D /09/2008

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000027415

APPLICANT DREW TURNER PHONE 352 208-8821

ADDRESS 1707 SW 27TH PLACE OCALA FL 34471

OWNER JOHN & KAREN DEARDORFF PHONE 352 274-1548

ADDRESS 861 NW BLACKBERRY CIRCLE LAKE CITY FL 32055

CONTRACTOR COASTAL CRAFTSMENS PHONE 352 369-1444

LOCATION OF PROPERTY 90W, TR BROWN RD, TR NASH, TL BLACKBERRY CR, TL ON FIRST ROAD, 11TH LOT ON LEFT

TYPE DEVELOPMENT POOL ENCLOSURE ESTIMATED COST OF CONSTRUCTION 9345.00

HEATED FLOOR AREA _____ TOTAL AREA _____ HEIGHT ____ STORIES ____

FOUNDATION _____ WALLS _____ ROOF PITCH _____ FLOOR _____

LAND USE & ZONING A-3 MAX. HEIGHT _____

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO. _____

PARCEL ID 17-3S-16-02168-110 SUBDIVISION BLACKBERRY FARMS

LOT 10 BLOCK ____ PHASE ____ UNIT ____ TOTAL ACRES _____

		CGC047465	X Andrew T. Turner
Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor

EXISTING	X08-328	BK	WR	N
Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident

COMMENTS: IMPACT FEE EXEMPT/ACCESSORY USE, NOC ON FILE

Check # or Cash 7556

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power _____ date/app. by Foundation _____ date/app. by Monolithic _____ date/app. by

Under slab rough-in plumbing _____ date/app. by Slab _____ date/app. by Sheathing/Nailing _____ date/app. by

Framing _____ date/app. by Rough-in plumbing above slab and below wood floor _____ date/app. by

Electrical rough-in _____ date/app. by Heat & Air Duct _____ date/app. by Peri. beam (Lintel) _____ date/app. by

Permanent power _____ date/app. by C.O. Final _____ date/app. by Culvert _____ date/app. by

M/H tie downs, blocking, electricity and plumbing _____ date/app. by Pool _____ date/app. by

Reconnection _____ date/app. by Pump pole _____ date/app. by Utility Pole _____ date/app. by

M/H Pole _____ date/app. by Travel Trailer _____ date/app. by Re-roof _____ date/app. by

BUILDING PERMIT FEE $ 50.00 CERTIFICATION FEE $ 0.00 SURCHARGE FEE $ 0.00

MISC. FEES $ 0.00 ZONING CERT. FEE $ ____ FIRE FEE $ 0.00 WASTE FEE $ _____

FLOOD DEVELOPMENT FEE $ ____ FLOOD ZONE FEE $ ____ CULVERT FEE $ ____ **TOTAL FEE** 50.00

INSPECTORS OFFICE _____ CLERKS OFFICE CH

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

NOTICE OF COMMENCEMENT

STATE OF FLORIDA, COUNTY OF COLUMBIA
I HEREBY CERTIFY, that the above and foregoing is a true copy of the original filed in this office.
P. DeWITT CASON, CLERK OF COURTS
By Shaun Teagle Deputy Clerk
Date 10-2-2008

CLERK CIRCUIT COURT COLUMBIA COUNTY, FLORIDA

Inst:200812018118 Date:10/2/2008 Time:2:00 PM
DC,P.DeWitt Cason,Columbia County Page 1 of 1 B:1159 P:1505

This Instrument Prepared By:
Name: _____
Address: _____

Permit No. # _____
Tax Folio/Parcel ID: 17-3S-16-02168-110
State: FLORIDA
County: Columbia County

The undersigned herby gives notice that improvement(s) will be made to certain real property. In accordance with Chapter 713, Florida Statues, the following information is provided in the Notice of Commencement:

1. Description of property (legal description, lot, block and street address if available): Lot 10 Blackberry Farms S/D WD 1036-1438, WD 1141-2036
8611 NW Blackberry Circle Lake City 32055

2. General description of improvement: SCREENED POOL ENCLOSURE

3. Owner name/address: John & Karen Deardorff
1707 SW 27th Place Ocala FL

3b. Interest in property: Owner
3c. Name and address of fee simple title holder (if other than owner): N/A

4. Contractor – Qualifier Name and Address: Coastal Craftsmen Aluminum –dba - William Woodard
1406 SW 15 Avenue – Ocala - Florida - 34471

5. Surety – Name and Address: N/A

Amount of bond: $ _____

6. Lender – Name and Address: N/A

7. Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1)(a) 7, Florida Statues: N/A

8. In addition to him/herself, Owner designates the following person(s) to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statues [Provide Name/Mailing Address]: N/A

9. NoC expiration date (one full year from the date of recording unless different date is specified): N/A

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART 1, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

_____ 9/18/08
Signature of Owner (or Owner's Authorized Officer/ Director/Partner/ Manager) Date

STATE OF FLORIDA
County of _____

The foregoing instrument was acknowledged before me this 18th day of September, 2008, by _____ (print name of person) as _____ type of authority, e.g. officer, trustee, attorney in fact) for _____ (name of party on behalf of whom instrument was executed).

_____ Seal:
Notary Public

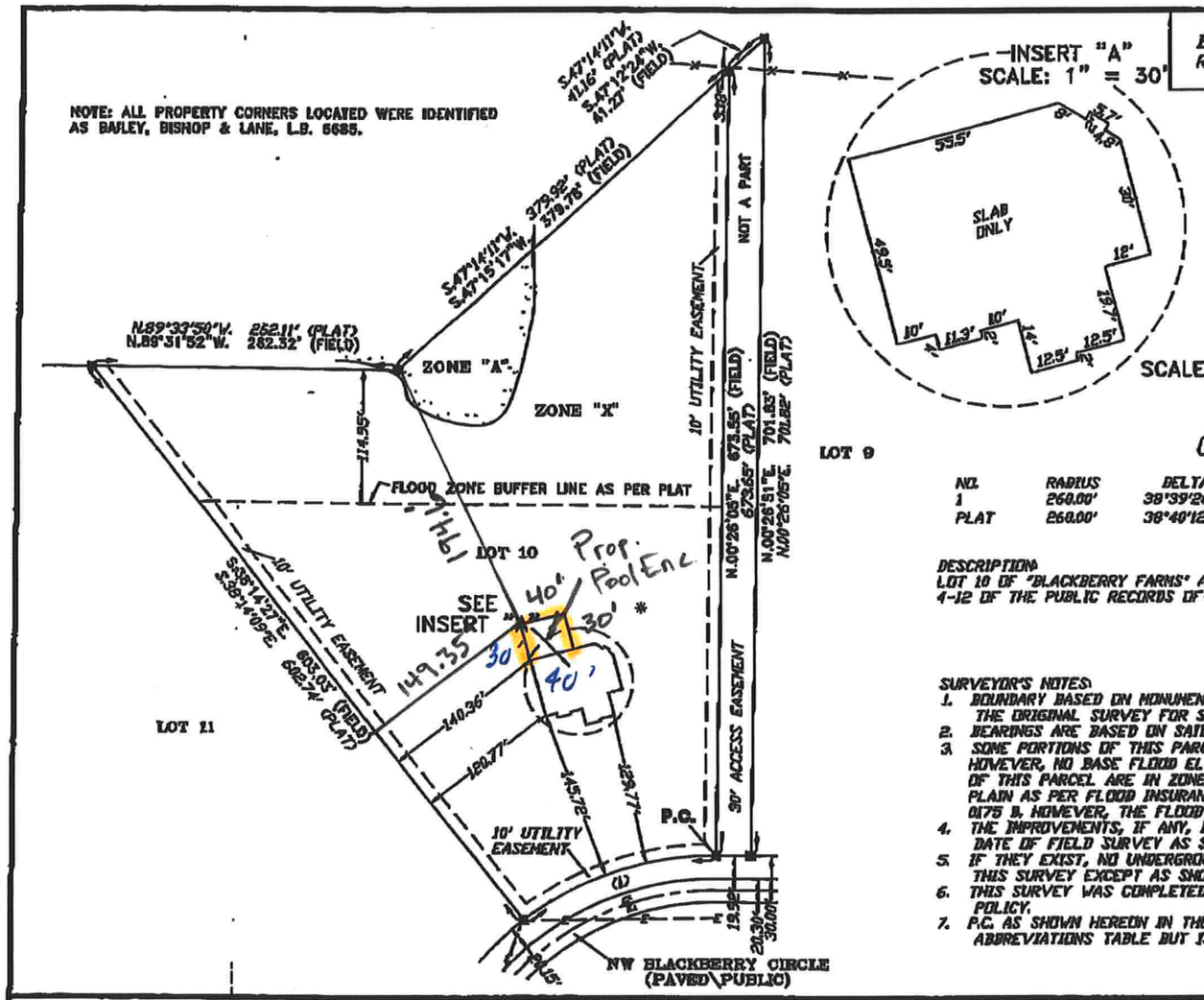
Personally Known _____ -OR- Produced Identification _____ Type of Identification Produced _____

Verification Pursuant to Section 92.525, Florida Statues: Under Penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

_____
Signature of Natural Person Signing Above

JENNIFER I. JONES
Comm# DD0762241
Expires 2/25/2012
Florida Notary Assn., Inc

BOUNDARY SURVEY IN SECTION 17, TOWNSHIP 3 SOUTH, RANGE 16 EAST, COLUMBIA COUNTY, FLORIDA.



SYMBOL LEGEND

- ■ 4"X4" CONCRETE MONUMENT FOUND
- □ 4"X4" CONCRETE MONUMENT SET
- ● IRON PIPE FOUND
- ○ IRON PIN AND CAP SET
- ⊕ POWER POLE
- ▲ WATER METER
- ℄ CENTERLINE
- ✲ WELL
- ◉ SATELLITE DISH
- Ⓣ TELEPHONE BOX
- ---E--- ELECTRIC LINES
- ---W--- WIRE FENCE
- ---●--- CHAIN LINK FENCE
- ---■--- WOODEN FENCE
- P.C. POINT OF CURVE

CURVE TABLE

NO.	RADIUS	DELTA	ARC	TANGENT	CHORD	CHORD BEARING
1	260.00'	38°39'26"	175.42'	91.20'	172.11'	N.71°06'36"E.
PLAT	260.00'	38°40'12"	175.48'		172.17'	N.71°05'59"E.

DESCRIPTION:
LOT 10 OF "BLACKBERRY FARMS" AS PER PLAT THEREOF RECORDED IN PRRD BOOK 1, PAGES 4-12 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

SURVEYOR'S NOTES:

1. BOUNDARY BASED ON MONUMENTATION FOUND IN ACCORDANCE WITH THE RETRACEMENT OF THE ORIGINAL SURVEY FOR SAID PLAT OF RECORD.
2. BEARINGS ARE BASED ON SAID PLAT OF RECORD.
3. SOME PORTIONS OF THIS PARCEL ARE IN ZONE "A" AND MAY BE SUBJECT TO FLOODING. HOWEVER, NO BASE FLOOD ELEVATION HAS BEEN DETERMINED FOR ZONE "A". SOME PORTIONS OF THIS PARCEL ARE IN ZONE "X" AND ARE DETERMINED TO BE OUTSIDE THE 500 YEAR FLOOD PLAIN AS PER FLOOD INSURANCE RATE MAP, DATED 6 JAN. 1988 COMMUNITY PANEL NO. 120070 0175 B. HOWEVER, THE FLOOD INSURANCE RATE MAPS ARE SUBJECT TO CHANGE.
4. THE IMPROVEMENTS, IF ANY, INDICATED ON THIS SURVEY DRAWING ARE AS LOCATED ON DATE OF FIELD SURVEY AS SHOWN HEREON.
5. IF THEY EXIST, NO UNDERGROUND ENCROACHMENTS AND/OR UTILITIES WERE LOCATED FOR THIS SURVEY EXCEPT AS SHOWN HEREON.
6. THIS SURVEY WAS COMPLETED WITHOUT THE BENEFIT OF A TITLE COMMITMENT OR A TITLE POLICY.
7. P.C. AS SHOWN HEREON IN THE SE CORNER OF THIS PARCEL IS NOT SHOWN ON THE PLAT'S ABBREVIATIONS TABLE BUT IS A COMMON SURVEY TERM FOR "POINT OF CURVE".

CERTIFIED TO:
JOHN DEARDORFF
MERCANTILE BANK
SIERRA TITLE, LLC
TICOR TITLE INSURANCE COMPANY

FIELD BOOK: SEE PAGE(S): FILE

SURVEYOR'S CERTIFICATION

I HEREBY CERTIFY THAT THIS SURVEY WAS MADE UNDER MY RESPONSIBLE CHARGE AND MEETS THE MINIMUM TECHNICAL STANDARDS AS SET FORTH BY THE FLORIDA BOARD OF PROFESSIONAL SURVEYORS AND MAPPERS IN CHAPTER 61G17-6, FLORIDA ADMINISTRATIVE CODE, PURSUANT TO SECTION 472.027, FLORIDA STATUTES.

06/03/08 FIELD SURVEY DATE
06/03/08 DRAWING DATE

SCOTT BRITT, PSM CERTIFICATION # 5267

NOTE: UNLESS IT BEARS THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER THIS DRAWING, SKETCH, PLAT OR MAP IS FOR INFORMATIONAL PURPOSES ONLY AND IS NOT VALID.

