix) Weight: 60 lb

12-7-0 12-7-0

LUMBER		BRACING	
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing directly applied or
BOT CHORD	2 X 4 SYP No.2		6-0-0 oc purlins.
OTHERS	2 X 4 SYP No.3	<b>BOT CHORD</b>	Rigid ceiling directly applied or 10-0-0 oc
			bracing.

REACTIONS (lb/size) 13=8/12-7-0, 2=239/12-7-0, 14=1/12-7-0, 22=85/12-7-0, 21=88/12-7-0, 20=85/12-7-0, 19=85/12-7-0, 18=85/12-7-0, 17=85/12-7-0, 16=90/12-7-0, 15=67/12-7-0

Max Horz 2=166(load case 6)

Max Uplift 13=-7(load case 4), 2=-209(load case 6), 22=-29(load case 7), 21=-74(load case 4), 20=-49(load case 4), 19=-54(load case 4), 18=-53(load case 4), 17=-53(load case 4), 16=-56(load case 4), 15=-41(load case 4)

Max Grav 13=8(load case 11), 2=239(load case 1), 14=3(load case 2), 22=94(load case 2), 21=88(load case 1), 20=86(load case 11), 19=85(load case 1), 18=86(load case 11), 17=85(load case 1), 16=90(load case 11), 15=67(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-44/7, 3-4=-29/21, 4-5=-21/7, 5-6=0/0, 6-7=0/0, 7-8=0/0, 8-9=0/0,

9-10=0/0, 10-11=0/0, 11-12=0/0, 12-13=0/0

**BOT CHORD** 2-22=0/0, 21-22=0/0, 20-21=0/0, 19-20=0/0, 18-19=0/0, 17-18=0/0, 16-17=0/0,

15-16=0/0, 14-15=0/0

4-22=-74/49, 6-21=-73/79, 7-20=-72/59, 8-19=-72/62, 9-18=-72/61, 10-17=-71/61,

11-16=-76/65, 12-15=-56/47

Continued on page 2

**WEBS** 



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T19G	GABLE	1	1	J1925	5175
1200000	1130	CABLE	'	,	Job Reference (optional)	

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## JOINT STRESS INDEX

2 = 0.45, 3 = 0.00, 3 = 0.15, 4 = 0.04, 5 = 0.04, 6 = 0.04, 7 = 0.03, 8 = 0.03, 9 = 0.03, 10 = 0.03, 11 = 0.04, 12 = 0.03, 15 = 0.03, 16 = 0.04, 17 = 0.03, 18 = 0.03, 19 = 0.03, 20 = 0.03, 21 = 0.04 and 22 = 0.03

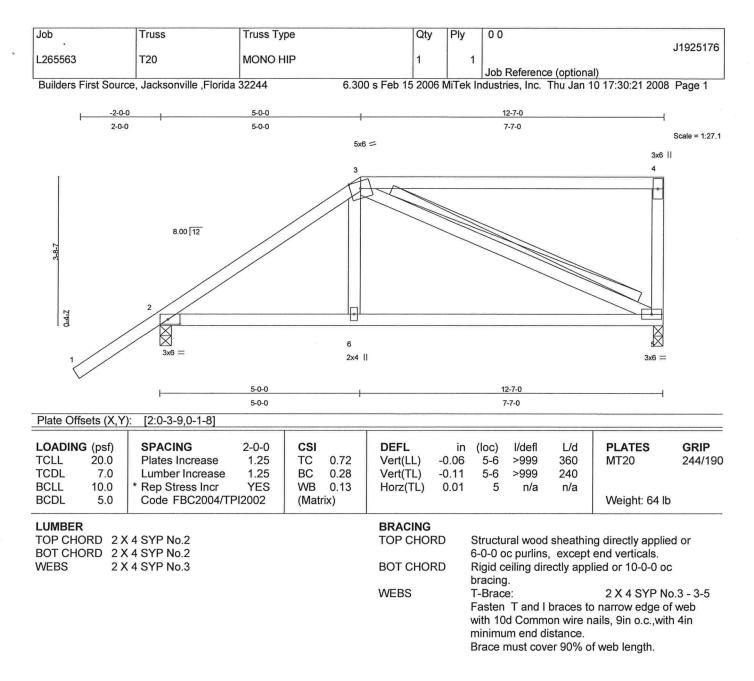
#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Provide adequate drainage to prevent water ponding.
- 5) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 1-4-0 oc.
- 9) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 7 lb uplift at joint 13, 209 lb uplift at joint 2, 29 lb uplift at joint 22, 74 lb uplift at joint 21, 49 lb uplift at joint 20, 54 lb uplift at joint 19, 53 lb uplift at joint 18, 53 lb uplift at joint 17, 56 lb uplift at joint 16 and 41 lb uplift at joint 15.

LOAD CASE(S) Standard

Julhis Les Truse Design Engineer Flonda PE No. 34889 1 109 Gassiel Bey Blyd Boynton Besch, FL 93435





REACTIONS (lb/size) 5=382/0-3-0, 2=524/0-3-8

Max Horz 2=177(load case 6)

Max Uplift 5=-108(load case 5), 2=-173(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-526/202, 3-4=-80/50, 4-5=-200/167

BOT CHORD 2-6=-238/381, 5-6=-242/379

WEBS 3-6=0/207, 3-5=-330/211

# JOINT STRESS INDEX

2 = 0.61, 3 = 0.69, 4 = 0.54, 5 = 0.54 and 6 = 0.15

Truse Design Engineer Florida FE No. 34899 1 100 Coastal Bay Blvd Boynton Beach, FL 33435







Job	Truss	Truss Type	Qty	Ply	00
L265563	T20	MONO HIP	1	1	J1925176
					Job Reference (optional)

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:21 2008 Page 2

#### NOTES

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 5 and 173 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida ME No. 23868 1199 Coestal Bay Blvri Boynton Beson, FL 22425



Job	Truss	Truss Type	Qty	Ply	00	٦
L265563	T21	MONO HIP	1	1	J1925177	
L203303	121	MONO THE	'	'	Job Reference (optional)	1

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:21 2008 Page 1

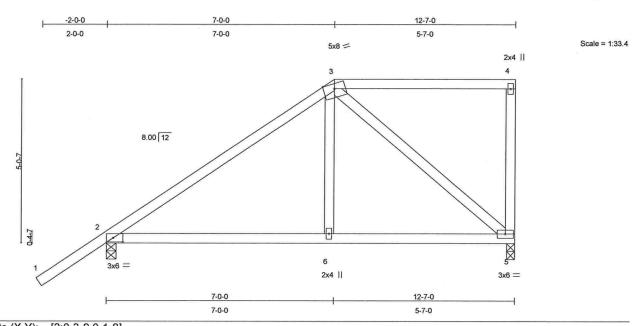


Plate Of	ffsets (X, Y	): [2:0-3-9,0-1-8]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.30	Vert(LL)	-0.04	2-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	-0.09	2-6	>999	240		
<b>BCLL</b>	10.0	* Rep Stress Incr	YES	WB	0.34	Horz(TL)	0.01	5	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 67 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

**WEBS** 2 X 4 SYP No.3

BRACING

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or

6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc

bracing.

**REACTIONS** (lb/size) 5=382/0-3-0, 2=524/0-3-8

Max Horz 2=220(load case 6)

Max Uplift 5=-108(load case 5), 2=-171(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-460/134, 3-4=-17/15, 4-5=-119/96

**BOT CHORD** 2-6=-202/295, 5-6=-203/292 **WEBS** 3-6=0/213, 3-5=-366/257

#### JOINT STRESS INDEX

2 = 0.41, 3 = 0.51, 4 = 0.56, 5 = 0.16 and 6 = 0.15

## **NOTES**

- 1) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi

Continued on page 2

January 10,2008

Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MITek connector Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building ocde. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	0 0	
L265563	T21	MONO HIP	1	1	J19251	177
	,				Job Reference (optional)	

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## NOTES

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 108 lb uplift at joint 5 and 171 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Flonda FE No. 34869 1100 Coastal Bay Blvd Boyoton Desen Et 20405





6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:22 2008 Page 1

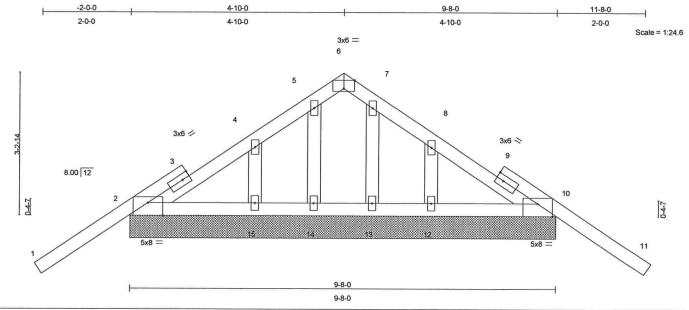


Plate Of	fsets (X,Y	): [2:0-4-0,0-3-7], [6:	0-3-0,Edg	e], [10:0	)-4-0,0-3	-7]						
LOADIN TCLL TCDL BCLL BCDL	20.0 7.0 10.0 5.0	SPACING Plates Increase Lumber Increase * Rep Stress Incr Code FBC2004/TF	2-0-0 1.25 1.25 NO Pl2002	CSI TC BC WB (Mat	0.32 0.06 0.02 rix)	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.03 -0.04 0.00	(loc) 11 11 10	I/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20 Weight: 53 lb	<b>GRIP</b> 244/190

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

OTHERS 2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or

6-0-0 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 6-0-0 oc

bracing.

**REACTIONS** (lb/size) 2=245/9-8-0, 10=245/9-8-0, 14=76/9-8-0, 15=101/9-8-0, 13=76/9-8-0,

12=101/9-8-0

Max Horz 2=108(load case 5)

Max Uplift 2=-217(load case 6), 10=-231(load case 7), 14=-36(load case 6),

15=-58(load case 7), 13=-16(load case 7), 12=-51(load case 6)

Max Grav 2=245(load case 1), 10=245(load case 1), 14=76(load case 1), 15=101(load case 2), 13=76(load case 1), 12=101(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/61, 2-3=-55/68, 3-4=-45/80, 4-5=-32/98, 5-6=-27/100, 6-7=-27/100,

7-8=-32/98, 8-9=-12/55, 9-10=-54/36, 10-11=0/61

BOT CHORD 2-15=-7/122, 14-15=-7/122, 13-14=-7/122, 12-13=-7/122, 10-12=-7/122 WEBS 5-14=-62/30, 4-15=-87/76, 7-13=-62/9, 8-12=-87/69

Julius Lee Trues Casign Engineer Florida PE No. 34869 1100 Castal Bay Blvd. Boynton Gesch. FL 33435

#### JOINT STRESS INDEX

2 = 0.70, 3 = 0.00, 3 = 0.16, 4 = 0.04, 5 = 0.03, 6 = 0.12, 7 = 0.03, 8 = 0.04, 9 = 0.00, 9 = 0.16, 10 = 0.70, 12 = 0.04, 13 = 0.02, 14 = 0.02 and 15 = 0.04

#### NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

January 10,2008

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors.
Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, Wi 53719 or the Truss Plate Institute, 583 D'Onofino Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	0 0	
L265563	T22G	GABLE	1	1	J19251	78
	1220	O' IDEE			Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:22 2008 Page 2

#### NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 1-4-0 oc.
- 8) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 217 lb uplift at joint 2, 231 lb uplift at joint 10, 36 lb uplift at joint 14, 58 lb uplift at joint 15, 16 lb uplift at joint 13 and 51 lb uplift at joint 12.

