

APPLICANTPATRICK WILSON

PHONE352.206.5459

ADDRESS6800SOUTHPOINT PKWY. # 300JACKSONVILLEFL32216

OWNERMARONDA HOMES INC. OF FLORIDA

PHONE352.206.5459

ADDRESS231SW TIMBER RIDGE DRIVELAKE CITYFL32024

CONTRACTORTHEODORE C.BROCK

PHONE904.227.3504

LOCATION OF PROPERTY90-W TO SR.247-S,TL TO C-252-B,TR TO TIMBER RIDGE,TL
AND IT'S THE 6TH LOT ON R.

TYPE DEVELOPMENTSFD/UTILITY

ESTIMATED COST OF CONSTRUCTION89950.00

HEATED FLOOR AREA1312.00

TOTAL AREA1799.00

HEIGHT21.00

STORIES1

FOUNDATIONCONC

WALLSFRAMED

ROOF PITCH6'12

FLOORCONC

LAND USE & ZONINGRSF-2

MAX. HEIGHT35

Minimum Set Back Requirments:

STREET-FRONT25.00

REAR15.00

SIDE10.00

NO. EX.D.U.0

FLOOD ZONEXPP

DEVELOPMENT PERMIT NO.

PARCEL ID10-4S-16-02856-106

SUBDIVISIONTIMBERLANDS

LOT6

BLOCK

PHASE

UNIT

TOTAL ACRES0.50

000001524

CRC1326913



Culvert Permit No.

Culvert Waiver

Contractor's License Number

Applicant/Owner/Contractor

18"X32'MITERED

07-0999

BLK

JTH

N

Driveway Connection

Septic Tank Number

LU & Zoning checked by

Approved for Issuance

New Resident

COMMENTS: ELEVATION CONFIRMATION LETTER REQUIRED.NOC ON FILE.

Check # or Cash861762

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power

Foundation

Monolithic

date/app. by

date/app. by

date/app. by

Under slab rough-in plumbing

Slab

Sheathing/Nailing

date/app. by

date/app. by

date/app. by

Framing

Rough-in plumbing above slab and below wood floor

date/app. by

date/app. by

Electrical rough-in

Heat & Air Duct

Peri. beam (Lintel)

date/app. by

date/app. by

date/app. by

Permanent power

C.O. Final

Culvert

date/app. by

date/app. by

date/app. by

M/H tie downs, blocking, electricity and plumbing

Pool

date/app. by

date/app. by

Reconnection

Pump pole

Utility Pole

date/app. by

date/app. by

date/app. by

M/H Pole

Travel Trailer

Re-roof

date/app. by

date/app. by

date/app. by

BUILDING PERMIT FEE \$450.00

CERTIFICATION FEE \$9.00

SURCHARGE FEE \$9.00

MISC. FEES \$0.00

ZONING CERT. FEE \$50.00

FIRE FEE \$0.00

WASTE FEE \$

FLOOD DEVELOPMENT FEE \$

FLOOD ZONE FEE \$25.00

CULVERT FEE \$25.00

TOTAL FEE568.00

INSPECTORS OFFICE

CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

For Office Use Only Application # 801-09 Date Received 1/3/08 By JW Permit # 1524 | 2662
Zoning Official BLK Date 08.01.08 Flood Zone Xpudat FEMA Map # N/A Zoning RSF-2
Land Use B2D Elevation N/A MFE 100' p.p.t River N/A Plans Examiner OK JIH Date 1-7-08
Comments Elevation Confirmation Letter Required

☒ NOC ☐ EH ☐ Deed or PA ☐ Site Plan ☐ State Road Info ☐ Parent Parcel #
☐ Dev Permit # ☐ In Floodway ☐ Letter of Authorization from Contractor
☐ Unincorporated area ☒ Incorporated area ☐ Town of Fort White ☐ Town of Fort White Compliance letter

Septic Permit No. 07-999 Patrick Wilson → Fax 352-206-5859
(904)-332-6367

Name Authorized Person Signing Permit Theodore C. Brock/Melissa McKague Phone (904)-296-1490

Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216

Owners Name Maronda Homes Inc. of Florida Phone (904)-296-1490

911 Address 231 SW Timber Ridge Dr. Lake City, FL 32024

Contractors Name Theodore C. Brock Phone (407)-227-3504

Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Tomas Ponce 4005 Maronda Way Sanford, FL 32771

Mortgage Lenders Name & Address Bank of America 250 Park Ave. S. #400 Winter Park, FL 32789

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress Energy

Property ID Number 10-45-10-02850-156 Estimated Cost of Construction 55,831.00

Subdivision Name Timberlands Lot 0 Block Unit Phase 1

Driving Directions Hwy 90, Left on 247 South; Right on 252B; Left on Timber Ridge 6th lot

on Right

Number of Existing Dwellings on Property

Construction of Residential Single Family Dwelling Total Acreage .50 Lot Size

Do you need a Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 21'

Actual Distance of Structure from Property Lines - Front 25.50' Side 38.5' Side 37.9' Rear 89.2'

Number of Stories 1 Heated Floor Area 1312 Total Floor Area 1799 Roof Pitch 6/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.



Owners Signature Theodore C. Brock

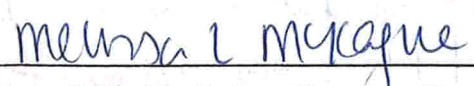
CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.



Contractor's Signature (Permitee) Theodore C. Brock

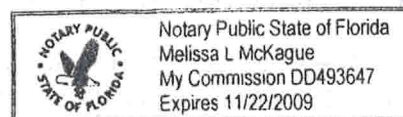
Contractor's License Number CRC 1326913
Columbia County
Competency Card Number _____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 10 day of December 2007.
Personally known XXX or Produced Identification _____



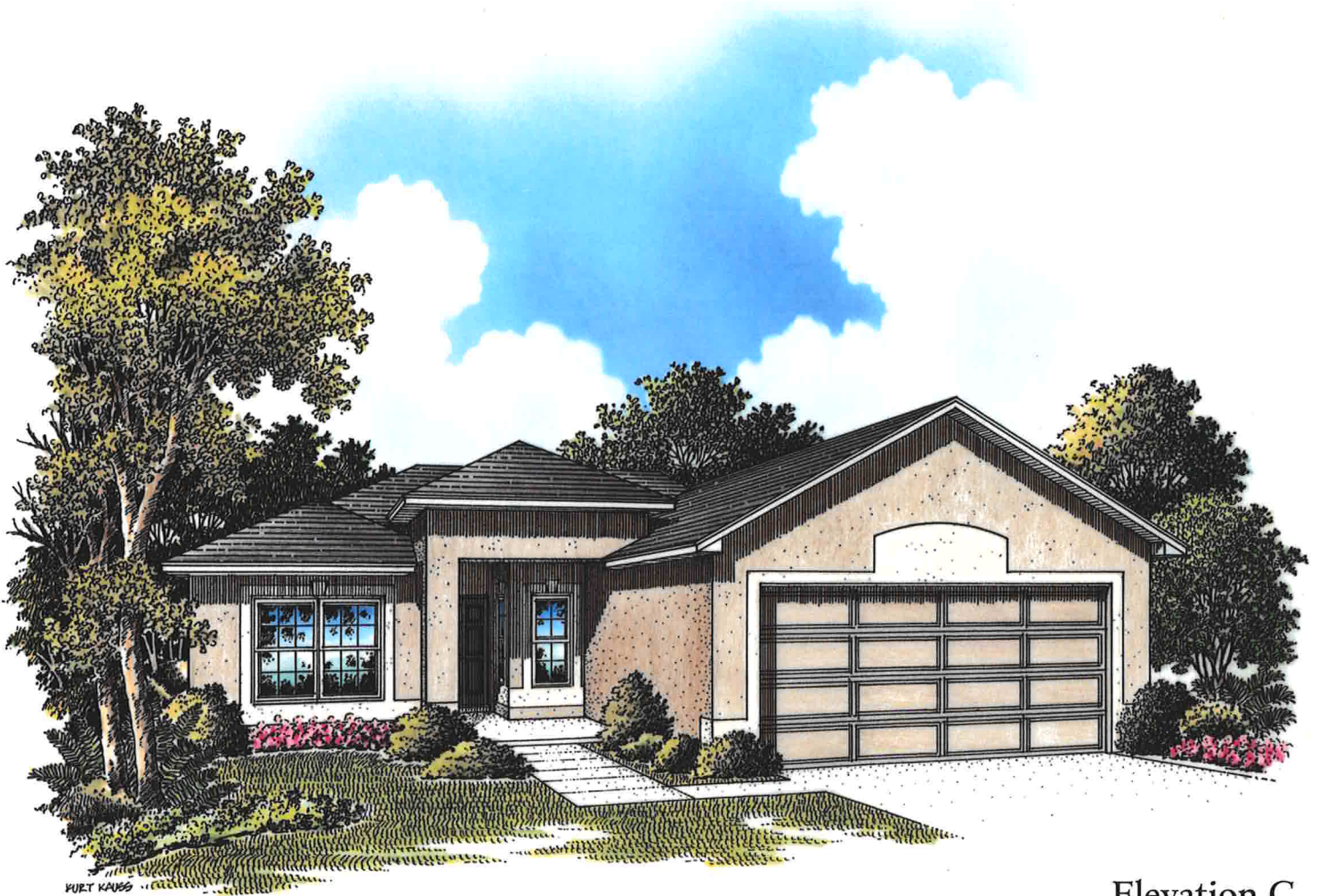
State of Florida Notary Signature (For the Contractor)
Melissa L. McKague

SEAL:



Montgomery

AMERICANA SERIES
3 Bedroom



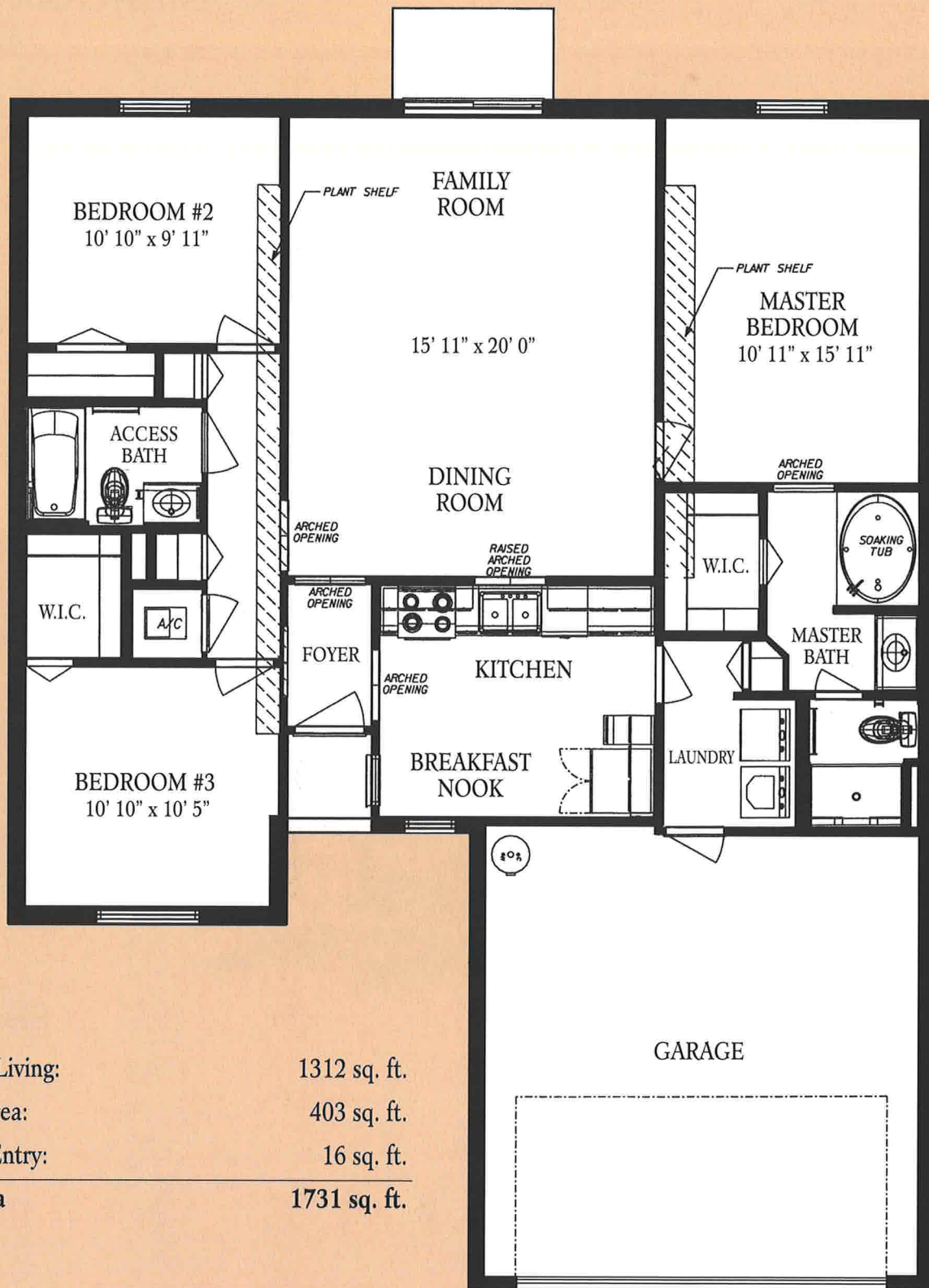
Elevation C

 **Maronda Homes**

Providing More Home for Less Money . . . It's the Way We Do Business!

Montgomery

AMERICANA SERIES
3 Bedroom



1st Floor Living:	1312 sq. ft.
Garage Area:	403 sq. ft.
Covered Entry:	16 sq. ft.
Total Area	1731 sq. ft.

Prices, plans, dimensions, features, specifications, materials and availability of Maronda Homes or communities are subject to change without prior notice or obligation. Maronda Homes illustrations are artist's depictions only and may differ from completed improvements. Copyright 2002 © Maronda Homes Inc. of Florida. All Rights Reserved.

 **Maronda Homes**
www.maronda.com

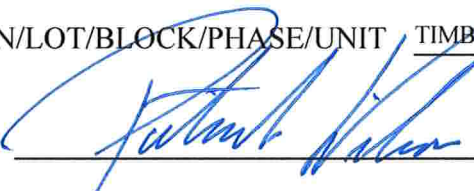


7/07

Columbia County Building Department Culvert Permit

Culvert Permit No.
000001524

DATE 01/17/2008 PARCEL ID # 10-4S-16-02856-106
APPLICANT PATRICK WILSON PHONE 352.206.5459
ADDRESS 6800 SOUTHPPOINT PKWY.#300 JACKSONVILLE FL 32216
OWNER MARONDA HOMES INC. OF FLORIDA PHONE 904.296.1490
ADDRESS 231 SW TIMBER RIDGE DRIVE LAKE CITY FL 32024
CONTRACTOR THEODORE C. BROCK PHONE 407.227.3504
LOCATION OF PROPERTY 90-W TO SR.247-S TO C-252-B,TR TO TIMBER RIDGE DR,TL
AND IT'S THE 6TH LOT ON R.

SUBDIVISION/LOT/BLOCK/PHASE/UNIT TIMBER RIDGE 6
SIGNATURE 

INSTALLATION REQUIREMENTS

☒

Culvert size will be 18 inches in diameter with a total length of 32 feet, leaving 24 feet of driving surface. Both ends will be mitered 4 foot with a 4 : 1 slope and poured with a 4 inch thick reinforced concrete slab.

INSTALLATION NOTE: Turnouts will be required as follows:

- a) a majority of the current and existing driveway turnouts are paved, or;
 - b) the driveway to be served will be paved or formed with concrete.
- Turnouts shall be concrete or paved a minimum of 12 feet wide or the width of the concrete or paved driveway, whichever is greater. The width shall conform to the current and existing paved or concreted turnouts.

☐

Culvert installation shall conform to the approved site plan standards.

☐

Department of Transportation Permit installation approved standards.

☐

Other _____

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED
DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21
Lake City, FL 32055
Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



NOTICE OF COMMENCEMENT

Inst:200812001046 Date:1/17/2008 Time:10:09 AM
14 DC, P. DeWitt Cason, Columbia County Page 1 of 1

Tax Parcel Identification Number 26621

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.

1. Description of property (legal description): 0/1 Timberlands
a) Street (job) Address: 31 SW Timber Ridge Dr
2. General description of improvements: Construction of a single family dwelling
3. Owner Information
a) Name and address: Maronda Homes Inc. of FL 6800 Southpoint Pkwy #300 Jax. FL 32216
b) Name and address of fee simple titleholder (if other than owner) _____
c) Interest in property _____
4. Contractor Information
a) Name and address: Theodore C. Brock 6800 Southpoint Pkwy #300 Jax. FL 32216
b) Telephone No.: (407)-227-3504 Fax No. (Opt.) _____
5. Surety Information
a) Name and address: _____
b) Amount of Bond: _____
c) Telephone No.: _____ Fax No. (Opt.) _____
6. Lender
a) Name and address: Bank of America 250 Park Ave S. #400 Winter Park, FL 32789
b) Phone No. _____
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
a) Name and address: Kampf Title and Guaranty Corp. P.O. Box 1359 200 W. 1st St.
b) Telephone No.: _____ Fax No. (Opt.) Winter Park, FL 32771
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(l)(b), Florida Statutes:
a) Name and address: Staff at Maronda Homes Inc. of FL
b) Telephone No.: (904)-296-1490 Fax No. (Opt.) _____
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified): _____

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

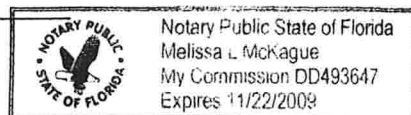
STATE OF FLORIDA
COUNTY OF COLUMBIA

10. [Signature]
Signature of Owner or Owner's Authorized Officer/Director/Partner/Manager
Theodore C. Brock
Print Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 3 day of January, 2008, by:
Theodore C. Brock as V.P. of Construction (type of authority, e.g. officer, trustee, attorney
fact) for Maronda Homes Inc. of FL (name of party on behalf of whom instrument was executed).

Personally Known XX OR Produced Identification _____ Type _____

Notary Signature Melissa L McKague Notary Stamp or Seal:



11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

[Signature]
Signature of Natural Person Signing (in line #10 above.)



Greater Lake City Regional Utility Authority

205 North Marion Avenue
LAKE CITY, FLORIDA 32055-3918
TELEPHONE: 386.719.5778 FAX: 386.719.5837
E-mail: customer.service@ci.lake-city.fl.us

APPLICATION FOR WATER/SEWER/GAS TAP-CAPACITY COMMITMENT

Project Name: Timberlands Date/Time: 12/10/07

Service Address: 231 SW Timber Ridge Dr.

Applicant Name: Maronda Homes Telephone Number: (904)-296-1490

Applicant's Agent: Melissa McKague Telephone Number: (904)-296-1490 ext. 117

Business Name: Maronda Homes Inc. of Telephone Number: (904)-296-1490

Parcel ID #'s: _____

Mailing Address: 6800 Southpoint Pkwy #300 Jacksonville, FL 32216

Requested Water Capacity: ☐ No ☒ Yes, in the amount of _____ gpd/gph
Requested Sewer Capacity: ☐ No ☐ Yes, in the amount of _____ gpd/gph
Requested Gas Capacity: ☐ No ☐ Yes, in the amount of _____ btu/unit

Application For: ☒ Water/Tap Size 3/4 ☐ Sewer/Tap Size _____
☐ Irrigation/Tap Size _____ ☐ Gas/Tap Size _____

Meter Size /Quantity: ☒ 3/4" ☐ 1" ☐ 1 1/2" ☐ 2" ☐ 6" ☐ Other Specify _____

Within City limits: ☐ Yes ☒ No

Fire Protection: ☒ No ☐ Yes, Diameter of new mainline _____
Quantity of new Hydrants: _____

Growth Management Zoned: _____ ☒ Residential ☐ Commercial ☐ Industrial

The above named applicant request that an inspection be made by the Greater Lake City Regional Utility Authority for verification of available services based on address, parcel numbers, lot numbers, etc. Upon confirmation of both capacity and or availability, the applicant will be notified and provided a "Cost Estimate/Tap & Impact Fees" summary.

NOTE: This is only an estimate, fees are subject to change.

Applicant: Melissa McKague

Date: 12/10/07

Service Available Date Requested

Date: ASAP

Billing will begin upon completion of tap.



Greater Lake City Regional Utility Authority

205 North Marion Avenue
LAKE CITY, FLORIDA 32055-3918
TELEPHONE: 386.719.5778 FAX: 386.719.5837
E-mail: customer.service@ci.lake-city.fl.us

OFFICE USE ONLY:

Wastewater Capacity Available

Director _____

Date _____

Collections/Distribution Available

Director _____

Date _____

Water Capacity Available

Director _____

Date _____

Natural Gas Available

Director _____

Date _____

The Greater Lake City Regional Utility Authority has reviewed the applicants request for availability/reserved capacity. Upon this evaluation service connection has been

☐ Accepted

☐ Declined due to _____

Greater Lake City Regional Utility Authority - Customer Service

Application Fee Paid

Application Number: _____

Amount: \$ _____ Check # _____ Type of Establishment _____

Additional Information:

Tap and Impact

Representative _____

Date _____



STATE OF FLORIDA
DEPARTMENT OF HEALTH

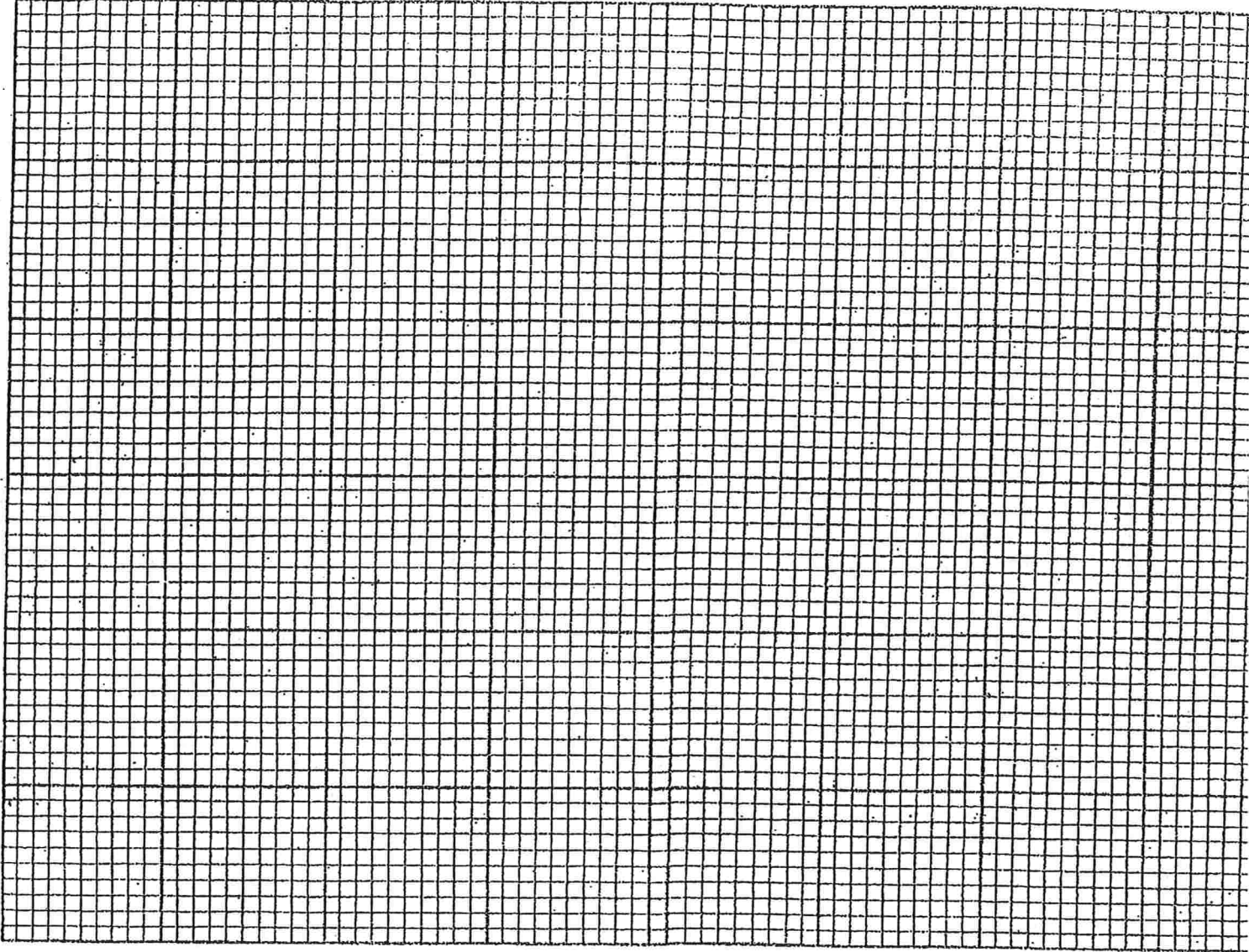
See attached.

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

Permit Application Number 07-0999

PART II - SITE PLAN

Scale: Each block represents 5 feet and 1 inch = 50 feet.