BRITT SURVEYING & ASSOCIATES, INC.
LAND SURVEYORS AND MAPPERS
630 WEST DUVAL STREET LAKE CITY, FLORIDA 32055
(386)752-7163 FAX (386)752-5573

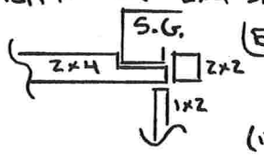
WORK ORDER # L-19339

William Woodard
Coastal Craftsmen Aluminum
1406 SW 15th Avenue
Ocala, FL 34474
352-369-1444

BRONZE MANSARD #08-4240 '13

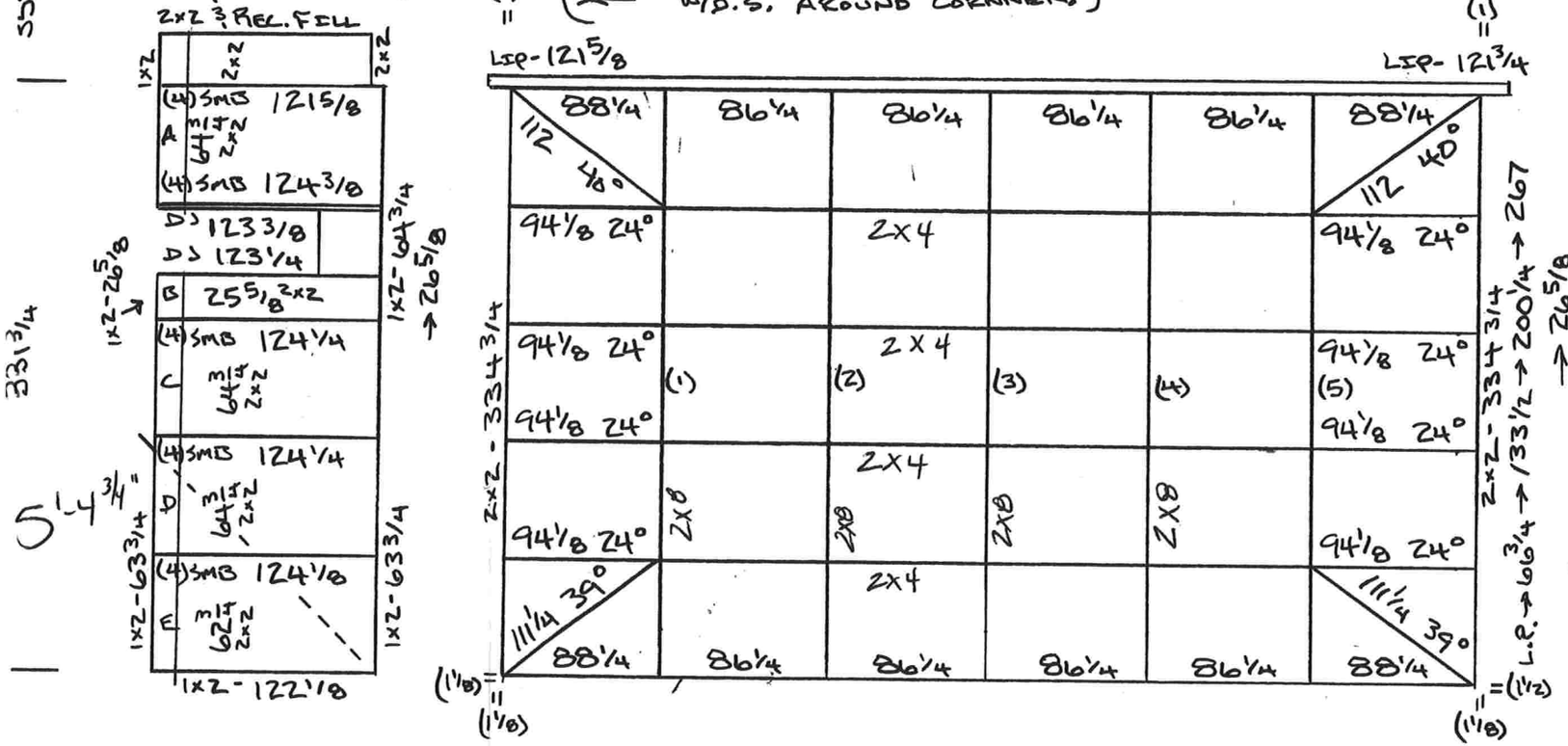
DEARDORFF 861 BLACKBERRY CIR. LAKE CITY

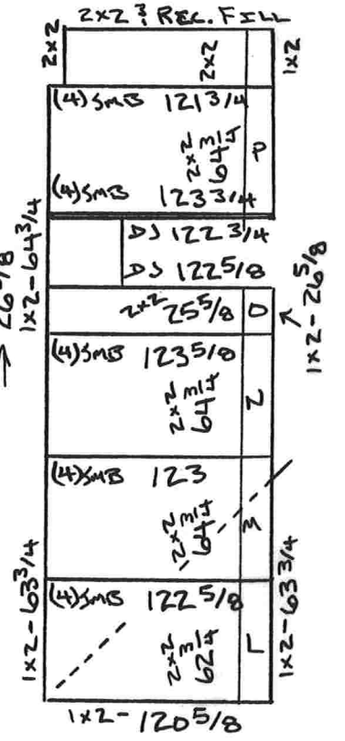
(1x4 NOUTCH AT TOP 2x4 S.M.B.)

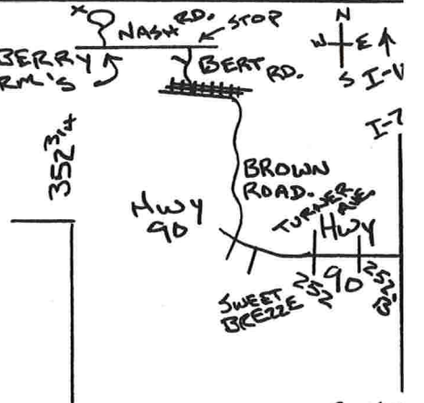


(EACH SIDE.)

(1) = (1) (← RUN S.G. TO END OF HOUSE W/D.S. AROUND CORNNER.)

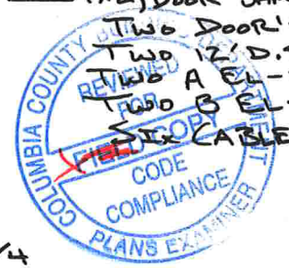


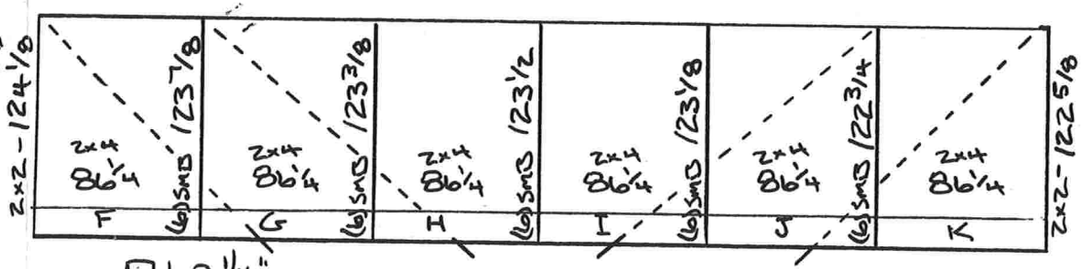




(MOMENT CON'T.)

BENNETT ENG.
2x4 P.B. PERLIN'
2x2, 2x4 CHAIR RAI
2x2, S.M.B. C.R. 8
1x2, DOOR JAM C.R. T
TWO DOOR'S
TWO 12' D.S.
TWO A EL-BOW'S
TWO B EL-BOW'
SIX CABLE'S @ 14





10'-4 1/8"

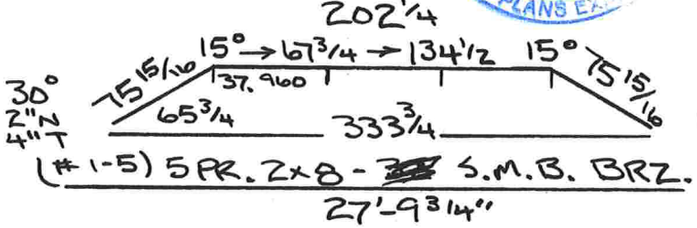
7'-2 1/4"

WALL 1085
ROOF 1310
2395 (SHOP)

L.F. S.G. 62'

B.O.J. TEE EN'S
2@ 2'x10'
B.O.J. LANI'S
1@ 3'x9'
1@ 12'x9'
1 WINN @ 14'x5'
245
2640 (FIELD)

IKE #61



(#1-5) 5 PR. 2x8 - ~~30~~ S.M.B. BRZ.

27'-9 3/4"

42.29

529 1/2

531 1/2

– END PAGE NEEDED By Property Owner Signature.

Columbia County Building Permit Application

Revised 9-23-04

For Office Use Only Application # 0810-05 Date Received 10/2 By JW Permit # 27415

Application Approved by - Zoning Official BLK Date 07.10.08 Plans Examiner (W) Date 10/3/08

Flood Zone X Development Permit N/A Zoning PRRD Land Use Plan Map Category A-3

Comments Impact Fee Exempt – Accessory Use

☑ NOC ☐ Proof of ownership ☑ EH ☐ Letter of Authorization

Applicants Name DREW TURNER Phone 352-208-8821

Address 1707 SW 27th Place OCALA FL 34471

Owners Name JOHN & KAREN DEARDORFF Phone —

911 Address 8101 NW Blackberry circle LAKE City 32055

Contractors Name COASTAL CRAFTSMEN - William Woodard Phone 352-369-1444

Address 14016 SW 15th AVE OCALA FL 34474

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Bennett Eng. P.O. BOX 214368 S. DAYTONA FL 32121

Mortgage Lenders Name & Address N/A

Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progressive Energy

Property ID Number 17-35-16-02168-110 Estimated Cost of Construction 9,345.00

Subdivision Name Blackberry FARMS Lot 10 Block ____ Unit ____ Phase ____

Driving Directions I 75 NORTH to 252 B west - to brown Rd right. TO Bert Road Right, to NASH ROAD left to Blackberry Farms right - House at end on coldesac.

Type of Construction Screen Pool Enclosure Number of Existing Dwellings on Property 1

Total Acreage 4.470 Lot Size ____ Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive

Actual Distance of Structure from Property Lines - Front 40/85± Side 30 (140±) Side 80 Rear 40

Total Building Height 10'-4⅛" Number of Stories 1 Heated Floor Area ____ Roof Pitch ____

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

OWNERS AFFIDAVIT: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning.

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

Owner Builder or Agent (Including Contractor)

STATE OF FLORIDA
COUNTY OF COLUMBIA

Sworn to (or affirmed) and subscribed before me
this 1st day of October 20 08.
Personally known X or Produced Identification ____

JENNIFER I. JONES
Comm# DD0762241
Expires 2/25/2012
Florida Notary Assn., Inc

Contractor Signature
Contractors License Number CGC047465
Competency Card Number ____
NOTARY STAMP/SEAL

Jennifer Jones
Notary Signature

LOT 10 BLACKBERRY FARMS S/D.
WD 1036-1438, WD 1141-2036,

DEARDORFF JOHN A &
KAREN L DEARDORFF
1707 SW 27TH PLACE
OCALA, FL 34471

17-3S-16-02168-110

Columbia County 2008 R
CARD 001 of 001
PRINTED 8/04/2008 13:57 BY JEFF
APPR 7/10/2006 DF

BUSE		AE?	HTD AREA	.000	INDEX	17316.00	DIST	3	PUSE 000000 VACANT
MOD	BATH		EFF AREA	71.670	E-RATE	.000	INDX	STR 17- 3S- 16	
EXW	FIXT		RCN				AYB	MKT AREA 01	0 BLDG
%	BDRM		%GOOD		BLDG VAL		EYB	(PUD1	0 XFOB
RSTR	RMS							AC 4.470	64,368 LAND
RCVR	UNTS		#FIELD CK:					NTCD	0 AG
%	C-W%		#LOC: BLACKBERRY FARMS					APPR CD	0 MKAG
INTW	HGHT							CNDO	64,368 JUST
%	PMTR							SUBD	0 CLAS
FLOR	STYS							BLK	
%	ECON							LOT	0 SOHD
HTTP	FUNC							MAP#	0 ASSD
A/C	SPCD								0 EXPT
QUAL	DEPR							TXDT 003	0 COTXBL
FNDN	UD-1								
SIZE	UD-2							BLDG TRAVERSE	
CEIL	UD-3								
ARCH	UD-4								
FRME	UD-5								
KTCH	UD-6								
WNDO	UD-7								
CLAS	UD-8								
OCC	UD-9								
COND	%								

SUB A-AREA % E-AREA SUB VALUE

PERMITS

NUMBER	DESC	AMT	ISSUED
26746	SFR	1,566	2/12/2008

SALE

BOOK	PAGE	DATE		PRICE
1141	2036	1/28/2008	Q V	95000
GRANTOR JAMES A & JUANA JO LYTTE				
GRANTEE JOHN A & KAREN L DEARDORFF				
1036	1438	1/25/2005	Q V	119900
GRANTOR DANIEL CRAPPS				
GRANTEE JAMES A & JUANA JO LYTTE				

TOTAL

EXTRA FEATURES

AE	BN	CODE	DESC	LEN	WID	HGHT	QTY	QL	YR	ADJ	UNITS	UT	PRICE	ADJ UT PR	SPCD %	%GOOD	XFOB VALUE

FIELD CK:

AE	LAND CODE	DESC	ZONE	TOPO	ROAD	UTIL	UD1	UD2	UD3	UD4	FRONT	BACK	DEPTH	DT	ADJUSTMENTS	UNITS	UT	PRICE	ADJ UT PR	LAND VALUE
Y	000000	VAC RES	A-1	0002	0007	0003									1.00 1.00 .80 1.00	4.470	AC	18000.000	14400.00	64,368

2008

Columbia County Property Appraiser

DB Last Updated: 8/5/2008

2008 Proposed Values

Tax Record | Property Card | Interactive GIS Map | Print

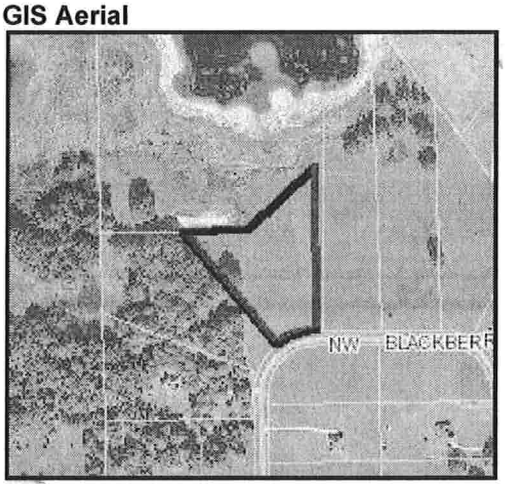
Parcel: 17-3S-16-02168-110

Search Result: 1 of 1

Owner & Property Info

Owner's Name	DEARDORFF JOHN A &		
Site Address	BLACKBERRY FARMS		
Mailing Address	KAREN L DEARDORFF 1707 SW 27TH PLACE OCALA, FL 34471		
Use Desc. (code)	VACANT (000000)		
Neighborhood	17316.00	**Tax District**	3
UD Codes	MKTA01	**Market Area**	01
Total Land Area	4.470 ACRES		
Description	LOT 10 BLACKBERRY FARMS S/D. WD 1036-1438, WD 1141-2036,		

GIS Aerial



Property & Assessment Values

Mkt Land Value	cnt: (1)	$64,368.00
Ag Land Value	cnt: (0)	$0.00
Building Value	cnt: (0)	$0.00
XFOB Value	cnt: (0)	$0.00
Total Appraised Value		$64,368.00

Just Value	$64,368.00
Class Value	$0.00
Assessed Value	$64,368.00
Exempt Value	$0.00
Total Taxable Value	$64,368.00

Sales History

Sale Date	Book/Page	Inst. Type	Sale Vlmp	Sale Qual	Sale RCode	Sale Price
1/28/2008	1141/2036	WD	V	Q		$95,000.00
1/25/2005	1036/1438	WD	V	Q		$119,900.00

Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
NONE						

Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
NONE						

Land Breakdown

Lnd Code	Desc	Units	Adjustments	Eff Rate	Lnd Value
000000	VAC RES (MKT)	4.470 AC	1.00/1.00/.80/1.00	$14,400.00	$64,368.00

Design Check List for Pool Enclosures (Page 4 of 4)

Example 4: Mansard Roof

Front wall @ eave: 42.29 (W) ft. x 10.33 (H) ft. = 436.86 (a) ft.² @ 100% = ... 436.86 ft.²

Front mansard rise: 3.00 (R) ft. x 1/2(27.89 (W1) ft. + 42.29 (W2) ft.) = 105.27 (b) ft.² @ 100% = ... 105.27 ft.²

Largest side wall: 27.64 (W) ft. x 10.33 (H) = 285.52 (c) ft.² @ 50% = ... 142.76 ft.²

Largest side mansard rise: 3 (R) ft. x 1/2(17.04 (W1) ft. + 27.64 (W2) ft.) = 67.02 (d) ft.² @ 50% = ... 33.51 ft.²

TOTAL = ... 718.40 ft.²

Total area / (233 ft.² / cable for 3/32") = 3 cable pairs
or
Total area / (445 ft.² / cable for 1/8") = 2 cable pairs

Side wall cable calculation: 285.52 (c) ft.² + 67.02 (d) ft.² = 352.54 ft.² @ 100% = ... 352.54 ft.²

Side wall area / (233 ft.² / cable for 3/32") = 2 cable(s)
or
Side wall area / (445 ft.² / cable for 1/8") = 1 cable(s)

Example 5: Dome Roof

Front dome wall @ eave: _____ (W) ft. x _____ (H) ft. = 0.00 (a) ft. @ 100% = ... 0.00 ft.²

Front dome rise: _____ (R) ft. x 1/2(_____ (W) ft.) = 0.00 (b) ft.² @ 100% = ... 0.00 ft.²

Largest side wall: _____ (W) ft. x _____ (H) = 0.00 (c) ft.² @ 50% = ... 0.00 ft.²

Largest side dome rise: _____ (R) ft. x _____ (W) ft. = 0.00 (d) ft.² @ 50% = ... 0.00 ft.²

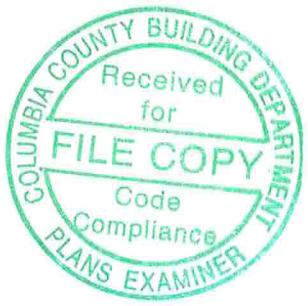
TOTAL = ... 0.00 ft.²

Total area / (233 ft.² / cable for 3/32") = 0 cable pairs
or
Total area / (445 ft.² / cable for 1/8") = 0 cable pairs

Side wall cable calculation: 0.00 (c) ft.² + 0.00 (d) ft.² = 0.00 ft.² @ 100% = ... 0.00 ft.²

Side wall area / (233 ft.² / cable for 3/32") = 0 cable(s)
or
Side wall area / (445 ft.² / cable for 1/8") = 0 cable(s)

Notes:



Design Check List for Pool Enclosures (Page 1 of 4)

I. Design Statement:

These plans have been designed in accordance with the Aluminum Structures Design Manual by Lawrence E. Bennett and are in compliance with the 2004 Florida Building Code Edition with 2006 Supplements, Chapter 20, ASM35 and The 2005 Aluminum Design Manual Part I-A & II-A; Exposure 'B'[☑] or 'C'[☐] or 'D'[☐]; Importance Factor 0.87 for 100 MPH and 0.77 for 110 MPH and higher; Negative I.P.C. 0.00; 110 MPH Wind Zone for 3 second wind gust; Basic Wind Pressure 14 ; Design pressures are 4 PSF for roofs & 13 PSF for walls. (see page 1ii for wind loads and design pressures) A 300 PLF point load is also considered for screen roof members.