LOAD CASE(S) Standard

Julius Lee Truse Design Enginser Florida PE No. 24866 1400 Coestsi Bay Blyd



Job	Truss	Truss Type	Qty	Ply	00
L265563	T23G	GABLE	1	1	J1925179
					Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 17:38:45 2008 Page 1

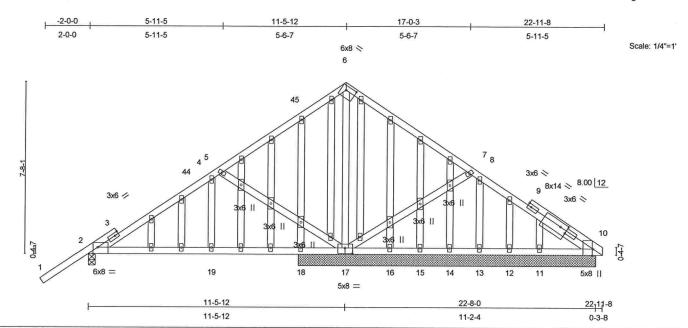


Plate Of	fsets (X,Y)	): [2	2:0-2-9,Edge], [6:0-	-5-4,0-2-4],	[9:0-3-1	2,0-1-8],	[10:0-3-8,Edge	], [17:0-	-4-0,0-3	-0], [29:1	-5-5,0-2-	8]	
LOADIN		1	SPACING	2-0-0	CSI	0.50	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	1	Plates Increase	1.25	TC	0.69	Vert(LL)	0.06	2-19	>999	360	MT20	244/190
TCDL	7.0		Lumber Increase	1.25	BC	0.25	Vert(TL)	-0.04	2-19	>999	240		
BCLL	10.0	*	Rep Stress Incr	NO	WB	0.87	Horz(TL)	-0.01	2	n/a	n/a		
BCDL	5.0		Code FBC2004/TF	PI2002	(Matr	rix)						Weight: 196 lb	

TOP CHORD	2 X
BOT CHORD	2 X

LUMBER

4 SYP No.2 4 SYP No.2

2 X 4 SYP No.3 WFBS **OTHERS** 2 X 4 SYP No.3

# BRACING

TOP CHORD

**BOT CHORD** 

oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing,

Structural wood sheathing directly applied or 6-0-0

Except:

10-0-0 oc bracing: 2-19,18-19 9-7-6 oc bracing: 17-18.

REACTIONS (lb/size) 10=249/13-3-8, 17=1373/13-3-8, 16=20/13-3-8, 15=12/13-3-8, 14=10/13-3-8,

13=488/13-3-8, 12=-48/13-3-8, 11=133/13-3-8, 18=35/13-3-8, 2=526/0-3-8

Max Horz 18=286(load case 5)

Max Uplift 10=-147(load case 7), 17=-1062(load case 7), 13=-351(load case 7), 12=-48(load case 1), 11=-87(load case 7), 18=-49(load case 4), 2=-476(load case 6)

Max Grav 10=289(load case 11), 17=1373(load case 1), 16=57(load case 2), 15=35(load case 2), 14=39(load case 2), 13=494(load case 11), 12=58(load case 7), 11=133(load

case 1), 18=95(load case 2), 2=540(load case 10)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 6-7=-269/370, 7-8=-173/88, 8-9=-163/180, 9-10=-247/59, 1-2=0/62, 2-3=-555/528,

3-44=-430/542, 4-44=-421/547, 4-5=-346/384, 5-45=-152/178, 6-45=-286/384

BOT CHORD 2-19=-306/420, 18-19=-306/420, 17-18=-430/420, 16-17=-51/107, 15-16=-51/107,

14-15=-51/107, 13-14=-51/107, 12-13=-51/107, 11-12=-51/107, 10-11=-51/107

**WEBS** 7-17=-213/300, 6-17=-914/877, 5-17=-656/893, 8-13=-464/440, 4-19=-236/153

#### JOINT STRESS INDEX

2 = 0.53, 3 = 0.00, 3 = 0.23, 4 = 0.34, 5 = 0.48, 6 = 0.94, 7 = 0.34, 8 = 0.34, 9 = 0.00, 9 = 0.58, 9 = 0.58, 10 = 0.29, 11 = 0.34, 12 = 0.3413 = 0.34, 14 = 0.34, 15 = 0.34, 16 = 0.34, 17 = 0.55, 18 = 0.34, 19 = 0.34, 20 = 0.34, 21 = 0.34, 22 = 0.34, 23 = 0.62, 24 = 0.34, 25 = 0.340.62, 26 = 0.34, 27 = 0.62, 28 = 0.34, 29 = 0.68, 30 = 0.34, 31 = 0.34, 32 = 0.62, 33 = 0.34, 34 = 0.62, 35 = 0.34, 36 = 0.34, 37 arguery 10,2008 Continue of page 20.34, 40 = 0.34, 41 = 0.34, 42 = 0.34 and 43 = 0.34

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connet Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building design and / or contractor per ANSI / TPI 1 as referenced by the building ode. For general guidance regarding storage, delivery, erect and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00
L265563	T23G	GABLE	1	1	J1925179
		57.522			Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 17:38:45 2008 Page 2

#### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS gable end zone and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 147 lb uplift at joint 10, 1062 lb uplift at joint 17, 351 lb uplift at joint 13, 48 lb uplift at joint 12, 87 lb uplift at joint 11, 49 lb uplift at joint 18 and 476 lb uplift at joint 2.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (plf)

Vert: 6-10=-114(F=-60), 1-44=-54, 44-45=-141(F=-87), 6-45=-114(F=-60), 2-10=-10

Julius Lee Truee Design Engineer Planda PE No. 34868 1 100 Coastel Bey Blvd.



Job	Truss	Truss Type	Qty	Ply	00
L265563	T24	COMMON	1	1	J192518
	121	COMMITTEE			Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 17:39:31 2008 Page 1

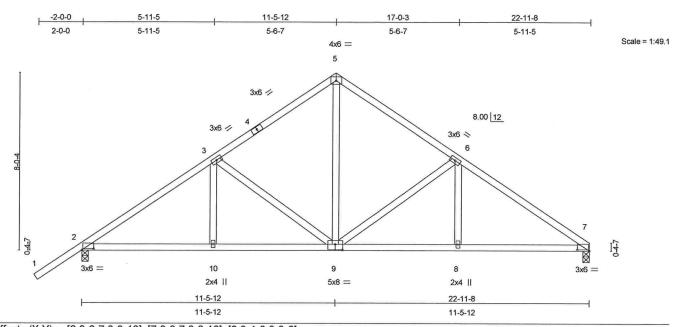


Plate Of	tsets (X,Y)	: [2:0-6-7,0-0-10], [7:	0-6-7,0-0-1	0], [9:0-4	4-0,0-3-0]							
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.34	Vert(LL)	0.13	7-8	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.28	Vert(TL)	0.11	7-8	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.90	Horz(TL)	0.03	2	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 121 lb	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 **WEBS** 2 X 4 SYP No.3 BRACING

TOP CHORD

Structural wood sheathing directly applied or 5-9-13

oc purlins.

**BOT CHORD** Rigid ceiling directly applied or 6-3-6 oc bracing.

REACTIONS (lb/size) 7=719/0-3-8, 2=851/0-3-8

Max Horz 7=239(load case 5)

Max Uplift 7=-435(load case 7), 2=-536(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 5-6=-740/962, 6-7=-1080/1295, 1-2=0/62, 2-3=-1057/1260, 3-4=-739/937, 4-5=-655/960

**BOT CHORD** 2-10=-851/797, 9-10=-851/797, 8-9=-889/822, 7-8=-889/822

**WEBS** 6-9=-359/551, 5-9=-864/433, 3-9=-327/505, 3-10=-276/182, 6-8=-305/187

#### JOINT STRESS INDEX

2 = 0.71, 3 = 0.43, 4 = 0.40, 5 = 0.50, 6 = 0.43, 7 = 0.71, 8 = 0.34, 9 = 0.35 and 10 = 0.34

# NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 435 lb uplift at Colflint and \$36 buplift at joint 2.

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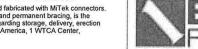
Marning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00
L265563	T24	COMMON	1	1	J1925180
					Job Reference (optional)

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LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T25	SPECIAL	1	1	J19251	81
220000	120	OI LOWLE			Job Reference (optional)	

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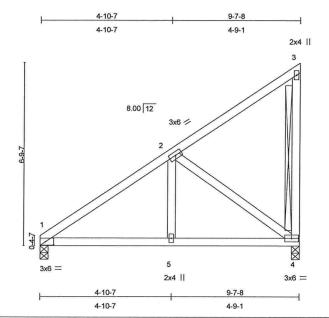


Plate Of	fsets (X,Y	(): [1:0-3-9,0-1-8]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.19	Vert(LL)	0.05	1-5	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.20	Vert(TL)	0.04	1-5	>999	240	A6000 6-9080	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.17	Horz(TL)	-0.00	1	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, , , , ,					Weight: 53 lb	

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

2 X 4 SYP No.3

BRACING

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

**WEBS** 

**BOT CHORD** 

T-Brace:

2 X 4 SYP No.3 - 3-4

Scale = 1:40.3

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 1=299/0-3-8, 4=299/0-3-8

Max Horz 4=204(load case 6)

Max Uplift 1=-130(load case 6), 4=-269(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-377/298, 2-3=-90/48, 3-4=-100/107 1-5=-170/257, 4-5=-170/257

**WEBS** 

**BOT CHORD** 

2-5=-307/163, 2-4=-305/560

#### JOINT STRESS INDEX

1 = 0.33, 2 = 0.29, 3 = 0.58, 4 = 0.38 and 5 = 0.12

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Continued on page 2

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T25	SPECIAL	1	1	,	J1925181
L203303	123	SPECIAL	1		Job Reference (optional)	

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## **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; porch left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 130 lb uplift at joint 1 and 269 lb uplift at joint 4.

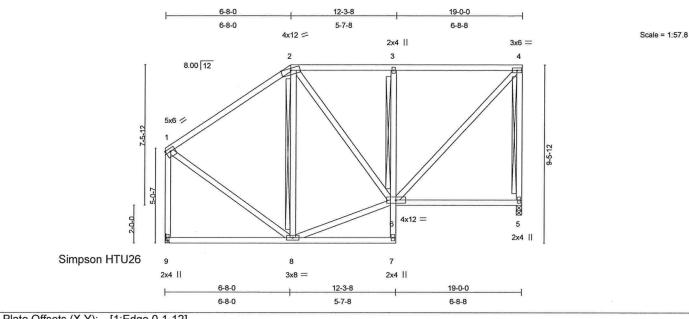
LOAD CASE(S) Standard

Julius Les Truss Design Engineer Plonda PE No. 34868 i 170 Coesial Bay Blvd. Boynton Beach, Ft. 33436



Job	Truss	Truss Type	Qty	Ply	00
L265563	T26	SPECIAL	1	1	J1925182
L200000	120	OI EGIAE			Job Reference (optional)

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- late Of	risets (X, Y	): [1:Eage,0-1-12]		I		T					i	
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.46	Vert(LL)	-0.05	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.23	Vert(TL)	-0.08	5-6	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.57	Horz(TL)	-0.01	9	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 149 lb	

LUMBER		BRACING		
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathin	g directly applied or
<b>BOT CHORD</b>	2 X 4 SYP No.2 *Except*		6-0-0 oc purlins, except	end verticals.
	3-7 2 X 4 SYP No.3	<b>BOT CHORD</b>	Rigid ceiling directly appl	ied or 10-0-0 oc
WEBS	2 X 4 SYP No.3 *Except*		bracing, Except:	
	1-9 2 X 4 SYP No.2		6-0-0 oc bracing: 7-8.	
			T-Brace:	2 X 4 SYP No.3 - 3-6
		MEDO	T Proces	2 V 4 CVD No 2 4 F

WEBS

T-Brace:

2-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 5=599/0-3-8, 9=599/Mechanical

Max Horz 5=140(load case 6)

Max Uplift 5=-181(load case 5), 9=-88(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 4-5=-559/367, 2-3=-379/236, 3-4=-389/242, 1-2=-458/211, 1-9=-561/248

**BOT CHORD** 5-6=-13/179, 6-7=0/64, 3-6=-338/234, 8-9=-31/39, 7-8=-22/16

**WEBS** 4-6=-337/537, 6-8=-50/301, 2-6=-106/132, 2-8=-239/98, 1-8=-68/324

# JOINT STRESS INDEX

1 = 0.74, 2 = 0.92, 3 = 0.48, 4 = 0.65, 5 = 0.68, 6 = 0.54, 7 = 0.36, 8 = 0.56 and 9 = 0.76

Codinger S. salese May Elva on. FL 35435

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	00
L265563	T26	SPECIAL	1	1	J1925182
1200000	120	OF EGIAL			Job Reference (optional)

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# **NOTES**

- Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Provide adequate drainage to prevent water ponding.
- 3) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 181 lb uplift at joint 5 and 88 lb uplift at joint 9.