Notes:

Site Plan submitted by:

Jessie

Signature

Abner

Agent

Title

Plan Approved ☒

Not Approved ☐

12-31-07

Date 2007

By

M 02

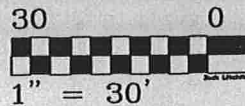
Columbia

County Health Department

ALL CHANGES MUST BE APPROVED BY THE COUNTY HEALTH DEPARTMENT

PROPOSED BUILDING LAYOUT

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE
16 EAST, COLUMBIA COUNTY, FLORIDA



LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
- = FOUND 1/2" REBAR & CAP L.B. 6894
- = SET 1/2" REBAR & CAP L.B. 6894
- = FOUND 3/4" IRON PIPE
- = FOUND 4" X 4" CONC. MO NO IDENTIFICATION
- = SET 4" X 4" CONC. MON. P.S.M. 5582
- ✕ = SET NAIL & DISK P.S.M. 5
- ✕ = FOUND NAIL & DISK
- ⊠ = FOUND 6" X 6" S.R.D. R/W MON.
- ⊞ = CATV RISER
- ⊞ = TELEPHONE PEDESTAL
- ⊞ = WOOD POWER POLE

**THIS IS
CERTIFIED**

NOT VALID WITH
A FLORIDA LICENSE
TO THIS MAP

I HEREBY CERTIFY
THAT THE SURVEY IS
TRUE AND CORRECT
TO MY SUPERVISION
AND THE MINIMUM
REQUIREMENTS OF THE
BOARD OF LAND SURVEYORS
AND STATUTES, AND

BY: *[Signature]*
JAMES E. BRIN

DATE: *10/24/07*



BRIN

4607 N

PHONE:

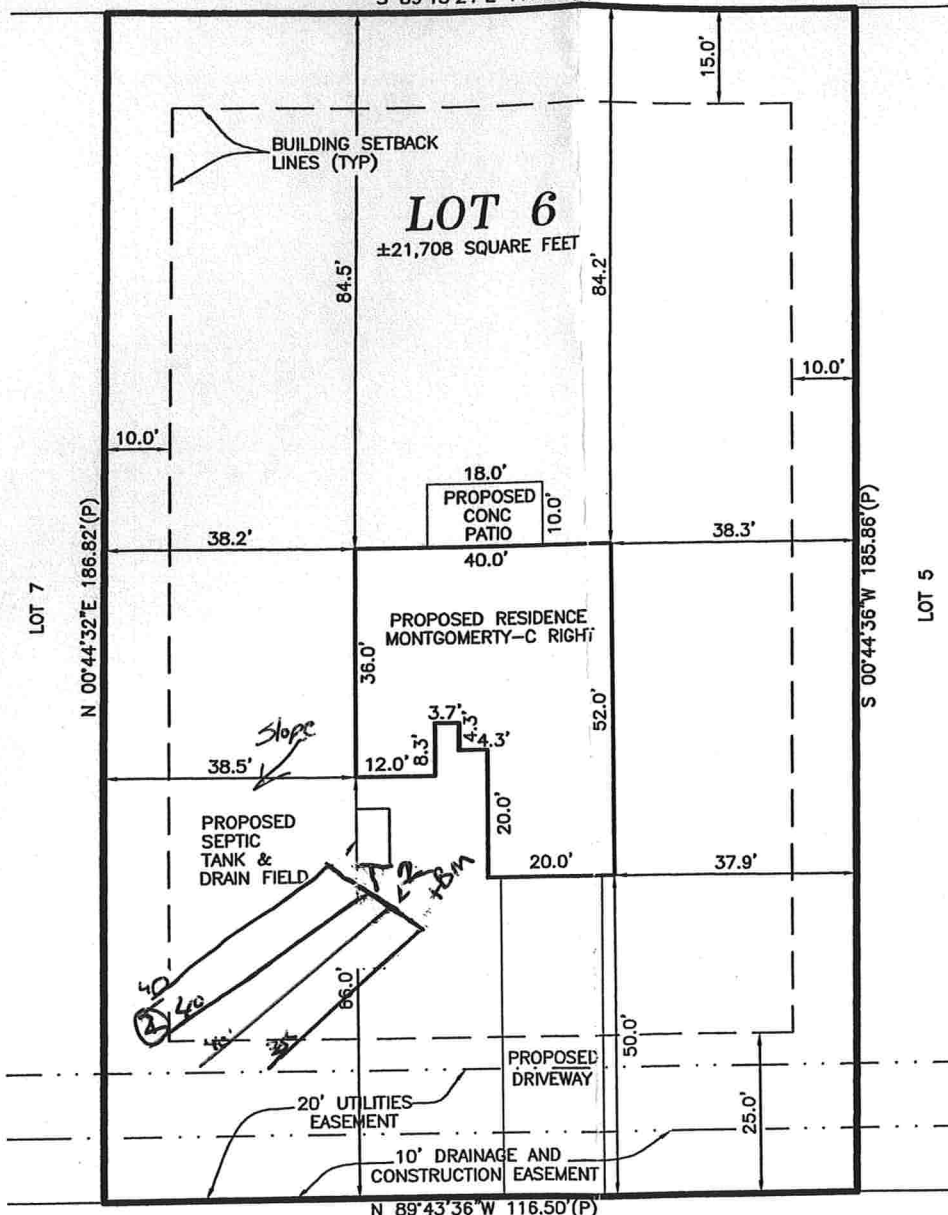
SCALE: 1" = 30'

DATE: 9/24/07

THIS IS NOT A BOUNDARY

PREPARED FOR: MARC

S 89°15'24"E 116.50'(P)



SW TIMBER RIDGE DRIVE

60' RIGHT-OF-WAY

NOTE:

INION OF THIS SURVEYOR, ACCORDING TO THE
FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE
UNIT PANEL NO. 120070-0175-B, DATED 1-6-88,
PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA
DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS
SHOWN ON SAID MAP. INFORMATION FROM THE FEDERAL
FLOOD MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE
MAP, SHOWN ON THIS MAP, WAS CURRENT AS OF THE
DATE. MAP REVISIONS AND AMENDMENTS ARE
USUALLY MADE BY LETTER AND MAY NOT BE REFLECTED
ON THIS CURRENT MAP.

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE
DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS
SURVEYOR HAS NOT PERFORMED A SEARCH OF THE
PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF
TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT
THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH
CLAIMS ARE NOT CERTIFIED HEREON.

This instrument prepared by:
William J. Haley, Esquire
Brannon, Brown,
Haley & Bullock, P. A.
P. O. Box 1029
Lake City, FL 32056-1029

Inst:200712024085 Date:10/26/2007 Time:4:45 PM
Doc Stamp-Deed:3276.00
28 DC, P. DeWitt Cason, Columbia County Page 1 of 2

SPECIAL WARRANTY DEED

THIS INDENTURE, made this 10th day of October, 2007, between **RML HOLDINGS, INC.**, a Florida corporation, having a mailing address of 703 NW Blackberry Circle, Lake City, Florida 32055, hereinafter referred to as Grantor, and **MARONDA HOMES, INC. OF FLORIDA**, a Florida corporation, having a mailing address of 3993 West First Street, Sanford, Florida 32771, hereinafter referred to as Grantee.

WITNESSETH: That said Grantor, for and in consideration of the sum of \$10.00 and other good and valuable considerations to said Grantor in hand paid by said Grantee, the receipt and sufficiency of which are hereby acknowledged, have granted, bargained and sold to the said Grantee, and Grantee's successors and assigns forever, the following described land, situate, lying and being in **Columbia** County, Florida, to-wit:

Lot(s) 2, 3, 4, 5, 6, 7, 8, 21, 22, 23, 25, and 26, **TIMBERLANDS, PHASE 1**, according to the plat thereof, as recorded in Plat Book 9, pages 26-27, public records of Columbia County, Florida,.

PARCEL NO. Part of 10-4S-16-02856-000 and 15-4S-16-02990-000

SUBJECT TO: Taxes and special assessments for the year 2007 and subsequent years; restrictions, reservations, rights of way for public roads, easements of record, if any; and zoning and any other governmental restrictions regulating the use of the lands.

and said Grantor does hereby fully warrant the title to said land, and will defend the same against the lawful claims of all persons claiming by, through or under said Grantor.

IN WITNESS WHEREOF, Grantor has hereunto set its hand and seal the day and year first above written.

Signed, sealed and delivered
in the presence of:

RML HOLDINGS, INC., a Florida
corporation

Linda Brown
Print Name: Linda Brown

By: Margaret Lardizabal
Margaret Lardizabal
Vice President

Debbie G. Moore
Print Name: Debbie G. Moore

STATE OF FLORIDA
COUNTY OF COLUMBIA

The foregoing instrument was acknowledged before me this 10th day of October, 2007, by Margaret Lardizabal, as Vice President of RML Holdings, Inc., a Florida corporation, on behalf of said corporation, who is personally known to me.

Debbie G. Moore
Notary Public, State of Florida



FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs
Residential Whole Building Performance Method A

011 TM

Project Name: MONTGOMERY GAINESVILLE	Builder: MARONDA HOMES
Address: 231 SW Timber Ridge Dr	Permitting Office: Columbia
City, State: LAKE CITY, FL 32055	Permit Number: 26621
Owner: ELECTRIC	Jurisdiction Number: 221000
Climate Zone: North	

1. New construction or existing New ☐	12. Cooling systems
2. Single family or multi-family Single family ☐	a. Central Unit Cap: 30.6 kBtu/hr ☐
3. Number of units, if multi-family 1 ☐	SEER: 14.00 ☐
4. Number of Bedrooms 3 ☐	b. N/A ☐
5. Is this a worst case? Yes ☐	c. N/A ☐
6. Conditioned floor area (ft²) 1312 ft² ☐	13. Heating systems
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)	a. Electric Heat Pump Cap: 30.6 kBtu/hr ☐
a. U-factor: Description Area	HSPF: 8.70 ☐
(or Single or Double DEFAULT) 7a(Sngle Default) 118.0 ft² ☐	b. N/A ☐
b. SHGC:	c. N/A ☐
(or Clear or Tint DEFAULT) 7b. (Clear) 118.0 ft² ☐	14. Hot water systems
8. Floor types	a. Electric Resistance Cap: 50.0 gallons ☐
a. Slab-On-Grade Edge Insulation R=0.0, 149.0(p) ft ☐	EF: 0.92 ☐
b. N/A ☐	b. N/A ☐
c. N/A ☐	c. Conservation credits
9. Wall types	(HR-Heat recovery, Solar
a. Concrete, Int Insul, Exterior R=4.1, 874.0 ft² ☐	DHP-Dedicated heat pump)
b. Frame, Steel, Adjacent R=11.0, 231.0 ft² ☐	15. HVAC credits PT, ☐
c. N/A ☐	(CF-Ceiling fan, CV-Cross ventilation,
d. N/A ☐	HF-Whole house fan,
e. N/A ☐	PT-Programmable Thermostat,
10. Ceiling types	MZ-C-Multizone cooling,
a. Under Attic R=19.0, 1339.0 ft² ☐	MZ-H-Multizone heating)
b. N/A ☐	
c. N/A ☐	
11. Ducts	
a. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 150.0 ft ☐	
b. N/A ☐	

Glass/Floor Area: 0.09

Total as-built points: 18678

Total base points: 20151

PASS

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY: [Signature]

DATE: Gary Carmack 12/13/10

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: [Signature]

DATE: Melissa McKeown 12/13/10

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.



BUILDING OFFICIAL: _____

DATE: _____

¹ Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BSPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X SPM X SOF = Points				
.18	1312.0	18.59	4390.0	1.Single, Clear	E	1.0	6.0	16.0	47.92	0.97	743.0
				2.Single, Clear	E	1.0	8.0	40.0	47.92	0.99	1899.0
				3.Single, Clear	W	3.0	6.0	11.0	43.84	0.73	351.0
				4.Single, Clear	W	1.0	6.0	11.0	43.84	0.97	467.0
				5.Single, Clear	E	1.0	6.0	16.0	47.92	0.97	743.0
				6.Single, Clear	W	1.0	6.0	24.0	43.84	0.97	1020.0
				As-Built Total:		118.0			5223.0		
WALL TYPES											
Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Adjacent	231.0	0.70	161.7	1. Concrete, Int Insul, Exterior	4.1		874.0	1.13		992.0	
Exterior	874.0	1.70	1485.8	2. Frame, Steel, Adjacent	11.0		231.0	1.00		231.0	
Base Total:				1105.0		1647.5		As-Built Total:		1105.0 1223.0	
DOOR TYPES											
Area X BSPM = Points				Type	Area X SPM = Points						
Adjacent	17.0	2.40	40.8	1.Exterior Insulated	20.0 4.10 82.0						
Exterior	20.0	6.10	122.0	2.Adjacent Wood	17.0 2.40 40.8						
Base Total:				37.0		162.8		As-Built Total:		37.0 122.8	
CEILING TYPES											
Area X BSPM = Points				Type	R-Value		Area X SPM X SCM = Points				
Under Attic	1312.0	1.73	2269.8	1. Under Attic	19.0		1339.0	2.34 X 1.00		3133.3	
Base Total:				1312.0		2269.8		As-Built Total:		1339.0 3133.3	
FLOOR TYPES											
Area X BSPM = Points				Type	R-Value		Area X SPM = Points				
Slab	149.0(p)	-37.0	-5513.0	1. Slab-On-Grade Edge Insulation	0.0		149.0(p)	-41.20		-6138.8	
Raised	0.0	0.00	0.0								
Base Total:				-5513.0		As-Built Total:		149.0		-6138.8	
INFILTRATION											
Area X BSPM = Points				Area X SPM = Points							
1312.0 10.21 13395.5				1312.0 10.21				13395.5			

SUMMER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

BASE				AS-BUILT						
Summer Base Points: 16352.6				Summer As-Built Points: 16958.8						
Total Summer Points	X System Multiplier	=	Cooling Points	Total Component (System - Points)	X Cap Ratio	X Duct Multiplier (DM x DSM x AHU)	X System Multiplier	X Credit Multiplier	=	Cooling Points
16352.6	0.3250		5314.6	(sys 1: Central Unit 30600btuh ,SEER/EFF(14.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS) 16959 1.00 (1.08 x 1.147 x 0.86) 0.244 0.950 4210.0 16958.8 1.00 1.072 0.244 0.950 4210.0						

WINTER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

BASE				AS-BUILT							
GLASS TYPES											
.18 X Conditioned X BWPM = Points Floor Area				Type/SC	Overhang Ornt Len Hgt		Area X WPM X WOF = Points				
.18	1312.0	20.17	4763.0	1.Single, Clear	E	1.0	6.0	16.0	26.41	1.02	429.0
				2.Single, Clear	E	1.0	8.0	40.0	26.41	1.01	1065.0
				3.Single, Clear	W	3.0	6.0	11.0	28.84	1.08	343.0
				4.Single, Clear	W	1.0	6.0	11.0	28.84	1.01	319.0
				5.Single, Clear	E	1.0	6.0	16.0	26.41	1.02	429.0
				6.Single, Clear	W	1.0	6.0	24.0	28.84	1.01	697.0
				As-Built Total:				118.0	3282.0		
WALL TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Adjacent	231.0	3.60	831.6	1. Concrete, Int Insul, Exterior	4.1		874.0	6.42		5611.1	
Exterior	874.0	3.70	3233.8	2. Frame, Steel, Adjacent	11.0		231.0	5.20		1201.2	
Base Total:				As-Built Total:		1105.0		6812.3			
DOOR TYPES Area X BWPM = Points				Type			Area X WPM = Points				
Adjacent	17.0	11.50	195.5	1.Exterior Insulated			20.0	8.40		168.0	
Exterior	20.0	12.30	246.0	2.Adjacent Wood			17.0	11.50		195.5	
Base Total:				As-Built Total:		37.0		363.5			
CEILING TYPES Area X BWPM = Points				Type	R-Value		Area X WPM X WCM = Points				
Under Attic	1312.0	2.05	2689.6	1. Under Attic	19.0		1339.0	2.70 X 1.00		3615.3	
Base Total:				As-Built Total:		1339.0		3615.3			
FLOOR TYPES Area X BWPM = Points				Type	R-Value		Area X WPM = Points				
Slab	149.0(p)	8.9	1326.1	1. Slab-On-Grade Edge Insulation	0.0		149.0(p)	18.80		2801.2	
Raised	0.0	0.00	0.0								
Base Total:				As-Built Total:		149.0		2801.2			
INFILTRATION Area X BWPM = Points								Area X WPM = Points			
1312.0 -0.59 -774.1						1312.0		-0.59 -774.1			

WINTER CALCULATIONS**Residential Whole Building Performance Method A - Details**

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

BASE				AS-BUILT						
Winter Base Points: 12511.5				Winter As-Built Points: 16100.2						
Total Winter Points	X System Multiplier	= Heating Points		Total Component (System - Points)	X Cap Ratio (DM x DSM x AHU)	X Duct Multiplier (1.060 x 1.169 x 0.88)	X System Multiplier 0.392	X Credit Multiplier 0.950	= Heating Points 6563.2	
12511.5	0.5540	6931.4		(sys 1: Electric Heat Pump 30600 btuh ,EFF(8.7) Ducts:Unc(S),Con(R),Int(AH),R6.0 16100.2 1.000	1.00	1.095	0.392	0.950	6563.2	
				16100.2	1.00	1.095	0.392	0.950	6563.2	

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

BASE				AS-BUILT					
WATER HEATING				Tank	EF	Number of	X	Tank X	Multiplier X Credit = Total
Number of	X	Multiplier	= Total	Volume		Bedrooms		Ratio	Multiplier
Bedrooms									
3		2635.00	7905.0	50.0	0.92	3		1.00	2635.00 1.00 7905.0
				As-Built Total:					7905.0

CODE COMPLIANCE STATUS

BASE						AS-BUILT					
Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	= Total Points
5315		6931		7905	20151	4210		6563		7905	18678

PASS

Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , ,

PERMIT #:

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum: .3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	✓
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	✓
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	✓
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	✓
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	✓
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	✓
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	✓

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked air breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	✓
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	✓
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	✓
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	✓
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	✓

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 86.0

The higher the score, the more efficient the home.

ELECTRIC, Lot: , Sub: , Plat: , , ,

1. New construction or existing	New	___	12. Cooling systems	
2. Single family or multi-family	Single family	___	a. Central Unit	Cap: 30.6 kBtu/hr
3. Number of units, if multi-family	1	___		SEER: 14.00
4. Number of Bedrooms	3	___	b. N/A	___
5. Is this a worst case?	Yes	___	c. N/A	___
6. Conditioned floor area (ft ²)	1312 ft ²	___		___
7. Glass type ¹ and area: (Label reqd. by 13-104.4.5 if not default)		___	13. Heating systems	
a. U-factor:	Description Area	___	a. Electric Heat Pump	Cap: 30.6 kBtu/hr
(or Single or Double DEFAULT)	7a(Sngle Default) 118.0 ft ²	___		HSPF: 8.70
b. SHGC:		___	b. N/A	___
(or Clear or Tint DEFAULT)	7b. (Clear) 118.0 ft ²	___	c. N/A	___
8. Floor types		___	14. Hot water systems	
a. Slab-On-Grade Edge Insulation	R=0.0, 149.0(p) ft	___	a. Electric Resistance	Cap: 50.0 gallons
b. N/A	___	___		EF: 0.92
c. N/A	___	___	b. N/A	___
9. Wall types		___	c. Conservation credits	___
a. Concrete, Int Insul, Exterior	R=4.1, 874.0 ft ²	___	(HR-Heat recovery, Solar	___
b. Frame, Steel, Adjacent	R=11.0, 231.0 ft ²	___	DHP-Dedicated heat pump)	___
c. N/A	___	___	15. HVAC credits	PT, ___
d. N/A	___	___	(CF-Ceiling fan, CV-Cross ventilation,	___
e. N/A	___	___	HF-Whole house fan,	___
10. Ceiling types		___	PT-Programmable Thermostat,	___
a. Under Attic	R=19.0, 1339.0 ft ²	___	MZ-C-Multizone cooling,	___
b. N/A	___	___	MZ-H-Multizone heating)	___
c. N/A	___	___		___
11. Ducts		___		___
a. Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0, 150.0 ft	___	___		___
b. N/A	___	___		___

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant features.

Builder Signature: Melina McKeage Date: 12/13/07

Address of New Home: 231 SW Timber Ridge Dr City/FL Zip: Lake City, FL 32055



*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStarTM designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4.
EnergyGauge® (Version: FLRCSB v4.5)



Project Summary Entire House MARONDA HOMES

Job: MONTGOMERY
Date:
By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: 407-321-0064

Project Information

For: MONTGOMERY

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Outside db	33 °F
Inside db	70 °F
Design TD	37 °F

Summer Design Conditions

Outside db	92 °F
Inside db	75 °F
Design TD	17 °F
Daily range	M
Relative humidity	50 %
Moisture difference	52 gr/lb

Heating Summary

Structure	24549 Btuh
Ducts	2428 Btuh
Central vent (100 cfm)	4048 Btuh
Humidification	0 Btuh
Piping	0 Btuh
Equipment load	31025 Btuh

Sensible Cooling Equipment Load Sizing

Structure	13023 Btuh
Ducts	5274 Btuh
Central vent (100 cfm)	1860 Btuh
Blower	0 Btuh

Infiltration

Method	Simplified
Construction quality	Average
Fireplaces	0

	Heating	Cooling
Area (ft ²)	1261	1261
Volume (ft ³)	10088	10088
Air changes/hour	0.45	0.23
Equiv. AVF (cfm)	76	39

Use manufacturer's data	n
Rate/swing multiplier	0.97
Equipment sensible load	19552 Btuh

Latent Cooling Equipment Load Sizing

Structure	2160 Btuh
Ducts	1179 Btuh
Central vent (100 cfm)	3516 Btuh
Equipment latent load	6855 Btuh

Equipment total load	26406 Btuh
Req. total capacity at 0.74 SHR	2.2 ton

Heating Equipment Summary

Make TEMPSTAR
Trade HEAT PUMP
Model N4H330AKA

Efficiency	8.7 HSPF
Heating input	
Heating output	0 Btuh @ 47°F
Temperature rise	0 °F
Actual air flow	0 cfm
Air flow factor	0.000 cfm/Btuh
Static pressure	0.60 in H2O
Space thermostat	

Cooling Equipment Summary

Make TEMPSTAR
Trade HEAT PUMP
Cond N4H330AKA
Coil FEM4X3600

Efficiency	14 SEER
Sensible cooling	22644 Btuh
Latent cooling	7956 Btuh
Total cooling	30600 Btuh
Actual air flow	935 cfm
Air flow factor	0.051 cfm/Btuh
Static pressure	0.60 in H2O
Load sensible heat ratio	0.75

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.



wrightsoft

Right-Suite Residential 6.0.90 RSR21115

C:\Documents and Settings\gcarmack\My Documents\Man J Calcs 2007\GAINESVILLE\MONTGOMERY .rrp Calc

2007-Sep-12 12:53:22

Page 1

Project Information

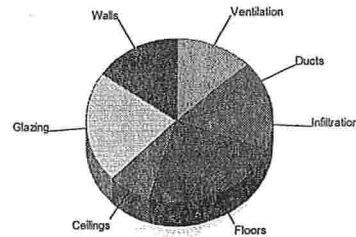
For: MONTGOMERY

Design Conditions

Location:		Indoor:		Heating	Cooling
Gainesville, FL, US		Indoor temperature (°F)		70	75
Elevation: 151 ft		Design TD (°F)		37	17
Latitude: 30°N		Relative humidity (%)		30	50
		Moisture difference (gr/lb)		10.6	52.0
Outdoor:		Heating	Cooling		
Dry bulb (°F)	33	92			
Daily range (°F)	-	19 (M)			
Wet bulb (°F)	-	77			
Wind speed (mph)	15.0	7.5			
		Infiltration:			
		Method	Simplified		
		Construction quality	Average		
		Fireplaces	0		

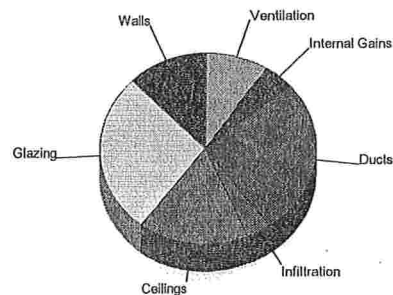
Heating

Component	Btuh/ft²	Btuh	% of load
Walls	4.4	4760	15.3
Glazing	47.0	6861	22.1
Doors	0.0	0	0.0
Ceilings	1.8	2379	7.7
Floors	19.9	7487	24.1
Infiltration	2.5	3063	9.9
Ducts		2428	7.8
Piping		0	0.0
Humidification		0	0.0
Ventilation		4048	13.0
Adjustments		0	
Total		31025	100.0



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls	2.3	2507	12.4
Glazing	37.6	5492	27.2
Doors	0.0	0	0.0
Ceilings	2.6	3385	16.8
Floors	0.0	0	0.0
Infiltration	0.6	719	3.6
Ducts		5274	26.2
Ventilation		1860	9.2
Internal gains		920	4.6
Blower		0	0.0
Adjustments		0	
Total		20156	100.0



Overall U-value = 0.199 Btuh/ft²-°F

ERROR: negative wall area in MASTER BDR - check windows.