Notes: Wind velocity zones and exposure category is determined by local code. Design pressures and conversion multipliers are on page 1-ii.

II. Host Structure Adequacy Statement:

I have inspected and verify that the host structure is in good repair and attachments made to the structure will be solid.

Stephanie Broderick Phone: 352-369-1444
Contractor / Authorized Rep* Name (please print)

Date: 10/01/08
Contractor / Authorized Rep* Signature

RET-BD DEARDORFF 861 NW BLACKBERRY CIRCLE LAKE CITY
Job Name & Address

Note: If the total of beam span & upright height exceeds 50' or upright height exceeds 16', site specific engineering is required.

III. Building Permit Application Package contains the following:	**Yes**	**No**
A. Project name & address on plans	☑	☐
B. Site plan or survey with enclosure location	☑	☐
C. Contractor's / Designer's name, address, phone number, & signature on plans	☑	☐
D. Site exposure form completed	☑	☐
E. Enclosure layout drawing @ 1/8" or 1/10" scale with the following:	☑	☐
1. Plan view with host structure, enclosure length, projection from host structure, and all dimensions	☑	☐
2. Front and side elevation views with all dimensions & heights	☑	☐
Note: All mansard wall drawings shall include mansard panel at the top of the wall.		
3. Beam location (show in plan & elevation view) & size (Table 1.1 & 1.6)	☑	☐

Roof frame member allowable span conversions from 120 MPH wind zone, "B" Exposure to _____ MPH wind zone and / or☐"C" or☐"D" Exposure for load width of _____:

Note: Conversion factors do not apply to members subject to point load (P).

Look up span in appropriate 120 MPH span table and apply the following formula:

Span @ 120 MPH — Required Converted Span / Height

0.00 (b or d) x 1.00 (b or d) x 1.00 (b or d) = _____

Wind Zone Multiplier (see page 1ii) — Exposure Multiplier (see page 1ii)

4. Upright location (show in plan & elevation view) & size (Table 1.3 & 1.6)	☑	☐
5. Chair rail & girt size, length, & spacing (Table 1.4)	☑	☐
6. Eave rail size, length, spacing and stitching of (Table 1.2)	☑	☐

*** Must have attended Engineer's Continuing Education Class within the past two years.**

Design Check List for Pool Enclosures (Page 2 of 4)

Wall frame member allowable span conversions from 120 MPH wind zone, "B" Exposure to 0.00 MPH wind zone and / or ☐ "C" or ☐ "D" Exposure for load width of 1.00 :
Look up span in appropriate 120 MPH span table and apply the following formula:

Span / Height @ 120 MPH or ___ MPH → 0.00 (b or d) x 1.00 (b or d) x 1.00 (b or d) = _____ ← Required Converted Span / Height

Wind Zone Multiplier ** ↑ ; Exposure Multiplier (see page 1ii) ↑

	Yes	No
7. Enclosure roof diagonal bracing in plan view	☑	☐
8. Knee braces length, location, & size (Table 1.7)	☐	☑
9. Wall cables or K-bracing sizes shown in wall views	☑	☐
IV. Highlight details from the Aluminum Structures Design Manual:	**Yes**	**No**
A. Beam & purlin tables with size, thickness, spacing, & spans / lengths (Tables 1.1 & 1.2 or 1.9.1 & 1.9.2)	☑	☐
B. Upright & girt tables with size, thickness, spacing, & spans / lengths (Tables 1.3 & 1.4)	☑	☐
C. Table 1.6 with beam & upright combination	☑	☐
D. Connection details to be use such as:		
1. Beam to upright	☑	☐
2. Beam to wall	☐	☑
3. Beam to beam	☐	☑
4. Chair rail, purlins, & knee braces	☑	☐
5. Extruded gutter connections	☑	☐
6. Angle to deck and / or sole plate	☑	☐
7. Anchors go through pavers into concrete	☑	☐
8. Minimum footing and / or knee wall details	☑	☐
9. Cable or K- brace details Section 1	☑	☐

Wall area calculations for cables:
W = wall width, H = wall height, R = rise
W1 = width @ top of mansard, W2 = width @ top of wall

E. Select footing from examples in manual.
Example 1: Flat Roof

Front wall @ eave: _____ (W) ft. x _____ (H) ft. = 0.00 (a) ft.² @ 100% = ... 0.00 ft.²

Largest side wall: _____ (W) ft. x _____ (H) ft. = 0.00 (b) ft.² @ 50% = ... 0.00 ft.²

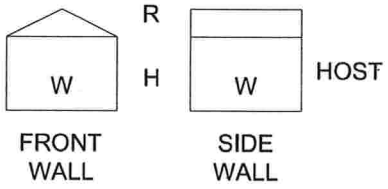
TOTAL = ... 0.00 ft.²

Total area / (233 ft.² / cable for 3/32") = 0 cable pairs
or
Total area / (445 ft.² / cable for 1/8") = 0 cable pairs

Side wall cable calculation: 0.00 (b) ft.² @ 100% = ... 0.00 ft.²

Side wall area / (233 ft.² / cable for 3/32") = 0 cable(s)
or
Side wall area / (445 ft.² / cable for 1/8") = 0 cable(s)

Design Check List for Pool Enclosures (Page 3 of 4)



Example 2: Gable Roof

Front wall @ eave: ____ft. (W) x ____ft. (H) = 0.00 ft.² (a) @ 100% = ... 0.00 ft.²

Front gable rise: ____ft. (R) x 1/2(____ft. (W)) = 0.00 ft.² (b) @ 100% = ... 0.00 ft.²

Largest side wall: ____ft. (W) x ____ (H) = 0.00 ft.² (c) @ 50% = ... 0.00 ft.²

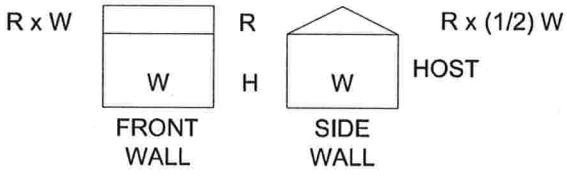
Largest side gable rise: ____ft. (R) x ____ft. (W) = 0.00 ft.² (d) @ 50% = ... 0.00 ft.²

TOTAL = ... 0.00 ft.²

Total area / (233 ft.² / cable for 3/32") = 0 cable pairs
or
Total area / (445 ft.² / cable for 1/8") = 0 cable pairs

Side wall cable calculation: 0.00 ft.² (c) + 0.00 ft.² (d) = 0.00 ft.² @ 100% = ... 0.00 ft.²

Side wall area / (233 ft.² / cable for 3/32") = 0 cable(s)
or
Side wall area / (445 ft.² / cable for 1/8") = 0 cable(s)



Example 3: Transverse Gable Roof

Front wall @ eave: ____ft. (W) x ____ft. (H) = 0.00 ft.² (a) @ 100% = ... 0.00 ft.²

Front gable rise: ____ft. (R) x ____ft. (W) = 0.00 ft.² (b) @ 100% = ... 0.00 ft.²

Largest side wall: ____ft. (W) x ____ (H) = 0.00 ft.² (c) @ 50% = ... 0.00 ft.²

Largest side gable rise: ____ft. (R) x 1/2 (____ft. (W)) = 0.00 ft.² (d) @ 50% = ... 0.00 ft.²

TOTAL = ... 0.00 ft.²

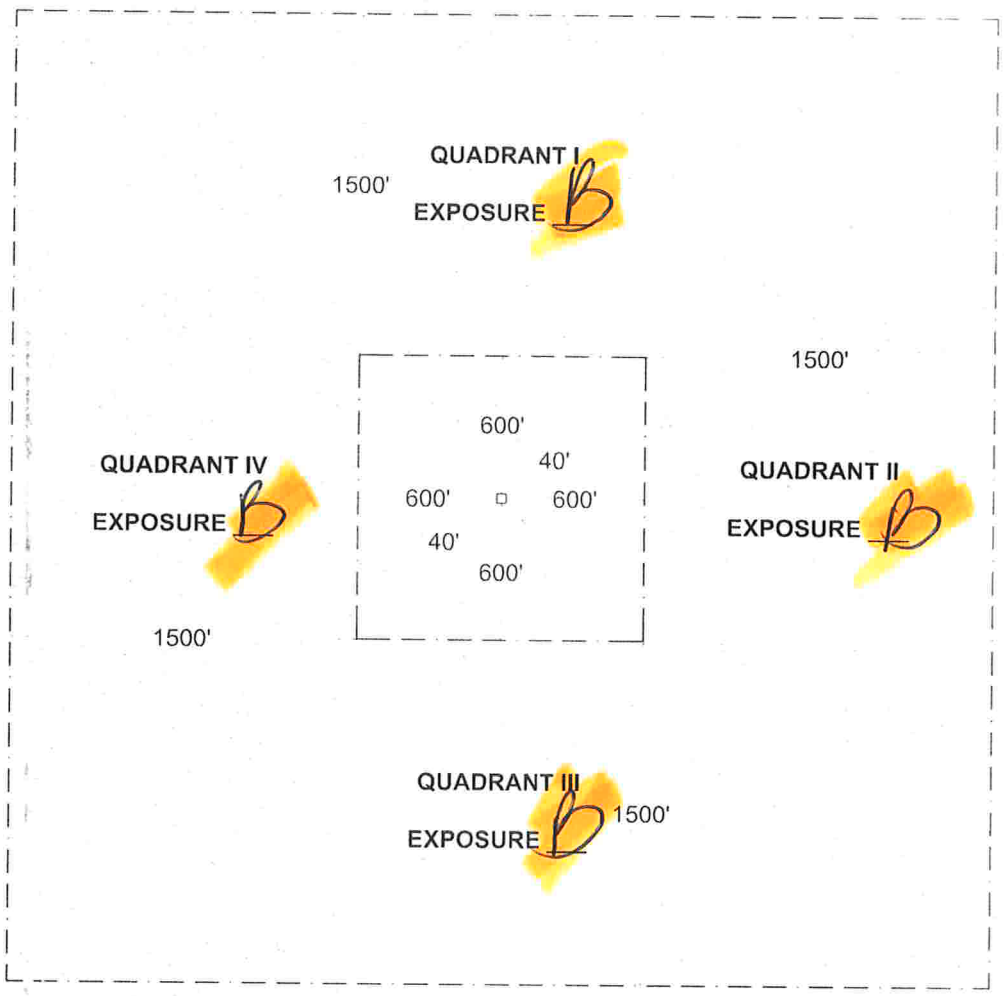
Total area / (233 ft.² / cable for 3/32") = 0 cable pairs
or
Total area / (445 ft.² / cable for 1/8") = 0 cable pairs

Side wall cable calculation: 0.00 ft.² (c) + 0.00 ft.² (d) = 0.00 ft.² @ 100% = ... 0.00 ft.²

Side wall area / (233 ft.² / cable for 3/32") = 0 cable(s)
or
Side wall area / (445 ft.² / cable for 1/8") = 0 cable(s)

SITE EXPOSURE EVALUATION FORM

Sht 1



NOTE: ZONES ARE MEASURED FROM STRUCTURE OUTWARD

SITE

SCALE: 1" = 800'

USING THE FOLLOWING CRITERIA, EVALUATE EACH QUADRANT AND MARK IT AS 'B', 'C', OR 'D' EXPOSURE. 'C' OR 'D' EXPOSURE IN ANY QUADRANT MAKE THE SITE THAT EXPOSURE.

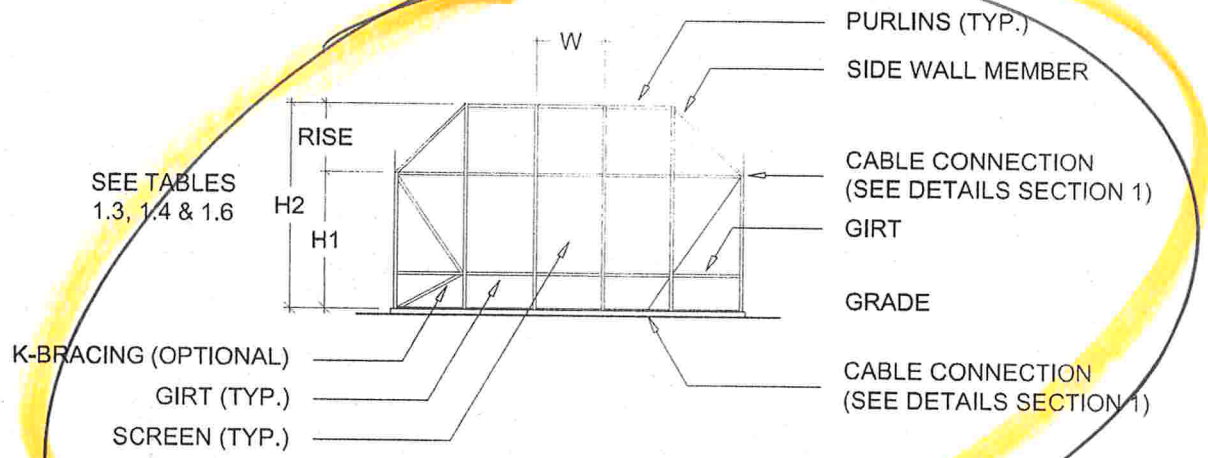
EXPOSURE C:
1. OPEN TERRAIN FOR MORE THAN 1,500 FEET IN ANY QUADRANT.
2. ANY 'C' EXPOSURE FOR GREATER THAN 600 FEET IN ANY QUADRANT.
3. NO SHORT TERM CHANGES IN 'B', 2 YEARS BEFORE SITE EVALUATION AND BUILD OUT WITHIN 3 YEARS, SITE WILL BE 'B'.
4. FLAT, OPEN COUNTRY, GRASSLANDS, PONDS AND OCEAN OR SHORELINES IN ANY QUADRANT FOR GREATER THAN 1,500 FEET.