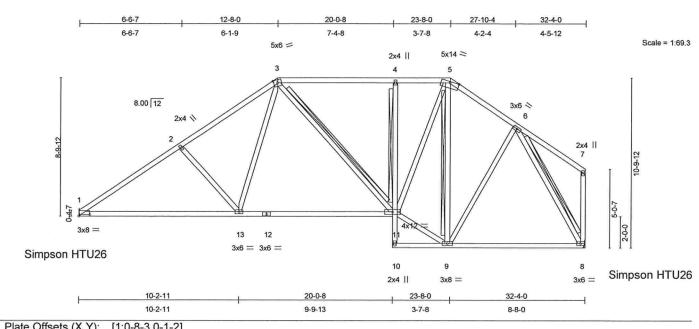
LOAD CASE(S) Standard

Julius Lee Truse Cesian Endineer Florida PE No. 24868 1 109 Ceastel Bay Blvd Boynton Beson, FL 99496



Job	Truss	Truss Type	Qty	Ply	00	٦
L265563	T27	SPECIAL	1	4	J1925183	É
L203303	127	SPECIAL	1		Job Reference (optional)	

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LOADIN	IG (psf)	SPACING	2-0-0	csı		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.31	Vert(LL)	-0.20	1-13	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.54	Vert(TL)	-0.40	1-13	>976	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.53	Horz(TL)	0.07	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TI	PI2002	(Mat							Weight: 221 lb	

LUMBER		BRACING		
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathin	g directly applied or
<b>BOT CHORD</b>	2 X 4 SYP No.2 *Except*		4-7-15 oc purlins, excep	
	4-10 2 X 4 SYP No.3	<b>BOT CHORD</b>	Rigid ceiling directly appl	lied or 6-0-0 oc
WEBS	2 X 4 SYP No.3 *Except*		bracing. Except:	
	7-8 2 X 4 SYP No.2		T-Brace:	2 X 4 SYP No.3 -
				4-11

WEBS

T-Brace:

2 X 4 SYP No.3 -

3-11, 5-9, 6-8 Fasten T and I braces to narrow edge of web

with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 1=1028/Mechanical, 8=1028/Mechanical

Max Horz 1=233(load case 5)

Max Uplift 1=-198(load case 6), 8=-173(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1565/752, 2-3=-1355/745, 3-4=-938/626, 4-5=-925/620, 5-6=-821/524,

6-7=-90/82, 7-8=-125/105

BOT CHORD 1-13=-643/1241, 12-13=-400/952, 11-12=-400/952, 10-11=-16/0, 4-11=-337/220,

9-10=-90/0, 8-9=-236/503

WEBS 2-13=-291/294, 3-13=-177/419, 3-11=-146/128, 5-11=-365/810, 5-9=-507/204,

9-11=-224/722, 6-9=-113/289, 6-8=-983/474

Julius Lee Truse Design Engineer Flonda PE No. 34898 1100 Geastel Bay Blvd Boynton Besch, FL 22421

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T27	SPECIAL	1	1		J1925183
					Job Reference (optional)	

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# JOINT STRESS INDEX

1 = 0.67, 2 = 0.33, 3 = 0.71, 4 = 0.72, 5 = 0.38, 6 = 0.35, 7 = 0.60, 8 = 0.64, 9 = 0.67, 10 = 0.47, 11 = 0.96, 12 = 0.37 and 13 = 0.49

## **NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 198 lb uplift at joint 1 and 173 lb uplift at joint 8.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 24868 1 190 Gessiel Bay Slvd Bovnton Besch, Ft. 33435



Job	Truss	Truss Type	Qty	Ply	00
L265563	T28	SPECIAL	2	1	J1925184
L200000	120	OF EGINE	_		Job Reference (optional)

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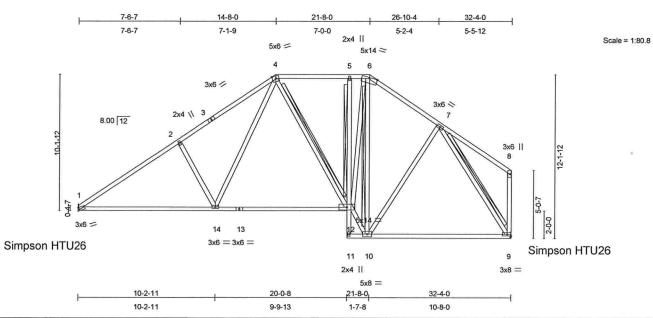


Plate Of	Plate Offsets (X,Y): [1:Edge,0-2-10]													
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP		
TCLL	20.0	Plates Increase	1.25	TC	0.44	Vert(LL)	-0.21	9-10	>999	360	MT20	244/190		
TCDL	7.0	Lumber Increase	1.25	BC	0.58	Vert(TL)	-0.42	1-14	>916	240				
BCLL	10.0	* Rep Stress Incr	YES	WB	0.71	Horz(TL)	0.12	9	n/a	n/a				
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)	, ,					Weight: 232 lb			

LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 \*Except\* 5-11 2 X 4 SYP No.3 **WEBS** 2 X 4 SYP No.3 \*Except\*

8-9 2 X 4 SYP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-5-15 oc purlins, except end verticals.

**BOT CHORD** Rigid ceiling directly applied or 6-0-0 oc bracing. Except:

T-Brace:

2 X 4 SYP No.3 -5-12

**WEBS** 

T-Brace:

2 X 4 SYP No.3 -4-12, 6-10, 7-9

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 1=1028/Mechanical, 9=1028/Mechanical

Max Horz 1=270(load case 5)

Max Uplift 1=-244(load case 6), 9=-201(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1567/726, 2-3=-1374/762, 3-4=-1260/792, 4-5=-793/574, 5-6=-781/571,

6-7=-845/546, 7-8=-116/104, 8-9=-157/132

**BOT CHORD** 1-14=-608/1217, 13-14=-318/848, 12-13=-318/848, 11-12=-265/0, 5-12=-150/126,

10-11=-94/22, 9-10=-251/554

**WEBS** 2-14=-354/355, 4-14=-291/484, 4-12=-190/156, 6-12=-397/1068, 10-12=-272/1082,

6-10=-878/273, 7-10=-70/216, 7-9=-972/456

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T28	SPECIAL	2	1	J19251	184
LEGGGGG	120	OI LOW L	_		Job Reference (optional)	

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## JOINT STRESS INDEX

1 = 0.81, 2 = 0.33, 3 = 0.39, 4 = 0.69, 5 = 0.52, 6 = 0.79, 7 = 0.34, 8 = 0.26, 9 = 0.62, 10 = 0.55, 11 = 0.73, 12 = 0.85, 13 = 0.35 and 14 = 0.46

## NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 244 lb uplift at joint 1 and 201 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Lee fruee Design Engineer Florida PE No. 34868 I 188 Geastal Bay Blord Poynton Beach, FL 33435



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T20	UID	4	,	J192518	5
L200000	T30	HIP	1	1	Job Reference (optional)	

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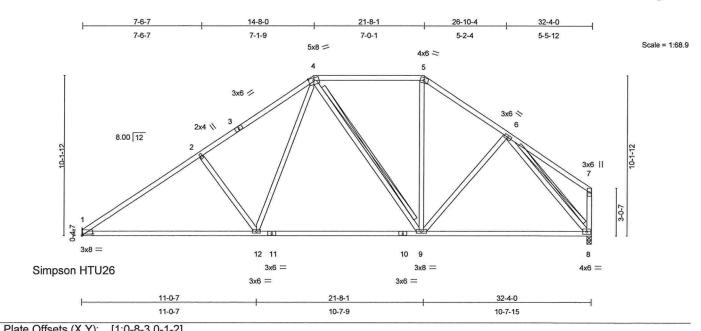


Plate Of	iseis (A, 1	). [1.0-0-3,0-1-2]										
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.51	Vert(LL)	-0.28	1-12	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.57	Vert(TL)	-0.56	1-12	>683	240	100000000000000000000000000000000000000	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.49	Horz(TL)	0.05	8	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 187 lb	

.UMBER		BRACING
OP CHORD	2 X 4 SYP No.2	TOP CHOR

BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 \*Except\* **WEBS** 

7-8 2 X 4 SYP No.2

**IORD** 

4-5-12 oc purlins, except end verticals. **BOT CHORD** Rigid ceiling directly applied or 7-8-0 oc

bracing.

**WEBS** T-Brace: 2 X 4 SYP No.3 - 4-9, 6-8

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Structural wood sheathing directly applied or

Brace must cover 90% of web length.

**REACTIONS** (lb/size) 1=1028/Mechanical, 8=1028/0-3-8

Max Horz 1=270(load case 5)

Max Uplift 1=-208(load case 6), 8=-185(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1557/728, 2-3=-1323/721, 3-4=-1128/752, 4-5=-768/562, 5-6=-993/594,

6-7=-172/122, 7-8=-195/147

1-12=-609/1210, 11-12=-319/854, 10-11=-319/854, 9-10=-319/854, 8-9=-326/723

**WEBS** 2-12=-350/351, 4-12=-250/465, 4-9=-243/175, 5-9=-103/267, 6-9=-103/171,

6-8=-1011/480

# JOINT STRESS INDEX

1 = 0.76, 2 = 0.33, 3 = 0.38, 4 = 0.64, 5 = 0.62, 6 = 0.35, 7 = 0.34, 8 = 0.74, 9 = 0.56, 10 = 0.45, 11 = 0.65 and 12 = 0.48

Continued on page 2

**BOT CHORD** 



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T30	HIP	1	1	J192	25185
			1		Job Reference (optional)	

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## NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 208 lb uplift at joint 1 and 185 lb uplift at joint 8.

LOAD CASE(S) Standard

Les Gesign Engineer s PE No. 24866 Gesstal Bay Blvd on Resch. Ft. 20426



Job	Truss	Truss Type	Qty	Ply	00	ì
, ,	T24	CDECIAL	2	١,		J1925186
L265563	T31	SPECIAL	3	1	Job Reference (optional)	

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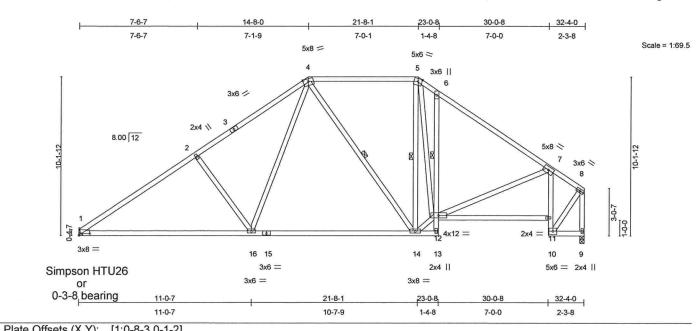


Plate Of	Piate Offsets (A, T). [1.0-0-3,0-1-2]													
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP		
TCLL	20.0	Plates Increase	1.25	TC	0.37	Vert(LL)	-0.27	1-16	>999	360	MT20	244/190		
TCDL	7.0	Lumber Increase	1.25	BC	1.00	Vert(TL)	-0.53	1-16	>719	240				
BCLL	10.0	* Rep Stress Incr	YES	WB	0.48	Horz(TL)	0.13	9	n/a	n/a				
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 214 lb			

11	М		_	
 	IVI	н	-	ĸ

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 \*Except\*

6-13 2 X 4 SYP No.3, 7-10 2 X 4 SYP No.3

**WEBS** 

2 X 4 SYP No.3 \*Except\*

8-9 2 X 4 SYP No.2

# BRACING

TOP CHORD

**BOT CHORD** 

Rigid ceiling directly applied. Except:

1 Row at midpt

6-12

Structural wood sheathing directly applied or

4-6-4 oc purlins, except end verticals.