Duct System Summary

Entire House

MARONDA HOMES

Job: MONTGOMERY
 Date:
 By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: 407-321-0064

Project Information

For: MONTGOMERY

	Heating	Cooling
External static pressure	0.60 in H2O	0.60 in H2O
Pressure losses	0.00 in H2O	0.00 in H2O
Available static pressure	0.60 in H2O	0.60 in H2O
Supply / return available pressure	0.43 / 0.17 in H2O	0.43 / 0.17 in H2O
Lowest friction rate	2.857 in/100ft	2.857 in/100ft
Actual air flow	935 cfm	935 cfm
Total effective length (TEL)	21 ft	

Supply Branch Detail Table

Name	Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
MASTER BDR	h 4970	200	200	4.286	7	x0	VIFx	10.0	0.0	st5
MASTER BATH	h 2370	50	50	8.571	5	x0	VIFx	5.0	0.0	st5
UTILITY ROOM	c 863	50	50	2.857	4	x0	VIFx	15.0	0.0	st1
FAMILY/DINING RM	c 3991	240	240	2.857	8	x0	VIFx	15.0	0.0	ST1
KITCHEN	c 2069	100	100	4.286	5	x0	VIFx	10.0	0.0	ST1
BDR #2	h 3335	120	120	2.857	6	x0	VIFx	15.0	0.0	ST3
BATHROOM	h 763	20	20	4.286	4	x0	VIFx	10.0	0.0	ST2
BDR #3	h 3880	120	120	4.286	6	x0	VIFx	10.0	0.0	ST4
MASTER TOILET	c 794	35	35	2.857	4	x0	VIFx	15.0	0.0	st5

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	675	675	2.857	622	14	0 x 0	VinIFlx	st1
ST2	Peak AVF	20	20	4.286	369	4	0 x 0	VinIFlx	
ST3	Peak AVF	120	120	2.857	527	6	0 x 0	VinIFlx	
ST4	Peak AVF	120	120	4.286	613	6	0 x 0	VinIFlx	
st5	Peak AVF	285	285	2.857	595	10	0 x 0	VinIFlx	

Bold/Italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x0	935	935	6.0	2.857	670	16	x0 0		VIFx	



ENGINEERING CONSULTANTS IN GEOTECHNICAL • ENVIRONMENTAL • CONSTRUCTION MATERIALS TESTING

REPORT OF SOIL DENSITY BY NUCLEAR METHODS

Client:..... Maronda Homes
 Address:..... 7478 SW 60th Avenue, Ste B
 City, State, Zip Code:..... Ocala, FL 34476
 Date of Test:..... February 1, 2008
 Test Location:..... Timber Lands, Lot 6 – 255 S.W. Timber Ridge Drive
 Area of Test:..... Building Pad & Footing

Test Methods: In Place Compaction Test per ASTM D-2922 Nuclear Method
 MAXIMUM DRY DENSITY PER AASHTO T-180 (MODIFIED PROCTOR)

TEST LOCATION	LAB DENSITY lb/ft ³	PERCENT OPTIMUM MOISTURE	DRY DENSITY lb/ft ³	PERCENT FIELD MOISTURE	PERCENT DENSITY
Center of Pad final grade	103.7	12.0	100.8	8.3	97.2
Center of N. Footing final grade	103.7	12.0	98.8	9.6	95.3
Center of S. Footing final grade	103.7	12.0	98.9	10.8	95.4

GTI is not responsible for determining thickness of fill soils. Above mentioned tests represent that location only. No other warranties are expressed or implied.

Lift = 12" Thickness

Field Technician: WC

Remarks: Permit No. 22622

The above tests were performed and reported
 in accordance with the referenced specification

John C. Dorman, Jr., Ph.D., P.E.
 Florida Registration No. 52612

2/11/08

26621

FIELD DENSITY WORKSHEET

CLIENT MARONDA HOMES DATE 1 FEB 08
LAKE CITY PROJECT NO. _____
PROJECT NAME TIMBER LANDS LOT 6 PERMIT NO. # 26621
EARTH CONTRACTOR 231 SW TIMBER RIDGE DR. TESTED BY JWL
COMPACTION REQUIREMENT (%) 95% ☐ Standard Proctor Patrick FIELD CONTACT
☐ Modified Proctor
TOTAL ON-SITE TIME _____ MILES FROM OFFICE _____
☐ Limerock ☐ Subgrade ☐ Pipe Backfill ☒ Building Pad ☒ Building Footing ☐ Other _____

TEST LOCATION	LAB PROCTOR		TEST DEPTH	PROBE DEPTH	% MOIST.	WET DENSITY (PCF)	DRY DENSITY (PCF)	% COMP.
	DENS.	OMC						
CTR. OF PAD	103.7	12.0	12"	12"	8.3	109.2	100.8	97.2
CTR. OF N.F.T.G.	2	1	1	1	9.6	108.3	98.8	95.3
CTR. OF S.F.T.G.	2	1	1	1	10.8	109.6	98.9	95.4

REMARKS _____

- * Density failed to meet minimum project requirement
** Retest indicates minimum density requirement was obtained.
() Client is aware of unsatisfactory test results.

Maronda Systems

Maronda Systems 4005 Maronda Way Sanford FL 32771 (407) 321-0064 Fax (407) 321-3913
 Engineer of Record: Tomas Ponce, P.E. 367 Medallion PL. Chuluota, FL 32766 FL PE # 50068,
 Design Criteria: TPI Design: Matrix Analysis TEE-LOK software

PLAN JOB #	LOT	ADDRESS	DIV/SUB	MODEL	MONTGOMERY C 3
1TM00601	6-1		ORA-1TM	MGMC3 RIGHT	VAULTED

This structure was designed in accordance with, and meets the requirements of TPI standards and the FLORIDA 2004 BUILDING CODE for 125 M.P.H. Wind Zone.

Truss loading is in accordance with ASCE 7-02. These trusses are designed for an enclosed building.

Truss ID	Run Date	Drawing Reviewed	Truss ID	Run Date	Drawing Reviewed	No. of Eng. Dwg's: 58
Layout	1-19-07	✓	JS4	2-1-07	✓	Roof Loads-
V	7-27-05		JS5	2-1-07		TC Live: 16.0 psf
HIP	11-2-06		JS6	2-1-07		TC Dead: 7.0 psf
C	7-9-07		JS7	2-1-07		BC Live: 10.0 psf
C1	2-1-07		JSGRD1	5-3-07		BC Dead: 10.0 psf
C2	2-1-07		MHSGRD1	2-1-07		Total 43.0 psf
FG2	2-1-07		S1B	3-6-07		
G1	2-1-07		S1C	2-1-07		DurFac- Lbr: 1.25
GRD1	5-3-07		S1D	2-1-07		DurFac- Plt: 1.25
H1A	2-1-07		T1	2-1-07		O.C. Spacing: 24.0"
H2A	2-1-07		T1A	2-1-07		
HGRD1A	2-1-07		V1	2-1-07		
HGRD3	5-3-07		V2	2-1-07		
HS1	5-3-07		V3	6-5-07		
HS2	5-3-07		V3A	2-1-07		
J2	2-1-07		V30	2-1-07		
J	2-1-07		V31	2-1-07		
J1	2-1-07		V32	2-1-07		
J3	2-1-07		V33	2-1-07		
J4	2-1-07		V34	2-1-07		
J5	2-1-07		V4	6-5-07		
J5A	2-1-07		V5	6-5-07		
J6	2-1-07		V6	6-5-07		
J6A	2-1-07					
J7	2-1-07					
J11	2-1-07					
J7A	2-1-07					
J12	5-3-07					
JGRD1	5-3-07					
JGRD1A	5-3-07					
JGRD3	2-1-07					
JS	2-1-07					
JS1	2-1-07		INV #	DESC	QNTY	
JS2	2-1-07		18331	THD26	8	
JS3	2-1-07		18330	THD28	1	
			18329	THD28-3	1	
			18360	SKH26R	2	
			18361	SKH26L	2	
			18363	JUS26	3	
			18370	THJ26		
			SEAT PLATES		127	

DATE: OCT 12 2007

GENERAL NOTES

Trusses are not marked in any way to identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for more information. All other permanent bracing design is the responsibility of the Building Designer.

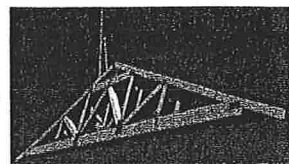
NOTAS GENERALES

Los trusses no están marcados de ningún modo que identifique la frecuencia o localización de restricción lateral y arriostre diagonal temporales. Use las recomendaciones de manejo, instalación, restricción y arriostre temporal de los trusses. Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Placas de Metal para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen BCSI-B3 - Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios para más información. El resto de los diseños de arriostres permanentes son la responsabilidad del Diseñador del Edificio.

HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES.

- ⚠ Warning! Don't overload the crane.
¡Advertencia! ¡No sobrecarga la grúa!
- ⚠ Never use banding alone to lift a bundle.
Do not lift a group of individually banded bundles.
Nunca use sólo los empaques para levantar un paquete.
No levante un grupo de empaques individuales.



- ✓ A single lift point may be used for bundles with trusses up to 45'.
Two lift points may be used for bundles with trusses up to 60'.
Use at least 3 lift points for bundles with trusses greater than 60'.

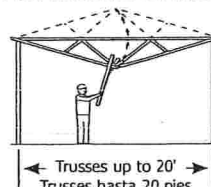
Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies.
Puede usar dos puntos de levantar para paquetes más de 60 pies.
Use por lo menos tres puntos de levantar para paquetes más de 60 pies.

- ⚠ Warning! Do not over load supporting structure with truss bundle.
¡Advertencia! No sobrecargue la estructura apoyada con el paquete de trusses.
- ✓ Place truss bundles in stable position.
Puse paquetes de trusses en una posición estable.

INSTALLATION OF SINGLE TRUSSES BY HAND INSTALACIÓN POR LA MANO DE TRUSSES INDIVIDUALES

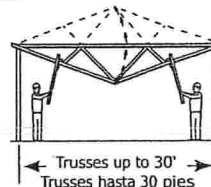
- ✓ Trusses 20' or less, support at peak.

Levante del pico los trusses de 20 pies o menos.



- ✓ Trusses 30' or less, support at quarter points.

Levante de los cuartos de tramo los trusses de 30 pies o menos.

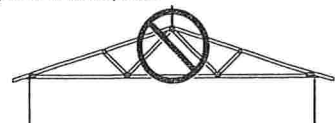


HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUAL

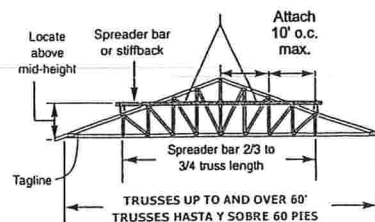
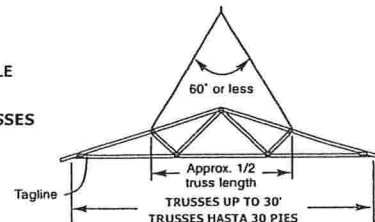
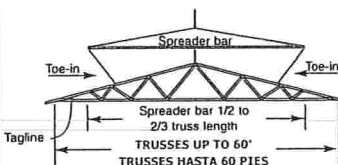
- ✓ Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalado y el truss está asegurado en los soportes.

- ⚠ Warning! Using a single pick-point at the peak can damage the truss.
¡Advertencia! El uso de un solo lugar para levantar en el pico puede hacer daño al truss.



HOISTING RECOMMENDATIONS FOR SINGLE TRUSSES RECOMENDACIONES PARA LEVANTAR TRUSSES INDIVIDUALES



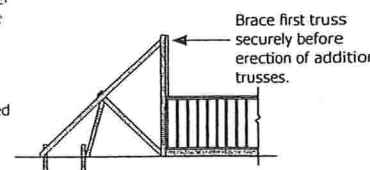
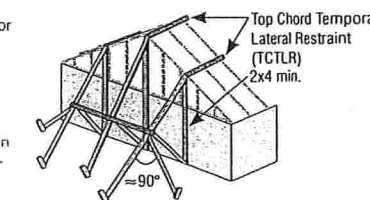
TEMPORARY RESTRAINT & BRACING RESTRICCIÓN Y ARRIOSTRE TEMPORAL

- ⚠ Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.
Vea el resumen BCSI B2 - Restricción/Arriostre Temporal y Instalación de los Trusses para más información.

- ✓ Locate ground braces for first truss directly in line with all rows of top chord temporary lateral restraint (see table in the next column).
Coloque los arriostres de tierra para el primer truss directamente en línea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la table en la próxima columna).

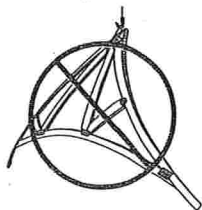


- ⚠ Do not walk on unbraced trusses.
No camine en trusses sueltos.



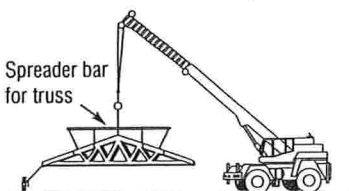
HANDLING — MANEJO

- ⚠ Avoid lateral bending. — Evite la flexión lateral.



- ⚠ Use special care in windy weather or near power lines and airports.

Utilice cuidado especial en días ventosos o cerca de cables eléctricos o de aeropuertos.



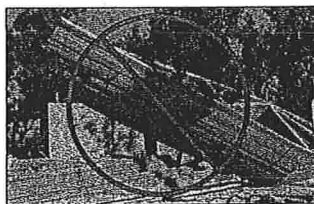
- ✓ Use proper rigging and hoisting equipment.

Use equipo apropiado para levantar e improvisar.



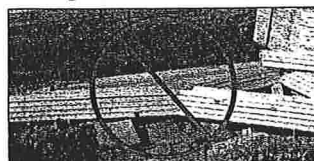
- ⚠ Do not store unbraced bundles upright.

No almacene verticalmente los trusses sueltos.



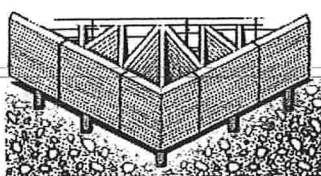
- ⚠ Do not store on uneven ground.

No almacene en tierra desigual.



- ⚠ The contractor is responsible for properly receiving, unloading and storing the trusses at the jobsite.

El contratista tiene la responsabilidad de recibir, descargar y almacenar adecuadamente los trusses en la obra.



- ✓ If trusses are to be stored horizontally, place blocking of sufficient height beneath the stack of trusses at 8' to 10' on center.

For trusses stored for more than one week, cover bundles to prevent moisture gain but allow for ventilation.

Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for more detailed information pertaining to handling and jobsite storage of trusses.

Si los trusses estarán guardados horizontalmente, ponga bloqueando de altura suficiente detrás de la pila de los trusses.

Para trusses guardados por más de una semana, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

Vea el folleto BCSI Guía de Buena Práctica para el Manejo, Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Placas de Metal para información más detallada sobre el manejo y almacenado de los trusses en área de trabajo.

TEPS TO SETTING TRUSSES

AS MEDIDAS DE LA INSTALACIÓN DE LOS TRUSSES

- 1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chord temporary lateral restraint and diagonal bracing (see below). 7) Repeat process on groups of four trusses until all trusses are set.

- 1) Instale los arriostres de tierra. 2) Instale el primero truss y áte seguramente al arriostre de tierra. 3) Instale los próximos cuatro trusses con restricción lateral temporal de miembro corto (vea abajo). 4) Instale el arriostre diagonal de la cuerda superior (vea abajo). 5) Instale arriostre diagonal para los planos de los miembros secundarios para estable los primeros cinco trusses (vea abajo). 6) Instale la restricción lateral temporal y arriostre diagonal para la cuerda inferior (vea abajo). 7) Repita éste procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.

Refer to BCSI-B2 Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information.

Vea el resumen BCSI-B2 - Instalación de Trusses y Arriostre Temporal para mayor información.

RESTRAINT/BRACING FOR ALL PLANES OF TRUSSES

RESTRICCIÓN/ARRIOSTRE EN TODOS PLANOS DE TRUSSES.

This restraint & bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.

Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas 3x2 y 4x2.

TOP CHORD — CUERDA SUPERIOR

Truss Span Longitud de Tramo	Top Chord Temporary Lateral Restraint (TCLTR) Spacing Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30' Hasta 30 pies	10' o.c. max. 10 pies máximo
30' to 45' 30 a 45 pies	8' o.c. max. 8 pies máximo
45' to 60' 45 a 60 pies	6' o.c. max. 6 pies máximo
60' to 80'* 60 a 80 pies*	4' o.c. max. 4 pies máximo

*Consult a Professional Engineer for trusses longer than 60'.
*Consulte a un Ingeniero para trusses de mas de 60 pies.

- See BCSI-B2 for TCLTR options.
Vea el BCSI-B2 para las opciones de TCLTR.

Refer to BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for Gable End Frame restraint/bracing/reinforcement information.

Para información sobre restricción/arriostre/refuerzo para armazón de hastial vea el resumen BCSI-B3 - Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios.

WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS

ATERAL RESTRAINT DIAGONAL BRACING RE VERY IMPORTANT LA RESTRICCIÓN DIAGONAL Y EL ARRIOSTRE DIAGONAL ON MUY IMPORTANTES!

10'-15' max. Same spacing as bottom chord Lateral Restraint
Some chord and web members not shown for clarity.

BOTTOM CHORD — CUERDA INFERIOR

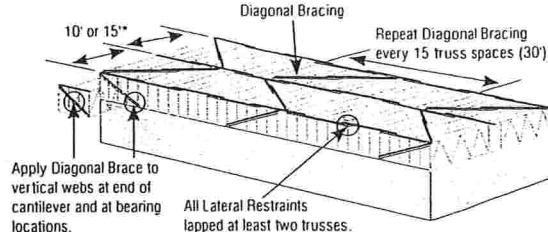
Lateral Restraints - 2x4x12' or greater lapped over two trusses.
Bottom chords
Diagonal Braces every 10 truss spaces (20' max.)
Some chord and web members not shown for clarity.

RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES

LA RESTRICCIÓN Y EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

- Refer to BCSI-B7 Summary Sheet - Temporary & Permanent Restraint/Bracing for Parallel Chord Trusses for more information.

Vea el resumen BCSI-B7 - Restricción y Arriostre Temporal y Permanente de Trusses de Cuerdas Paralelas para más información.



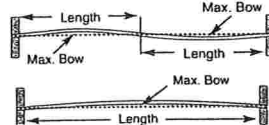
Apply Diagonal Brace to vertical webs at end of cantilever and at bearing locations.

All Lateral Restraints lapped at least two trusses.

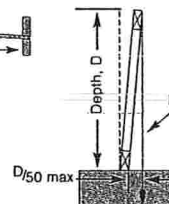
*Top chord Temporary Lateral Restraint spacing shall be 10' o.c. max. for 3x2 chords and 15' o.c. for 4x2 chords.

INSTALLING — INSTALACION

- Tolerances for Out-of-Plane.
Tolerancias para Fuera-de-Plano.



- Tolerances for Out-of-Plumb.
Tolerancias para Fuera-de-Plomada.



Out of Plumb	Out of Plumb
D/50	D (ft.)
1/4"	1'
1/2"	2'
3/4"	3'
1"	4'
1-1/4"	5'
1-1/2"	6'
1-3/4"	7'
2"	≥8'

Max. Bow	Truss Length
3/4"	12.5'
7/8"	14.6'
1"	16.7'
1-1/8"	18.8'
1-1/4"	20.8'
1-3/8"	22.9'
1-1/2"	25.0'
1-3/4"	29.2'
2"	≥33.3'

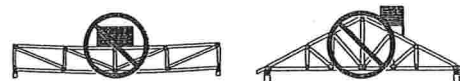
CONSTRUCTION LOADING — CARGA DE CONSTRUCCION

- Do not proceed with construction until all lateral restraint and bracing is securely and properly in place.

No proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma apropiada y segura.

- Do not exceed maximum stack heights. Refer to BCSI-B4 Summary Sheet - Construction Loading for more information.

No exceda las máximas alturas recomendadas. Vea el resumen BCSI-B4 Carga de Construcción para mayor información.



- Do not overload small groups or single trusses.
No sobrecargue pequeños grupos o trusses individuales.
- Never stack materials near a peak.
Nunca amontone material cerca del pico.
- Place loads over as many trusses as possible.
Coloque las cargas sobre tantos trusses como sea posible.
- Position loads over load bearing walls.
Coloque las cargas sobre las paredes soportantes.

ALTERATIONS — ALTERACIONES

- Refer to BCSI-B5 Summary Sheet - Truss Damage, Jobsite Modifications & Installation Errors.

Vea el resumen BCSI-B5 Daños de trusses, Modificaciones en la Obra y Errores de Instalación.

- Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.
No corte, altere o perforo ningún miembro estructural de los trusses, a menos que esté específicamente permitido en el dibujo del diseño del truss.

- Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin una autorización previa del Fabricante de Trusses, pueden reducir o eliminar la garantía del Fabricante de Trusses.

NOTE: The Truss Manufacturer and Truss Designer rely on the presumption that the Contractor and crane operator (if applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the Contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competent party. The methods and procedures outlined in this document are intended to ensure that the overall construction techniques employed will put the trusses into place SAFELY. These recommendations for handling, installing, restraining and bracing trusses are based upon the collective experience of leading personnel involved with truss design, manufacture and installation, but must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified Building Designer or Contractor. It is not intended that these recommendations be interpreted as superior to the Building Designer's design specification for handling, installing, restraining and bracing trusses and it does not preclude the use of other equivalent methods for restraining/bracing and providing stability for the walls, columns, floors, roofs and all the interrelated structural building components as determined by the Contractor. Thus, WTCA and TPI expressly disclaim any responsibility for damages arising from the use, application, or reliance on the recommendations and information contained herein.



6300 Enterprise Lane • Madison, WI 53719
608/274-4849 • www.sbcindustry.com



218 N. Lee St., Ste. 312 • Alexandria, VA 22314
703/683-1010 • www.tpinst.org

Maronda Systems

MARONDA SYSTEMS

4005 Maronda Way

Sanford, FL 32771

(407) 321-0064

Fax (407) 321-3913

Date: November 1, 2006

To: Building Department

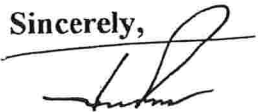
**From: Maronda Systems
Tomas Ponce
Professional Engineer
State of Florida #0050068**

Subject: Valley Trusses

All valley trusses labeled V-1 through 100 are covered under the general valley sheet provided in the truss package signed and sealed by the engineer of record. The connections are noted on the structural info sheet of the plans. All criteria of the valley trusses are noted on the general sheet.

If you have any questions please feel free to call at 407-321-0064.

Sincerely,



Tomas Ponce, P.E.

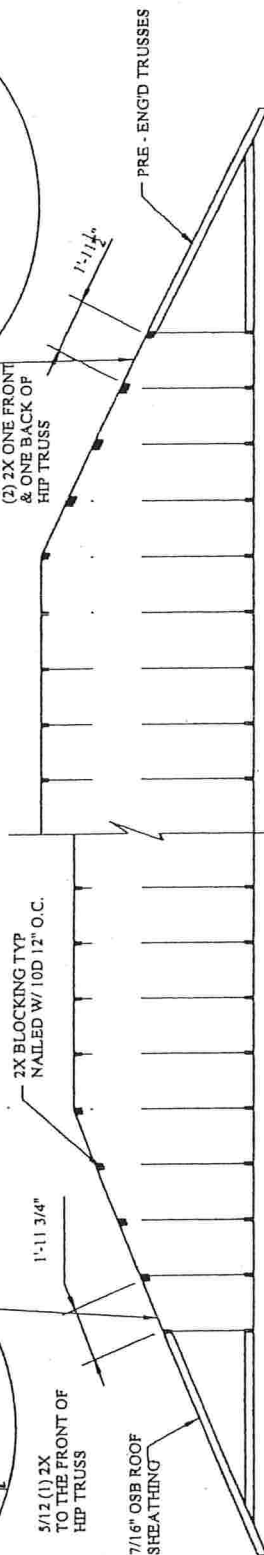
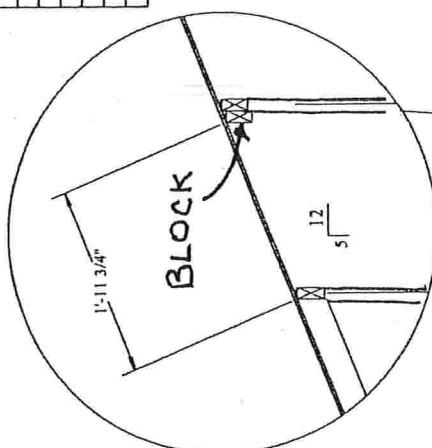
Date: 11/1/06

APA-FORM NO. TT-083

SPAN RATING AND BLOCKING RECOMMENDATIONS FOR USE OVER HIP-ROOFS SUPPORT AT 24" O.C.

Roof Slope	Panel Span Rating			48/24
	24/18	32/18	40/20	
≤ 1:12	OK	OK	OK	OK
≤ 2:12	OK	OK	OK	OK
≤ 3:12	OK	OK	OK	OK
≤ 4:12	OK	OK	OK	OK
≤ 5:12	OK	OK	OK	OK
≤ 6:12	OK	OK	OK	OK
≤ 8:12	OK	OK	OK	OK
≤ 7:12	OK	OK	OK	OK
≤ 8:12	OK	OK	OK	OK
≤ 9:12	OK	OK	OK	OK
≤ 10:12	OK	OK	OK	OK
≤ 11:12	OK	OK	OK	OK
≤ 12:12	OK	OK	OK	OK
≤ 13:12	OK	OK	OK	OK
≤ 14:12	OK	OK	OK	OK

NOTE: SHADED AREA REQUIRES 2X LUMBER BLOCKING BETWEEN TRUSS AT UN SUPPORTED PANEL. EDGES IN ITP ROOF ARE PAS



NOTE: NO BLOCKING IS REQUIRED ON 4/12 AND BELOW PITCHED ROOFS

WARNING:

WARNING: A COPY OF THIS DRAWING TO BE GIVEN TO
 THE ARCHITECT, CONTRACTOR, AND THE
 INSPECTOR.

ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 for TPI) For specific riss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 583 D'Donofrio Drive, Madison, Wisconsin 53719).

D'Onofrio Drive, Madison, Wisconsin 53719).

Eng Job:	WO: HIPDETAIL
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Dwg: T1: HTP

Dsgnr: TTY Chk: 11/2/2006

TC	live	16	0	ref	libr	DF: 1-25
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PLT	DF:	1.25
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[illegible]

TPI-02/FBC

[illegible]

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bending and handling. Temporary bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the trusses shall be applied to trusses until after all fastening and bracing is completed.

Samford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
267 Merchiston Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\work\Jobs\Job-1\COMN-1.prx

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the trustee designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility for any errors or omissions. The designer disclaims any responsibility for the design and installation of trusses. This truss has been designed in accordance with the building component in accordance with ANS/PD-1, 1995, and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has the design assumed for storage or occupancy loads. The design designer is not responsible for the design of the truss to be supported by shearwalls or other foundation systems. Where bottom chords in tension are not fully braced by a maximum spacing of 10'-0" girders, they should be braced at the property applied load rating. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A36, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for the quality of the work. Any discrepancies are to be put in writing before cutting or fabricating. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with milk fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x4 plate is 6" wide x 4" long. Slots (holes) shall be parallel to the plate length specified. Double cut ends of web members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain loading and/or erection stresses. This truss is not to be fabricated with the restraint treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/J77P1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling. Temporary and permanent installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection for trusses requires such temporary bracing during installation. The supervision of erection of trusses shall be monitoring. The supervision of erection of trusses shall be done by the contractor and persons experienced in the trade. The contractor shall seek professional advice that be sought if needed. Concentration of design loads shall be sought. Design loads shall not be applied to trusses greater than the design loads than the weight of the erection shall be applied to trusses until after all fastening and bracing is completed.

CANT: 0- 7- 0

EXCEPT AS SHOWN PLATES ARE MITEK MT20

WARNING:

Maronda Systems

4005 MARONDA WAY
Sanford, FL. 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
3267 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\C.prx

Customer: MONTGOMERY C WO: MGMC

III: C

Qty: 2

UC: 2x 4 SP #2
BC: 2x 4 SP #2
BC: 2x 4 SP #2
VB: 2x 4 SP #3

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. User defined loadcases have been considered in this design.

This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O.

```

MULTIPLE LOADCASES -- This design is the
composite result of multiple loadcases.
L. and R. End verticals designed for wind
W/ndled per ASCE 7-02, Cbc, V= 125psf
H= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0
Bld type= part L= 10.6 ft W= 10.6 ft Truss
in End zone, TCDL= 4.2 psf BCDL= 6.0 psf
--MAX. REACTIONS PER BEARING LOCATION--
X-loc Bset Vset Horiz Uplift Y T
0-11-0 1 509 16 -696 B H
9-8-0 1 509 0 -696 B H
PROVIDE UPLIFT CONNECTION PER SCHEDULE

```

Unbalanced load case(s) have been checked.

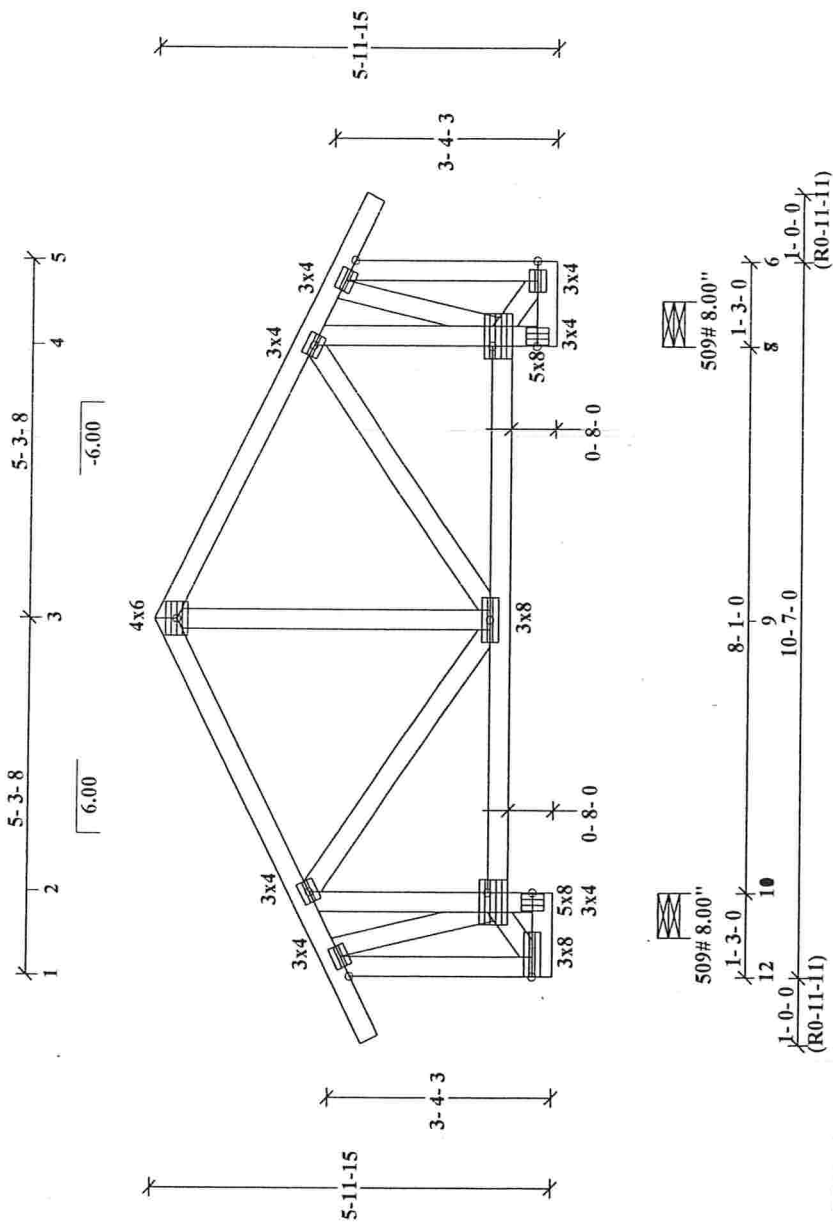
	Joint Locations											
1)	0	0	0	5)	10	7	0	9)	5	3	8	
2)	1	3	3	6)	10	7	0	10)	1	3	0	
3)	5	3	8	7)	9	4	0	11)	1	3	0	
4)	9	4	0	8)	9	4	0	12)	0	0	0	

In-Plant Quality Assurance with Cq = 1.00

-----GLOBAL MAX DEFLECTIONS-----

	LL	TL
in/petio		/petio
T-t/V		T-t/V

	in./ratio	in./ratio	in./ratio
L. Cant.	0.00/999	0.00/999	12
I. Span	-0.01/999	-0.02/999	4-3
R. Cant.	0.00/999	0.00/999	6
Griz.	0.00	0.00	NA
Long term deflection factor			= 1.50



CANT: 0- 7- 0
Scale=0.3486

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY C
ERECTING CONTRACTOR. BRACING WARNING:

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of
the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss
members only to reduce buckling length. Provisions must be made to anchor internal bracing at ends and specified
members determined by the building designer. Additional bracing of the overall structure may be required. (See HB-91
of TPI) For more information, contact building designer (Truss Plus Institute, TPI is located at 983
O'Donofrio Drive, Madison, Wisconsin 53719).
Component Engineering by: Wisconsin Designers, P.A., 818 Sandspide Rd. Elkhorn, NC 27623

Eng Job: MGMC

Dwg: Dsgmr: ECH Chk:

17/07/2007	Lbr DF: 1.25	16.0	psf
	Plt DF: 1.25	7.0	psf
	O.C.: 2 - 0 - 0	10.0	psf
	FBC-04/TPI-02	10.0	psf
	Code: FLA	43.0	psf
	v5.5.94145		
	TOTAL		

TOTAL	43.0	psf	v5.5.6-94145
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MGMC

Job: MONTGOMERY C

Customer: MONTGOMERY C

WO: MGMC

TI: C1

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses.

This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climate records for wind and snow loads, local specifications of special steel or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10' 0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for the erection of the truss. Discrepancies are to be put in writing before cutting of plates. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bundling and handling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

Unbalanced load case(s) have been checked.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

User defined loadcases have been considered in this design.

TC: FORCE...CSI, Cr...BC...FORCE...CSI.

1-2 74 0.12 A 11-10 -58 0.02

2-3 376 0.15 A 9-8 81 0.05

3-4 444 0.18 A 8-7 193 0.12

4-5 387 0.31 A 7-6 68 0.10

GLOBAL MAX DEFLECTIONS-----

LL

in./Ratio

in./Ratio

Int(s)

L.Cant. 0.00/xxx 0.00/xxx 11

I.Span 0.07/999 0.06/999 5-4

Horiz. 0.00 0.01 NA

Long term deflection factor = 1.50

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases.

L. and R. End verticals designed for wind

WindLoad per ASCE 7-02, C&C, V= 125mph,

E= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= part I= 10.6 ft W= 10.6 ft Truss

In END zone, TCDL= 4.2 psf, BCDL= 6.0 psf

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-63/74,2-3=-247/376,

3-4=-236/444,4-5=-291/387,

BOT CHORD 11-10=-58/53,10-9=-474/584,

9-8=-54/81,8-7=-61/193,7-6=-51/68,

9-2=-394/523,

Webs 11-1=-55/20,11-9=-53/84,

1-9=-47/88,2-8=-188/257,8-3=-90/117,

3-7=-89/77,7-4=-65/205,7-5=-207/270,

6-5=-344/458,

X-LOC Bset Vert Horiz Uplift Y Type

0-11-0 1 548 37 -724 B H Roll

10-3-0 1 461 0 -611 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

WEBC...FORCE...CSI, Cr...WEB...FORCE...CSI, C

10-9 584 0.10 B 8-3 117 0.03

9-2 523 0.08 C 3-7 -89 0.04

11-1 -55 0.23 C 7-4 205 0.05

11-9 84 0.03 C 7-5 270 0.09

1-9 88 0.03 C 6-5 458 0.25

2-8 257 0.08 C

Joint Locations-----

1) 0-0-0 5) 10-7-0 9) 1-1-4

2) 1-1-4 6) 10-7-0 10) 1-1-4

3) 3-11-8 7) 6-7-8 11) 0-0-0

4) 6-7-8 8) 3-11-8

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

Scale = 0.3322

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO

ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI). For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oro Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenport, NC 27932

Eng Job: MGMC

Dwg:

Dsgnr: ECH

Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: MGMC

TI: C1

2/1/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.3505-83652

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion PL Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\1.prx

DESIGN INFORMATION

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FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for the erection of the truss. Discrepancies are to be put in writing before cutting of plates. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

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TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

Unbalanced load case(s) have been checked.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

User defined loadcases have been considered in this design.

TC: FORCE...CSI, Cr...BC...FORCE...CSI.

1-2 74 0.12 A 11-10 -58 0.02

2-3 376 0.15 A 9-8 81 0.05

3-4 444 0.18 A 8-7 193 0.12

4-5 387 0.31 A 7-6 68 0.10

GLOBAL MAX DEFLECTIONS-----

LL

in./Ratio

in./Ratio

Int(s)

L.Cant. 0.00/xxx 0.00/xxx 11

I.Span 0.07/999 0.06/999 5-4

Horiz. 0.00 0.01 NA

Long term deflection factor = 1.50

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases.

L. and R. End verticals designed for wind

WindLoad per ASCE 7-02, C&C, V= 125mph,

E= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= part I= 10.6 ft W= 10.6 ft Truss

In END zone, TCDL= 4.2 psf, BCDL= 6.0 psf

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-63/74,2-3=-247/376,

3-4=-236/444,4-5=-291/387,

BOT CHORD 11-10=-58/53,10-9=-474/584,

9-8=-54/81,8-7=-61/193,7-6=-51/68,

9-2=-394/523,

Webs 11-1=-55/20,11-9=-53/84,

1-9=-47/88,2-8=-188/257,8-3=-90/117,

3-7=-89/77,7-4=-65/205,7-5=-207/270,

6-5=-344/458,

X-LOC Bset Vert Horiz Uplift Y Type

0-11-0 1 548 37 -724 B H Roll

10-3-0 1 461 0 -611 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

WEBC...FORCE...CSI, Cr...WEB...FORCE...CSI, C

10-9 584 0.10 B 8-3 117 0.03

9-2 523 0.08 C 3-7 -89 0.04

11-1 -55 0.23 C 7-4 205 0.05

11-9 84 0.03 C 7-5 270 0.09

1-9 88 0.03 C 6-5 458 0.25

2-8 257 0.08 C

Joint Locations-----

1) 0-0-0 5) 10-7-0 9) 1-1-4

2) 1-1-4 6) 10-7-0 10) 1-1-4

3) 3-11-8 7) 6-7-8 11) 0-0-0

4) 6-7-8 8) 3-11-8

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

Scale = 0.3322

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO

ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI). For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oro Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenport, NC 27932

Eng Job: MGMC

Dwg:

Dsgnr: ECH

Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: MGMC

TI: C1

2/1/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.3505-83652

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion PL Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\1.prx

2

77) 0-0-0
V= 125mph,
Kzt= 1.0
6 ft Truss
= 6.0 psf

 $C_q = 1$

```

ar
t(s):
      FORCE..CSI.
      0 0.69
      0 0.69
      0 0.52

```

Scale = 0 3553

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
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Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of
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members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified
locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91
of TPI) For specific mass bracing requirements, consult building designer. (Truss Plate Institute, TPI is located at 583
D'Orofino Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenton, NC 27932

Eng Job: MGMC	WO: MGMC
Dwg:	TI: FG2
Dsgnr: ECH	2/1/2007
TC Live 16.0 psf	lbr DF: 1.25
TC Dead 7.0 psf	Plt DF: 1.25
BC Live 10.0 psf	O.C.: 2- 0- 0
BC Dead 10.0 psf	TPI-02/FBC-04
	Code: FLA
TOTAL 43.0 psf	v4 7 3505-83655

DESIGN INFORMATION

This design is for an individual building component and is based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to transportation, handling, shipment and installation.

This test must be designed as an individual building component in accordance with ANSI/APA 1-195 and RD5-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building Designer, the design loadings shown must be checked to be sure that the chain loads are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where braced by sheathing unless otherwise braced. Lateral motion chords in tension are not fully braced laterally by a maximum applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel in accordance with ASTM A 653, 30 gauge, unless otherwise shown.

FABRICATION NOTES

prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted material. Members shall be cut for tight fitting wood to wood framing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be 5" wide at the joint unless otherwise shown. A 3x4 plate shall be 3" long. A 6x8 plate is 6" wide x 8" long. Double holes on an parallel to the plate length specified. Double holes on members shall be at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain loading, and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANS/ITPI 1-1905.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bracing and bundling, delivery and installation to avoid damage. Temporary bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary erection bracing is always required such temporary bracing for trusses requires avoid toppling and dismantling. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Consideration of construction loads greater than design loads shall not be applied to trusses at any time. The erection of trusses shall be completed and then applied to trusses will after all fastening and bracing is completed.

```

UC: 2x 4 SP #2
UC: 2x 4 SP #2
WB: 2x 4 SP #3
WB: 2x 4 SP #3
REPL: 2x 4 SP #3

```

```

Wind loads have been checked perpendicular
to the face of the truss.
Non-Plant Quality Assurance with Cq= 1
-----MAX. REACTIONS PER BEARING LOCATION-----
Loc Bset Vert Horiz Uplift Y Typ
0- 4- 0 1 127 -227 -116 B P
9- 8- 0 1 127 -227 -112 B P
PROVIDE UPLIFT CONNECTION PER SCHEDULE
HORIZONTAL CONNECTION PER SCHEDULE
.WEB..FORCE..CSI. Cr .WEB..FORCE..C
21- 2 140 0.06 C 16- 7 162 0
20- 3 90 0.18 C 15- 8 104 0
19- 4 104 0.42 C 14- 9 90 0
18- 5 162 0.70 C 13-10 140 0
17- 6 -83 0.35 C
-----GLOBAL MAX DEFLECTIONS-----

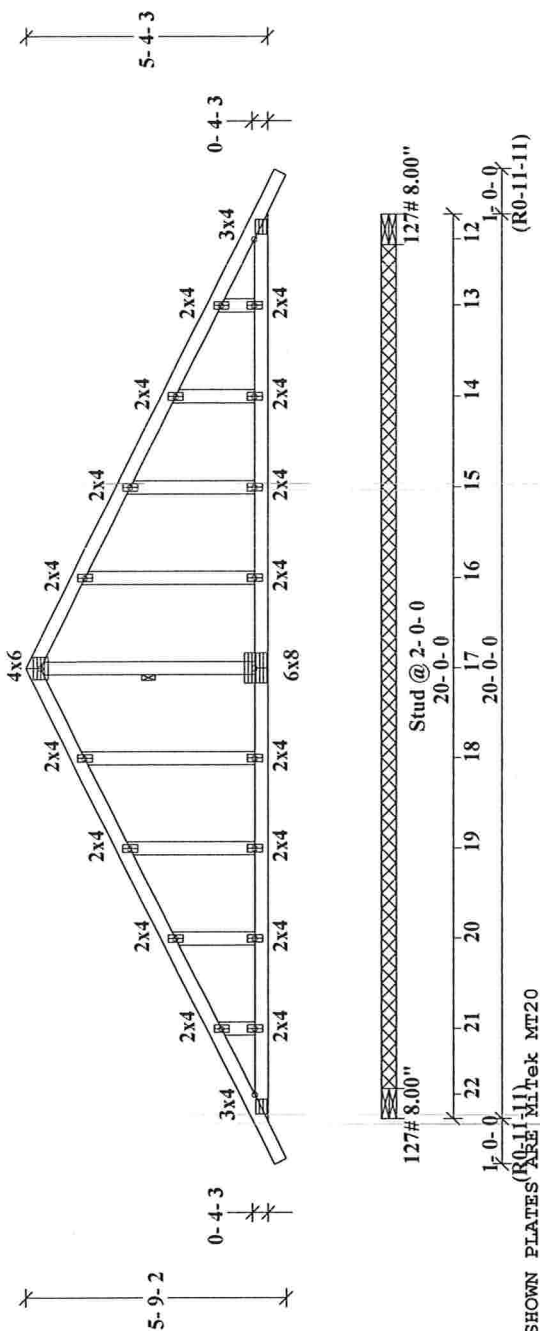
```

I.Span	in./Ratio	in./Ratio	Jnt(s).
Horiz.	0.00/999	0.00/999	11-10
	0.00	0.01	NA
Long term deflection factor = 1.50			

Analysis based on Simplified Analog Model.
All COMPRESSION Chords are assumed to be
continuously braced unless noted otherwise.
C&C, V= 1.25mph,
Wtd per ASCE 7-02,
H= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0
Bld Type= encl I= 20.0 ft W= 20.0 ft Truss
in END zone, TCDI= 4.2 psf, BCDI= 6.0 psf
Impact Resistant Covering and/or Glazing Req

Bracing Schedule:

Web	Brace Pt	Joint to Joint
	1/2	6-17
Bracing shown is for visual purposes only.		
FORCES (lb) - Max Compression/Max Tension		
TOP CHORD	1-2=-111/255	2-3=-48/196,
3-4=-39/142	4-5=-37/95	5-6=-41/134,
6-7=-48/134	7-8=-95/556	8-9=-142/4,
9-10=-171/16	10-11=-255/32,	
BOT CHORD	1-21=0/0	21-20=0/0
20-19=0/0	18-17=0/0	17-16=0/0
16-15=0/0	14-13=0/0	13-11=0/0,
Web	21-2=-97/140	20-3=-89/90,
19-4=-91/104	18-5=-93/162	17-6=-83/33,
16-7=-93/162	15-8=-91/104	14-9=-89/90,
13-10=-97/140		



Scale = 0.2337

EXCEPT AS SHOWN PLATES (R0-11-11) ARE M1 Tek MT20

Maronda Systems
4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #00500686
367 Medallion Pl. Chuluota, FL 32766

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
ELECTRICIAN CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of
the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss
members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified
locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91
of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583
D'Oroville Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenboro, NC 27932

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\G1.prx

Name: MONTGOMERY C	Customer: MONTGOMERY C	WO: MGMC	TI: HGRD3	Qty: 2
DESIGN INFORMATION This design is for an individual building component and has been based on information provided by the client. The designer declines any responsibility for damages as a result of design errors or omissions. This design is the composite result of multiple loadcases. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. 2-PLY TRUSS fasten w/3"x0.120" nails in staggered pattern per nailing schedule: TC- 2 per ft. BC- 4 per ft. WEBS- 2 per ft. Distribute loads equally to each ply. This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O. ..TC.. ..FORCE..CSI.. Cr ..BCI.. ..FORCE..CSI.. 1- 2 -8702 0.37 C 10- 9 7731 0.54 2- 3 -10963 0.68 C 9- 8 7938 0.98 3- 4 -10963 0.68 C 8- 7 7938 0.98 4- 5 -8702 0.37 C 7- 6 7731 0.54 2 PLYS REQUIRED Linal Footage = 33.3				
FABRICATION NOTES Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for the accuracy of the drawings. Fabricator shall be responsible for checking all dimensions and materials before cutting or fabrication. Plates shall be installed over treetholes, knots or distorted grain. Members shall be cut for light fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum size based on the forces shown and may need to be increased for certain handling, and/or erection stresses. This truss is not to be fabricated without the required number of lumber nails shown. For additional information on Quality Control refer to ANSI/TPI 1-1995				
PRECAUTIONARY NOTES All bracing and erection recommendations are for reference only. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Precautionary bracing shall be erected prior to erection of other construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed				
EXCEPT AS SHOWN PLATES ARE MI TEK MT20				
WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB-9) of TPI. For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oro Drive, Madison, Wisconsin 53719). Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenon, NC 27932				
Maronda Systems 4005 MARONDA WAY Sanford, FL 32771 (407) 321-0064 Fax (407) 321-3913 TOMAS PONCE P.E. LICENSE #0050068 367 Mettallon Pl. Chuluota, FL 32768				
Eng Job: MGMC Dwg: Dsgnr: TLY Chk: TC Live 16.0 psf TC Dead 7.0 psf BC Live 10.0 psf BC Dead 10.0 psf TOTAL 43.0 psf Scale = 0.3266				
Joint Locations 1) 0- 0- 0 5) 12- 0- 0 9) 2- 9-12 2) 2- 9-12 6) 12- 0- 0 10) 0- 0- 0 3) 6- 0- 0 7) 9- 2- 4 4) 9- 2- 4 8) 6- 0- 0 TOTAL DESIGN LOADS Uniform PLF From PLF To TC Vert L+D -66 0- 0- 0 -66 12- 0- 0 BC Vert L+D -696 0- 0- 0 -696 12- 0- 0 Concentrated LBS Location BC Vert L+D -1423 6- 0-12 In-Plant Quality Assurance with Cq = 1.00 In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s): 1,2,4,5,8 FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=-8702/670,2-3=-10963/641, 3-4=-10963/641,4-5=-8702/670, BOT CHORD 10-9=-558/7731,9-8=-572/7938, 8-7=-572/7938,7-6=-558/7731, C Webs 2-9=-164/2482,2-8=-1/3216, 8-3=0/217,8-4=-1/3216,7-4=-164/2482,				

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climate records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for any other loading. The design assumes compression chords (top or bottom) are braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility to the client. The fabricator shall be responsible for putting in writing before cutting or fabricating plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym about the joint unless otherwise shown. A 5/4 plate is 5" wide x 4" long. A 6/8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

EXCEPT AS SHOWN PLATES ARE MiTek MT20

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HUD-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oroff Drive, Madison, Wisconsin 53719). Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenton, NC 27932

Unbalanced load case(s) have been checked. Analysis based on Simplified Analog Model. MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. PROVIDE MOMENT CONNECTION PER SCHEDULE:

Support 1 8021#
 ---MAX. REACTIONS PER BEARING LOCATION---
 X-Loc Bset Vert Horiz Uplift Y Type
 0- 4- 0 1 361 241 -536 B Fixed
 5-11- 4 1 111 149 -271 T Pin
 5-11- 4 1 90 0 0 B H Roll
 PROVIDE UPLIFT CONNECTION PER SCHEDULE
 PROVIDE HORIZONTAL CONNECTION PER SCHEDULE

WdLoad per ASCE 7-02, C&C, V= 125mph, B= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0
 Bld Type= encl I= 6.0 ft W= 6.0 ft Truss
 In END zone, TCDF= 4.2 psf, BCDL= 6.0 psf
 Impact Resistant Covering and/or Glazing Req
 ..TC.. FORCE.. CSI.. Cr ..BC.. ..FORCE.. CSI..
 1- 2 62 0.75 A 1- 3 0 0.34

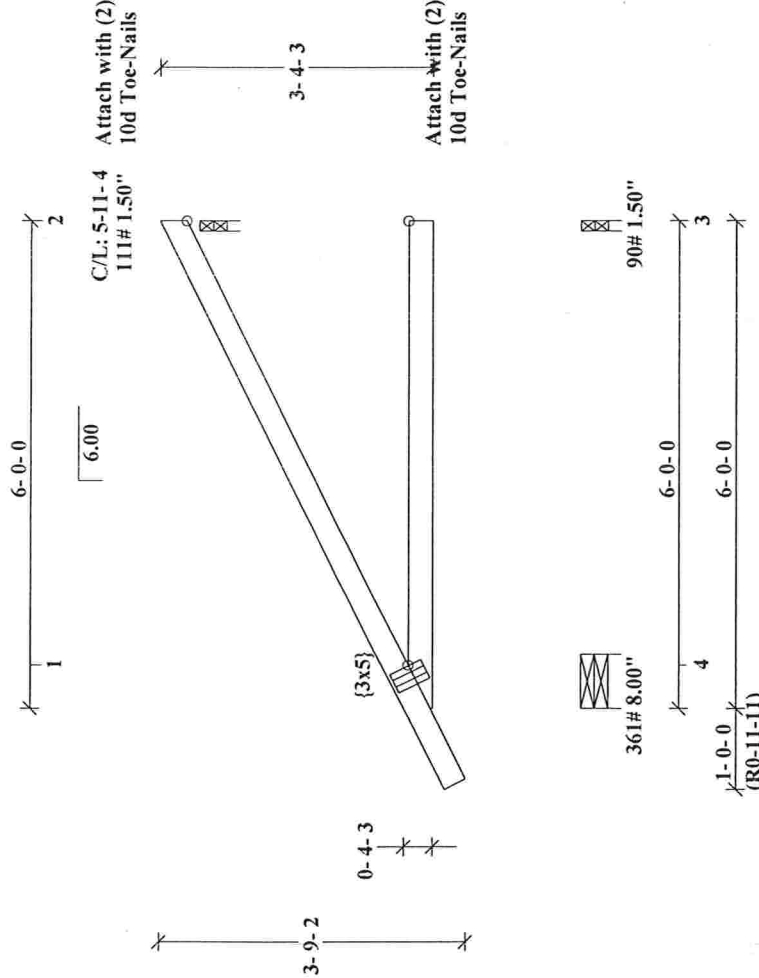
1) 0- 6- 7 3) 6- 0- 0
 2) 6- 0- 0 4) 0- 6- 7

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq= 1
 FORCES (lb) - Max Compression/Max Tension
 TOP CHORD 1-2=-11/62,
 BOT CHORD 1-3=0/0,
 -----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s)
 I.Span 0.37/170 0.14/456 2-1
 Horiz. 0.00 0.00 NA
 Long term deflection factor = 1.50



Over 3 Supports Scale = 0.4201

Eng Job: MGMC

Dwg:

Dsgnr:ECH Chk:

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: MGMC

TI: J

2/1/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.3505-83676

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\J.prx

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses.