EXPOSURE D: FLAT, UNOBSTRUCTED AREAS THAT ARE 1,500 FT INLAND FROM THE SHORE LINE AND ARE EXPOSED TO WIND FLOWING OVER WATER FOR A DISTANCE OF AT LEAST 1 MILE.

SITE IS EXPOSURE: B EVALUATED BY: William Woodward DATE: 10/21/08

SIGNATURE: _____ LICENSE #: CGC047465

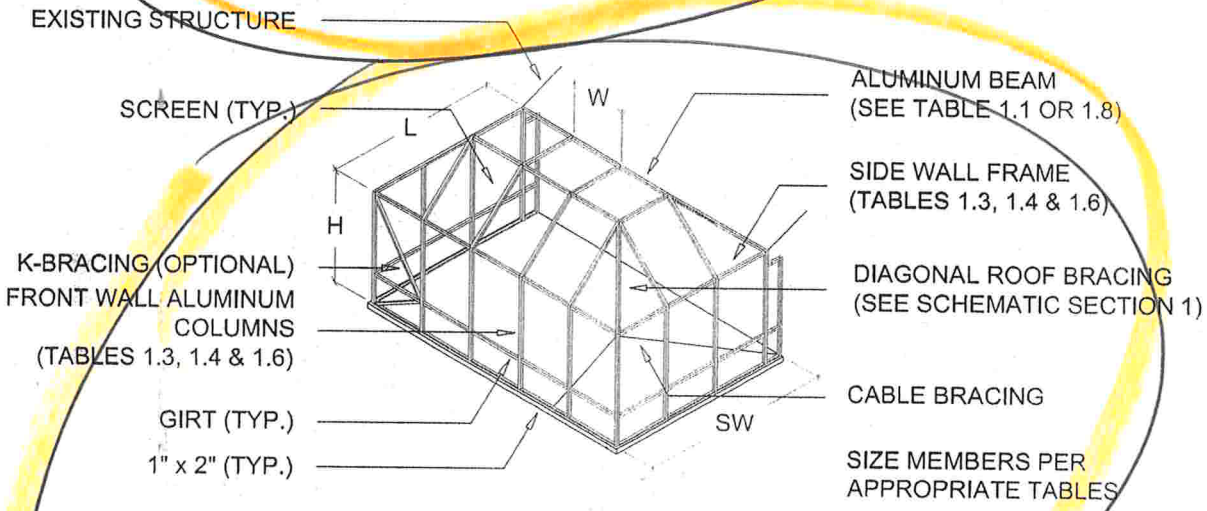
COLUMBIA COUNTY BUILDING DEPARTMENT
Received
for
FILE COPY
Code Compliance
PLANS EXAMINER



NOTE: USE H2 FOR CABLE AREA CALCULATION

TYPICAL MANSARD ROOF - FRONT WALL ELEVATION

SCALE: N.T.S.



TYPICAL MANSARD ROOF - ISOMETRIC

SCALE: N.T.S.

CONNECTION DETAILS AND NOTES ARE FOUND IN THE SUBSEQUENT PAGES.

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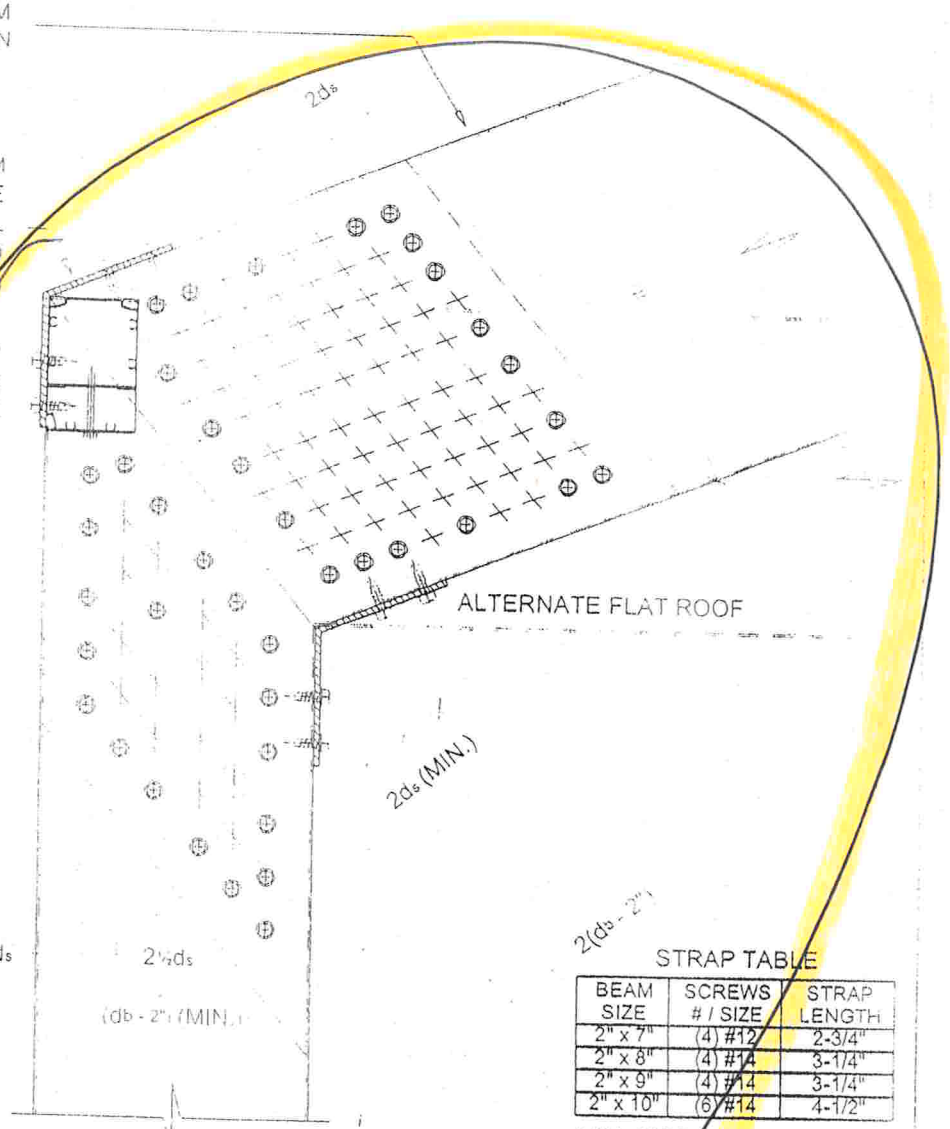
2" x 9" x 0.072" x 0.224" BEAM SHOWN

1-3/4" STRAP MADE FROM REQUIRED GUSSET PLATE MATERIAL (SEE TABLE FOR LENGTH AND # OF SCREWS REQUIRED)

WHEN FASTENING 2" x 2" THROUGH GUSSET PLATE USE #10 x 2" (3) EACH MIN.



ALL GUSSET PLATES SHALL BE A MINIMUM OF 5052 H-32 ALLOY OR HAVE A MINIMUM YIELD STRENGTH OF 23 ksi

db = DEPTH OF BEAM
ds = DIAMETER OF SCREW

2" x 6" x 0.050" x 0.120" UPRIGHT SHOWN

STRAP TABLE

BEAM SIZE	SCREWS # / SIZE	STRAP LENGTH
2" x 7"	(4) #12	2-3/4"
2" x 8"	(4) #14	3-1/4"
2" x 9"	(4) #14	3-1/4"
2" x 10"	(6) #14	4-1/2"

* ALL SCREWS 3/4" LONG

NOTES:
1. FILL OUTER SCREW POSITIONS FIRST UNTIL REQUIRED NUMBER OF SCREWS IS ACHIEVED.
2. SEE TABLE 1.6 FOR GUSSETT SIZE, SCREW SIZES, AND NUMBER.
3. GUSSET PLATES ARE REQUIRED ON ALL BEAMS 2" x 7" AND LARGER.
4. SCREW PATTERN LAYOUT W/ SPACING BETWEEN SCREWS GREATER THAN MINIMUM IS ALLOWED SO THAT EQUAL SPACING IS ACHIEVED.

BEAM SPLICE CUT, GUSSET PLATE CONNECTION & GUSSET SCREW PATTERN
BEAM TO POST MOMENT CONNECTION DETAIL

SCALE: 3" = 1'-0"

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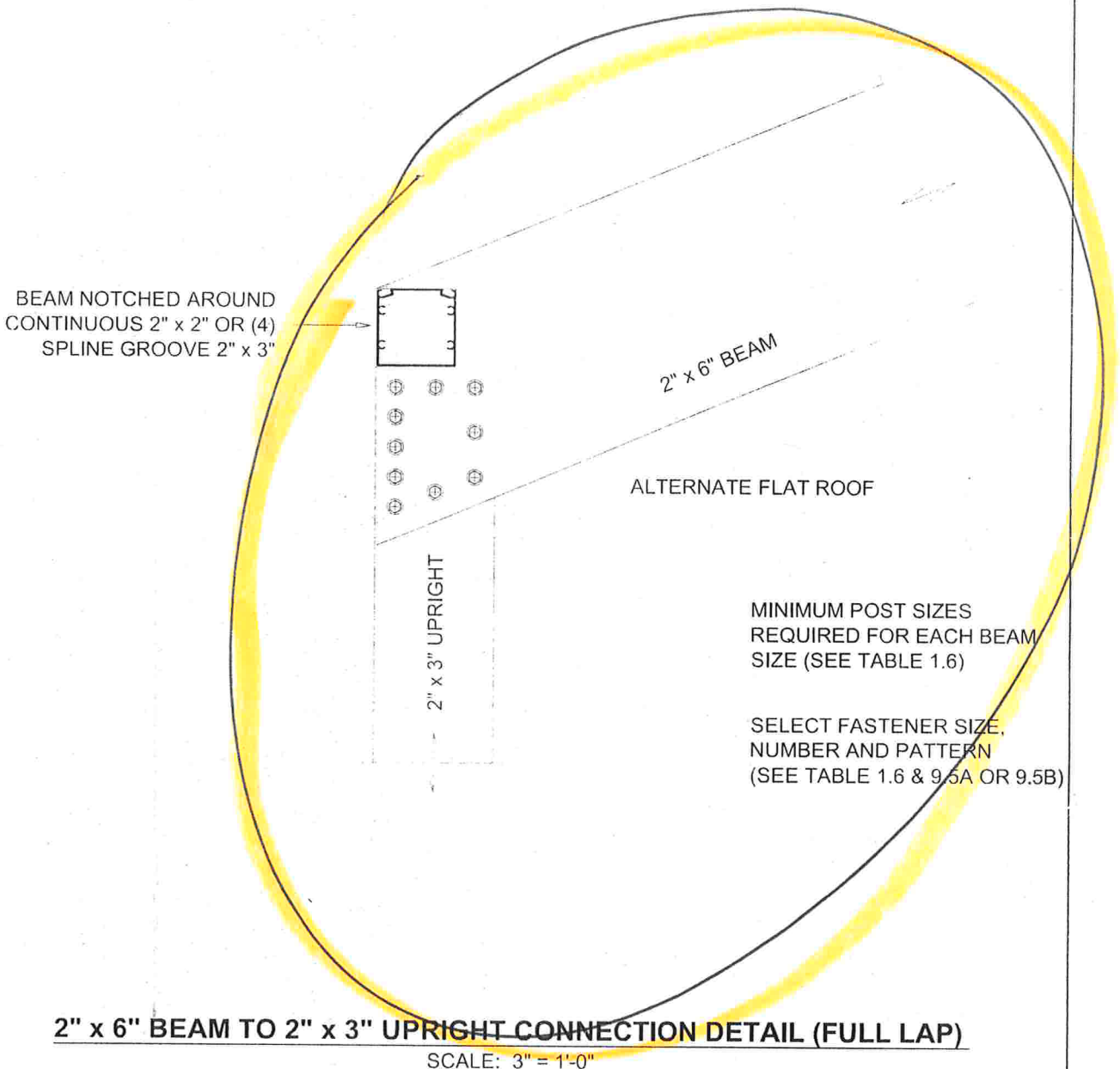

2" x 6" BEAM TO 2" x 3" UPRIGHT CONNECTION DETAIL (FULL LAP)

SCALE: 3" = 1'-0"

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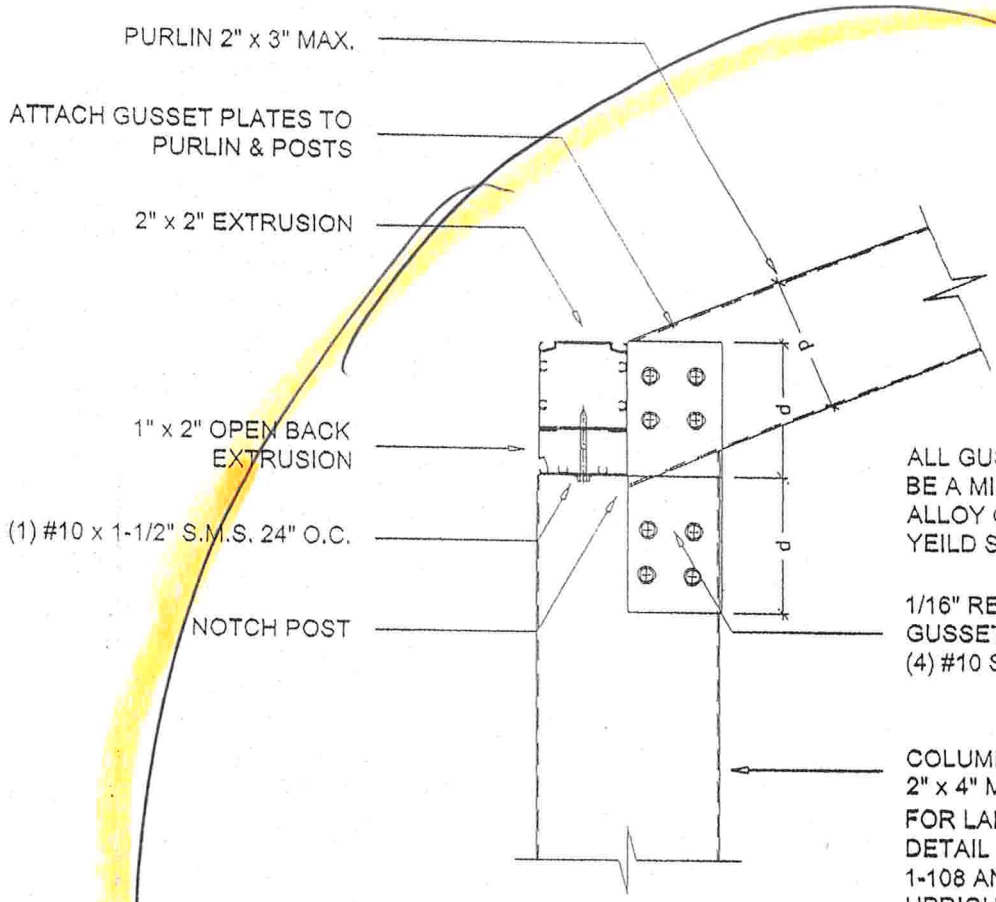