**WEBS** 

1 Row at midpt

4-14, 5-14

**REACTIONS** (lb/size) 1=1025/Mechanical, 9=1025/0-3-8

Max Horz 1=316(load case 5)

Max Uplift 1=-246(load case 6), 9=-197(load case 6)

## FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=-1545/730, 2-3=-1313/724, 3-4=-1119/754, 4-5=-765/570, 5-6=-1019/745,

6-7=-1164/607, 7-8=-658/355, 8-9=-1143/532

**BOT CHORD** 

1-16=-504/1196, 15-16=-220/849, 14-15=-220/849, 13-14=-157/16, 12-13=-188/0,

6-12=-289/255, 11-12=-245/673, 10-11=-610/301, 7-11=-585/325, 9-10=-24/59

WEBS

2-16=-341/345, 4-16=-242/465, 4-14=-233/165, 5-14=-320/89, 5-12=-355/637,

12-14=-94/931, 7-12=-93/256, 8-10=-416/917

on Engineer No. 34868 Stal Bay Blvd Stal Bay Blvd

# JOINT STRESS INDEX

1 = 0.73, 2 = 0.33, 3 = 0.38, 4 = 0.61, 5 = 0.49, 6 = 0.25, 7 = 0.52, 8 = 0.69, 9 = 0.42, 10 = 0.56, 11 = 0.74, 12 = 0.70, 13 = 0.70, 13 = 0.700.62, 14 = 0.88, 15 = 0.67 and 16 = 0.48

## NOTES

1) Unbalanced roof live loads have been considered for this design.

Continued on page 2

January 10,2008

👠 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI /TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T31	SPECIAL	3	1	J1925186	,
220000	101	OF ESTAL			Job Reference (optional)	

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### NOTES

- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 246 lb uplift at joint 1 and 197 lb uplift at joint 9.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Truss Design 248es 1,000 Cess No. 248es 1,000 Cess No. 248es Hoviton Besch, FL 33426



Job	Truss		Truss Type			Qty	Ply	0.0				J1925187
L265563	T32		SPECIAL			1	1	lob	Poforono	e (option	ol)	31925187
Builders First S	ource, Jackson	ville ,Florida	32244		6.300 s Fe	b 15 2006	MiTek I				aı) 10 17:30:31 20	08 Page 1
-2-0-0	6-11-	å .	11-7-8		17-8-8		22-	4-8		27-0-8	30-4-0	
2-0-0	6-11-		4-8-0		6-1-0	Į.	4-8		1	4-8-0	3-3-8	Scale = 1:59.0
				5x6 =	2x4	4x12	) <del>&gt;</del>					Scale - 1,59,0
				6	7	8						
7-1-8	8.00 12 7x14 MT20H =	4	4 \\					\\ //	9		10	<u>1.</u>
1-10-12_ 0-4-7	19		17		4x12 = 15 2x4	14 3x8	<u>//</u> =					11 1-10-15 PM 212
ı	2-3-8 2-3-8	8-5-8 6-2-0		14-7-8		17-8-8			27-0-8		30-4-0	Н
Plate Offsets (	Allow on	3,0-0-6], [3:0	0-7-0.0-2-41	6-2-0		3-1-0			9-4-0		3-3-8	
LOADING (ps TCLL 20.1 TCDL 7.1 BCLL 10.1 BCDL 5.1	f) SPACII D Plates I Lumber D * Rep Str	NG ncrease Increase	2-0-0 1.25 1.25 YES	BC 0	.92 V€ .95 V€ .43 Hc	EFL ert(LL) ert(TL) orz(TL)	in 0.34 1 -0.66 1 0.30		l/defl >999 >547 n/a	L/d 360 240 n/a	PLATES MT20 MT20H Weight: 19	<b>GRIP</b> 244/19 187/14
LUMBER TOP CHORD BOT CHORD	TC	RACING OP CHOR	2	-1-13	oc purlir	s, exce	ng directly app pt end verticals blied or 2-2-0 o	3.				

**REACTIONS** (lb/size) 12=957/0-3-8, 2=1086/0-3-8

Max Horz 2=267(load case 5)

Max Uplift 12=-193(load case 7), 2=-285(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1416/522, 3-4=-1699/771, 4-5=-1626/826, 5-6=-1596/847,

6-7=-1043/629, 7-8=-1041/630, 8-9=-1165/642, 9-10=-2139/1081, 10-11=-68/71,

11-12=-106/61

**BOT CHORD** 2-19=-344/1039, 18-19=-5/69, 3-18=-56/123, 17-18=-537/1423, 16-17=-286/997,

15-16=-51/0, 7-16=-159/95, 14-15=-62/67, 13-14=-486/1191, 12-13=-741/1702

**WEBS** 4-17=-392/293, 6-17=-321/644, 6-16=-147/231, 8-16=-197/388, 14-16=-209/891,

8-14=-101/181, 9-14=-400/319, 9-13=-401/778, 10-13=-498/354, 10-12=-1832/818

### JOINT STRESS INDEX

2 = 0.66, 3 = 0.91, 4 = 0.33, 5 = 0.42, 6 = 0.38, 7 = 0.33, 8 = 0.66, 9 = 0.51, 10 = 0.66, 11 = 0.67, 12 = 0.51, 13 = 0.47, 14 = 0.47, 0.80, 15 = 0.41, 16 = 0.77, 17 = 0.53, 18 = 0.00 and 19 = 0.56

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T32	SPECIAL	1	1		J1925187
L203303	132	OF EGIAL			Job Reference (optional)	

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### **NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are MT20 plates unless otherwise indicated.
- 6) All plates are 3x6 MT20 unless otherwise indicated.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 193 lb uplift at joint 12 and 285 lb uplift at joint 2.

LOAD CASE(S) Standard

em Pesion Endineer Ple No. Sideeb Peside Bay Elvi 7 Detach



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T22	SPECIAL	4			J1925188
1205505	133	SPECIAL	1	1	Job Reference (optional)	

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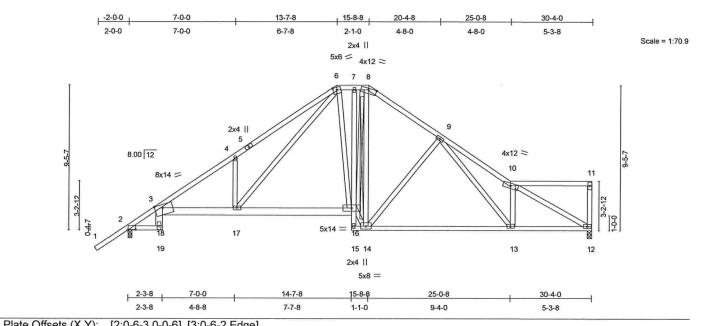


Plate Of	isets (A, Y	): [2:0-6-3,0-0-6], [3:	0-6-2,Eag	ej								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	-0.18	13-14	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.79	Vert(TL)	-0.37	13-14	>976	240	Total Control of Contr	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.99	Horz(TL)	0.20	12	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 224 lb	

LUMBER		BRACING		
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathir	ng directly applied or
<b>BOT CHORD</b>	2 X 4 SYP No.2 *Except*		4-2-6 oc purlins, except	end verticals.
	3-19 2 X 6 SYP No.1D, 16-18 2 X 6 SYP No.1D	BOT CHORD	Rigid ceiling directly app	lied or 6-0-0 oc
	7-15 2 X 4 SYP No.3		bracing.	
WEBS	2 X 4 SYP No.3	WEBS	T-Brace:	2 X 4 SYP No.3 -
				8-14

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c.,with 4in minimum end distance.

Brace must cover 90% of web length.

**REACTIONS** (lb/size) 12=957/0-3-8, 2=1086/0-3-8

Max Horz 2=325(load case 5)

Max Uplift 12=-202(load case 7), 2=-295(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1424/512, 3-4=-1809/788, 4-5=-1885/986, 5-6=-1784/1014,

6-7=-872/574, 7-8=-860/570, 8-9=-1032/601, 9-10=-1807/960, 10-11=-50/83.

11-12=-144/100

BOT CHORD 2-19=-346/1051, 18-19=-5/65, 3-18=-191/159, 17-18=-547/1485, 16-17=-214/888,

15-16=-337/0, 7-16=-249/321, 14-15=-101/166, 13-14=-387/1057, 12-13=-600/1429

WEBS 4-17=-378/351, 6-17=-525/939, 6-16=-274/233, 8-16=-242/607, 14-16=-191/1059,

8-14=-377/194, 9-14=-418/341, 9-13=-369/629, 10-13=-434/346, 10-12=-1586/721

Julius Lee Truse Design Engineer Florida PE No. 34868 1102 Grastal Bay Slvd

Continued on page 2



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T33	SPECIAL	1	1		J1925188
			Ľ		Job Reference (optional)	

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### JOINT STRESS INDEX

2 = 0.66, 3 = 0.84, 4 = 0.33, 5 = 0.39, 6 = 0.69, 7 = 0.33, 8 = 0.62, 9 = 0.42, 10 = 0.57, 11 = 0.29, 12 = 0.45, 13 = 0.43, 14 = 0.54, 15 = 0.49, 16 = 0.57, 17 = 0.63, 18 = 0.00 and 19 = 0.56

### **NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 202 lb uplift at joint 12 and 295 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Les Truse Design Engineer Florida PE No. 34866 Hoo Coustal Bay Slovi Aconton Lesch, FL 93436



Job	Truss	Truss Type	Qty	Ply	00
L265563	T34	SPECIAL	1	1	J1925189
2200000	104	SI ESI/LE			Job Reference (optional)

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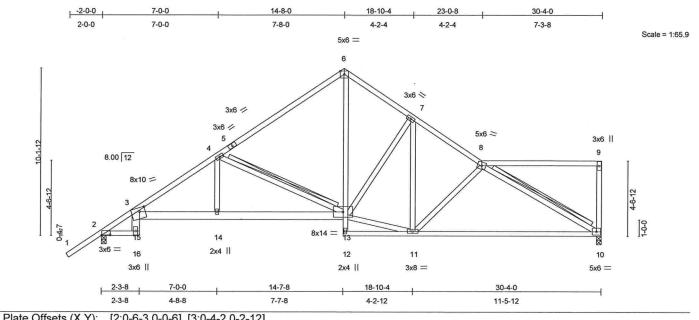


Plate Of	isets (A, r	). [2:0-6-3,0-0-6], [3:	0-4-2,0-2-	-12]							
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in (loc	) I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.96	Vert(LL)	-0.32 10-1°	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.98	Vert(TL)	-0.58 10-1	>617	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.57	Horz(TL)	0.17 1	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)					Weight: 198 lb	

LUMBER BRACING TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2 \*Except\*

3-16 2 X 6 SYP No.1D, 13-15 2 X 6 SYP No.1D

6-12 2 X 4 SYP No.3

**WEBS** 2 X 4 SYP No.3

**BOT CHORD** 

TOP CHORD Structural wood sheathing directly applied or

4-2-0 oc purlins, except end verticals. Rigid ceiling directly applied or 6-0-0 oc

bracing.

**WEBS** T-Brace:

2 X 4 SYP No.3 -

4-13, 8-10

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

**REACTIONS** (lb/size) 10=957/0-3-8, 2=1086/0-3-8

Max Horz 2=363(load case 5)

Max Uplift 10=-207(load case 7), 2=-299(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1423/506, 3-4=-1823/799, 4-5=-1119/545, 5-6=-1002/577,

6-7=-1044/606, 7-8=-1247/648, 8-9=-84/99, 9-10=-193/137

2-16=-348/1050, 15-16=-6/66, 3-15=-187/156, 14-15=-571/1504, 13-14=-571/1504,

12-13=-17/7, 6-13=-407/756, 11-12=-82/83, 10-11=-529/1249

**WEBS** 4-14=-55/411, 4-13=-739/437, 7-13=-298/227, 11-13=-226/918, 7-11=-106/216,

8-11=-398/326, 8-10=-1373/687

### JOINT STRESS INDEX

**BOT CHORD** 

2 = 0.66, 3 = 0.85, 4 = 0.41, 5 = 0.31, 6 = 0.88, 7 = 0.41, 8 = 0.66, 9 = 0.51, 10 = 0.57, 11 = 0.81, 12 = 0.44, 13 = 0.66, 14 = January 10,2008 Conthided 55 - page 2 and 16 = 0.56





Job	Truss	Truss Type	Qty	Ply	00
L265563	T34	SPECIAL	1	1	J1925189
220000	104	OI LOWE		į	Job Reference (optional)

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### **NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 207 lb uplift at joint 10 and 299 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Les Trues Design Engineer Florida PE No. 34868 Florida PE No. 34868 Boynton Beach, FL 32436



Job	Truss	Truss Type	Qty	Ply	00
L265563	T35	SPECIAL	1	1	J1925190
L200000	100	OI EGIAL	1	1.	Job Reference (optional)

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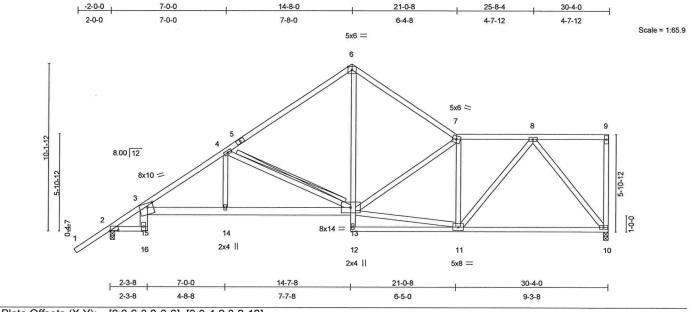


Plate Of	tsets (X, Y	): [2:0-6-3,0-0-6], [3:	0-4-2,0-2-	12]								
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.52	Vert(LL)	-0.15	1Ò-11	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.77	Vert(TL)	-0.28	14-15	>999	240	30.1.	
BCLL	10.0	* Rep Stress Incr	YES	WB	0.93	Horz(TL)	0.17	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 202 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 SYP No.2	TOP CHO

BOT CHORD 2 X 4 SYP No.2 \*Except\*

3-16 2 X 6 SYP No.1D, 13-15 2 X 6 SYP No.1D

6-12 2 X 4 SYP No.3

**WEBS** 2 X 4 SYP No.3 G

**WEBS** 

TOP CHORD Structural wood sheathing directly applied or 4-2-0 oc purlins, except end verticals. **BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

bracing, Except:

7-3-12 oc bracing: 15-16 9-10-6 oc bracing: 14-15.