This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climate records for wind or snow loads, project specifications or special requirements. The design loadings are based on the design assumptions shown on the design drawings. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformity with the fabricator's plans and to realize a continuing responsibility for the accuracy of the design. Any discrepancies are to be noted in writing before fabrication begins. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/4 plate is 5" wide x 4" long. A 6/8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during landing and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

TC: 2x 4 SP #2
BC: 2x 4 SP #2
[] or []: Plate(s) OFFSET from joint center. The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation.

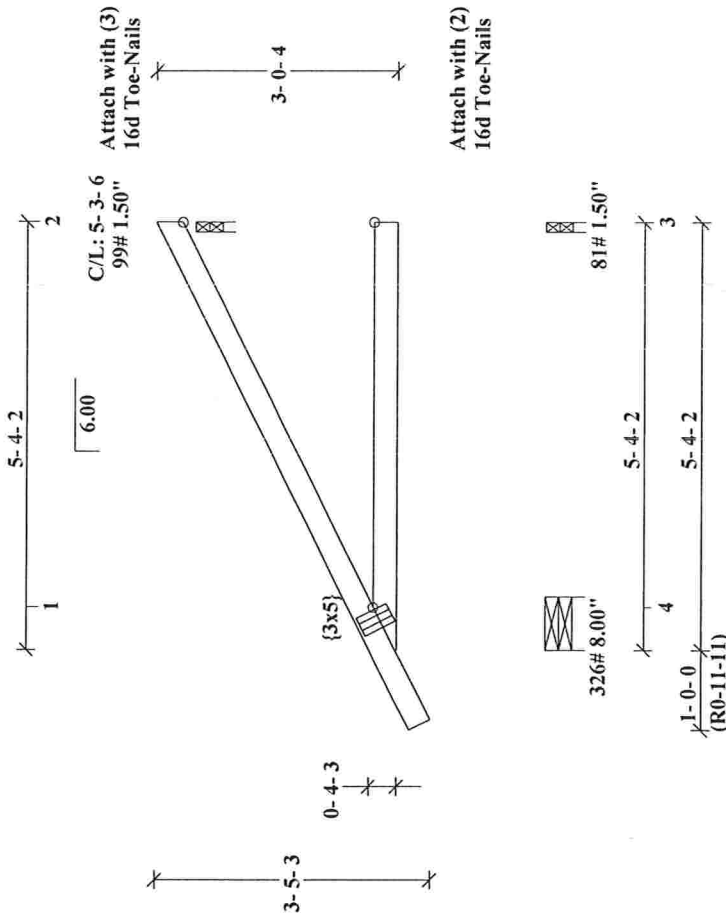
User defined loadcases have been considered in this design.

PROVIDE MOMENT CONNECTION PER SCHEDULE :

Support 1 6546#
FORCES (lb) - Max Compression/Max Tension
TOP CHORD 1-2=55/10,
BOT CHORD 1-3=0/0,

Analysis based on Simplified Analog Model. MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Windload per ASCE 7-02, C_sC_e V= 125mph, E= 15,0 ft, I= 1.00, Exp. Cat. B, K_{zt}= 1.0 Bld Type= encl L= 5.3 ft W= 5.3 ft Truss in END zone, TCDFL= 4.2 psf, BCDFL= 6.0 psf Impact Resistant Covering and/or Glazing Req. --TC. --FORCE. CSI. Cr. BC. --FORCE. CSI. 1-2 -55 0.73 A 1-3 0 0.27
-----GLOBAL MAX DEFLECTIONS-----
LL TL
I. Span in./Ratio in./Ratio Jnt(s).
Horiz. 0.24/225 0.09/604 2-1
Long term deflection factor = 1.50

Unbalanced load case(s) have been checked.
-----Joint Locations-----
1) 0-6-7 3) 5-4-2
2) 5-4-2 4) 0-6-7
This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.
In-Plant Quality Assurance with Cq= 1
-----MAX. REACTIONS PER BEARING LOCATION-----
X-Loc Bset Vert Horiz Uplift Y Type
0-4-0 1 326 221 -500 B Fixed
5-3-6 1 99 137 -249 T Pin
5-3-6 1 81 0 0 B H Roll
PROVIDE UPLIFT CONNECTION PER SCHEDULE
PROVIDE HORIZONTAL CONNECTION PER SCHEDULE



EXCEPT AS SHOWN PLATES ARE MiTek MT20

Over 3 Supports Scale = 0.4142

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050069

367 Medallion Pl. Chuluota, FL 32766

WARNING:
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Component Engineering by: Truss Engineering Co., P.A., 818 Seaside Rd, Edenton, NC 27932

Eng Job: MGMC	WO: MGMC
Dwg:	TI: J1
Dsgnr: ECH	Chk: 2/1/2007
TC Live 16.0 psf	Lbr DF: 1.25
TC Dead 7.0 psf	Plt DF: 1.25
BC Live 10.0 psf	O.C.: 2- 0- 0
BC Dead 10.0 psf	TPI-02/FBC-04
TOTAL 43.0 psf	Code: FTA
	v4.7.3505-83678

Unbalanced load case(s) have been checked.

Joint Locations

1) 0-6-7	3) 3-10-2
2) 3-10-2	4) 0-6-7

This truss is designed to bear on multiple supports. Interior bearing location should

A vertical cable is shown with a pulley at the top and a weight at the bottom. The weight is represented by a rectangle with horizontal lines inside, and a small circle at the top where the cable is attached. The cable is a single vertical line.

Over 3 Supports Scale = 0.44

Job: MGMC	WO: MGMC
Pr: ECH	TI: J2
Chk:	2/1/2007

head	7.0	psf
live	10.0	psf
dead	10.0	psf
	42.0	psf

Unbalanced load case(s) have been checked.

Joint Locations

1) 0- 6- 7 3) 2- 4- 2
2) 2- 4- 2 4) 0- 6- 7

This truss is designed to bear on multiple supports. Interior bearing locations should

X-Loc	Bset	Vert	Horiz	Uplift	Y	Type
0-0	0	1	168	101	-280	B Fix
2-3	6	1	44	64	-116	T P
2-3	6	1	36	0	0	B H Ro

with (2)
e-Nails

6-4

with (2)

Over 3 Supports Scale = 0 5408

WO: MGMC
TI: J3
2/1/2007
Lbr DF: 1.25
Plt DF: 1.25
O.C.: 2- 0- 0
TPI-02/FBC-04
Code: FIA
v4.7.3505-83683

Job: MONTGOMERY C

Customer: MONTGOMERY C

WO: MGMC

TI: J4

Qty: 3

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by the building designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climate records for wind or snow loads, project specifications or special applied loads. Unless shown, trusses have not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced in a maximum spacing of 10'-4" o.c. Connector plates shall be manufactured from a single piece of dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for right fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3/4" plate is 5' wide x 1" long. (A 1/2" plate is 3' wide x 8" long. 3/8" plate (holes) may parallel the nail length specified. Double end members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for lifting trusses through roof openings and for erecting trusses shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

TC: 2x 4 SP #2

BC: 2x 4 SP #2

[] or () : Plate(s) OFFSET from joint center.

The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation.

User defined loadcases have been considered in this design.

PROVIDE MOMENT CONNECTION PER SCHEDULE :

Support 1 121#

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-9/2,

BOT CHORD 1-3=0/0,

Analysis based on Simplified Analog Model. MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Windload per ASCE 7-02, C&C, V= 125mph, B= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0 Bld Type= encl I= 0.8 ft W= 0.8 ft Truss in END zone, TCDD= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req. .TC. .FORCE. .CSI. Cr .BC. .FORCE. .CSI. 1-2 -9 0.02 A 1-3 0 0.00 1-3 -9 0.02 A 1-3 0 0.00

-----GLOBAL MAX DEFLECTIONS-----

IL TL in./Ratio in./Ratio Jnt(s).

I Span 0.00/999 0.00/999 2-1

Horiz. 0.00 0.00 NA

Long term deflection factor = 1.50

Unbalanced load case(s) have been checked.

=====Joint Locations=====

1) 0- 6- 7 3) 0-10- 2

2) 0-10- 2 4) 0- 6- 7

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0- 4- 0 1 88 36 -159 B Fixed

0- 9- 6 1 17 26 -45 T Pin

0- 9- 6 1 14 0 0 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

EXCEPT AS SHOWN PLATES ARE MiTek MT20

Over 3 Supports Scale = 0.6080

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only, to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific inss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oroff Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenport, NC 27932

Eng Job: MGMC

Dwg: TI: J4

Dsgnr: ECH Chk: 2/1/2007

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: MGMC

TI: J4

2/1/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

V4.7.3505-83685

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\J4.prx

Unbalanced load case(s) have been checked.

==Joint Locations==

1) 0- 8- 2	3) 6-10- 0
2) 6-10- 0	4) 0- 8- 2

This truss is designed to bear on multiple supports. Interior bearing location should

be marked on truss. Shim or wedge if necessary to achieve full bearing.

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc	BSet	Vert	Horiz	Uplift	Y	Type
0- 4- 0	1	428	182	-569	B	Fix

6-9-4	1	123	124	-298	F	P
6-9-4 <td>1 <td>103 <td>0 <td>0 <td>B <th>F Ro</th> </td></td></td></td></td>	1 <td>103 <td>0 <td>0 <td>B <th>F Ro</th> </td></td></td></td>	103 <td>0 <td>0 <td>B <th>F Ro</th> </td></td></td>	0 <td>0 <td>B <th>F Ro</th> </td></td>	0 <td>B <th>F Ro</th> </td>	B <th>F Ro</th>	F Ro

PROVIDE UPLIFT CONNECTION PER SCHEDULE

CI-017

Over 3 Supports Scale = 0.4185

Eng Job: MGMC	WO: MGMC
Dwg:	TI: J5
Dsgnr: ECH	Chk: 2/1/2007
TC Live 16.0 psf	Lbr DF: 1.25
TC Dead 7.0 psf	Plt DF: 1.25
	0 C - 2 - 0 - 0

TC Dead	7.0	psf	PLT DF: 1.25
PC Ring	10.0	psf	O.C.: 2- 0- 0

Job: MONTGOMERY C		Customer: MONTGOMERY C		WO:MGMC		TI: J5A		Qty: 1	
DESIGN INFORMATION This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 30 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown. FABRICATION NOTES Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully embedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5" wide x 4" long. A 4x6 plate is 6" wide x 6" long. Slots (holes) with parallel ends shall be cut to the center of the truss unless otherwise shown. Connector plate sizes are to be determined based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995 PRECAUTIONARY NOTES All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for lifting trusses in a straight and plumb position for erection shall be designed and installed by others. Critical loads in erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed									
TC: 2x 4 SP #2 BC: 2x 4 SP #2 [] or []: Plate(s) OFFSET from joint center. The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation. User defined loadcases have been considered in this design. PROVIDE MOMENT CONNECTION PER SCHEDULE : Support 1 9261# FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=56/0, BOT CHORD 1-3=0/1,									
Analysis based on Simplified Analog Model. MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Wind per ASCE 7-02, C&C, V= 125mph, H= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0 Bld Type= encl L= 6.8 ft W= 6.8 ft Truss in END zone, TCDF= 4.2 psf, BCDF= 6.0 psf Impact Resistant Covering and/or Glazing Req ..TC.. FORCE..CSI.. Cr ..BC.. FORCE..CSI.. 1- 2 -56 0.99 A 1- 3 1 0.44 -----GLOBAL MAX DEFLECTIONS----- IL TL I. Span in./Ratio in./Ratio Jnt(s) . Horiz. 0.48/165 0.18/435 2-1 Long term deflection factor = 1.50 Unbalanced load case(s) have been checked. -----Joint Locations----- 1) 0- 8- 2 3) 6-10- 0 2) 6-10- 0 4) 0- 8- 2 This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing. In-Plant Quality Assurance with Cq= 1 ---MAX. REACTIONS PER BEARING LOCATION--- X-Loc Bset Vert Horiz Uplift Y Type 0- 0-14 1 362 189 -457 B Fixed 6- 9- 4 1 123 129 -311 T Pin 6- 9- 4 1 103 0 0 B H Roll PROVIDE UPLIFT CONNECTION PER SCHEDULE									
Diagram showing truss structure with dimensions and labels. Labels: 2-10-15, 6-10-0, 4.50, 103# 1.50", 362# 1.75", 6-10-0, 3, 4, 0-4-3, (3x5), Attach with (4) 16d Toe-Nails, Attach with (2) 16d Toe-Nails									
EXCEPT AS SHOWN PLATES ARE Mitek MT20 Over 3 Supports Scale = 0.4504									
Maronda Systems 4005 MARONDA WAY Sanford, FL 32711 (407) 321-0064 Fax (407) 321-3913 TOMAS PONCE P.E. LICENSE #0050068 367 Medallion Pl. Chuluota, FL 32766 Design: Matrix Analysis Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\J5A.prx									
WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for final support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB-91 of TPI) For specific truss bracing information, contact building designer. (Truss Plate Institute, TPI is located at 383 D'Onefro Drive, Madison, Wisconsin 53719) Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932 Eng Job: MGMC Dwg: J5A Dsgnr: ECH Chk: 2/1/2007 TC Live 16.0 psf TC Dead 7.0 psf BC Live 10.0 psf BC Dead 10.0 psf TOTAL 43.0 psf Lbr DF: 1.25 Plt DF: 1.25 O.C.: 2- 0- 0 TPI-02/FBC-04 Code: FTA v4.7.3505-83692									

Job: MONTGOMERY C	Customer: MONTGOMERY C	WO: MEMC	TI: J6	Qty: 1
DESIGN INFORMATION This design is for an individual building component and has been based on information provided by the client. The designer assumes no responsibility for the accuracy or completeness of any facts or figures furnished by the client and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or specific project and accepts no responsibility for fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by the Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climate records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid diaphragm, they should be braced at a maximum height of 10 feet above the base plate. All steel shall be manufactured from 30 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown. FABRICATION NOTES Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knuckles, knots or distorted members. All connections shall be made in full bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/8" plate is 5" wide x 4" long. A 6/8" plate is 6" wide x 8" long. Slots (holes) web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995. PRECAUTIONARY NOTES All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bundling and handling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all flashing and bracing is completed.				
TC: 2x 4 SP #2 BC: 2x 4 SP #2 [] or [{}] Plate(s) OFFSET from joint center. The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation. User defined loadcases have been considered in this design. PROVIDE MOMENT CONNECTION PER SCHEDULE : Support 1 3716# FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=-34/0, BOT CHORD 1-3=0/1,				
Analysis based on Simplified Analog Model. MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Windload per ASCE 7-02. C&C, V= 125mph, E= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0 Bld Type= encl L= 4.2 ft W= 4.2 ft Truss in END zone, TCCL= 4.2 psf, BCCL= 6.0 psf Impact Resistant Covering and/or Glazing Req. ..TC ..FORCE..CSI. Cr .BC. ..FORCE..CSI. 1- 2 -34 0 42 A 1- 3 1 0.16 -----GLOBAL MAX DEFLECTIONS----- LL TL I. Span in./Ratio in./Ratio Unt(s). Horiz. 0.08/483 0.03/999 2-1 Long term deflection factor = 1.50				
Unbalanced load case(s) have been checked. Joint Locations: 1) 0- 8- 2 3) 4- 2- 0 2) 4- 2- 0 4) 0- 8- 2 This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing. In-Plant Quality Assurance with Cq= 1 ---MAX. REACTIONS PER BEARING LOCATION--- X-Loc Bset Vert Horiz Uplift Y Type 0- 4- 0 1 286 134 -461 B Fixed 4- 1- 4 1 75 84 -203 T Pin 4- 1- 4 1 63 0 0 B H Roll PROVIDE UPLIFT CONNECTION PER SCHEDULE				
Eng Job: MEMC Dwg: 2/1/2007 Dsgr: ECH Chk: TC Live 16.0 psf TC Dead 7.0 psf BC Live 10.0 psf BC Dead 10.0 psf TOTAL 43.0 psf Over 3 Supports Scale = 0.4722				

Qty: 1

Unbalanced load case(s) have been checked.

Joint Locations

1) 0-8-2 3) 4-2-0
2) 4-2-0 4) 0-8-2

MAX.	REACTS PER BEARING LOCATION	Bset	Vert	Horiz	Uplift	Y	Type
0-0-14	1	220	135	-328	B	Fixed	
4-1-4	1	75	84	-204	T	P	
4-1-4	1	63	0	0	B	H	Roll

Attach with (3)
16d Toe-Nails

Attach with (2) 16d Toe-Nails

Over 3 Supports Scale = 0-5478

Eng Job: MCMC	WO: MCMC
Dwg:	TI: J6A
Dsgnr: ECH	2/1/2007
Chk:	
TC Live	16.0 psf
TC Dead	7.0 psf
BC Live	10.0 psf
BC Dead	10.0 psf
	43.0 psf

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\J6A.prx

MGMC

DESIGN INFORMATION

This design is for an individual building component and is based on information provided by the client. The designer declines any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or specific control with regard to fabrication, handling, shipment and installation of trusses.

This truss has been designed in accordance with the NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by a Building Designer, the designer shall be responsible for checking to be sure that the design shown is in agreement with the local building codes, local climatic conditions for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knots, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all finishing and bracing is completed

Job: MONTGOMERY C

Customer: MONTGOMERY C

WO: MGMC

TI: J7A

Qty: 1

TC: 2x 4 SP #2

BC: 2x 4 SP #2

[] or []: Plate(s) OFFSET from joint center.

The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation.

User defined loadcases have been considered in this design.

PROVIDE MOMENT CONNECTION PER SCHEDULE :

Support 1 430#

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-13/2,

BOT CHORD 1-3=0/1,

Analysis based on Simplified Analog Model. MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Windload per ASCE 7-02, C6C, V= 125mph, H= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0 Bld Type= encl I= 1.5 ft W= 1.5 ft Truss in END zone, TCDF= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req. .TC. .FORCE. CSI. Cr .BC. .FORCE. CSI. 1- 2 -13 0.05 A 1- 3 1 0.02

-----GLOBAL MAX DEFLECTIONS-----

LL TL in./Ratio in./Ratio Jnt(s).

I. Span 0.00/999 0.00/999 2-1

Horiz. 0.00 0.00 NA

Long term deflection factor = 1.50

Unbalanced load case(s) have been checked.

1) 0- 8- 2 3) 1- 6- 0

2) 1- 6- 0 4) 0- 8- 2

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq= 1

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc Bset Vert Horiz Uplift Y Type

0- 0-14 1 77 49 -115 B Fixed

1- 5- 4 1 28 32 -76 T Pin

1- 5- 4 1 24 0 0 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

0-10-15

0-4-3

77# 1.75"

24# 1.50"

1-6-0

1 1 2

4.50

C/L: 1-5-4

28# 1.50"

Attach with (2) 16d Toe-Nails

0-10-15

Attach with (2) 16d Toe-Nails

EXCEPT AS SHOWN PLATES ARE

MiTek MT20

Over 3 Supports

Scale = 0.6500

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, consult building designer (Truss Plate Institute, TPI is located at 583 D'Oro Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Eden, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Eng Job: MGMC

Dwg: TI: J7A

Dsgnr: ECH Chk: 2/1/2007

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: MGMC

TI: J7A

2/1/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

V4.7.3505-83708

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\J7A.prx

MGMC

Job: MONTGOMERY C

Customer: MONTGOMERY C

WO: MGMC

TI: J11

Qty: 4

DESIGN INFORMATION

This design is for an individual building component and is based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climate records for wind or snow loads, proper specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuous. Bottom chords in spans are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall be fabricated per AWS D1.1 and shall be for fire retardant treated wood. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed

TC: 2x 4 SP #2

BC: 2x 4 SP #2

Unbalanced load case(s) have been checked.

[] or []: Plate(s) OFFSET from joint center.

The Joint Detail Report must be included with any submittal, inspection, and/or fabrication documentation.

User defined loadcases have been considered in this design.

PROVIDE MOMENT CONNECTION PER SCHEDULE :

Support 1 2997#

..TC: ..FORCE..CSI. Cx ..BC. ..FORCE..CSI.

1- 2 -31 0.34 A 1- 3 1 0.13

-----GLOBAL MAX DEFLECTIONS-----

LL

in./Ratio in./Ratio Jnt(s).

I.Span 0.06/630 0.02/999 2-1

Horiz. 0.00 0.00 NA

Long term deflection factor = 1.50

Analysis based on Simplified Analog Model.