SIDE WALL TO PURLIN DETAIL

SCALE: 3" = 1'-0"

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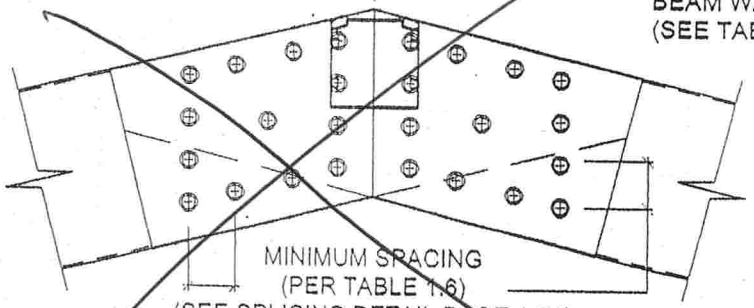
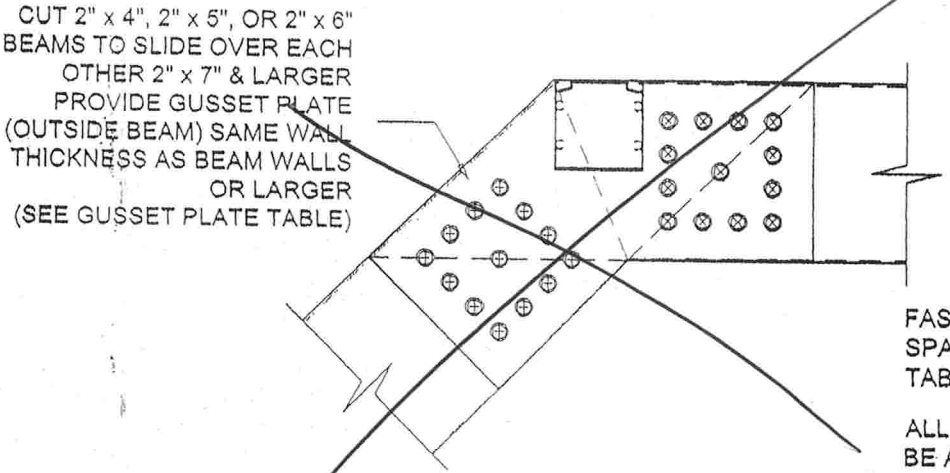


TYPICAL SIDE PLATE CONNECTION DETAIL

SCALE: 3" = 1'-0"



TYPICAL SIDE PLATE CONNECTION DETAIL - MANSARD ROOF

SCALE: 3" = 1'-0"

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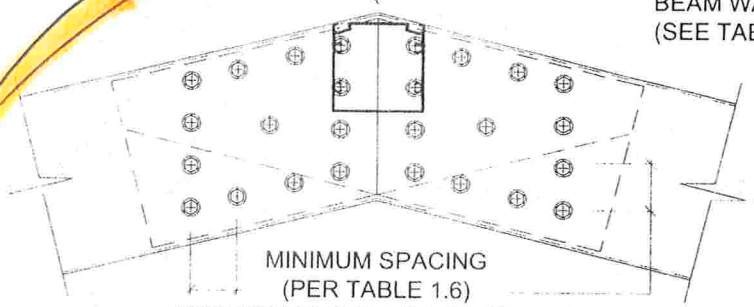
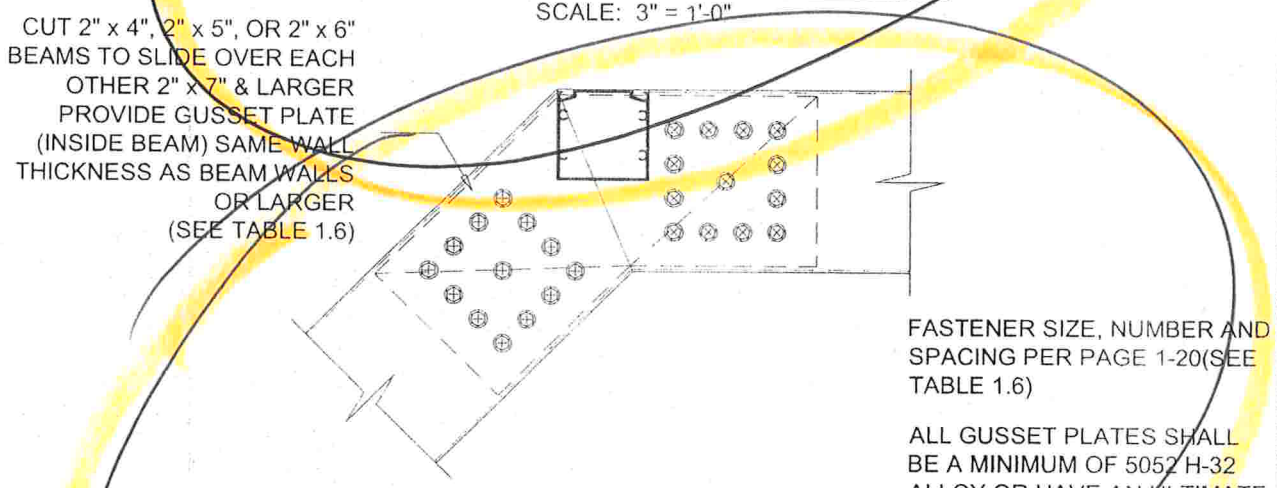


ALTERNATE SIDE PLATE CONNECTION DETAIL
GUSSET PLATE MOUNTED INTERNALLY
SCALE: 3" = 1'-0"



ALTERNATE SIDE PLATE CONNECTION DETAIL - MANSARD ROOF
GUSSET PLATE MOUNTED INTERNALLY
SCALE: 3" = 1'-0"

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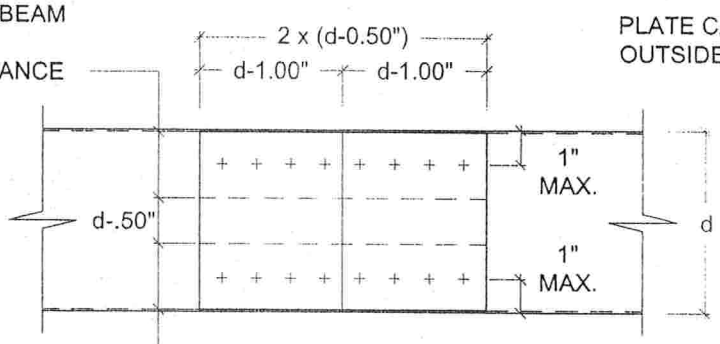
BEAM SPLICE SHALL BE MIN. BEAM HEIGHT MINUS 1/2" AND 2 x (d - .50") LENGTH

d = HEIGHT OF BEAM

SPLICE LOCATED 1/4 TO 1/3 BEAM SPAN STAGGERED EACH SIDE OF BEAM

PLATE CAN BE INSIDE OR OUTSIDE BEAM OR LAP CUT



DENOTES SCREW PATTERN NOT NUMBER OF SCREWS

FASTENER SIZE, NUMBER AND SPACING (SEE TABLE 1.6)

Screw Size	ds (in.)	Minimum Distance and Spacing of Screws* Edge to Center 2ds (in.)	Center to Center 2-1/2ds (in.)	Gusset Plate Beam Size	Thickness (in.)
#8	0.16	3/8	7/16	2" x 7" x 0.055" x 0.120"**	1/16 = 0.063
#10	0.19	3/8	1/2	2" x 8" x 0.072" x 0.224"	1/8 = 0.125
#12	0.21	7/16	9/16	2" x 9" x 0.072" x 0.224"	1/8 = 0.125
#14 or 1/4"	0.25	1/2	5/8	2" x 9" x 0.082" x 0.306"	1/8 = 0.125
5/16"	0.31	5/8	3/4	2" x 10" x 0.092" x 0.369"	1/4 = 0.25

* refers to each side of splice

** use for 2" x 4" and 2" x 6" also

Note:

1. All gusset plates shall be minimum 5052 H-32 Alloy or have a minimum yield of 30 ksi.

TYPICAL BEAM SPLICE DETAIL

SCALE: 3" = 1'-0"

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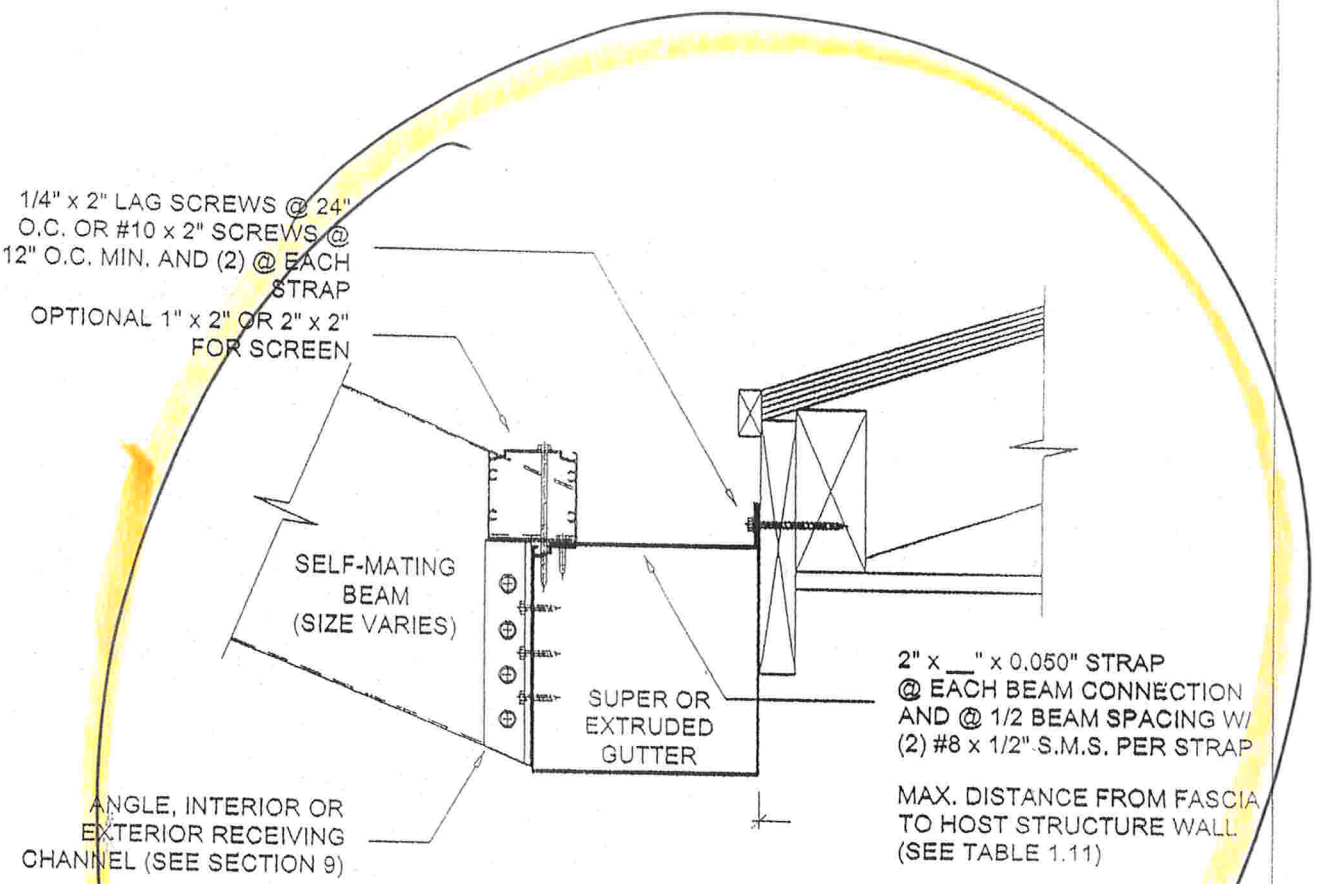

ALTERNATE SELF MATING BEAM CONNECTION TO SUPER OR EXTRUDED GUTTER

SCALE: 3" = 1'-0"

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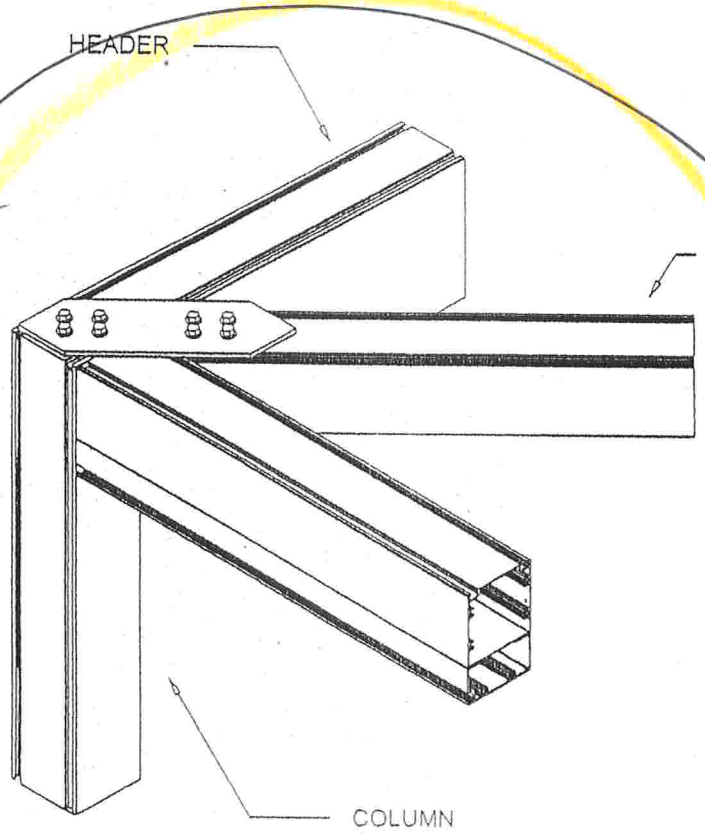

WIND BRACE CONNECTION DETAIL

SCALE: 3" = 1'-0"

NOTES:

1. Wind bracing shall be provided at each side wall panel when enclosure projects more than three panels from host structure. Structures of four or more panels shall be spaced for even number of panels for opposing wind bracing.
2. Cut brace parts with min. 12" lap of larger and smaller brace.
3. Cut receiving channel with angle.