T-Brace:

2 X 4 SYP No.3 -

4-13

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance. Brace must cover 90% of web length.

**REACTIONS** (lb/size) 10=957/0-3-8, 2=1086/0-3-8

Max Horz 2=384(load case 5)

Max Uplift 10=-216(load case 5), 2=-300(load case 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1423/508, 3-4=-1825/804, 4-5=-1115/547, 5-6=-1001/579,

6-7=-1090/591, 7-8=-1104/625, 8-9=-50/111, 9-10=-116/99

**BOT CHORD** 2-16=-380/1050, 15-16=-9/65, 3-15=-191/160, 14-15=-582/1504, 13-14=-582/1504,

12-13=0/78, 6-13=-366/712, 11-12=-63/144, 10-11=-217/661

**WEBS** 4-14=-62/420, 4-13=-741/437, 7-13=-383/293, 11-13=-366/990, 7-11=-642/355,

8-11=-316/707, 8-10=-1021/505

Cogineer : 34666 Bay Blvd :n. ft 99496

Continued on page 2





Job	Truss	Truss Type	Qty	Ply	00	
L265563	T35	SPECIAL	1	1	J19	925190
	300 to 80 00 W				Job Reference (optional)	

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### JOINT STRESS INDEX

2 = 0.66, 3 = 0.85, 4 = 0.41, 5 = 0.31, 6 = 0.98, 7 = 0.57, 8 = 0.48, 9 = 0.28, 10 = 0.63, 11 = 0.44, 12 = 0.80, 13 = 0.58, 14 = 0.33, 15 = 0.00 and 16 = 0.56

### **NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 216 lb uplift at joint 10 and 300 lb uplift at joint 2.

LOAD CASE(S) Standard

Julius Lire Truse Design Engineer Floride FE No. 23868 1 109 Coastel Bay Blvd. Boynton Besch. FL 98495



Job	Truss	Truss Type	Qty	Ply	00	٦
L265563	T36	SPECIAL	1	1	J1925191	
1203303	150	OF ECIAL	"	'	Job Reference (optional)	

6.300 s Feb 15 2006 MiTek Industries, Inc. Thu Jan 10 17:30:36 2008 Page 1

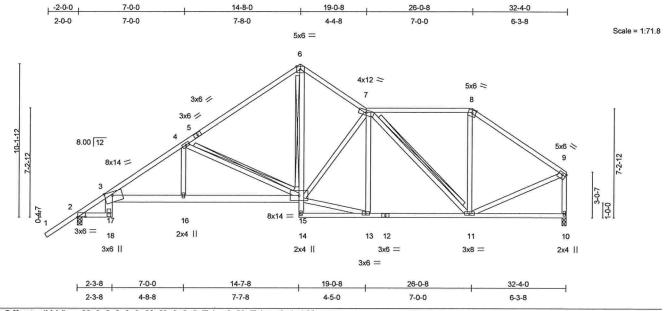


Plate Of	fsets (X,Y	): [2:0-6-3,0-0-6], [3:	0-6-2,Edg	e], [9:Ed	dge,0-1-1	12]						
LOADIN	G (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.50	Vert(LL)	-0.16	16-17	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.82	Vert(TL)	-0.31	16-17	>999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.40	Horz(TL)	0.18	10	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 217 lb	

LUMBER		BRACING
TOP CHORD	2 X 4 SYP No.2	TOP CHORD

BOT CHORD 2 X 4 SYP No.2 \*Except\*

3-18 2 X 6 SYP No.1D, 15-17 2 X 6 SYP No.1D

6-14 2 X 4 SYP No.3

**WEBS** 2 X 4 SYP No.3 \*Except\*

9-10 2 X 4 SYP No.2

**BOT CHORD** 

Structural wood sheathing directly applied or 3-11-6 oc purlins, except end verticals. Rigid ceiling directly applied or 6-3-12 oc

bracing. Except:

T-Brace:

2 X 4 SYP No.3 -

6-15

**WEBS** 

T-Brace:

2 X 4 SYP No.3 -4-15, 7-11

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in

minimum end distance.

Brace must cover 90% of web length.

**REACTIONS** (lb/size) 2=1150/0-3-8, 10=1021/0-3-8

Max Horz 2=341(load case 5)

Max Uplift 2=-308(load case 6), 10=-227(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1530/564, 3-4=-1972/876, 4-5=-1243/612, 5-6=-1125/644,

6-7=-1185/678, 7-8=-779/520, 8-9=-1015/526, 9-10=-988/494

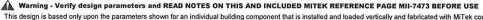
**BOT CHORD** 2-18=-385/1132, 17-18=-8/69, 3-17=-199/161, 16-17=-628/1630, 15-16=-628/1630,

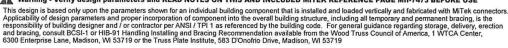
14-15=0/57, 6-15=-492/919, 13-14=-77/133, 12-13=-406/1163, 11-12=-406/1163,

**WEBS** 4-16=-63/429, 4-15=-766/449, 7-15=-429/324, 13-15=-342/1064, 7-13=-205/122,

7-11=-546/242, 8-11=-19/257, 9-11=-265/788

Continued on page 2







	Job	Truss	Truss Type	Qty	Ply	00	٦
	L265563	T36	SPECIAL	1	1	J1925191	i
1	L203303	130	OF EGIAL	1		Job Reference (optional)	1

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### JOINT STRESS INDEX

2 = 0.71, 3 = 0.88, 4 = 0.41, 5 = 0.34, 6 = 0.83, 7 = 0.73, 8 = 0.72, 9 = 0.70, 10 = 0.74, 11 = 0.71, 12 = 0.39, 13 = 0.57, 14 = 0.52, 15 = 0.74, 16 = 0.33, 17 = 0.00 and 18 = 0.61

### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 308 lb uplift at joint 2 and 227 lb uplift at joint 10.

LOAD CASE(S) Standard

Julius Les Truss Design Engineer Florida PE No. 24868 1100 Ceastal Bay Blvd. Boynton Besch. FL 33435



Job	Truss	Truss Type	Qty	Ply	0 0	
		0050141				J1925192
L265563	T37	SPECIAL	1	1		
			1	1	Job Reference (optional)	

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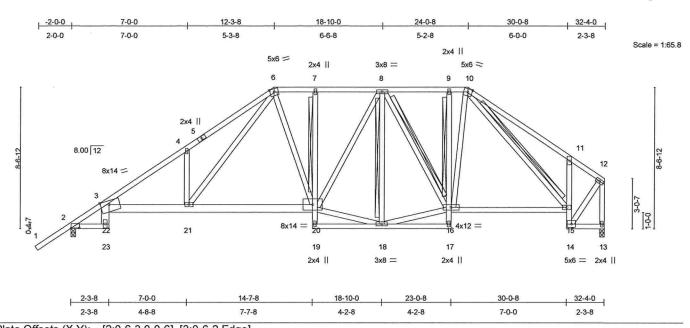


Plate Of	tsets (X, Y	): [2:0-6-3,0-0-6], [3:	0-6-2,Edg	ej							
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in (lo	c) I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.38	Vert(LL)	0.18 21-2	2 >999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.96	Vert(TL)	-0.33 21-2	2 >999	240		
BCLL	10.0	* Rep Stress Incr	YES	WB	0.67	Horz(TL)	0.25 1	3 n/a	n/a		
BCDL	5.0	Code FBC2004/TF	212002	(Mat	rix)					Weight: 257 lb	

LUMBER		BRACING		
TOP CHORD	2 X 4 SYP No.2	TOP CHORD	Structural wood sheathing	ng directly applied or
BOT CHORD	2 X 4 SYP No.2 *Except*		3-11-15 oc purlins, exce	ept end verticals.
	3-23 2 X 6 SYP No.1D, 20-22 2 X 6 SYP No.1D	<b>BOT CHORD</b>	Rigid ceiling directly app	lied or 2-2-0 oc
	7-19 2 X 4 SYP No.3, 9-17 2 X 4 SYP No.3		bracing. Except:	
	11-14 2 X 4 SYP No.3		T-Brace:	2 X 4 SYP No.3 -

**WEBS** 2 X 4 SYP No.3 \*Except\* 12-13 2 X 4 SYP No.2

**WEBS** 

T-Brace:

2 X 4 SYP No.3 -8-18, 8-16, 10-15

7-20, 9-16

Fasten T and I braces to narrow edge of web with 10d Common wire nails, 9in o.c., with 4in minimum end distance.

Brace must cover 90% of web length.

REACTIONS (lb/size) 2=1150/0-3-8, 13=1021/0-3-8

Max Horz 2=297(load case 5)

Max Uplift 2=-297(load case 6), 13=-173(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/62, 2-3=-1531/583, 3-4=-1953/885, 4-5=-2013/1063, 5-6=-1927/1085, TOP CHORD

6-7=-1117/685, 7-8=-1118/688, 8-9=-933/602, 9-10=-939/603, 10-11=-865/579,

11-12=-656/358, 12-13=-1137/533

2-23=-449/1134, 22-23=-12/69, 3-22=-208/174, 21-22=-623/1601, 20-21=-393/1074 **BOT CHORD** 

, 19-20=0/60, 7-20=-161/105, 18-19=-53/101, 17-18=-67/0, 16-17=0/59,

9-16=-80/101, 15-16=-247/878, 14-15=-588/283, 11-15=-288/275, 13-14=-23/54

**WEBS** 4-21=-349/316, 6-21=-488/888, 6-20=-186/273, 8-20=-145/320, 18-20=-268/900,

Continued on page 18=-445/173, 8-16=-152/99, 16-18=-294/1007, 10-16=-231/506, 10-15=-411/204,

January 10,2008

Engineer 'eay elva :n. YL 35435

🛕 Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE

This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building code. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00	
L265563	T37	SPECIAL	1	1		J1925192
LLCCCCC	101	0, 20, 12			Job Reference (optional)	

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### JOINT STRESS INDEX

2 = 0.71, 3 = 0.91, 4 = 0.33, 5 = 0.39, 6 = 0.46, 7 = 0.33, 8 = 0.59, 9 = 0.33, 10 = 0.62, 11 = 0.50, 12 = 0.67, 13 = 0.40, 14 = 0.40, 0.55, 15 = 0.69, 16 = 0.82, 17 = 0.46, 18 = 0.89, 19 = 0.50, 20 = 0.65, 21 = 0.62, 22 = 0.00 and 23 = 0.61

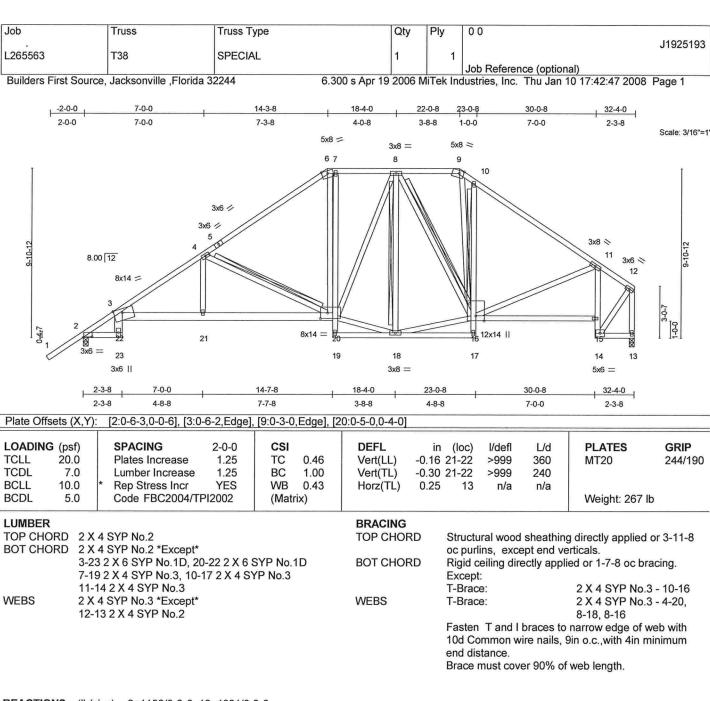
### NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 3x6 MT20 unless otherwise indicated.
- 6) The following joint(s) require plate inspection per the Tooth Count Method when this truss is chosen for quality assurance
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 297 lb uplift at joint 2 and 173 lb uplift at joint 13.