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

Windload per ASCE 7-02, C&C, V= 125mph, H= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= encl L= 3.8 ft W= 3.8 ft Truss in END zone, TCCL= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-31/6,

BOT CHORD 1-3=0/1,

=====Joint Locations=====

1) 0- 8- 2 3) 3- 9- 0

2) 3- 9- 0 4) 0- 8- 2

This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cq= 1

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc BSet Vert Horiz Uplift Y Type

0- 4- 0 1 264 121 -432 B Fixed

3- 8- 4 1 68 76 -183 T Pin

3- 8- 4 1 57 0 0 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

3-9-0

1

2

4.50

C/L: 3-8-4

68# 1.50"

Attach with (2)

16d Toe-Nails

1-9-1

Attach with (2)

16d Toe-Nails

264# 8.00"

57# 1.50"

3-9-0

4

3

1-5-4

(RI-5-2)

3-9-0

EXCEPT AS SHOWN PLATES ARE MITek MT20

Over 3 Supports Scale = 0.5949

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Place Institute, TPI is located at 583 D'Oro Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Eden, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\J11.prx

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the rrus designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or warranty control with regard to fabrication, handling, shipment and installation of rruuses. This rruus has been designed in accordance with the requirements set forth in ASTM F1957 and NDS-907 as part of the building design by a Building Designer. When reviewed for approval by a Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, rruus has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in elevation are not fully braced laterally by a properly spaced rruus ceiling, they should be designed for minimum spanning of 6' o.c. The designers shall be responsible for determining if the proposed advanced steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or disordered grain. Members shall be cut for tight fitting wood to wood joints. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be 5" wide x 4" long. A 6"x6" plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise noted. Connector plate sizes are minimum sizes based on the following: 1) 100% of the truss members are to be hand-banded and/or erection struts. This truss is to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to: ANSI/TPI-1:1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with pertinent industry publications. Trusses are to be braced with perpendicular bracing during erection. Trusses are to be braced with perpendicular bracing during dismantling and bracing. Delivery and installation are to avoid damage to temporary and permanent bracing for holding trusses in place. Temporary bracing is to be installed by the contractor. Temporary bracing is to be installed by the contractor. Careful handling is essential and erection bracing is always required. Normal precautionary bracing for movement required during temporary bracing during erection is required to avoid loosening and distortion. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No load other than the weight of the erection shall be applied to trusses until after all fastening and bracing is completed.

```

TC:  2x 4  SP #2
BC:  2x 4  SP #2
WB:  2x 4  SP #3

All COMPRESSION Chords are assumed to be
continuously braced unless noted otherwise.
User defined loadcases have been considered
in this design.

This truss is designed in accordance with
the 2004 Florida Building Code section
2306.1 and referenced standards, U.N.O.

-----MAX. REACTIONS PER BEARING LOCATION-----
X-Loc      Bset  Vert  Horiz  Uplift  Y  Type
9-10-10     1    950   263      -524  B  Pin
9-10-12     1    963      0    -545  B  H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE
PROVIDE HORIZONTAL CONNECTION PER SCHEDULE
..TC.. FORCE.. CSI.. Cz ..BC.. ..FORCE..CSI..
1 - 2  -1443  0.51  C  6 - 5  1327  0.50
2 - 3   -82  0.54  C  5 - 4  1302  0.60

-----GLOBAL MAX DEFLECTIONS-----
LL          TL
in./Ratio  in./Ratio  Unt(s)
I.Span    -0.05/999  -0.11/758  3-2(L)
Horiz.     0.01      0.02      NA

Long term deflection factor = 1.50
(L): Deflection is from local inner panel

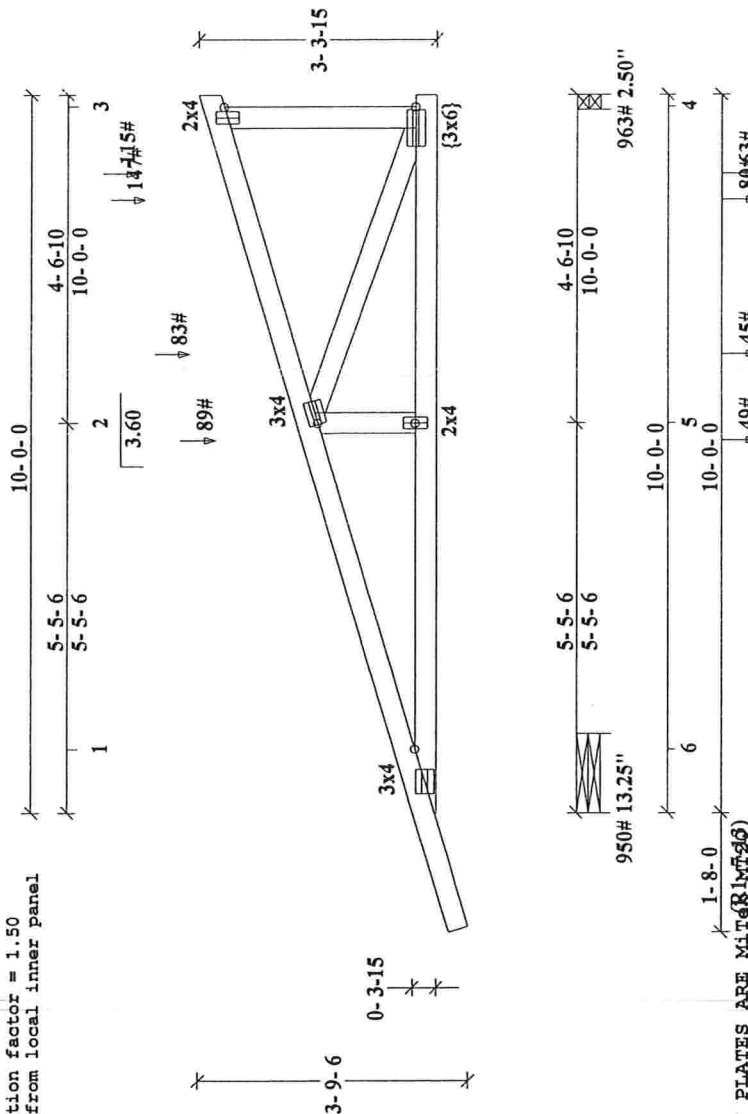
```

```

MULTIPLE LOADCASES -- This design is the
composite effect of multiple loadcases.
Right End vertical designed for wind.
WInd= 15.0 ft, CSc, V= 125mph,
H= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0
Bld Type= encl L= 10.0 ft W= 10.0 ft Truss
in END zone, TCdL= 4.2 psf, BCDL= 6.0 psf
Impact Resistant Covering and/or Glazing Req.
In-Plant Quality Assurance with Cq = 1.00
In-Plant Quality Assurance, per
sec. 3.2.4 of TPI-1-02, for joint(s):
2
FORCES (lb) - Max Compression/Max Tension
TOP CHORD 1-2=-1443/606, 2-3=-827/26,
BOT CHORD 6-5=-794/1327, 5-4=-795/1302,
Webs 5-2=0/369, 2-4=-1400/855,
3-3=-265/226,
..WEBC..CSFI..Cr ..WEBC..FORCE..CSI
5-2 369 0.12 C 4- 3 -265 0.
2-4 -1400 0.61 C

```

10-0-0



Scale = 0 3694

EXCEPT AS SHOWN PLATES ARE MITSUBISHI

WABNTNC:

**READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR. BRACING WARNING:**

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design. It is intended to provide lateral support for the building during construction. It is not intended to provide lateral support for the building members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See SRS-513 of TPI). For design of the bracing system, consult building designer. (Tross Plate Institute, TPI is located 318-531-0260/ro).

Donner, Madison, Wisconsin 53710.

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenlon, NC 27932

Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Metallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: T:\TEE-LOK\Work\Jobs\MEMC\MONTGOMERY C\TGBD1.prx

Eng Job: MCMC	WO: MCMC
Dwg:	TI: JGRD1
Dsgnr: TLY	Chk: 5/3/2007

TC Live	16.0	psf	Lbr DF: 1
TC Dead	7.0	psf	Plt DF: 1
BC Live	10.0	psf	O.C.: 2-
BC Dead	10.0	psf	FBC-04/TP
TOTAL	43.0	psf	Code: FLA
			v5.4 18-5

Titm: 10000

WO: MGMC
TI: JGRD1
5/3/2007

Lbr DF: 1
Plt DF: 1
O.C.: 2-
FBC-04/TP
Code: FLA
v5.4.18-5

Name: MONTGOMERY C		Customer: MONTGOMERY C		WO:MGMC		TT: JGRD1A		Qty:1	
DESIGN INFORMATION This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O. -----MAX. REACTIONS PER BEARING LOCATION----- X-LOC BSet Vert Horiz Uplift Y Type 0- 6-10 1 873 263 0 -545 B H Roll 9-10-12 1 963 0 -545 B H Roll PROVIDE UPLIFT CONNECTION PER SCHEDULE PROVIDE HORIZONTAL CONNECTION PER SCHEDULE ..TC.. ..FORCE.. CSI.. Cr.. BC.. ..FORCE.. CSI.. 1- 2 -1443 0.51 C 6- 5 1327 0.50 2- 3 -82 0.54 C 5- 4 1302 0.60 unless otherwise shown. FABRICATION NOTES Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the information provided by the client and the building code for each jurisdiction. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over bolt holes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x4 plate is 6" wide x 4" long. Slot (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling, and/or erection stresses. This truss is not to be fabricated or installed in a manner other than that shown, unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995 PRECAUTIONARY NOTES All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bracing and bundling. Delivery and installation of truss members shall be done in a manner that does not damage the truss in a straight and plumb position and for installing lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erection shall be applied to trusses until after all bracing and bundling is completed.									
MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. Right end vertical designed for wind. Windload per ASCE 7-02, C&C, V= 125mph, H= 15.0 ft, Z= 1.00, Exp. Cat. B, Kzt= 1.0 Bld Type= encl I= 10.0 ft W= 10.0 ft Truss in END zone, TCCL= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req In-Plant Quality Assurance with Cq = 1.00 sec. 3.2.4 of TPI-1-02, for joint(s): 2 FORCES (lb) - Max Compression/Max Tension TOP CHORD 1-2=-1443/640, 2-3=-82/26, BOT CHORD 6-5=-794/1327, 5-4=-795/1302, Webs 5-2=0/369, 2-4=-1400/855, 4-3=-265/226, ..WEB.. ..FORCE.. CSI.. Cr.. WEB.. ..FORCE.. CSI.. Cr.. 5- 2 -369 0.12 C 4- 3 -265 0.24 C 2- 4 -1400 0.61 C Joint Locations 1) 0- 0- 0 3) 10- 0- 0 5) 5- 5- 6 2) 5- 5- 6 4) 10- 0- 0 6) 0- 0- 0 -----TOTAL DESIGN LOADS----- Uniform PLF From To TC Vert L+D -97 0- 0- 0 -97 3-10-14 TC Vert L+D -46 3-10-14 -46 10- 0- 0 BC Vert L+D -67 0- 0- 0 -67 3-10-14 BC Vert L+D -40 3-10-14 -40 10- 0- 0 Concentrated LBS Location TC Vert L+D -89 5- 2- 8 TC Vert L+D -83 6- 4-14 TC Vert L+D -147 8- 6- 8 TC Vert L+D -115 8-10-14 BC Vert L+D -49 5- 2- 8 BC Vert L+D -45 6- 4-14 BC Vert L+D -80 8- 6- 8 BC Vert L+D -63 8-10-14 -----GLOBAL MAX DEFLECTIONS----- LL in./Ratio in./Ratio Jnt(s) I. Span -0.05/999 -0.11/756 3-2(L) Horiz. 0.01 0.02 NA Long term deflection factor = 1.50 (L): Deflection is from local inner panel									
EXCEPT AS SHOWN PLATES ARE MITEK MT20									
Scale = 0.3849									
Maronda Systems 4005 MARONDA WAY Saintford, FL 32771 (407) 321-0064 Fax (407) 321-3913 TOMAS PONCE P.E. LICENSE #0050068 367 Medallion Pl. Chuluota, FL 32768 Design: Matrix Analysis									
WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING IS TO BE GIVEN TO THE EFFECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-51 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 383 Duxbury Compcon Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenboro, NC 27932)									
Eng Job: MGMC Dwg: TTY Dsgnr: TTY Chk: TC Live 16.0 psf TC Dead 7.0 psf BC Live 10.0 psf BC Dead 10.0 psf TOTAL 43.0 psf									
WO: MGMC TI: JGRD1A 5/3/2007 Lbr DF: 1.25 Plt DF: 1.25 O.C.: 2- 0- 0 FSC-04/TPI-02/NDS-01 Code: FLA V5.4.18-5897									

Job: MONTGOMERY C

Customer: MONTGOMERY C

WO: MGMC

TI: JGRD3

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5" wide x 4" long. A 4x6 plate is 6" wide x 8" long. Slots (holes) parallel to the plate face specified shall be 1/8" wide with a maximum spacing of 12" o.c. at the end of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling, and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for loading trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses and supporting structures. The erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Consumption of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erecors shall be applied to trusses until after all fastening and bracing is completed

TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

User defined loadcases have been considered in this design.

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc Bset Vert Horiz Uplift Y Type

0- 6-10 1 374 85 -450 B Pin

4- 6- 4 1 263 0 -271 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

.WEB. . .FORCE. .CSI. Cr

3- 2 240 0.10 C

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. Right End vertical designed for wind. Windload per ASCE 7-02, C&C, V= 125mph, H= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0 Bid Type= encl I= 12.0 ft W= 4.6 ft Truss in END zone, TC DL= 4.2 psf, BC DL= 6.0 psf Impact Resistant Covering and/or Glazing Req FORCES (Lb) - Max Compression/Max Tension TOP CHORD 1-2=-43/38, BOT CHORD 4-3=-32/44, Webs 3-2=-139/240, .TC. . .FORCE. .CSI. Cr . .BC. . .FORCE. .CSI. .

1- 2 -43 0.32 C 4- 3 44 0.24

---GLOBAL MAX DEFLECTIONS---

LL TL

in./Ratio in./Ratio Jnt(s)

I. Span -0.02/412 -0.04/921 3-4

Horiz. -0.01 -0.02 NA

Long term deflection factor = 1.50

1) 0- 0- 0 3) 4- 7- 8

2) 4- 7- 8 4) 0- 0- 0

TOTAL DESIGN LOADS

Uniform PLF From To

TC Vert L+D -46 - 1- 8- 0 -46 4- 7- 8

BC Vert L+D -40 0- 0- 0 -40 4- 7- 8

Concentrated LBS Location

TC Vert L+D -17 0-10- 3

TC Vert L+D -28 1- 9-10

TC Vert L+D -44 3- 9-10

BC Vert L+D -14 0-10- 3

BC Vert L+D -24 1- 9-10

BC Vert L+D -36 3- 9-10

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 583 D'Onefro Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenon, NC 27932

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.4700

Eng Job: MGMC

Dwg: JGRD3

Dgnr: TLY Chk: 2/1/2007

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: MGMC

TI: JGRD3

2/1/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FTA

v4.7.3505-81718

Maronda Systems

4005 MARONDA WAY

Saunder, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #00050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\JGRD3.prx

MGMC

Job: MONTGOMERY C

Customer: MONTGOMERY C

WO: MGMC

TI: JS1

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and is based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climate records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for any other loads. Trusses are to be designed as compression chords (top or bottom) are to be braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for each verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates to be put in writing before cutting or fabrication. Plates to be put in writing before cutting or fabrication. Members shall be cut for light fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with units fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/4 plate is 5" wide x 4" long. A 6/8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-58/149,

BOT CHORD 1-3=-18/23,

Unbalanced load case(s) have been checked. Analysis based on Simplified Analog Model. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Shim or wedge if necessary to achieve full bearing.

In-Plant Quality Assurance with Cp=1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc Bset Vert Horiz Uplift Y Type

0-4-0 1 276 0 -313 B H Roll

5-3-6 1 120 212 -90 T Pin

5-3-6 1 110 0 -22 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

PROVIDE HORIZONTAL CONNECTION PER SCHEDULE

Joint Locations

1) 0-5-8 3) 5-4-2

2) 5-4-2 4) 0-5-8

Windload per ASCE 7-02, C&C, V= 125mph, B= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= encl I= 40.0 ft W= 5.3 ft Truss in INT zone, TCDL= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

..TC..FORCE..CSI..Cr..BC..FORCE..CSI.

1-2 149 0.34 A 1-3 23 0.29

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s).

I.Span -0.06/196 -0.11/486 3-1

Horiz. 0.00 0.00 NA

Long term deflection factor = 1.50

Diagram

Diagram showing truss structure with dimensions and labels:

- Dimensions: 5-4-2, 6.00, 120# 1.50", 1-11-8, 110# 1.50", 276# 8.00", 5-4-2, 5-4-2, 1-0-0 (R0-11-1), 0-4-3, 3-5-3, 1-0-12, 2.72, 0-8-0, 1-0-0, 4, 3, 2, 1
- Labels: Attach with (2) 16d Toe-Nails, Attach with (3) 16d Toe-Nails, C/L: 5-3-6 120# 1.50"

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing. Wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, consult building designer (Truss Plate Institute, TPI is located at 583 D'Onofrio Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl, Chuluota, FL 32766

Design: Matrix Analysis

Over 3 Supports Scale = 0.4731

Eng Job: MGMC

Dwg: TI: JS1

Dsgnr: ECH Chk: 2/1/2007

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: MGMC

TI: JS1

2/1/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2-0-0

TPI-02/FBC-04

Code: FLA

v4.7.3505-83721

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\JS1.prx

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 3x4 plate is 5" wide x 4" long. A 6x6 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plates shall not be installed on the basis of the truss shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and building, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during erection. When erected, the design loadings shall be maintained. The provision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed.

Job: MONTGOMERY C

Customer: MONTGOMERY C

WO:MGMC

TT:JS2

Qty:1

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=42/191,

BOT CHORD 1-3=16/16,

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s)

I.Span 0.06/xxx 0.01/xxx 3-1

Horiz. 0.00 0.00 NA

Long term deflection factor = 1.50

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

Shim or wedge if necessary to achieve full bearing.

Windload per ASCE 7-02, C&C, V= 125mph,

B= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= encl L= 40.0 ft W= 3.8 ft Truss

in INT zone, TC&C= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

..TC.. FORCE.. CSI.. Cr ..BC.. FORCE.. CSI..

1- 2 191 0.43 A 1- 3 -16 0.31

Unbalanced load case(s) have been checked.

Joint Locations

1) 0- 5- 8 3) 3-10- 2

2) 3-10- 2 4) 0- 5- 8

User defined loadcases have been considered in this design.

In-Plant Quality Assurance with CG= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0- 4- 0 1 211 0 -409 B H Roll

3- 9- 6 1 86 284 -134 T Pin

3- 9- 6 1 79 0 -36 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

PROVIDE HORIZONTAL CONNECTION PER SCHEDULE

Diagram showing truss structure with dimensions and labels.

Labels include: 3-10-2, 6.00, C/L: 3-9-6, 86# 1.50", 1-6-10, Attach with (3) 16d Toe-Nails, 0-8-10, 2.72, 0-4-3, 2-8-3, 0-8-0, 211# 8.00", 79# 1.50", 3-10-2, 3-10-2, 1-0-0, (R0-11-11)

EXCEPT AS SHOWN PLATES ARE

Over 3 Supports Scale = 0.5572

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HB-91 or TPI) For specific mass bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 583 O'Conor Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Eden, NC 27032

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\JS2.prx

Eng Job: MGMC

Dwg: TT: JS2

Dsgnr: ECH Chk: 2/1/2007

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: MGMC

TI: JS2

2/1/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.3505-83723

MGMC

Job: MONTGOMERY C

Customer: MONTGOMERY C

WO: MGMC

TI: JS3

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by the building structure. Where bracing is not shown, it is to be provided by the building designer. Trusses shall be properly applied rigid ceiling, they should be braced to maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall be fabricated over knifed edges or ordered knifed. Members shall be knifed in accordance with the bearing. Connector plates shall be located on both sides of the truss with nails fully imbedded and shall be bent about the joint unless otherwise shown. A 5/4 plate is 5" wide x 4" long. A 6/8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with recognized industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erections shall be applied to trusses until after all fastening and bracing is completed

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-26/119,

BOT CHORD 1-3=-9/9,

-----GLOBAL MAX DEFLECTIONS-----

LL

in./Ratio in./Ratio Jnt(s).

I. Span 0.01/xxxx 0.00/xxxx 3-1

Horiz. 0.00 0.00 NA

Long term deflection factor = 1.50

Analysis based on Simplified Analog Model. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Shim or wedge if necessary to achieve full bearing.

Windload per ASCE 7-02, C&C, V= 125mph, H= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0, Bld Type= encl I= 2.3 ft W= 2.3 ft Truss in END zone, TCDF= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req ..TC. ..FORCE. CSI. Cr ..BC. ..FORCE. CSI. 1- 2 119 0.17 A 1- 3 -9 0.11

Unbalanced load case(s) have been checked.

Joint Locations

1) 0- 5- 8 3) 2- 4- 2

2) 2- 4- 2 4) 0- 5- 8

User defined loadcases have been considered in this design.

In-Plant Quality Assurance with Cq= 1

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc BSet Vert Horiz Uplift Y Type

0- 4- 0 1 147 0 -288 B B Roll

2- 3- 6 1 52 178 -79 T Pin

2- 3- 6 1 48 0 -21 B B Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

EXCEPT AS SHOWN PLATES ARE M&T MT20

WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 583 D'Ondra Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Eden, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\JS3.prx

Eng Job: MGMC

Dwg: JS3

Dsgnr: ECH Chk: 2/1/2007

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: MGMC

TI: JS3

2/1/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.3505-83725

Over 3 Supports

Scale = 0.6425

Job: MONTGOMERY C	Customer: MONTGOMERY C	WO: MGMC	TI: JS4	Qty: 1
DESIGN INFORMATION This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility for errors or omissions with regard to this design. This truss has been designed as an individual building component in accordance with ANSI/TPI-1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.				
FABRICATION NOTES Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of members and fully extend beyond the joint. Stale (holes) 4" long. A 6x8 plate is 6" wide x 8" long. Stale (holes) web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI-1-1995				
PRECAUTIONARY NOTES All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.				
Analysis based on Simplified Analog Model. ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Shim or wedge if necessary to achieve full bearing. Welded per ASCE 7-02, C&C, V= 125mph, H= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0 Bld Type= encl L= 0.8 ft W= 0.8 ft Truss in END zone, TCDFL= 4.2 psf, BCDFL= 6.0 psf Impact Resistant Covering and/or Glazing Req .TC.. FORCE..CSI.. Cr ..BC.. FORCE..CSI.. 1-2 47 0.03 A 1-3 2 0.01				
Unbalanced load case(s) have been checked. ---MAX. REACTIONS PER BEARING LOCATION--- X-Loc BSet Vert Horiz Uplift Y Type 0- 4- 0 1 82 0 -168 B H Roll 0- 9- 6 1 19 71 -26 T H Pin 0- 9- 6 1 17 0 -3 B H Roll PROVIDE UPLIFT CONNECTION PER SCHEDULE				
The diagram shows a cross-section of a truss system. Key features include: - A main horizontal member labeled "3x4". - Vertical dimensions: "0-10-2" (total height), "1" (from top chord to main member), "2" (from main member to bottom chord). - Horizontal dimensions: "6.00" (width of main member), "C/L: 0-9-6" (centerline width), "19# 1.50" (nail pattern). - Labels for connections: "Attach with (2) 16d Toe-Nails" at the top and bottom. - Other dimensions: "0-8-12", "0-0-8 Attach with (2) 16d Toe-Nails", "0-0-8", "0-8-0", "82# 8.00" 17# 1.50", "0-10-2", "1-0-0", "0-10-2", "(R0-11-1)", "0-4-3", "1-2-3", "0-4-3", "2.72".				
EXCEPT AS SHOWN PLATES ARE Mitek MT20 Over 3 Supports Scale = 0.7586				
WARNING: READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See IIIB-91 of TPI) For specific truss bracing requirements, contact building designer (Truss Plate Institute, TPI is located at 583 D'Oroville Drive, Madison, Wisconsin 53719). Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Edenton, NC 27932				
Maronda Systems 4005 MARONDA WAY Sanford, FL 32771 (407) 321-0064 Fax (407) 321-3913 TOMAS PONCE P.E. LICENSE #0050069 367 Medallion Pl. Chuluota, FL 32766				
Design: Matrix Analysis				

on check

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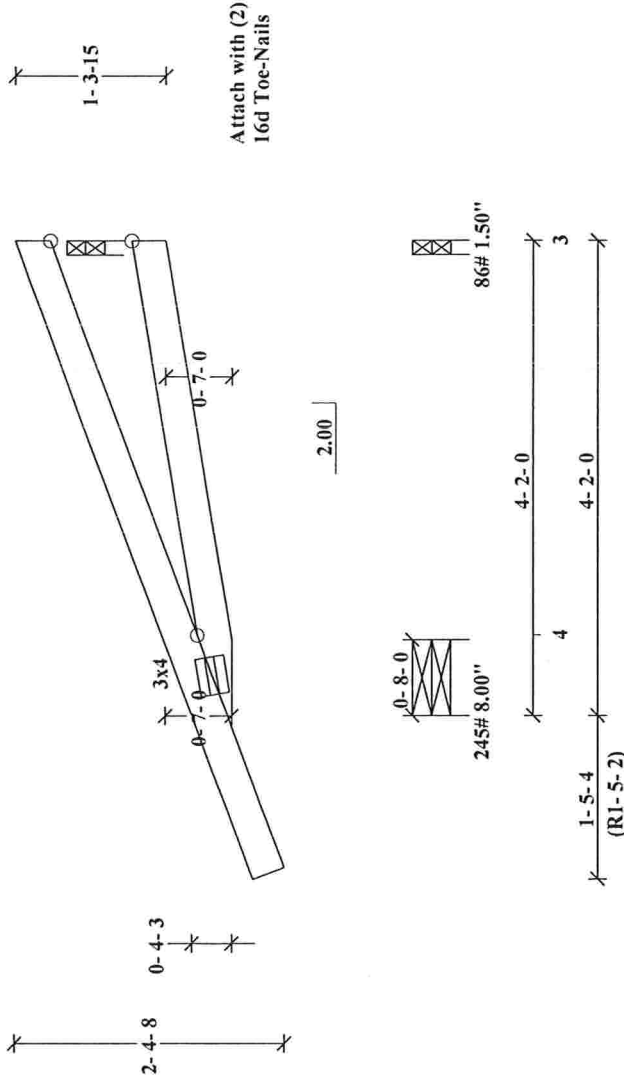
=====Joint Locations=====
1) 0-8-8      3) 4-2-0
2) 4-2-0      4) 0-8-8

User defined loadcases have been considered
in this design.