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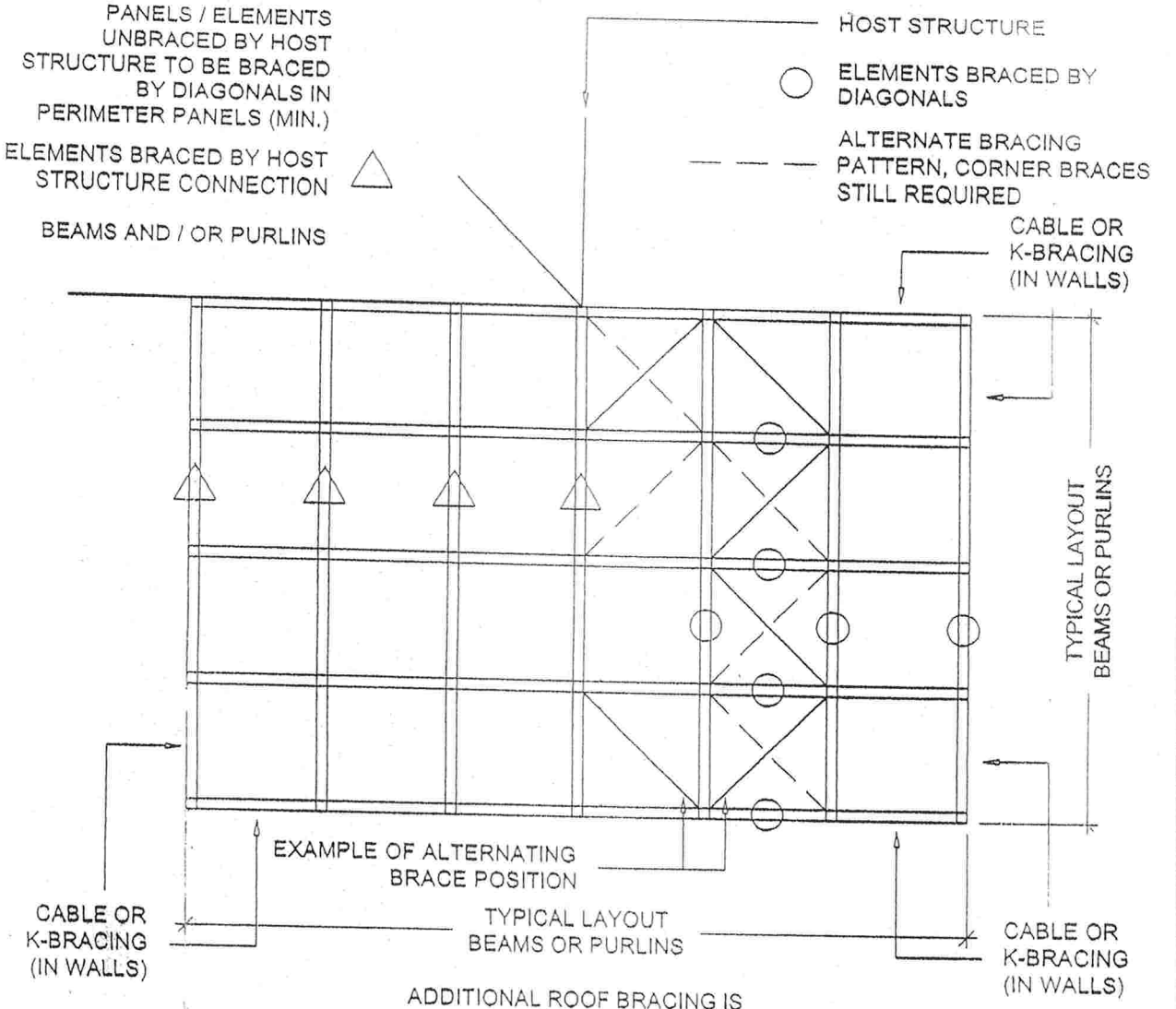

EACH DIAGONAL TO BE FASTENED EACH END W/ (2) EACH #10 S.M.S. (MIN.)

ADDITIONAL ROOF BRACING IS REQUIRED FOR ALL SIDE WALLS LARGER THAN 4 PANELS. NUMBER OF PANELS SHOULD BE EVEN TO PERMIT POSITION OF BRACES ALTERNATING

2 x 2 (MIN) ROOF DIAGONAL, MEET WALL AT WALL BRACING AT CORNERS (TYP.)

(POOL ENCLOSURE SCREEN ROOF MAY BE FLAT, GABLE, MANSARD, DOME, OR HIP)

POOL ENCLOSURE DIAGONAL BRACING - SCHEMATIC PLAN VIEW

SCALE: 3/8" = 1'-0"

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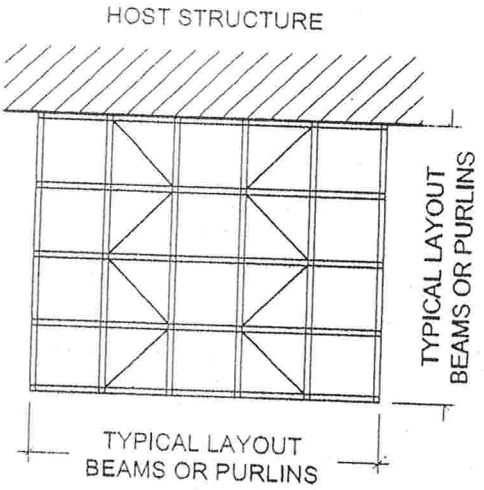
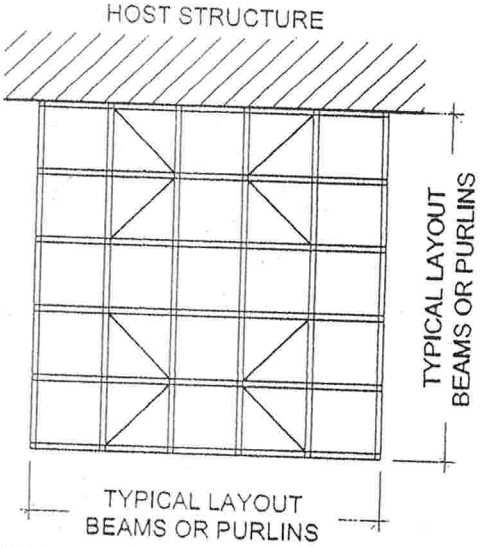


WIND BRACING PATTERN
TYPICAL FOR EVEN NUMBER OF SIDE PANELS OVER 4

SCALE: 3/16" = 1'-0"



WIND BRACING PATTERN
TYPICAL FOR ODD NUMBER OF SIDE PANELS OVER 4

SCALE: 3/16" = 1'-0"

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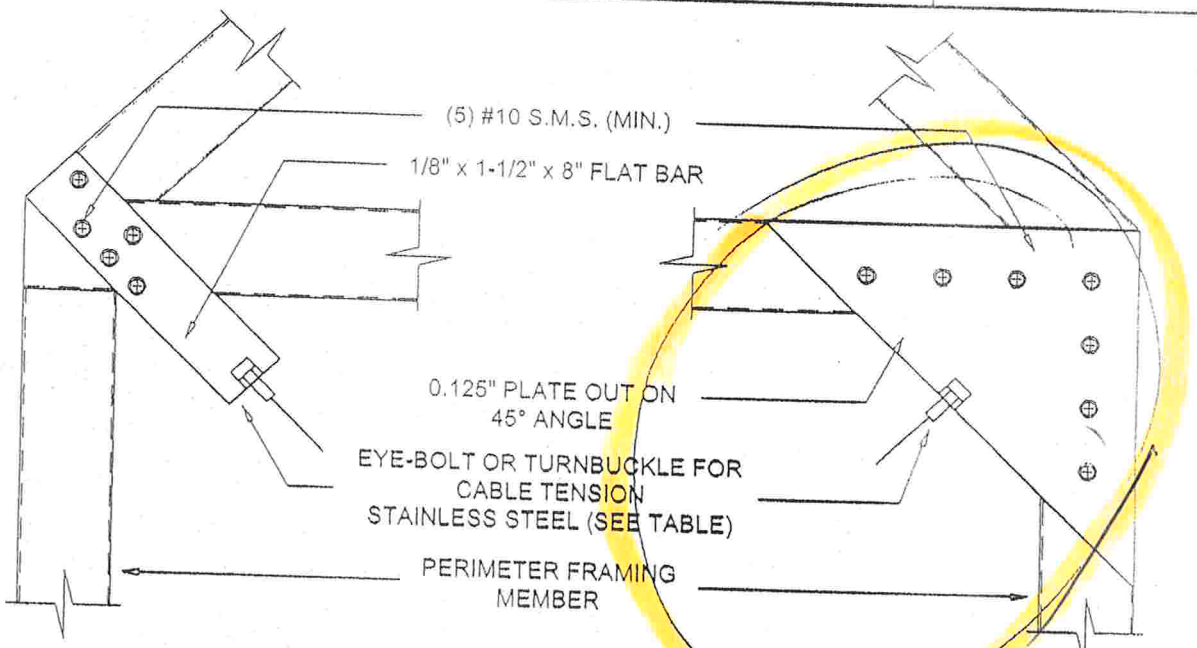
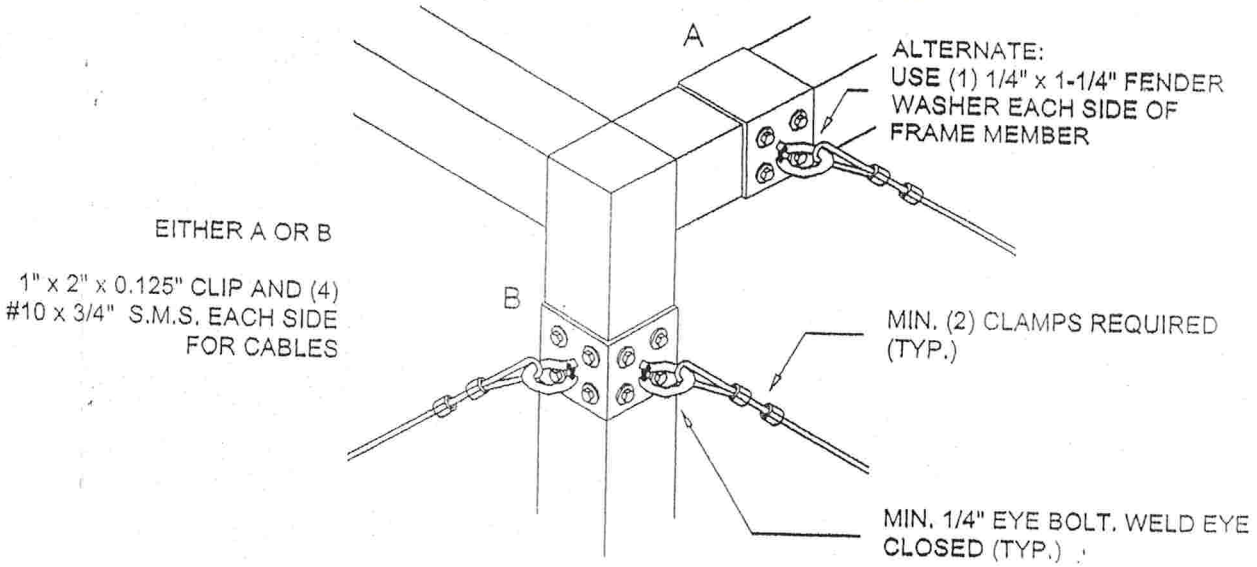


TYPICAL CABLE CONNECTIONS AT CORNER - DETAIL 1

SCALE: 3" = 1'-0"



ALTERNATE TOP CORNER OF CABLE CONNECTION - DETAIL 1A

SCALE: 3" = 1'-0"

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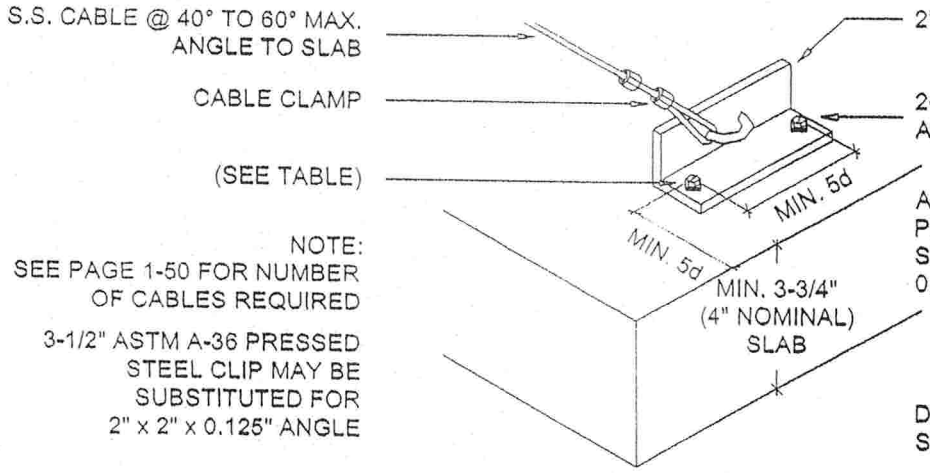
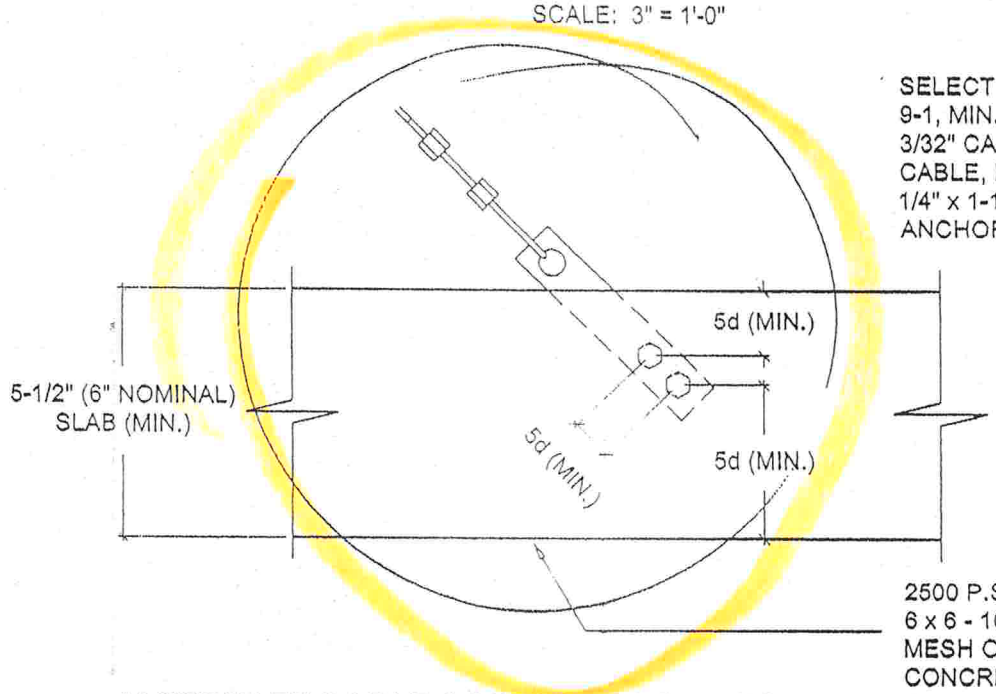


ALTERNATE CABLE CONNECTION AT SLAB DETAIL - DETAIL 2B

SCALE: 3" = 1'-0"



ALTERNATE CABLE CONNECTIONS AT FOUNDATION - DETAIL 2C

SCALE: 3" = 1'-0"

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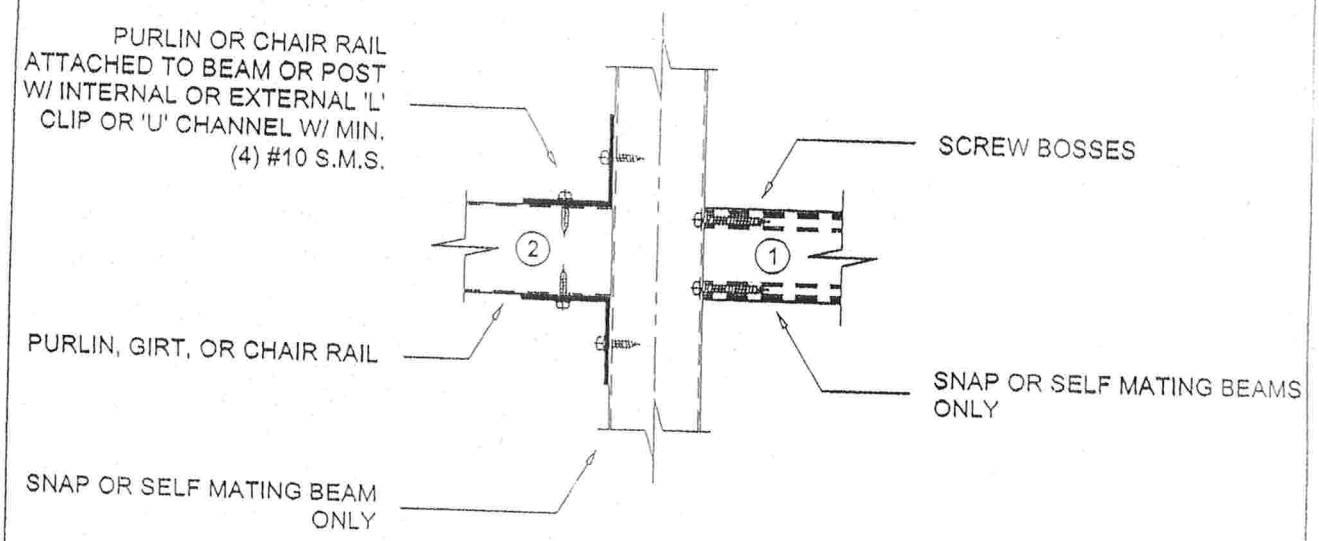

PURLIN TO BEAM OR GIRT TO POST DETAIL

SCALE: 3" = 1'-0"

(1) FOR WALLS LESS THAN 6'-8" FROM TOP OF PLATE TO CENTER OF BEAM CONNECTION OR BOTTOM OF TOP RAIL THE GIRT IS DECORATIVE AND SCREW HEADS MAY BE REMOVED AND INSTALLED IN PILOT HOLES

(2) FOR ALL OTHER PURLINS AND GIRTS IF THE SCREW HEADS ARE REMOVED THEN THE OUTSIDE OF THE CONNECTION MUST BE STRAPPED FROM GIRT TO POST WITH 0.050" x 1-3/4" x 4" STRAP AND (4) #10 x 3/4" S.M.S. SCREWS TO POST AND GIRT

IF GIRT IS ON BOTH SIDES OF THE POST THEN STRAP SHALL BE 6" LONG AND CENTERED ON THE POST AND HAVE A TOTAL (12) #10 x 3/4" S.M.S.