LOAD CASE(S) Standard

us Les se Cesign Engineer se Cesign Engles NO Cesiul Bay Blod onton Besch, FL 33435





REACTIONS (lb/size) 2=1150/0-3-8, 13=1021/0-3-8

Max Horz 2=334(load case 5)

Max Uplift 2=-306(load case 6), 13=-179(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/62, 2-3=-1530/572, 3-4=-1970/885, 4-5=-1245/623, 5-6=-1135/654, 6-7=-958/640,

7-8=-946/629, 8-9=-808/567, 9-10=-1078/772, 10-11=-1160/601, 11-12=-655/351,

12-13=-1138/526

BOT CHORD 2-23=-426/1132, 22-23=-11/69, 3-22=-202/165, 21-22=-633/1625, 20-21=-633/1625,

19-20=0/48, 7-20=-238/247, 18-19=-56/98, 17-18=-46/25, 16-17=0/69, 10-16=-287/315,

15-16=-241/668, 14-15=-607/298, 11-15=-584/323, 13-14=-24/59

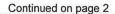
WEBS 4-21=-68/428, 4-20=-752/438, 8-18=-374/137, 18-20=-192/774, 8-20=-153/308,

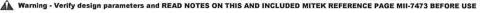
16-18=-225/834, 8-16=-174/124, 9-16=-398/497, 11-16=-131/262, 12-14=-411/912,

6-20=-129/254

Johns Lee Truss Design Engineer Florida FE No. 34869 1100 Coastal Bay Blvd Bovoton Basch, Et. 33436

January 10,2008





This design is based only upon the parameters shown for an individual building component that is installed and loaded vertically and fabricated with MiTek connectors. Applicability of design parameters and proper incorporation of component into the overall building structure, including all temporary and permanent bracing, is the responsibility of building designer and / or contractor per ANSI / TPI 1 as referenced by the building occel. For general guidance regarding storage, delivery, erection and bracing, consult BCSI-1 or HIB-91 Handling Installing and Bracing Recommendation available from the Wood Truss Council of America, 1 WTCA Center, 6300 Enterprise Lane, Madison, WI 53719 or the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Job	Truss	Truss Type	Qty	Ply	00
L265563	T38	SPECIAL	1	1	J1925193
					Job Reference (optional)

6.300 s Apr 19 2006 MiTek Industries, Inc. Thu Jan 10 17:42:47 2008 Page 2

### JOINT STRESS INDEX

2 = 0.73, 3 = 0.90, 4 = 0.43, 5 = 0.32, 6 = 0.64, 7 = 0.34, 8 = 0.65, 9 = 0.35, 10 = 0.75, 11 = 0.95, 12 = 0.71, 13 = 0.43, 14 = 0.57, 15 = 0.77, 16 = 0.30, 17 = 0.61, 18 = 0.75, 19 = 0.40, 20 = 0.89, 21 = 0.34, 22 = 0.00 and 23 = 0.63

### **NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS and C-C Exterior(2) zone; end vertical right exposed; Lumber DOL=1.60 plate grip DOL=1.60. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 306 lb uplift at joint 2 and 179 lb uplift at joint 13.

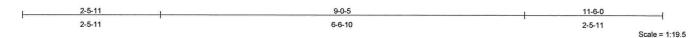
LOAD CASE(S) Standard

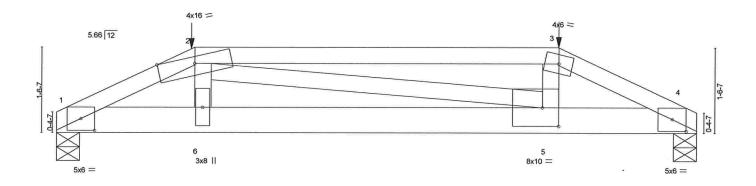
Julius Lee Truse Design Engineer Flonda FE No. 34869 1 109 Geestel Bay Blvd Boynton Besch, FL 93496



Job	Truss	Truss Type	Qty	Ply	00
L265563	T39	HIP	1	_	J1925194
L203303	139	nie	1	2	Job Reference (optional)

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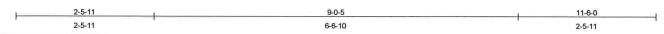


Plate Of	fsets (X,Y	): [1:0-3-0,0-2-9], [4:	0-3-0,0-2-	9], [5:0-	3-8,0-4-0	0]						
LOADIN	IG (psf)	SPACING	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plates Increase	1.25	TC	0.33	Vert(LL)	-0.12	5-6	>999	360	MT20	244/190
TCDL	7.0	Lumber Increase	1.25	BC	0.68	Vert(TL)	-0.24	5-6	>563	240	Value de Accession.	
BCLL	10.0	* Rep Stress Incr	NO	WB	0.42	Horz(TL)	0.03	4	n/a	n/a		
BCDL	5.0	Code FBC2004/TF	PI2002	(Mat	rix)						Weight: 113 lb	

LUMBER	
TOP CHORD	2 X 4 SYP No.2
<b>BOT CHORD</b>	2 X 6 SYP No.1D
WEBS	2 X 4 SYP No.3

### BRACING

TOP CHORD

Structural wood sheathing directly applied or

5-3-6 oc purlins.

**BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (lb/size) 1=3052/0-4-15, 4=3052/0-4-15

Max Horz 1=14(load case 5)

Max Uplift 1=-832(load case 4), 4=-832(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-6331/1744, 2-3=-5815/1616, 3-4=-6176/1700 **BOT CHORD** 1-6=-1558/5636, 5-6=-1641/5961, 4-5=-1509/5500 **WEBS** 2-6=-677/2613, 2-5=-192/57, 3-5=-653/2517

### JOINT STRESS INDEX

1 = 0.75, 2 = 0.74, 3 = 0.63, 4 = 0.73, 5 = 0.26 and 6 = 0.41

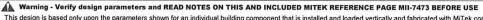
### **NOTES**

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc. Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

2) Hinhalanced ager live loads have been considered for this design.

January 10,2008



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Job	Truss	Truss Type	Qty	Ply	00	
1 205552	T20	LUID			J192	25194
L265563	T39	HIP	1	2	Job Reference (optional)	

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### NOTES

- 4) Wind: ASCE 7-02; 110mph (3-second gust); h=19ft; TCDL=4.2psf; BCDL=3.0psf; Category II; Exp B; enclosed; MWFRS; Lumber DOL=1.60 plate grip DOL=1.60.
- 5) Provide adequate drainage to prevent water ponding.
- 6) \*This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) All bearings are assumed to be SYP No.2 crushing capacity of 565.00 psi
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 832 lb uplift at joint 1 and 832 lb uplift at joint 4.

Loading has been calculated by the truss manufacturer. It is the responsibility of the Architect/Engineer of Record to verify and approve the loading.

### LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-54, 2-3=-56(F=-2), 3-4=-54, 1-4=-491(F=-481)

Concentrated Loads (lb)

Vert: 2=-22(F) 3=-22(F)

Julius Les Trues Design Engineer Flonds PE No. 24888 1109 Coastel Bay Blvd.

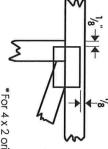


## Symbols

# PLATE LOCATION AND ORIENTATION



\*Center plate on joint unless dimensions indicate otherwise. Dimensions are in inches. Apply securely seat plates to both sides of truss and



\*For 4 x 2 orientation, locate plates 1/8" from outside edge of truss and vertical web.



\*This symbol indicates the required direction of slots in connector plates

## PLATE SIZE

4 × 4

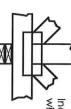
dimension is the length paralle The first dimension is the width perpendicular to slots. Second to slots.

## LATERAL BRACING



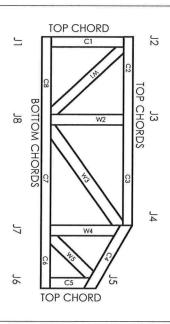
continuous lateral bracing. Indicates location of required

### BEARING



which bearings (supports) occur. Indicates location of joints at

# Numbering System



JOINTS AND CHORDS ARE NUMBERED CLOCKWISE AROUND THE TRUSS STARTING AT THE LOWEST JOINT FARTHEST TO THE LEFT.

WEBS ARE NUMBERED FROM LEFT TO RIGHT

# CONNECTOR PLATE CODE APPROVALS

ICBO

3907, 4922

BOCA

SBCCI

9667, 9432A

WISC/DILHR 960022-W, 970036-N

NER

561





MiTek Engineering Reference Sheet: MII-7473

## Failure to Follow Could Cause Property Damage or Personal Injury General Safety Notes

- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 2 Cut members to bear tightly against each other.
- ω Place plates on each face of truss at each joint and embed fully. Avoid knots and wane at joint locations.
- Unless otherwise noted, locate chord splices at 1/4 panel length (± 6" from adjacent joint.)
- 5 Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- 0 Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.

7

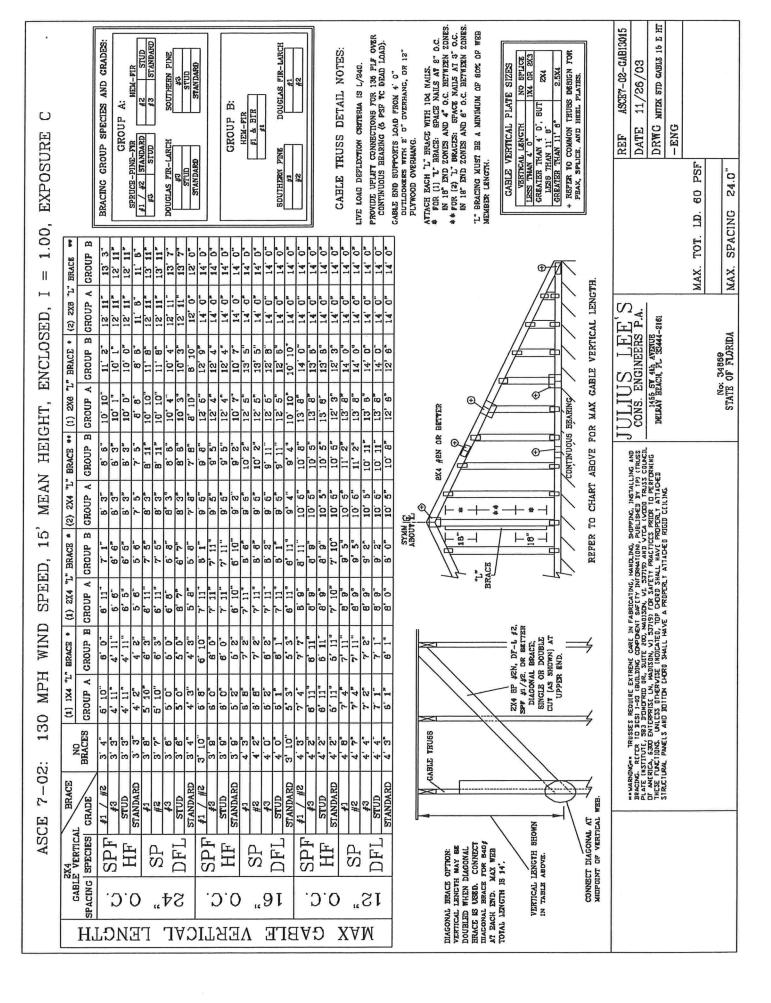
Camber is a non-structural consideration and