--Plant Quality Assurance with Cq=1
-----MAX. REACTIONS PER BEARING LOCATION-----
X-loc      Bset      Vert      Horiz      Uplift      Y      TYPE
0-0-0      1      245      0      -437      B      R
4-1-4      1      94      230      -182      T      R
4-1-4      1      86      0      -33      B      R

```

C/L: 4-1-4
94# 1 50"



All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be landed with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and trusses are to be braced and supported during erection. Bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than those specified shall be avoided. Trusses shall not be loaded other than by the weight of the erections shall be applied to trusses until after all bracing and bracing is completed.

Over 3 Supports Scale = 0.5890

4005 MARONDA WAY
Sanford, FL. 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl, Chuluota, FL 32766

Eng Job: MGMC	
Dwg:	
Dsgnr: ECH	Chk:
TC Live	16.0 psf
TC Dead	7.0 psf
BC Live	10.0 psf
BC Dead	10.0 psf
TOTAL	43.0 psf

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\JS6.prx

DESIGN INFORMATION

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FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the design and specifications. The fabricator shall be responsible for each verification and fabrication. Plates shall not be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for light fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/4 plate is 5" wide x 4" long. A 6/8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling, and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during handling and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

MGMC

Job: MONTGOMERY C

Customer: MONTGOMERY C

WO: MGMC

TI: JS7

Qty: 1

TC: 2x 4 SP #2

BC: 2x 4 SP #2

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. This truss is designed to bear on multiple supports. Interior bearing locations should be marked on truss. Shim or wedge if necessary to achieve full bearing.

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-14/57,

BOT CHORD 1-3=-3/3,

-----GLOBAL MAX DEFLECTIONS-----

IL

in./Ratio

in./Ratio

in./Ratio

Int(s)

I.Span

Horiz.

0.00

0.00

0.00

2-1

NA

Long term deflection factor = 1.50

Analysis based on Simplified Analog Model. All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise. Shim or wedge if necessary to achieve full bearing.

Windrod per ASCE 7-02, C&C, V= 125mph, H= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0 Bld Type= encl I= 1.5 ft W= 1.5 ft Truss in END zone, TCDF= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req. .TC. .FORCE..CSI. Cr .BC. .FORCE..CSI. 1-2 57 0.07 A 1-3 3 0.04

Unbalanced load case(s) have been checked.

Joint Locations

1) 0- 8- 8 3) 1- 6- 0

2) 1- 6- 0 4) 0- 8- 8

User defined loadcases have been considered in this design.

In-Plant Quality Assurance with Cq= 1

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc BSet Vert Horiz Uplift Y Type

0- 4- 0 1 131 0 -248 B H Roll

1- 5- 4 1 34 89 -65 T Pin

1- 5- 4 1 31 0 -10 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

Diagram showing truss structure with dimensions and labels.

0-4-3

1-4-8

1-6-0

1

2

4.50

C/L: 1-5-4

34# 1.50"

3x4

0-1-11 0-1-11

2.00

0-8-0

131# 8.00"

31# 1.50"

1-6-0

4

3

1-6-0

(RI-5-2)

Attach with (2) 16d Toe-Nails

Attach with (2) 16d Toe-Nails

WARNING:

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Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Eden, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chuluota, FL 32766

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Over 3 Supports Scale = 0.7242

Eng Job: MGMC

Dwg: TI: JS7

Dsgnr: ECH Chk: 2/1/2007

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: MGMC

TI: JS7

2/1/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

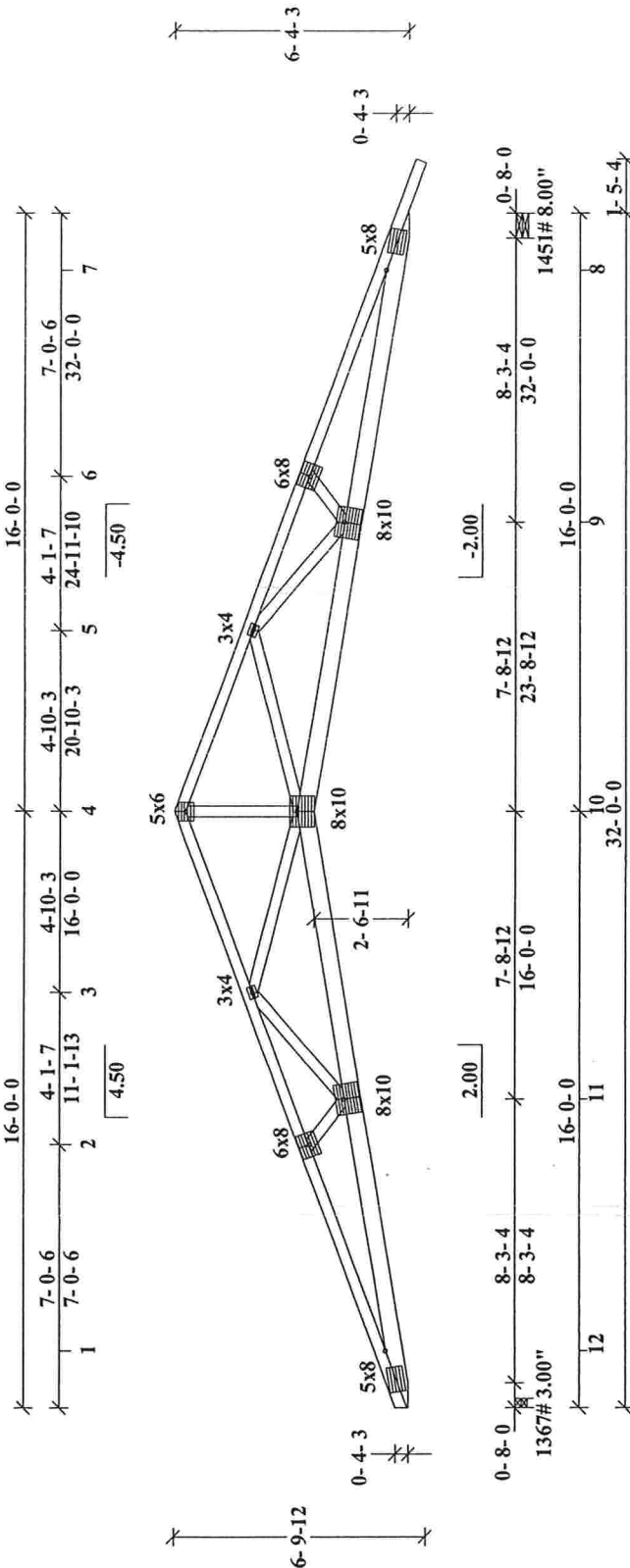
v4.7.3505-83735

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\JS7.prx

[illegible]

TOTAL 43.0 psf v5.4.18-83796



MGMC

Job: MONTGOMERY C

Customer: MONTGOMERY C

WO: MGMC

TI: SLD

Qty: 2

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or details furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and controls no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climate records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by shoring unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10 ft. o.c. Connector plates shall be designed for a minimum of 20 in. of unbraced length, meeting ASTM A 633, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for right fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. (5x6 piers is 5" wide x 4" high, 6x8 piers are 6" wide x 8" high, 8x10 piers are 8" wide x 10" high) Double end plates shall be used on all end members unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995.

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and action for bracing is always required. Normal precautions for erection of trusses shall be followed during erection. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed.

Customer: MONTGOMERY C

WO: MGMC

TI: SLD

Qty: 2

TC: 2x 4 SP #2

BC: 2x 6 SP #2

WB: 2x 4 SP #3

ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

WindLoad per ASCE 7-02, C6C, V= 125mph, H= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0

Bld Type= encl I= 32.0 ft W= 32.0 ft Truss in INT zone, TCDL= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

..TC.. ..FORCE..CSI.. Cr ..BC.. ..FORCE..CSI..

1- 2 -4854 0.78 A 12-11 4575 0.76

2- 3 -4892 0.43 A 11-10 3990 0.74

3- 4 -3372 0.39 A 10- 9 3990 0.74

4- 5 -3372 0.39 A 9- 8 4575 0.76

5- 6 -4692 0.43 A

6- 7 -4854 0.78 A

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. User defined loadcases have been considered in this design.

In-Plant Quality Assurance with Cq= 1

In-Plant Quality Assurance, per sec. 3.2.4 of TPI-1-02, for joint(s):

4

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-4854/1935, 2-3=-4692/1886, 3-4=-3372/1453, 4-5=-3372/1453, 5-6=-4692/1886, 6-7=-4854/1935, ..FORCE..CSI..

BOT CHORD 12-11=-1742/4575, 11-10=-1537/3990, 10-9=-1537/3990, 9-8=-1742/4575, 8-7=-1537/3990, 7-6=-1537/3990, 6-5=-1537/3990, 5-4=-1537/3990, 4-3=-1537/3990, 3-2=-1537/3990, 2-1=-1537/3990, 1-0=-1537/3990, 0-1=-1537/3990, 1-2=-1537/3990, 2-3=-1537/3990, 3-4=-1537/3990, 4-5=-1537/3990, 5-6=-1537/3990, 6-7=-1537/3990, 7-8=-1537/3990, 8-9=-1537/3990, 9-10=-1537/3990, 10-11=-1537/3990, 11-12=-1537/3990, 12-13=-1537/3990, 13-14=-1537/3990, 14-15=-1537/3990, 15-16=-1537/3990, 16-17=-1537/3990, 17-18=-1537/3990, 18-19=-1537/3990, 19-20=-1537/3990, 20-21=-1537/3990, 21-22=-1537/3990, 22-23=-1537/3990, 23-24=-1537/3990, 24-25=-1537/3990, 25-26=-1537/3990, 26-27=-1537/3990, 27-28=-1537/3990, 28-29=-1537/3990, 29-30=-1537/3990, 30-31=-1537/3990, 31-32=-1537/3990, 32-33=-1537/3990, 33-34=-1537/3990, 34-35=-1537/3990, 35-36=-1537/3990, 36-37=-1537/3990, 37-38=-1537/3990, 38-39=-1537/3990, 39-40=-1537/3990, 40-41=-1537/3990, 41-42=-1537/3990, 42-43=-1537/3990, 43-44=-1537/3990, 44-45=-1537/3990, 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MGMC

Job: MONTGOMERY C

Customer: MONTGOMERY C

WO: MGMC

TI: V1

Qty: 3

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer assumes no responsibility for design errors or omissions, or for the accuracy of the information furnished by the client, or for the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slot (holes) run parallel to the plate length specified. Double cuts on web members shall meet in the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for conditions of loading or field conditions. This truss is to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for raising internal forces shall be designed and installed by others. Credit loading is essential and erection bracing shall be maintained. No erection bracing shall be removed until the permanent bracing is installed between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

TC: 2x 4 SP #2

BC: 2x 4 SP #2

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

Windload per ASCE 7-02, C6C, V= 125mph, B= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt= 1.0

Bld Type= encl I= 3.0 ft W= 3.0 ft Truss

In END zone, TCDL= 4.2 psf, BCDL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

-----GLOBAL MAX DEFLECTIONS-----

TL

in./Ratio in./Ratio Jnt(s).

I. Span 0.00/xxxx 0.00/xxxx 4

Horiz. 0.00 0.00 NA

Long term deflection factor = 1.50

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. User defined loadcases have been considered in this design.

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc BSet Vert Horiz Uplift Y Type

0- 1-12 1 129 -343 -168 B Pin

2-10- 4 1 129 0 -168 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

PROVIDE HORIZONTAL CONNECTION PER SCHEDULE

=====Joint Locations=====

1) 0- 0- 0 3) 3- 0- 0 5) 0- 0- 0

2) 1- 6- 0 4) 3- 0- 0

In-Plant Quality Assurance with Cq= 1

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-145/208,2-3=-145/208,

BOT CHORD 5-4=-148/184,

..TC.. ..FORCE..CSI. Cr ..BC. ..FORCE..CSI.

1- 2 208 0.05 A 5- 4 184 0.06

2- 3 208 0.05 A

EXCEPT AS SHOWN PLATES ARE MITEK MT20

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 583 D'Oroff Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenon, NC 27932

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #00030068
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Eng Job: MGMC

Dwg: TI: V1

Dsgnr: ECH Chk: 2/1/2007

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: MGMC

TI: V1

2/1/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.3505-83756

Scale = 0.9205

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\1.prx

Job: MONTGOMERY C

Customer: MONTGOMERY C

WO:MGMC

TI:V2

Qty:3

DESIGN INFORMATION

This design is for an individual building component and has been designed in accordance with the design of the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or design furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accept no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where bottom chords in tension are not fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be manufactured from 1/4" gage 30,000 psi minimum yield strength steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or disordered grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with ends fully embedded and shall be set about 1/4" from the ends of the truss members. 4x6 truss shall be 4x6x8" long. A 6x8 plate is 6" wide x 8" long. Shear plates run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry practices and are not to be altered without the particular attention of the designer. Temporary and permanent installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

Windload per ASCE 7-02, C&C, V= 125mph,

B= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0

Bld Type= encl L= 6.0 ft W= 6.0 ft Truss

in END zone, TCDFL= 4.2 psf, BCDFL= 6.0 psf

Impact Resistant Covering and/or Glazing Req

-----GLOBAL MAX DEFLECTIONS-----

in./Ratio in./Ratio Jnt(s).

I. Span 0.01/xxxx 0.01/xxxx 4

Horiz. 0.00 0.00 NA

Long term deflection factor = 1.50

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. User defined loadcases have been considered in this design.

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc BSet Vert Horiz Uplift Y Type

0- 1-12 1 82 -184 -103 B Pin

5-10- 4 1 81 0 -104 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

.WEB. .FORCE..CSI. Cx

5- 2 347 0.09 C

1- 2 156 0.07 A 6- 5 146 0.11

2- 3 116 0.07 A 5- 4 197 0.11

Joint Locations

1) 0- 0- 0 3) 6- 0- 0 5) 3- 0- 0

2) 3- 0- 0 4) 6- 0- 0 6) 0- 0- 0

In-Plant Quality Assurance with Cq= 1

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-104/156, 2-3=-104/116,

BOT CHORD 6-5=-88/146, 5-4=-80/197,

WEBS 5-2=-205/347,

.TC. .FORCE..CSI. Cx .BC. .FORCE..CSI.

Diagram

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.7377

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING: Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, consult building designer (Truss Plate Institute, TPI is located at 583 D'Oro Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #00050068

367 Medallion Pl, Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\V2.prx

Eng Job: MGMC

Dwg: TI: V2

Dsgnr: ECH Chk: 2/1/2007

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: MGMC

TI: V2

2/1/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FTA

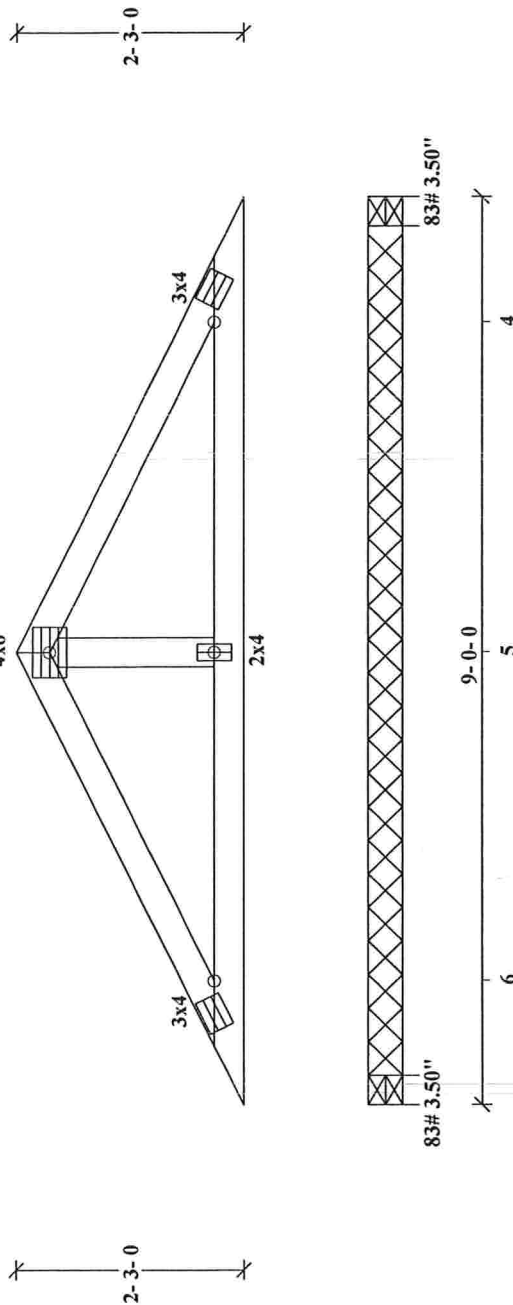
v4.7.3505-83758

	Joint Locations										
1)	0	0	0	3)	9	0	0	5)	4	6	0
2)	4	6	0	4)	9	0	0	6)	0	0	0

In-Plant Quality Assurance with Cq=1

1)	0	-	0	-	0	3)	9	-	0	-	0	5)	4	-	6	-	0
2)	4	-	6	-	0	4)	9	-	0	-	0						
In-Plant Quality Assurance with Cq= 1																	
FORCES (lb) - Max Compression/Max Tension																	
TOP CHORD 1-2=-259/251, 2-3=-253/251,																	
BOT CHORD 6-5=-200/305, 5-4=-186/364,																	
Webs 5-2=-395/618,																	
...TC...FORCE..CSI. Cr ..BC. ...FORCE..CSI.																	
1-2	-259	0.17	A	6	-	5	305	0.22									
2-3	-253	0.17	A	5	-	4	364	0.23									

A diagram showing a rectangular block resting on an inclined plane. A spring scale is attached to the bottom of the block and hangs vertically down to a horizontal surface. An arrow points to the right from the horizontal surface.



Scale = 0.5210

Saintford, FL 32271
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

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Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27931.

Eng Job: MGMC

Dwg:

TC Live	16.0	psf
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TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL	43.0	psf
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WO: MGMC

TI: V30

2/1/2007

Libr DF: 1.25

Plt DF: 1.25

U.C.: 2-0-0
EPT-02/EBC-04

Code: FLA

v4.7.3505-837

Job: MONTGOMERY C

Customer: MONTGOMERY C

WO:MGMC

TI:V31

Qty:1

TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

DESIGN INFORMATION

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FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knobholes, knots or distorted grain. Members shall be cut for tight fitting wood to wood bearing. Connector plates shall be located on both faces of the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5x4 plate is 5" wide x 4" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the web unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1993

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominating. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

Multiple Loadcases -- This design is the composite result of multiple loadcases. User defined loadcases have been considered in this design.

-----MAX. REACTIONS PER BEARING LOCATION-----

X-Loc Bset Vert Horiz Uplift Y Type

0- 1-12 1 341 -701 -456 B Pin

10- 5- 4 1 333 0 -456 B H Roll

PROVIDE UPLIFT CONNECTION PER SCHEDULE

PROVIDE HORIZONTAL CONNECTION PER SCHEDULE

-----GLOBAL MAX DEFLECTIONS-----

LL TI

I Span in./Ratio in./Ratio Jnt(s).

Horiz. 0.11/999 0.09/999 3-2

Long term deflection factor = 1.50

Joint Locations

1) 0- 8- 8 3) 11- 3- 8 5) 6- 0- 0

2) 6- 0- 0 4) 11- 3- 8 6) 0- 8- 8

In-Plant Quality Assurance with Cq= 1

FORCES (lb) - Max Compression/Max Tension

TOP CHORD 1-2=-346/551,2-3=-344/548,

BOT CHORD 6-5=-211/356,5-4=-346/270,

Webs 5-2=-10/58,

..TC.. ..FORCE..CSI.. Cr ..BC.. ..FORCE..CSI..

1- 2 551 0.34 A 6- 5 356 0.25

2- 3 548 0.36 A 5- 4 -346 0.29

EXCEPT AS SHOWN PLATES ARE MITEK MT20

Scale = 0.4767

WARNING:

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Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd. Eden, NC 27932

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

TOMAS PONCE P.E. LICENSE #0050068

367 Medallion Pl. Chulueta, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\V31.prx

Eng Job: MGMC

Dwg: TI: V31

Dsgnr: ECH Chk: 2/1/2007

TC Live 16.0 psf

TC Dead 7.0 psf

BC Live 10.0 psf

BC Dead 10.0 psf

TOTAL 43.0 psf

WO: MGMC

TI: V31

2/1/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

TPI-02/FBC-04

Code: FLA

v4.7.3505-83763

MGMC

Job: MONTGOMERY C

Customer: MONTGOMERY C

WO: MGMC

TI: V33

Qty: 1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by side members against specified lateral movement. Web members in compression shall be fully braced laterally by a properly applied rigid ceiling, they should be braced at a maximum spacing of 10'4" o.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members and the end of framing web members shall be cut square. Connector plates shall be centered on the truss with nails fully imbedded and shall be sym. about the joint unless otherwise shown. A 5/4 plate is 5" wide x 4" long. A 6/8 plate is 6" wide x 8" long. Stiles (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during bundling and handling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. L. and R. End verticals designed for wind. Windload per ASCE 7-02, C6C, V= 125mph, B= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0 Bld Type= encl I= 18.0 ft W= 10.6 ft Truss in END zone, TCDL= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req. .TC. .FORCE. .CSI. Cr .BC. .FORCE. .CSI. 1- 2 293 0.39 A 6- 5 0 0.25 2- 3 -178 0.39 A 5- 4 44 0.25 -----GLOBAL MAX DEFLECTIONS----- LL TL in./Ratio in./Ratio Jnt(s). I. Span 0.16/730 0.13/895 3-2 Horiz. 0.01 0.01 NA Long term deflection factor = 1.50

Joint Locations

1) 3- 8- 8	3) 14- 3- 8	5) 9- 0- 0
2) 9- 0- 0	4) 14- 3- 8	6) 3- 8- 8

In-Plant Quality Assurance with Cq= 1
---MAX. REACTIONS PER BEARING LOCATION---
X-Loc Bset Vert Horiz Uplift Y Type
0- 1-12 1 269 44 -399 B Pin
10- 5- 4 1 269 0 -399 B H Roll
PROVIDE UPLIFT CONNECTION PER SCHEDULE
.WEB. .FORCE. .CSI. Cr .WEB. .FORCE. .CSI. Cr
6- 1 280 0.12 C 5- 3 317 0.10 C
1- 5 -317 0.18 C 4- 3 280 0.12 C
5- 2 413 0.10 C

Diagram

Warnings

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Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\V33.prx

Table

TC: 2x 4 SP #2	Eng Job: MGMC	WO: MGMC
BC: 2x 4 SP #2	Dwg:	TI: V33
WB: 2x 4 SP #3	Dsgnr: ECH	Chk: 2/1/2007
All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.	TC Live 16.0 psf	Lbr DF: 1.25
User defined loadcases have been considered in this design.	TC Dead 7.0 psf	Plt DF: 1.25