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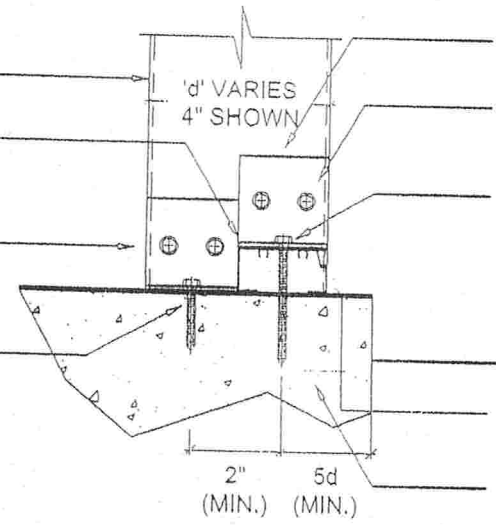
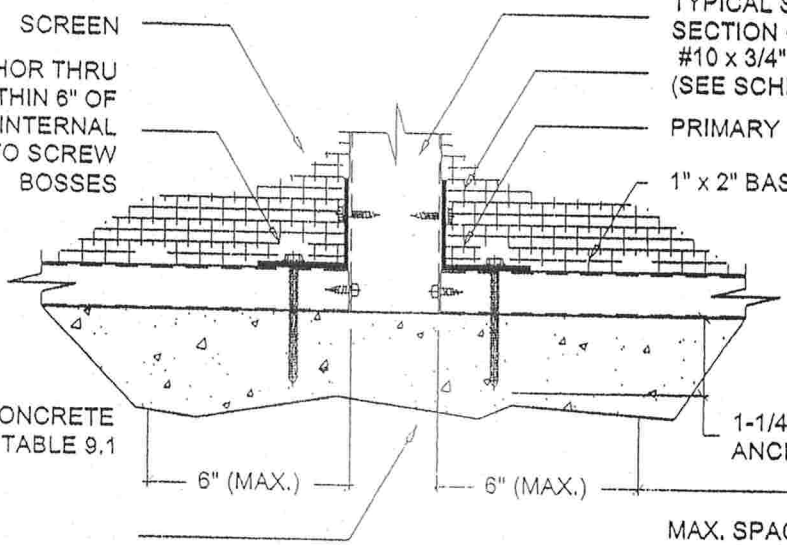


BOLT Ø	* 5d DISTANCE	4d
1/4"	1-1/4"	1"
3/8"	1-7/8"	1-1/2"

SIDE VIEW



FRONT VIEW

2" x 4" OR LARGER SELF MATING OR SNAP SECTION POST TO DECK DETAILS

SCALE: 3" = 1'-0"

NOTE:
1. FOR SIDE WALLS OF 2" x 4" OR SMALLER ONLY ONE ANGLE IS REQUIRED.
2. PREDRILL PAVERS W/ MIN. 1/4" MASONRY BIT.

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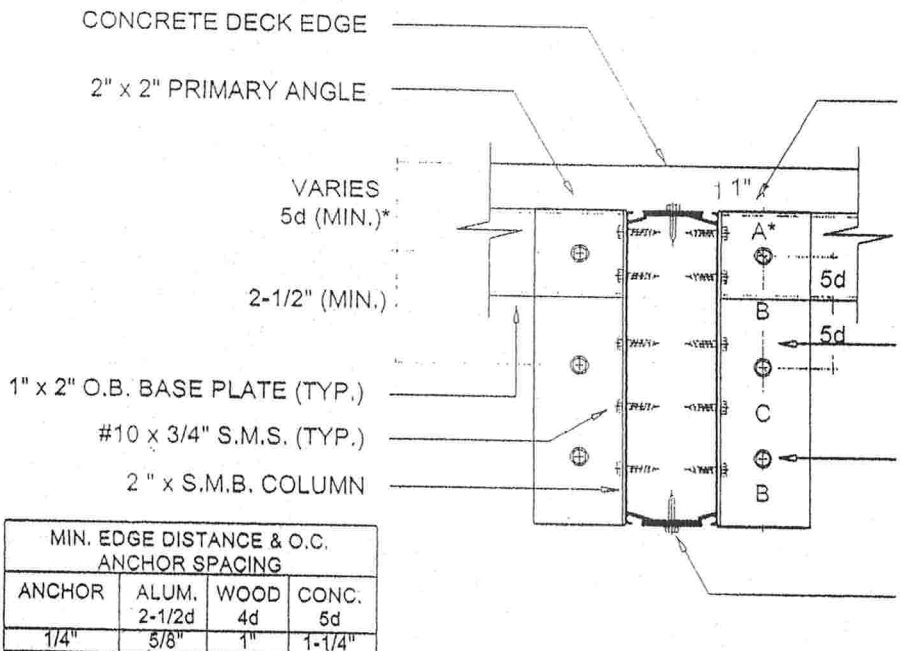
DETAIL ILLUSTRATES TYPICAL 2" x 4" S.M.B. THRU 2" x 9" SUB CONNECTIONS



MIN. EDGE DISTANCE & O.C. ANCHOR SPACING			
ANCHOR	ALUM. 2-1/2d	WOOD 4d	CONC. 5d
1/4"	5/8"	1"	1-1/4"
5/16"	25/32"	1-1/4"	1-9/16"
3/8"	15/16"	1-1/2"	1-7/8"

TOP VIEW POST TO DECK DETAIL

SCALE: 3" = 1'-0"

Primary and Secondary Anchor Schedule

Column Size	Secondary Angle				Maximum Number and Spacing Anchors											
	Angle	Number of Anchors			1/4"				5/16"				3/8"			
	Length "L"	1/4"	5/16"	3/8"	#	"A"	"B"	"C"	#	"A"	"B"	"C"	#	"A"	"B"	"C"
2 x 4	2"	4	4	4	4	1"	1"	1"	4	1"	1"	1"	4	1"	1"	1"
2 x 5	3"	4	4	4	4	1"	1-1/2"	-	4	1"	1-1/2"	-	4	1"	1-1/2"	-
2 x 6	4"	4	4	4	4	1"	2"	-	4	1"	2"	-	4	1"	2"	-
2 x 7	5"	6	4	4	6	1"	5/8"	1-7/8"	4	1"	2-1/2"	-	4	1"	2-1/2"	-
2 x 8	6"	6	4	4	6	1"	5/8"	2-3/8"	4	1"	3"	-	4	1"	3"	-
2 x 9	7"	6	6	4	6	1"	5/8"	2-7/8"	6	1"	13/16"	2-7/8"	4	1"	3-1/2"	-
2 x 10	8"	8	6	6	8	1"	5/8"	2"	6	1"	13/16"	3-3/16"	6	1"	3/4"	3-1/4"

Example:
Calculate the number of anchors required: 1.5 x beam span / 2 x beam spacing x roof wind pressure (PSF) = total #;
If 1.5 x 30'/2 x 6' x 10 PSF = = 1350# and 1/4" x 1/4" Tapcon in tension @ 5d = 427# / ea. (see table 9.1)
then 1350# / 427# / ea. = 3.16 ea. use (3) ea., secondary angle not required
Actual Edge Distance Example:
From edge of concrete to fastener = 2" / dia. of 0.25" = 8d
Note:
For attachment to wood deck substitute wood fasteners for concrete fasteners & calculate the required number of fasteners using tables from section 9.

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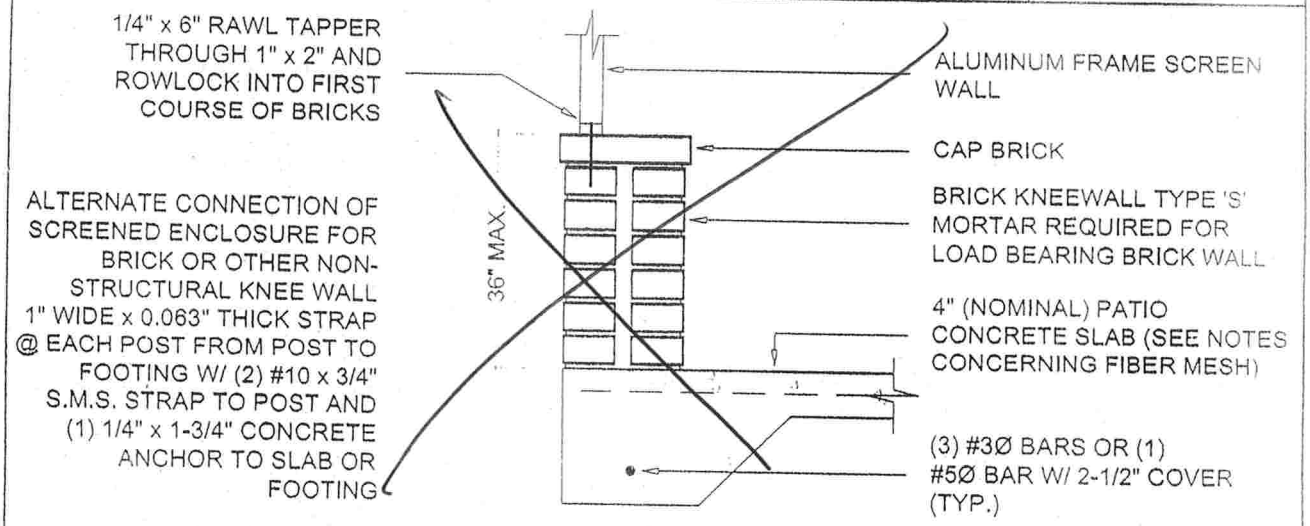
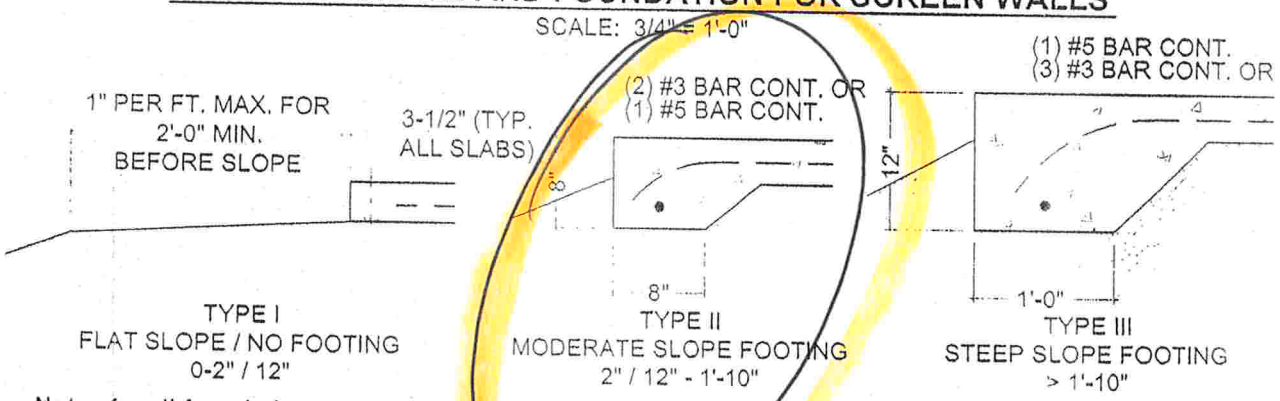


BRICK KNEEWALL AND FOUNDATION FOR SCREEN WALLS

SCALE: 3/4" = 1'-0"



TYPE I
FLAT SLOPE / NO FOOTING
0-2" / 12"

TYPE II
MODERATE SLOPE FOOTING
2" / 12" - 1'-10"

TYPE III
STEEP SLOPE FOOTING
> 1'-10"

Notes for all foundation types:

1. The foundations shown are based on a minimum soil bearing pressure of 1,500 PSF. Bearing capacity of soil shall be verified prior to placing slab by field soil test (soil penetrometer) or a soil testing lab.
2. The slab / foundation shall be cleared of debris, roots and compacted prior to placement of concrete.
3. No footing is required except when addressing erosion until the slab width in the direction of the primary beams exceeds the span per table on page 1-69, then a type II slab is required under the load bearing wall only unless the side wall exceeds 16' in height or the enclosure is in a "C" exposure catagory in which case a type II footing is required.
4. Monolithic slabs and footings shall be minimum 2,500 psi concrete with 6 x 6 - 10 x 10 welded wire mesh or crack control fiber mesh; Fibermesh® Mesh, InForce™ e3™ (Formerly Fibermesh MD) per manufacturer's specification may be used in lieu of wire mesh. All slabs / footings shall be allowed to cure for 7 days before installing anchors.
5. If local codes require a minimum footing use Type II footing or footing section required by local code. Local codes govern.