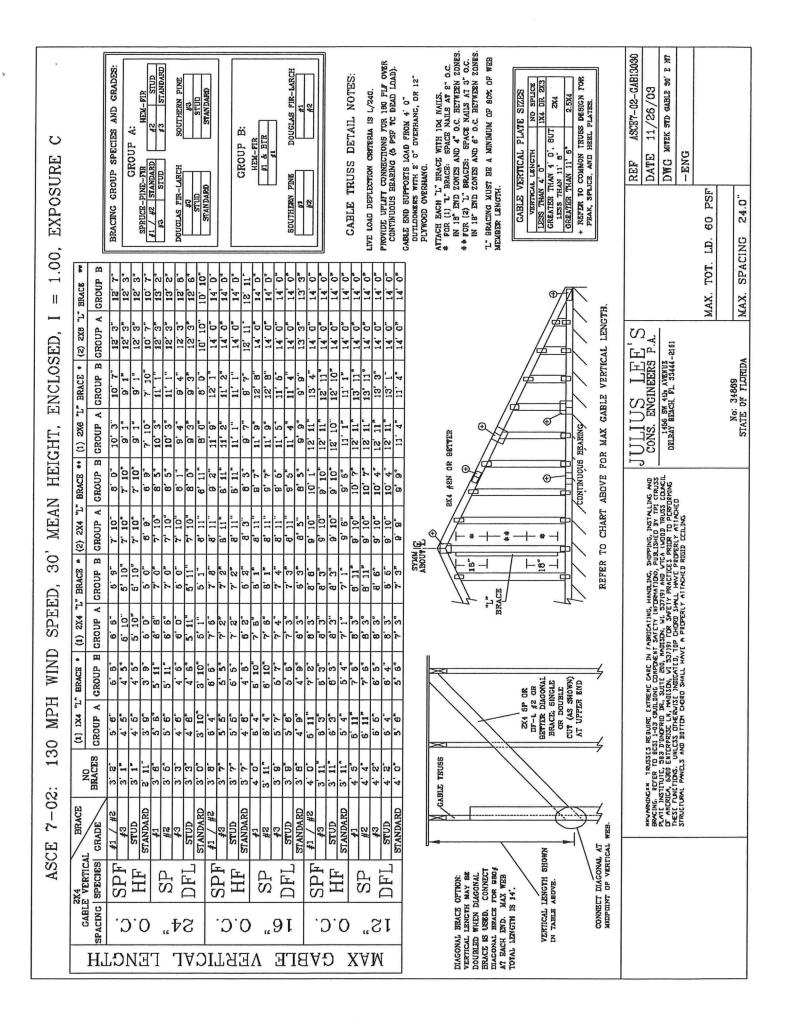
is the responsibility of truss fabricator. General

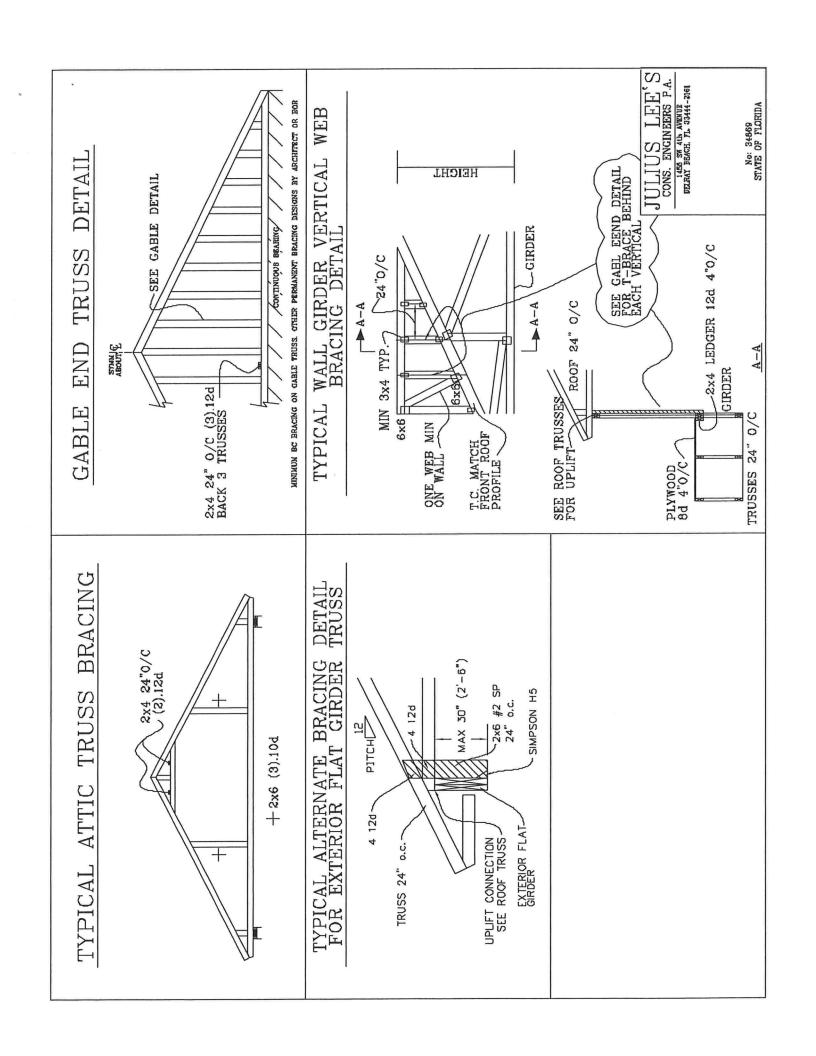
practice is to camber for dead load deflection

- œ Plate type, size and location dimensions shown indicate minimum plating requirements.
- Lumber shall be of the species and size, and in all respects, equal to or better than the grade specified.
- Top chords must be sheathed or purlins provided at spacing shown on design.
- 11. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed unless otherwise noted
- Anchorage and / or load transferring connections to trusses are the responsibility of others unless shown.
- 13. Do not overload roof or floor trusses with stacks of construction materials
- 14. Do not cut or alter truss member or plate without prior approval of a professional engineer.
- Care should be exercised in handling erection and installation of trusses.

# 1993 MiTek® Holdings, Inc.







## 2X4 2X4 2X4 CHORD

PIGGYBACK DETAIL

888 200 TOP

REFER TO SEALED DESIGN FOR DASHED PLATES.

SPACE PIGGYBACK VERTICALS AT 4' OC MAX

SO THAT ONE SPLICE TOP AND BOTTOM CHORD SPLICES MUST BE STAGGERED IS NOT DIRECTLY OVER ANOTHER.

ATTACH VERTICAL WEBS TO PIGGYBACK BOTTOM CHORD MAY BE OMITED. TRUSS TOP CHORD WITH 1.5X3 PLATE.

ATTACH PURLINS TO TOP OF FIAT TOP CHORD. IF PIGGYBACK IS SOLID LUMBER OR THE BOTTOM CHORD IS OMITTED, PURLINS MAY BE APPLIED BENEATH THE TOP CHORD OF SUPPORTING TRUSS.

REFER TO ENCINEER'S SEALED DESIGN POR REQUIRED PURLIN SPACING.

THIS DETAIL IS APPLICABLE FOR THE POLLOWING WIND CONDITIONS:

110 MPH WIND, 30 MEAN HGT, ASCE 7-02, CLOSED BLDC, LOCATED ANYWHERE IN ROOF, 1 MI FROM COAST 1. EXP C. WIND TC DL=5 FSF, WIND BC DL=5 FSF GLOSED BLDG, LOCATED ANYWHERE IN ROOF WIND TC DL-5 FSF, WIND GC DL-5 FSF, WIND GC DL-5 FSF.

FRONT FACE (E,\*) PLATES MAY BE OFFSET FROM BACK FACE PLATES AS LONG AS BOTH FACES ARE SPACED 4' OC MAX.

CLOSED EXP. C. 130 MPH WIND, 30' MEAN BCT, ASCE 7-02, BLDG, LOCATED ANYWHERE IN ROOF, CAT II, WIND TC DL=6 PSF, WIND BC DL=6 PSF

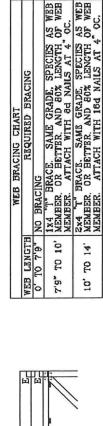
2X12

MAX SIZE OF 2 #2 OR BETTER

凸

图

20' FLAT TOP CHORD MAX SPAN



ATTACH TRULOX PLATES WITH (8) 0.120° X 1.375° NAILS, OR EQUAL, PER FACE PER PLY. (4) NAILS IN EACH MEMBER TO BE CONNECTED. REFER TO DRAWING 160 TL FOR TRULOX INFORMATION.

1.5X4

1.5X4

1.5X4

1.5X3

**5**X6

**5X8** 

5X6

335

2.5X4

2.5X4

2X4 4X8

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d S SPANS S,

OR 3X6 TRULOX AT 4' ROTATED VERTICALLY

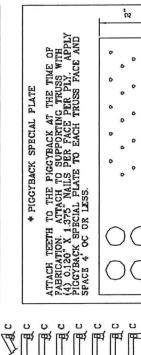
4XB

538

**6X5** 

5X5

5X4



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D-SPLICE

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EITHBR PLATE
LOCATION IS
ACCEPTABLE

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THIS DRAWING REPLACES DRAWINGS 634,018 634,017 ALPINE PIGGYBACK SPECIAL PLATE. O.S. 3X6 TRULOX PIGCYBACK WITH \*ATTACH MAX []

0,001 UIII	MAX LOADING	REF PIGCYBA	YBA
ENGINE	55 PSF AT	DATE 09/12/0	8/0
1400 SW 4th AVENUE	1.33 DUR. FAC.	DRWGMITEK	ST
DIMENT BOACH, IN DOTTE-SIDE	50 PSF AT 1.25 DUR. FAC.	-ENG JL	
	47 PSF AT 1.15 DUR. FAC.		
No: 34869 STATE OF PLORIDA	SPACING 24.0"		

NAVARNINGEM TRISSES REGLIRE EXTREME CARE IN FARRICATING, HANDLING, SHIPPING, INSTALLING AND BAZING. REFER TO EXELLED GRILLING CHPORNING SAFETY INFORMATION, PLEISTING BY THI (TRISSS PRACTICE REFER TO EXELLED BY THE INSTITUTE, 383 DEDOCTION DES, SUITE COD, MADISON, VI. 33739 AND WITCH COOD TRISS COUNCIL DESTROYERSEE LIN HADISON, MI 23739 FOR SAFETY PRACTICES FRIEND TO PERFERRING TO PERFERRING TO PERFERRING TO PERFERRING TO PERFERRING TO PERFERRING STRUCTURAL, DATESS DIHERVISE TO PROTECT AND STRUCT TO PERFER TO STRUCTURAL. AND BOTTON CHOOL DESCRIPTION.