FORCES (lb) - Max Compression/Max Tension	BC Live 10.0 psf	O.C.: 2- 0- 0
TOP CHORD 1-2=-91/293, 2-3=-178/174, 6-5=0/0, 5-4=-34/44,	BC Dead 10.0 psf	TPI-02/FBC-04
WETS CHORD 6-1=-139/280, 1-5=-317/44, 5-2=-235/413, 5-3=-9/317, 4-3=-163/280,	TOTAL 43.0 psf	Code: FLA

Scale = 0.3587

MGMC

Job: MONTGOMERY C

Customer: MONTGOMERY C

WO:MGMC

TI:V34

Qty:1

DESIGN INFORMATION

This design is for an individual building component and has been based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications and/or designs furnished to the truss designer by the client and the correctness or accuracy of this information as it may relate to a specific project and accepts no responsibility or exercises no control with regard to fabrication, handling, shipment and installation of trusses.

This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1995 and NDS-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by the building designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind or snow loads, project specifications or special applied loads. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes truss members are fully braced against lateral movement by sheathing or other specified means. Where bracing is required, it should be provided by a properly applied rigid ceiling. They should be braced at a maximum spacing of 10'4" s.c. Connector plates shall be manufactured from 20 gauge hot dipped galvanized steel meeting ASTM A 653, Grade 40, unless otherwise shown.

FABRICATION NOTES

Prior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates shall not be installed over knotholes, knots or distorted grain. Members shall be cut in right fitting wood to wood joint. Connectors shall be located on both faces of the joint unless otherwise shown. A 5/8" plate is 5" wide x 4" long. A 6/8" plate is 6" wide x 4" long. Slots (holes) run parallel to the plate length specified. Double cuts on web members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum sizes based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1-1995

PRECAUTIONARY NOTES

All bracing and erection recommendations are to be followed in accordance with accepted industry publications. Trusses are to be handled with particular care during erection and handling. Temporary bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and erection bracing is always required. Normal precautionary action for trusses requires such temporary bracing during installation between trusses to avoid toppling and dominoing. The supervision of erection of trusses shall be under the control of persons experienced in the installation of trusses. Professional advice shall be sought if needed. Concentration of construction loads greater than the design loads shall not be applied to trusses at any time. No loads other than the weight of the erectors shall be applied to trusses until after all fastening and bracing is completed

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. L and R. End verticals designed for wind Wndload per ASCE 7-02, C&C, V= 125mph, H= 15.0 ft, I= 1.00, Exp.Cat. B, Kzt=1.0 Bld Type= encl L= 19.6 ft W= 10.6 ft Truss in END zone, TCDL= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req

Joint Locations

1) 4- 6- 0	3) 15- 1- 0	5) 9- 9- 8
2) 9- 9- 8	4) 15- 1- 0	6) 4- 6- 0

In-Plant Quality Assurance with Cq= 1
---MAX. REACTIONS PER BEARING LOCATION---
X-Loc Bset Vert Horiz Uplift Y Type
0- 1-12 1 501 -300 -599 B Pin
10- 5-4 1 501 -300 -599 B H Roll
PROVIDE UPLIFT CONNECTION PER SCHEDULE
PROVIDE HORIZONTAL CONNECTION PER SCHEDULE
---FORCE---CSI. Cr. WEB. --FORCE--CSI. Cr.
6- 1 478 0.20 C 5- 3 322 0.13 C
1- 5 -322 0.19 C 4- 3 478 0.20 C
5- 2 246 0.06 C

LL in./Ratio in./Ratio Jnt(s)
I.Span 0.17/716 0.14/878 3-2
Horiz. 0.01 0.02 NA
Long term deflection factor = 1.50

GLOBAL MAX DEFLECTIONS-----

1- 2	430	0.43	A	6- 5	300	0.24
2- 3	430	0.43	A	5- 4	-300	0.24

TC: ..FORCE..CSI. Cr. BC. ..FORCE..CSI.
1- 2 430 0.43 A 6- 5 300 0.24
2- 3 430 0.43 A 5- 4 -300 0.24

TC: 2x 4 SP #2
BC: 2x 4 SP #2
WB: 2x 4 SP #3

ALL COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.
User defined loadcases have been considered in this design.

FORCES (lb) - Max Compression/Max Tension
TOP CHORD 1-2=-377/430,2-3=-377/430,
BOT CHORD 6-5=-42/300,5-4=-300/55,
Webs 6-1=-371/478,1-5=-322/320,
5-2=-35/246,5-3=-220/322,4-3=-371/478,

EXCEPT AS SHOWN PLATES ARE MiTek MT20

Scale = 0.3444

WARNING:
READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO
ERECTING CONTRACTOR. BRACING WARNING:
Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which must be considered by the building designer. Bracing shown is for lateral support of truss members only to reduce buckling length. Provisions must be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-91 of TPI) For specific truss bracing requirements, contact building designer. (Truss Plate Institute, TPI is located at 583 D'Oro Drive, Madison, Wisconsin 53719).
Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenton, NC 27932

Eng Job: MGMC
Dwg:
Dsgnr:ECH Chk:
TC Live 16.0 psf
TC Dead 7.0 psf
BC Live 10.0 psf
BC Dead 10.0 psf
TOTAL 43.0 psf

WO: MGMC
TI: V34
2/1/2007
Lbr DF: 1.25
Plt DF: 1.25
O.C.: 2- 0- 0
TPI-02/FBC-04
Code: FLA
v4.7.3505-83772

Maronda Systems

4005 MARONDA WAY
Sanford, FL 32771
(407) 321-0064 Fax (407) 321-3913
TOMAS PONCE P.E. LICENSE #0050068
367 Medallion Pl. Chuluota, FL 32766

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\V34.prx

NAME: MONTGOMERY C

Customer: MONTGOMERY C

WO: MGMC

TI: V6

Qty: 1

DESIGN INFORMATION

TC: 2x 4 SP #2

BC: 2x 4 SP #2

WB: 2x 4 SP #3

GB: 2x 4 SP #3

MULTIPLE LOADCASES -- This design is the composite result of multiple loadcases. Right End vertical designed for wind. WindLoad per ASCE 7-02, C&C, V= 125mph, B= 15.0 ft, I= 1.00, Exp. Cat. B, Kzt= 1.0 Bld type= encl I= 18.0 ft W= 12.6 ft Truss in END zone, TCDF= 4.2 psf, BCDL= 6.0 psf Impact Resistant Covering and/or Glazing Req

MGMC

DESIGN INFORMATION

his design is for an individual building component and is based on information provided by the client. The designer disclaims any responsibility for damages as a result of faulty or incorrect information, specifications or designs furnished to the truss designer by the client or the correctness or accuracy of this information as it relates to a specific project and accepts no responsibility or liability for control with regard to erection, handling, shipment and installation of trusses. This truss has been designed as an individual building component in accordance with ANSI/TPI 1-1992 and 35-97 to be incorporated as part of the building design by a Building Designer. When reviewed for approval by a Building Designer, the design loadings shown must be checked to be sure that the data shown are in agreement with the local building codes, local climatic records for wind and snow loads, project specifications or special requirements. Unless shown, truss has not been designed for storage or occupancy loads. The design assumes compression chords (top or bottom) are continuously braced by sheathing unless otherwise specified. Where from chords in tension are not fully braced laterally by a sheathing applied rigid ceiling, they should be braced at a maximum spacing of 10'-0" o.c. Connector plates shall be unlubricated from 20 gauge hot dipped galvanized steel unless ASTM A 653, Grade 40, unless otherwise shown.

ABRICATION NOTES

rior to fabrication, the fabricator shall review this drawing to verify that this drawing is in conformance with the fabricator's plans and to realize a continuing responsibility for such verification. Any discrepancies are to be put in writing before cutting or fabrication. Plates all not be installed over knotholes, knots or distorted grain. Members shall be cut for right fitting wood to wood joint. Connector plates shall be located on both faces of truss with nails fully imbedded and shall be sym. about joint unless otherwise shown. A 3x4 plate is 5" wide x 1" long. A 6x8 plate is 6" wide x 8" long. Slots (holes) parallel to the plate length specified. Double cuts on members shall meet at the centroid of the webs unless otherwise shown. Connector plate sizes are minimum as based on the forces shown and may need to be increased for certain handling and/or erection stresses. This truss is not to be fabricated with fire retardant treated lumber unless otherwise shown. For additional information on Quality Control refer to ANSI/TPI 1995

RECAUTIONARY NOTES

1 bracing and erection recommendations are to be followed in accordance with accepted industry practices. Trusses are to be handled with particular care during banding and bundling, delivery and installation to avoid damage. Temporary and permanent bracing for holding trusses in a straight and plumb position and for resisting lateral forces shall be designed and installed by others. Careful handling is essential and action bracing is always required. Normal precautionary action for trusses requires such temporary bracing during erection. The supervision of erection of trusses shall be the responsibility of persons experienced in the erection and installation of trusses. Pre-erection shall be sought needed. Concentration of construction loads greater than design loads shall not be applied to trusses at any time. o loads other than the weight of the erection shall be applied to trusses until after all fastening and bracing is completed

Customer: MONTGOMERY C

WO: MGMC

TI: V6

Qty: 1

Analysis based on Simplified Analog Model.

All COMPRESSION Chords are assumed to be continuously braced unless noted otherwise.

User defined loadcases have been considered in this design.

Wind loads have been checked perpendicular to the face of the truss.

In-Plant Quality Assurance with Cq = 1.00

---MAX. REACTIONS PER BEARING LOCATION---

X-Loc Bset Vert Horiz Uplift Y Type

0- 1-12 1 88 -661 -298 B Pin

12- 5-12 1 57 -67 -76 B Pin

PROVIDE UPLIFT CONNECTION PER SCHEDULE

PROVIDE HORIZONTAL CONNECTION PER SCHEDULE

Joint Locations

1) 0- 0- 0 6) 11- 0- 0 11) 7- 0- 0

2) 3- 0- 0 7) 12- 7- 8 12) 5- 0- 0

3) 5- 0- 0 8) 12- 7- 8 13) 3- 0- 0

4) 7- 0- 0 9) 11- 0- 0 14) 0- 0- 0

5) 9- 0- 0 10) 9- 0- 0

This truss is designed in accordance with the 2004 Florida Building Code section 2306.1 and referenced standards, U.N.O.

-----GLOBAL MAX DEFLECTIONS-----

LL TL

in./Ratio in./Ratio Jnt(s).

I Span 0.00/999 0.00/999 2-1

Horiz. 0.01 0.02 NA

Long term deflection factor = 1.50

Diagram

EXCEPT AS SHOWN PLATES ARE Mitek MT20

Scale = 0.4058

Maronda Systems

4005 MARONDA WAY

Sanford, FL 32771

(407) 321-0064 Fax (407) 321-3913

Design: Matrix Analysis

Profile Path: C:\TEE-LOK\Work\Jobs\MGMC\MONTGOMERY C\V6.prx

WARNING:

READ ALL NOTES ON THIS SHEET. A COPY OF THIS DRAWING TO BE GIVEN TO ERECTING CONTRACTOR. BRACING WARNING:

Bracing shown on this drawing is not erection bracing, wind bracing, portal bracing or similar bracing which is a part of the building design and which shall be provided by the building designer. Bracing shown is for lateral support of truss members by end bracing. For erection bracing, the truss shall be made to anchor lateral bracing at ends and specified locations determined by the building designer. Additional bracing of the overall structure may be required. (See HIB-01 of TPI) For specific post bracing, contact building designer. (Truss Plate Institute, TPI is located at 381 D'Ondra Drive, Madison, Wisconsin 53719).

Component Engineering by: Truss Engineering Co., P.A., 818 Soundside Rd, Edenon, NC 27032

Eng Job: MGMC

WO: MGMC

TI: V6

Dsgnr: ECH Chk:

6/5/2007

Lbr DF: 1.25

Plt DF: 1.25

O.C.: 2- 0- 0

FBC-04/TPI-02/NDS-01

Code: FLA

TOTAL 43.0 psf

COLUMBIA COUNTY OFFICIAL SEAL

OCCUPANCY

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection

This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-4S-16-02856-106

Building permit No. 000026621

Use Classification SFD/UTILITY

Fire: 32.10

Permit Holder THEODORE C. BROCK

Waste: 83.75

Owner of Building MARONDA HOMES INC. OF FLORIDA

Total: 115.85

Location: 231 SW TIMBER RIDGE DR., LAKE CITY, FL

Date: 05/19/2008

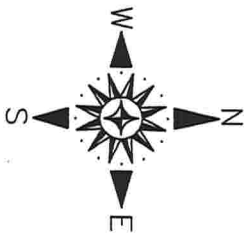
Building Inspector

POST IN A CONSPICUOUS PLACE
(Business Places Only)



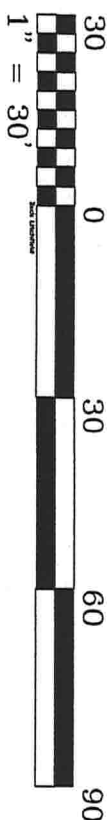
Wayne A. Love

LEGAL DESCRIPTION:
LOT SIX (6) OF "TIMBERLANDS" AS PER PLAT
THEREOF, AS RECORDED IN PLAT BOOK "D", PAGE 2
OF THE PUBLIC RECORDS OF COLUMBIA COUNTY,
FLORIDA.



PROPOSED BUILDING LAYOUT

IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE
16 EAST, COLUMBIA COUNTY, FLORIDA



CERTIFIED TO:

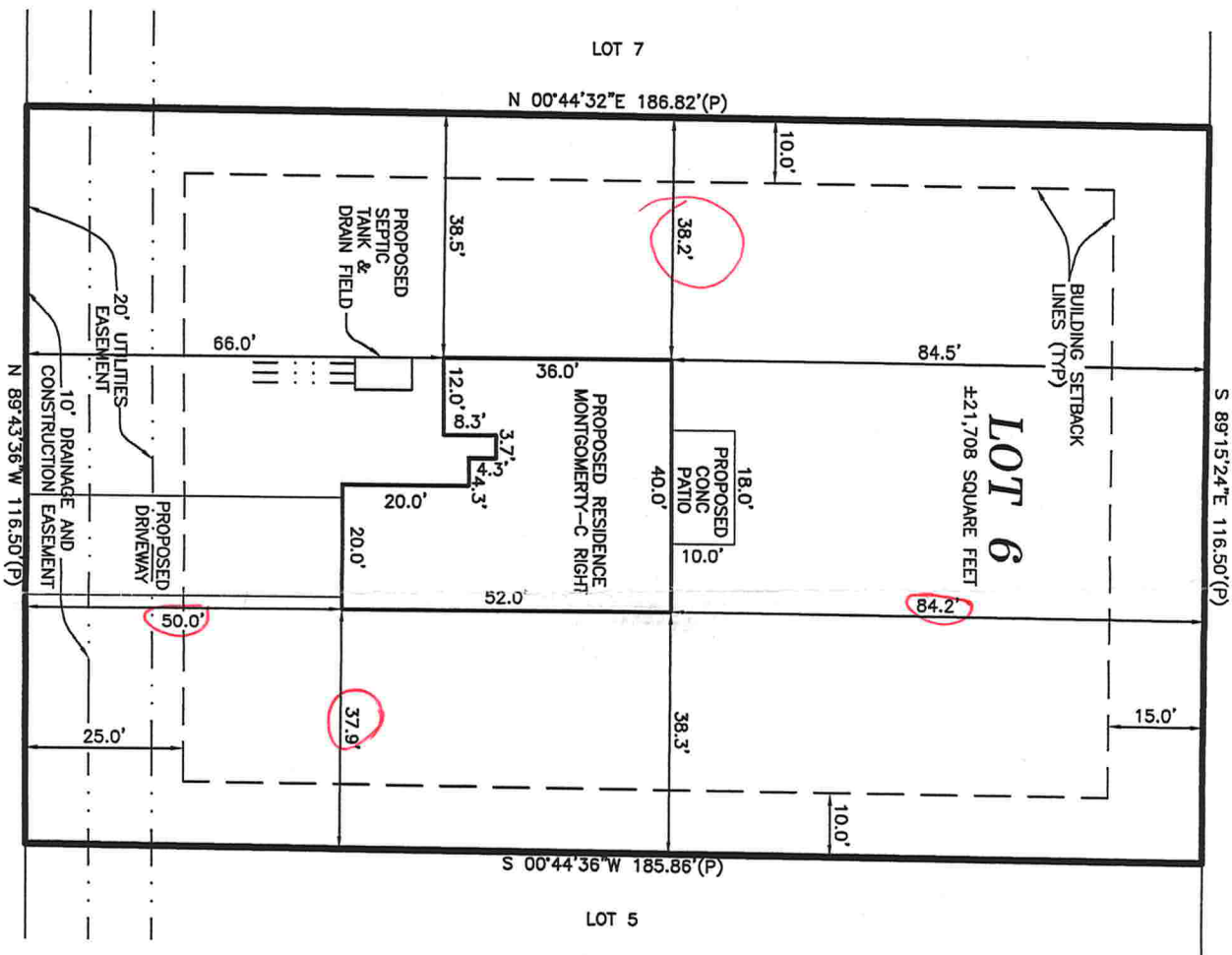
- 1) MARONDA HOMES

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR
"TIMBERLANDS" IS AS FOLLOWS: FRONT
25', REAR 15', SIDE 10'

SURVEYOR NOTES:

- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- 2) ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.



SW TIMBER RIDGE DRIVE

60' RIGHT-OF-WAY

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY AFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

LEGEND:

- = FOUND 1/2" REBAR NO IDENTIFICATION
- ⊙ = FOUND 1/2" REBAR & CAP
- L.B. 6894
- = SET 1/2" REBAR & CAP
- L.B. 6894
- ⊙ = FOUND 3/4" IRON PIPE
- = FOUND 4" X 4" CONC. MON.
- NO IDENTIFICATION
- = SET 4" X 4" CONC. MON.
- P.S.M. 5582
- ✕ = SET NAIL & DISK P.S.M. 5582
- ✕ = FOUND NAIL & DISK
- ⊠ = FOUND 6" X 6" S.R.D.
- R/W MON.
- ⊠ = CATV RISER
- ⊠ = TELEPHONE PEDESTAL
- ⊠ = WOOD POWER POLE

ABBREVIATIONS:

- A = ARC LENGTH
- A/C = AIR CONDITIONER
- ALUM = ALUMINUM
- ASPH = ASPHALT
- AVE = AVENUE
- BLK = BLOCK
- BLVD = BOULEVARD
- BM = BENCHMARK
- C = CALCULATED FROM MEASURED
- CATV = CABLE TELEVISION
- C/B = CONCRETE BLOCK
- CL = CENTER LINE
- CLF = CHAIN LINK FENCE
- CM = CONCRETE MONUMENT
- CMP = CORRUGATED METAL PIPE
- CONC = CONCRETE
- DOT = DEPARTMENT OF TRANSPORTATION
- ELEC = ELECTRIC
- ELEV = ELEVATION
- FND = FOUND
- FNC = FENCE
- (L) = PER LEGAL DESCRIPTION
- LB = LICENSED SURVEYOR BUSINESS
- (M) = FIELD MEASURED
- MH = MANHOLE
- P = PLAT
- PB = PLAT BOOK
- P.U.E. = PUBLIC UTILITIES EASEMENT
- R/W = RIGHT OF WAY

THIS IS NOT A BOUNDARY SURVEY CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF LAND SURVEYORS, PURSUANT TO SECTION 472.027, FLORIDA STATUTES, AND CHAPTER 62617-6, FLORIDA ADMINISTRATIVE CODE.

BY *James E. Brinkman* PSM FLA. CERT# 5582

DATE: 10/31/07



BRINKMAN SURVEYING & MAPPING INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL. 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'

"THE BENCHMARK IN QUALITY SERVICE"

DRAWN BY: ZL

DATE: 9/24/07

CHECKED BY: J.B.

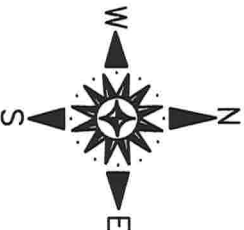
THIS IS NOT A BOUNDARY SURVEY

PREPARED FOR: MARONDA HOMES

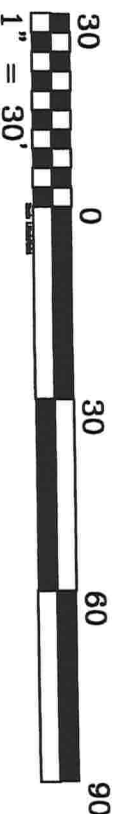
DRAWING NUMBER
302-07

LEGAL DESCRIPTION:
LOT SIX (6) OF "TIMBERLANDS" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK "D", PAGE 2 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

BOUNDARY SURVEY
IN SECTION 10, TOWNSHIP 4 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA



S 89°15'24"E 116.50'(P)
S 89°14'44"E 116.50'(M)
S 89°15'24"E 594.51'(P)
S 89°14'44"E 594.51'(M)



CERTIFIED TO:
1) MARONDA HOMES

BEARING NOTE:
BEARINGS SHOWN HEREON ARE REFERRED TO AN ASSUMED VALUE OF N. 89°43'36" W., FOR THE NORTH RIGHT-OF-WAY LINE OF SW TIMBER RIDGE DRIVE.

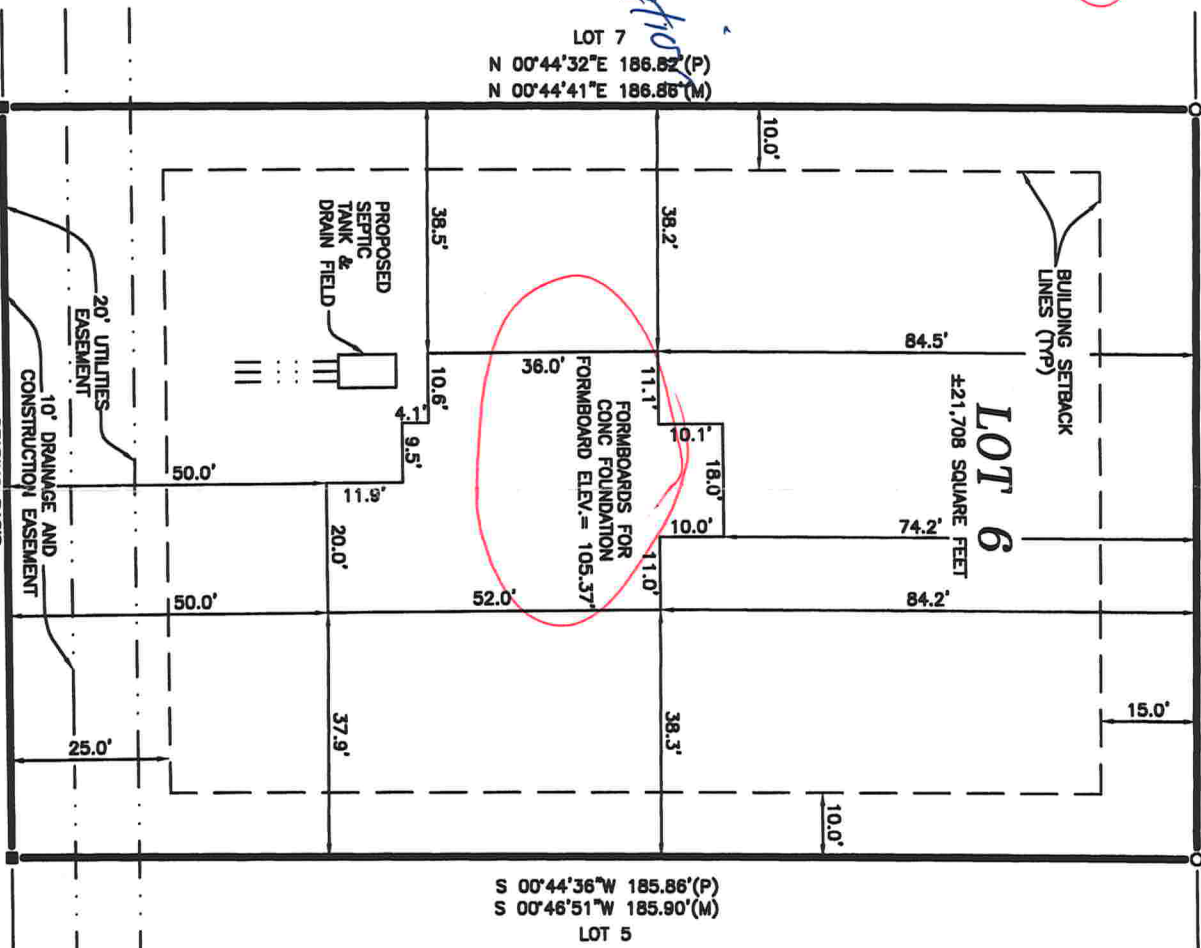
BUILDING SETBACK NOTE:
BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10'

FLOOD NOTE:
IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-8-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

SURVEYOR NOTES:

- 1) TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
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- 5) THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE TITLE BLOCK).
- 6) AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

Handwritten notes:
"26621" circled in red.
"Columbia County" written in blue.
"Timber Ridge" written in red.



BEARING BASIS
N 89°43'36" W 116.50'(P)
N 89°43'36" W 116.35'(M)

SW TIMBER RIDGE DRIVE

60' RIGHT-OF-WAY
±20' ASPHALT ROAD

EDGE OF PAVEMENT

TITLE NOTE:

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL. THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

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- = FOUND 1/2" REBAR & CAP
- = SET 1/2" REBAR & CAP
- = FOUND 3/4" IRON PIPE
- = FOUND 4" X 4" CONC. MON.
- = SET 4" X 4" CONC. MON.
- = SET NAIL & DISK P.S.M. 5582
- ✕ = FOUND NAIL & DISK
- ✕ = FOUND 6" X 6" S.R.D.
- ✕ = FOUND 6" X 6" S.R.D.
- ✕ = CATV RISER
- ✕ = TELEPHONE PEDESTAL
- ✕ = WOOD POWER POLE

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- P.U.E. = PUBLIC UTILITIES EASEMENT
- R/W = RIGHT OF WAY

CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

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BY: *James E. Brinkman*
JAMES E. BRINKMAN, FSM - FLA. CERT# 5582
DATE: 2/6/08



BRINKMAN SURVEYING & MAPPING, INC.

4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609
PHONE: (352) 374-7707 FAX: (352) 374-8757

SCALE: 1" = 30'
DATE: 2/5/08
FIELD WORK COMPLETED 2/4/08
DRAWN BY: ZL
CHECKED BY: J.B.

PREPARED FOR: MARONDA HOMES
FIELDBOOK 93, PAGE 45
DRAWING NUMBER 302-07



HOMETEAM
PEST DEFENSE®

26621

TREATMENT WORKORDER

☐ Termite Baiting System w/Tubes-under-the slab

☐ Treat Only

☒ Tubes-under-the slab and Treat

☐ Bora-Care

DATE CALLED IN:	2-4	DATE OF SCHEDULE:	2-5
TIME CALLED IN:		TIME SCHEDULE:	11-11:30

JOB NAME:	Mgeonda	SUBDIVISION:	timberland
JOB ADDRESS:	231 S.W. Timber Ridge Dr Lake City, FL		
BILLING NAME:		BILLING PHONE:	
BILLING ADDRESS:			
PRO		26621	
TERMINES	CALLED IN BY:	PHONE:	PERMIT NUMBER:

LOT & MODEL NUMBER: Lot 6

DATE & TIME COMPLETED: 2-5-08

SQUARE FOOT: 1999 LINEAR FOOT: BLOCKVOIDS:

SLAB TYPE: mono TYPE OF FILL: sand

APPROX. DEPTH OF FOOTING: Outside: Inside:

☐ Addition ☐ Spot Treat ☐ Pool Addition ☐ Driveway

☐ Final/Completion ☐ Other

PESTICIDE USED: Immax Pro TOTAL APPLIED: 100gals

PERCENT (%) USED: 190 STICKER POSTED: yes

PRICE PER SQ. FT. =	TOTAL FOR P.T.	
	ADDITIONAL	
	TAX:	
/ /	TOTAL AMOUNT	\$

X

X TECHNICIAN: CHAD

I hereby acknowledge the satisfactory completion of the above described work.