D PIGGY

847,045

\*

8 1/4"

**1CK** 

## DETAIL TRUSS VALLEY

HORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. HORD 2X3(\*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER. WEBS 2X4 SP #3 OR BETTER. TOP CHORD BOT CHORD

- \* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
- (2) 16d BOX (0.135" X 3.5") NAILS TOE—NAILED FOR FEC 2004 110 MPH, ASCE 7-02 110 MPH WIND OR (3) 16d FOR ASCE 7-02 130 MPH WIND. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C. RESIDENTIAL, WIND TC DL=5 PSF. ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: \*

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD. 444

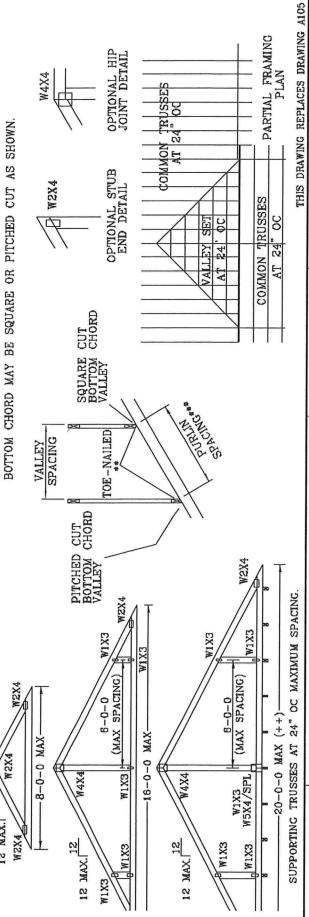
LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0". +

4-0-0 MAX

12 MAX.

CUT FROM 2X6 OR LARGER AS REQ'D

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



JOLIOS L	1455 SW 445, AVI DRIBAY BRACH, FL 33
BEACH HIGH TRUSTES REGIDE EXTREME CARE IN FABRICATING, MANDLING, SHIPPING, INSTALLING AND PRACTICE OF THE PROPERTY OF THE PROP	PLATE DISTRICTION, SEED DOUGHERD BY, SUIT END, MADISON, VI, SETTIN PROFILED, PUBLISHED BY THE HISTORY OF ARRENT COUNCIL OF ARRESTED BY, MADISON, VI, SETTIN PROFILED BY THE YOUR TOUR THANKS COUNCIL WHICK FUNCTIONS. UKE'SS OTHERWAYSE HOW ARRESTED TO CHOOD SHALL HAVE PROFILEY ATTACKED STRUCK THANKS AND MOTION CHOOD SHALL HAVE PROFILEY. ATTACKED STRUCK DANKS AND MOTION CHOOD SHALL HAVE FOR SHALL H

20 20 PSF REF VALLEY DETAIL	L 7 15 PSF DATE 11/26/03	L 5 5 PSF DRWG VALTRUSS1103	L 0 0 PSF -ENG JL	LD. 32 40 PSF	C. 1.25 1.25	"VC CIVI
TC II	TC DL	BC DI	BC LL	TOT. LD.	DUR.FAC. 1.25	SDACTNC
US LEE S	ERS P.A.	1455 SW 44h AVENUE IY BEACH, FL 33444-2161			No. 34860	TATE OF FLORIDA

# TOE-NAIL DETAIL

TOE-NAILS TO BE DRIVEN AT AN ANGLE OF APPROXIMATELY THIRTY DEGREES WITH THE PIECE AND STARTED APPROXIMATELY ONE-THIRD THE LENGTH OF THE NAIL FROM THE END OF THE MEMBER.

PER ANSI/AF&PA NDS-2001 SECTION 12.4.1 — EDGE DISTANCE, END DISTANCE, SPACINC: "EDGE DISTANCES, END DISTANCES AND SPACINGS FOR NAILS AND SPIKES SHALL BE SUFFICIENT TO PREVENT SPLITTING OF THE WOOD.

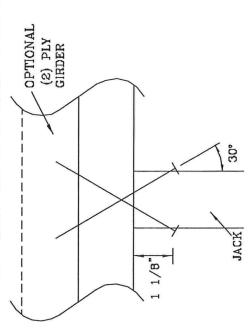
THE NUMBER OF TOE-NAILS TO BE USED IN A SPECIFIC APPLICATION IS DEPENDENT UPON PROPERTIES FOR THE CHORD SIZE, LUMBER SPECIES, AND NAIL TYPE, PROPER CONSTRUCTION PRACTICES AS WELL AS GOOD JUDGEMENT SHOULD DETERMINE THE NUMBER OF NAILS TO BE USED.

THIS DETAIL DISPLAYS A TOE-NAILED CONNECTION FOR JACK FRAMING INTO A SINGLE OR DOUBLE PLY SUPPORTING GIRDER.

COMMON TOE-NAILS
COMMON
(0.162"X3.5")
OF 16d (0.10
RESISTANCE
VERTICAL
MAXIMUM

NUMBER OF		SOUTHERN PINE	DOUGLAS	DOUGLAS FIR-LARCH	HEM-FIR	-FIR	SPRUCE	SPRUCE PINE FIR
TOE-NAILS	1 PLY	2 PLIES	2 PLIES 1 PLY	2 PLIES	1 PLY	1 PLY 2 PLIES	1 PLY 2 PLIES	2 PLIES
82	197#	256#	181#	234#	156#	#602	#791	188#
ဗ	#962	383#	271#	351#	234#	304#	#062	#862
4	394#	511#	361#	468#	312#	400+	#208	#268
Ó	483#	#669	452#	285#	#068	207#	384#	496#

ALL VALUES MAY BE MULTIPLIED BY APPROPRIATE DURATION OF LOAD FACTOR.



OPTIONAL (2) PLY GIRDER		CONDITIO
(2)	1 1/8"	ALTERNATIVE CONDITIO
	1	AJACK
	3060	\

THIS DRAWING REPLACES DRAWING 784040

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WANDANIA, TRUSSES REQUIRE EXTRECT CARE IN PREMICATIVE, AND SUPPING, INSTALLING AND PRACTICAL TO BEST 1-43 COULDING COMPOSENT SAFETY (INTORNATION, PUBLISHED BY TP3 TRAISS PACETY (INTORNATION, PUBLISHED BY TP3 TRAISSES, VIA SAFETY PRACTICES PRING THUS EXCENTIONS, UNICESS DIFERVISED (NOTICE), TIP CHERS SAFETY PRACTICES PRING TO PERPENHING THESE TRUSTICAL PANTELS AND BUTTON CHORD SHALL HAVE A PROPERTY ATTACHED RIGID CELLING.	

TOE-NAIL	09/12/07	CNTONAIL:103	: JL			
PSF REF	DATE	DRWG	-ENG			
PSF	PSF	PSF	PSF	PSF	1.00	
TC LL	TC DL	BC DL	BC IL	TOT. LD.	DUR. FAC.	SPACING
	CONS. ENGINEERS P.A. TC DI.	1455 SW 41h AVENUE DELKAV BEACH, FL 33444-2161			No. 34869	STATE OF FLORIDA
	98	200				

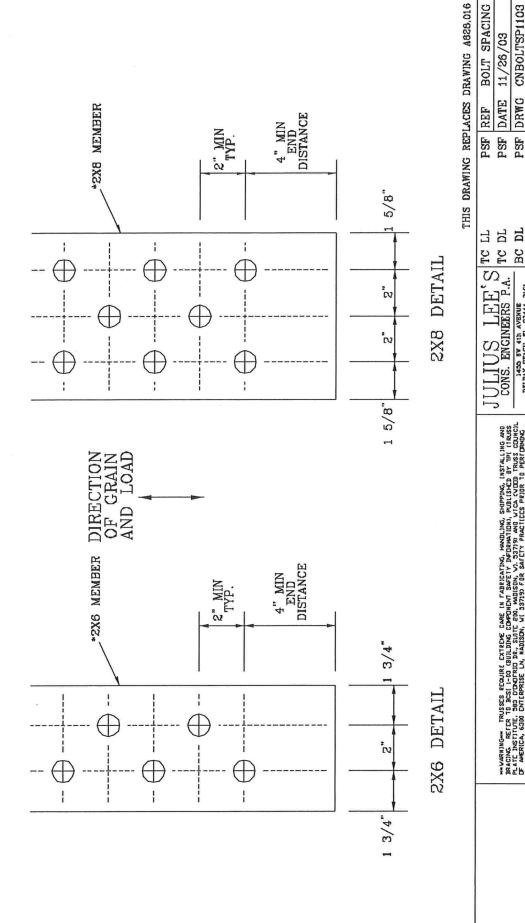
# TO GRAIN SPACING FOR LOAD APPLIED PARALLEL BOLT DIAMETER ,2,

\* GRADE AND SPECIES AS SPECIFIED ON THE ALPINE DESIGN.

BOLT HOLES SHALL BE A MINIMUM OF 1/32" TO A MAXIMUM OF 1/16" LARGER THAN BOLT DIAMETER.

BOLT TYPICAL LOCATION OF 1/2" DIAMETER THRU BOLTS. QUANTITIES AS NOTED ON SEALED DESIGN MUST BE IN ONE OF THE PATTERNS SHOWN BELOW.

WASHERS REQUIRED UNDER BOLT HEAD AND NUT



CNBOLTSP1103

DRWG -ENG

PSF PSF

BC DL BC LL

DELRAY BEACH, FL 3344-2161

DUR. FAC SPACING

No: 34869 STATE OF FLORIDA

TOT. LD.

# TRULOX CONNECTION DETAII

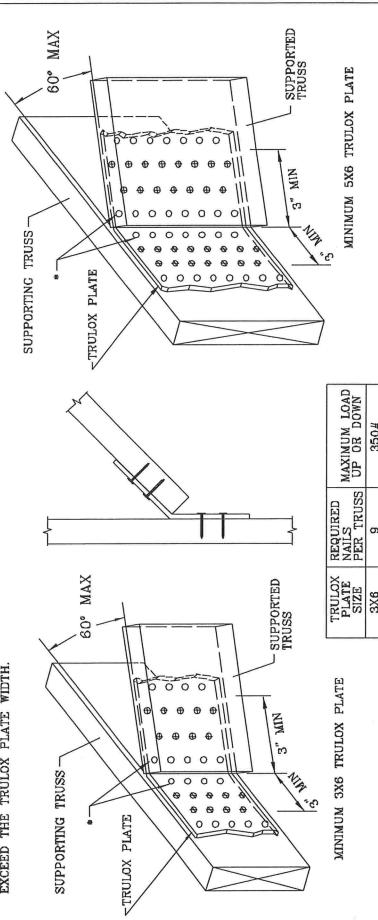
11 GAUGE (0.120" X 1.375") NAILS REQUIRED FOR TRULOX PLATE ATTACHMENT. FILL ROWS COMPLETELY WHERE SHOWN (\$\phi\$).

NAILS MAY BE OMITTED FROM THESE ROWS.

THIS DETAIL MAY BE USED WITH SO. PINE, DOUGLAS-FIR OR HEM-FIR CHORDS WITH A MINIMUM 1.00 DURATION OF LOAD OR SPRUCE-PINE-FIR CHORDS WITH A MINIMUM 1.15 DURATION OF LOAD. CHORD SIZE OF BOTH TRUSSES MUST EXCEED THE TRULOX PLATE WIDTH.

TRULOX PLATE IS CENTERED ON THE CHORDS AND BENT BETWEEN NAIL ROWS.

REFER TO ENGINEER'S SEALED DESIGN REFERENCING THIS DETAIL FOR LUMBER, PLATES, AND OTHER INFORMATION NOT SHOWN.



	IHIS DI	1,154,84	JLIUS LEE'S CONS. ENGINEERS P.A. 165 SW 4th AVENUE DELRAY BRACH, 7L, 38444-2761
320 <i>#</i>	#066		IOC .
			PING, JKSTALLJN LJSHED BY TP) ( 1 (NCDD TRUSS O RRIDR TO PERFER FERLY ATTACHED SID CELLING
6	15		GICATING, HAMILING, SHIPPING, INSTALLII FETY INCORNATION, PLIBLISCED BY TP. NY, NY, 337199, AND VICE AVOID TRUSS FOR SALEY, PRACFILES, PRINK TO PERPO PROPERLY ATTACHED RIGID CELLING.
9XE	9X9		FROURE EXTROME CARE IN FABRICATING, HAMBLING, SHIPPING, INSTALLING AND 1) -FOR (BUILDING DEPONENT SWETTY INFORMATION), PUBLISHED BY TP1 (IRAGS) PROPERTION, WILLIAM SALIE FOR MAISSON, VI. 38719 AND VICEA (NOTE) REMBERS COUNCED FROM TO PERTORNING SMILEN HAVE INDIDATED, TP CHORD SHALL HAVE PROPERLY ATTACHED BUILD CHORD SHALL HAVE PROPERLY ATTACHED
i.			**************************************
	6	9 350# 15 990#	9 350# 15 990#

1,158,989/R

THIS DRAWING REPLACES DRAWINGS 1,168,989 1,158,98 1,154,944 1,152,217 1,152,017 1,159,154 & 1,151,524

CNTRULOX1103

DRWG DATE

JI.

-ENG

11/26/03 TRULOX

REF

No: 34869 STATE OF FLORIDA