SLAB-FOOTING DETAILS

SCALE: 3/4" = 1'-0"

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Table 1.1A 120 Moment Connection

Allowable Spans for Primary Screen Roof Frame Members

Aluminum Alloy 6063 T-6

for Areas in Wind Zones up to 120 M.P.H., Exposure "B" and Latitudes Below 30°-30'-00" North (Jacksonville, FL. Uniform Load = 4 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered

Hollow Sections	Tributary Load Width 'W' = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb
2" x 2" x 0.050"	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb
2" x 2" x 0.090"	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb
2" x 3" x 0.045"	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb
2" x 4" x 0.050"	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb
2" x 5" x 0.062"	20'-5"	Pb	20'-5"	Pb	20'-5"	Pb	20'-5"	Pb	20'-5"	Pb	20'-5"	Pb	20'-4"	Ub

Self Mating Sections	Tributary Load Width 'W' = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 4" x 0.044 x 0.100"	12'-3"	Pb	12'-3"	Pb	12'-3"	Pb	12'-3"	Pb	12'-3"	Pb	12'-3"	Pb	12'-3"	Pb
2" x 5" x 0.050" x 0.100"	18'-5"	Pb	18'-5"	Pb	18'-5"	Pb	18'-5"	Pb	18'-5"	Pb	18'-5"	Pb	18'-5"	Pb
2" x 6" x 0.050" x 0.120"	23'-0"	Pb	23'-0"	Pb	23'-0"	Pb	23'-0"	Pb	23'-0"	Pb	22'-5"	Ub	21'-0"	Ub
2" x 7" x 0.055" x 0.120"	27'-0"	Pb	27'-0"	Pb	27'-0"	Pb	27'-0"	Pb	26'-2"	Ub	24'-4"	Ub	22'-10"	Ub
2" x 8" x 0.072" x 0.224"	48'-3"	Ud	43'-10"	Ud	40'-8"	Ud	38'-4"	Ud	36'-5"	Ud	34'-10"	Ud	33'-5"	Ud
2" x 9" x 0.072" x 0.224"	52'-11"	Ud	48'-1"	Ud	44'-8"	Ud	42'-0"	Ud	39'-11"	Ud	38'-2"	Ud	36'-6"	Ub
2" x 9" x 0.082" x 0.310"	56'-10"	Ud	51'-8"	Ud	47'-11"	Ud	45'-1"	Ud	42'-10"	Ud	40'-11"	Ud	39'-5"	Ud
2" x 10" x 0.092" x 0.369"	66'-0"	Ud	59'-11"	Ud	55'-8"	Ud	52'-5"	Ud	49'-9"	Ud	47'-7"	Ud	45'-9"	Ud

Snap Sections	Tributary Load Width 'W' = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd
2" x 3" x 0.045"	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd
2" x 4" x 0.045"	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd
2" x 6" x 0.062"	22'-2"	Pd	22'-2"	Pd	22'-2"	Pd	22'-2"	Pd	22'-2"	Pd	22'-2"	Pd	22'-2"	Pd
2" x 7" x 0.062"	26'-8"	Pd	26'-8"	Pd	26'-8"	Pd	26'-8"	Pd	26'-8"	Pd	26'-8"	Pd	26'-8"	Pd

Note:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. The structures designed using this section shall be limited to a maximum combined span and upright height of 50' and a maximum upright height of 16'. Structures larger than these limits shall have site specific engineering.
3. Span is measured from center of beam and upright connection to fascia or wall connection.
4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
5. Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable. Other conditions may offer better spans w/ enclosure site specific engineering.
6. Spans may be interpolated.
7. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

Example: Max. 'L' for 2" x 4" x 0.050" hollow section with 'W' = 5'-0" = 9'-1"

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Table 1.3A 110 Moment Connection
Allowable Post / Upright Heights for Primary Screen Wall Frame Members
Aluminum Alloy 6063 T-6

For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 #/sq. ft.

Hollow Sections	Tributary Load Width 'W' = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" / bending (b), deflection (d)													
2" x 2" x 0.044"	8'-4"	b	7'-2"	b	6'-4"	b	5'-8"	b	5'-2"	b	4'-9"	b	4'-5"	b
2" x 2" x 0.050"	9'-2"	b	7'-11"	b	6'-11"	b	6'-4"	b	5'-9"	b	5'-4"	b	4'-11"	b
2" x 2" x 0.090"	11'-5"	b	9'-10"	b	8'-9"	b	7'-11"	b	7'-4"	b	6'-10"	b	6'-5"	b
2" x 3" x 0.045"	11'-2"	d	9'-9"	b	8'-8"	b	7'-10"	b	7'-2"	b	6'-8"	b	6'-2"	b
2" x 4" x 0.050"	12'-6"	b	10'-9"	b	9'-6"	b	8'-7"	b	7'-11"	b	7'-4"	b	6'-10"	b
2" x 5" x 0.062"	19'-3"	b	16'-7"	b	14'-9"	b	13'-5"	b	12'-4"	b	11'-6"	b	10'-9"	b

Self Mating Sections	Tributary Load Width 'W' = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" / bending (b), deflection (d)													
2" x 4" x 0.044" x 0.100"	15'-1"	b	13'-0"	b	11'-7"	b	10'-6"	b	9'-8"	b	8'-11"	b	8'-5"	b
2" x 5" x 0.050" x 0.100"	18'-8"	b	16'-1"	b	14'-4"	b	12'-11"	b	11'-11"	b	11'-2"	b	10'-5"	b
2" x 6" x 0.050" x 0.120"	20'-11"	b	18'-0"	b	16'-1"	b	14'-7"	b	13'-5"	b	12'-6"	b	11'-9"	b
2" x 7" x 0.055" x 0.120"	22'-8"	b	19'-7"	b	17'-5"	b	15'-10"	b	14'-7"	b	13'-7"	b	12'-10"	b
2" x 8" x 0.072" x 0.224"	32'-7"	d	29'-3"	b	26'-2"	b	23'-10"	b	22'-0"	b	20'-7"	b	19'-4"	b
2" x 9" x 0.072" x 0.224"	35'-7"	b	30'-9"	b	27'-5"	b	25'-0"	b	23'-1"	b	21'-7"	b	20'-4"	b
2" x 9" x 0.082" x 0.310"	38'-4"	d	34'-10"	d	32'-1"	b	29'-3"	b	27'-1"	b	25'-4"	b	23'-10"	b
2" x 10" x 0.092" x 0.369"	44'-7"	d	40'-6"	d	37'-7"	d	35'-4"	d	32'-11"	b	30'-10"	b	29'-0"	b

Snap Sections	Tributary Load Width 'W' = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" / bending (b), deflection (d)													
2" x 2" x 0.044"	8'-10"	d	7'-8"	b	6'-9"	b	6'-0"	b	5'-5"	b	4'-11"	b	4'-7"	b
2" x 3" x 0.045"	11'-9"	b	9'-11"	b	8'-9"	b	7'-9"	b	7'-0"	b	6'-5"	b	5'-10"	b
2" x 4" x 0.045"	13'-9"	b	11'-8"	b	10'-3"	b	9'-1"	b	8'-3"	b	7'-6"	b	6'-11"	b
2" x 6" x 0.062"	24'-5"	d	22'-2"	d	19'-10"	b	17'-11"	b	16'-6"	b	15'-4"	b	14'-4"	b
2" x 7" x 0.062"	27'-7"	d	24'-7"	b	21'-10"	b	19'-10"	b	18'-3"	b	16'-11"	b	15'-10"	b

Note:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Using screen panel width 'W' select upright length 'H'.
3. Above heights do not include length of knee brace. Add vertical distance from upright to center of brace to beam connection to the above heights for total beam heights.
4. Site specific engineering required for pool enclosures over 30' in mean roof height.
5. height is to be measured from center of beam and upright connection to fascia or wall connection.
6. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height are designed to be residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. into the screw bosses and do not exceed 8'-0" in height.
7. Maximum beam size for 2"x 5" is a 2"x 7"x 0.055"x 0.120"
8. heights may be interpolated.
9. To convert heights to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

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Table 1.6A Moment Connection
Minimum Upright Sizes and Number of Screws for
Connection of Roof Beams To Wall Uprights or Beam Splicing

Beam/Upright or Post	Upright or Post/Beam	Minimum Purlin, Girt & Knee Brace Size	Notes	Minimum Number of Screws*			Beam Stitching Screw at 24" OC
				#8 x ½"	#10 x ½"	#12 x ½"	
2 x 4 SMB	2 x 4 SMB	2" x 2" x 0.044"	Moment Connection	8	6	4	#8
2 x 5 SMB	2 x 4 SMB	2" x 2" x 0.044"	Moment Connection	8	6	4	#8
2 x 6 SMB	2 x 4 SMB	2" x 2" x 0.044"	Moment Connection	10	8	6	#10
2 x 7 SMB	2 x 5 SMB	2" x 2" x 0.044"	Moment Connection	14	12	10	#12
2 x 8 SMB	2 x 6 SMB	2" x 3" x 0.044"	Moment Connection	16	14	12	#14
2 x 9 SMB	2 x 6 SMB	2" x 3" x 0.045"	Moment Connection	18	16	14	#14
2 x 9 SMB **	2 x 7 SMB	2" x 4" x 0.050"	Moment Connection	20	18	16	#14
2 x 10 SMB	2 x 8 SMB	2" x 5" x 0.050"	Moment Connection	20	18	16	#14

Screw Size	Minimum Distance and Spacing of Screws	
	Edge To Center	Center To Center
#8	5/16"	5/8"
#10	3/8"	3/4"
#12	1/2"	1"
#14 or 1/4"	3/4"	1-1/2"
5/16"	7/8"	1-3/4"
3/8"	1"	2"

Gusset Plate Thickness	
Beam Size	Thickness
2" x 7" x 0.055" x 0.120"	0.063"
2" x 8" x 0.072" x 0.224"	0.125"
2" x 9" x 0.072" x 0.224"	0.125"
2" x 9" x 0.082" x 0..306"	0.190"
2" x 10" x 0.092" x 0.369"	0.250"

* Refers to each side of the connection of the beam and upright and each side of splice connection.

Connection Example:

2" x 7" beam & 2" x 5" at beam & gusset plate, (14) #8 x 1/2" sms & upright & gusset plate (14) #8 x 1/2" sms ea. side of beam & upright.

** 0.082" wall thickness, 0.310" flange thickness

Note:

1. Connection of 2" x 6" to 2" x 3" shall use a full lap cut or 1/16" gusset plate
2. For beam splice connections the number of screws shown is the total for each splice with 1/2 the screws on each side of the cut.
3. The number of screws is based on the maximum allowable moment of the beam.
4. The number of deck anchors is based on RAWL R Tapper allowable load data for 2,500 psi concrete and / or equal anchors may be used. The number shown is the total use 1/2 per side.
5. Hollow splice connections can be made provided the connection is approved by the engineer.
6. If a larger than minimum upright is used the number of screws is the same for each splice with 1/2 the screws on each side of the cut.
7. All beam to upright connections for 2" x 7" beams or larger shall have an internal gusset plate except when a knee brace is used at the connection. Gusset plates are required for mansard, gabled and all spliced connections.
8. For gusset plate connections 2" x 9" beams or larger use 3/4" long screws.
9. The side wall upright shall have a minimum beam size as shown above, ie., a 2" x 4" upright shall have a 2" x 3" beam.
10. For minimum girt size read upright size as a beam and purlin size is minimum girt size. (i.e. 2" x 9" x 0.072" x 0.224" s.m.b. or 2" x 6" x 0.050 x 0.120" s.m.b. upright requires a 2" x 3" x 0.045" girt / chair rail.)

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Table 1.9.2A Moment Connection
Allowable Spans for Secondary Screen Roof Frame Members
Aluminum Alloy 6063 T-6

For Wind Zones up to 130 M.P.H., Exposure "B" and Latitudes North of 30°-30'-00" North (Jacksonville, FL)
Uniform Load = 15 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered

A. Sections Fastened To Beams With Clips

Hollow Sections	Tributary Load Width 'W' = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb
2" x 2" x 0.050"	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb
2" x 2" x 0.090"	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-3"	Ud	6'-11"	Ud	6'-9"	Ud	6'-7"	Ud
3" x 2" x 0.045"	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb
3" x 2" x 0.070"	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd	7'-6"	Ud	7'-4"	Ud	7'-1"	Ud	6'-10"	Ud
2" x 3" x 0.045"	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-3"	Ud	7'-0"	Ud	6'-10"	Ud	6'-7"	Ud
2" x 4" x 0.050"	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	8'-10"	Ub	8'-5"	Ub	8'-0"	Ub
2" x 5" x 0.062"	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd	13'-11"	Ud	13'-5"	Ub	12'-11"	Ub	12'-3"	Ub

Snap Sections	Tributary Load Width 'W' = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-10"	Ud
2" x 3" x 0.045"	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd	7'-2"	Ud	6'-11"	Ud	6'-9"	Ud	6'-6"	Ud
2" x 4" x 0.045"	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd	9'-0"	Ud	8'-9"	Ud	8'-6"	Ud	8'-2"	Ud

Uniform Load = 4 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered

B. Sections Fastened Through Beam Webs Into Screw Bosses

Hollow Sections	Tributary Load Width 'W' = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 3" x 0.050"	8'-2"	Ud	7'-10"	Ud	7'-6"	Ud	7'-3"	Ud	7'-0"	Ud	6'-10"	Ud	6'-7"	Ud
2" x 4" x 0.050"	11'-1"	Ub	10'-4"	Ub	9'-9"	Ub	9'-3"	Ub	8'-10"	Ub	8'-5"	Ub	8'-0"	Ub
2" x 5" x 0.062"	15'-8"	Ud	14'-11"	Ud	14'-5"	Ud	13'-11"	Ud	13'-5"	Ub	12'-11"	Ub	12'-3"	Ub

Snap Sections	Tributary Load Width 'W' = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	5'-11"	Ud	5'-8"	Ud	5'-6"	Ud	5'-4"	Ud	5'-2"	Ud	4'-11"	Ud	4'-10"	Ud

Note:
1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Span is measured from center of beam and upright connection to fascia or wall connection.
3. Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable. Other conditions may offer better spans w/ enclosure site specific engineering.
4. Spans may be interpolated.
5. 2" x 4" & 2" x 5" Hollow Girts shall be connected w/ an internal or external 1-1/2" x 1-1/2" x 0.044" angle.
6. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-il.

CHECK TABLE 1.6 FOR MINIMUM UPRIGHT SIZE FOR BEAMS.

Example:
Max. 'L' for 2" x 4" x 0.050" hollow section fastened to beam with clips with 'W' = 5'-0" = 8'-3"

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Table 1.4 110 Allowable Post / Girt / Chair Rail Spans, Header Spans & Upright Heights for Secondary Screen Wall Frame Members

Aluminum Alloy 6063 T-6

For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 # / sq. ft.

A. Sections As Horizontals Fastened To Posts With Clips

Hollow Sections	Tributary Load Width 'W' = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" or Span "L" / bending (b), deflection (d)													
2" x 2" x 0.044"	7'-5"	d	6'-5"	b	5'-8"	b	5'-1"	b	4'-8"	b	4'-3"	b	3'-11"	b
2" x 2" x 0.050"	7'-10"	d	7'-1"	b	6'-3"	b	5'-8"	b	5'-2"	b	4'-9"	b	4'-5"	b
2" x 2" x 0.090"	8'-11"	d	8'-2"	d	7'-10"	d	7'-1"	b	6'-7"	b	6'-1"	b	5'-9"	b
3" x 2" x 0.045"	8'-4"	d	7'-4"	b	6'-6"	b	5'-10"	b	5'-4"	b	4'-11"	b	4'-7"	b
3" x 2" x 0.070"	9'-5"	d	8'-6"	d	7'-9"	b	7'-0"	b	6'-5"	b	5'-11"	b	5'-7"	b
2" x 3" x 0.045"	8'-4"	d	7'-7"	d	7'-9"	d	6'-11"	d	6'-5"	d	5'-11"	b	5'-6"	b
2" x 4" x 0.050"	11'-2"	b	9'-7"	b	8'-6"	b	7'-9"	b	7'-1"	b	6'-7"	b	6'-1"	b
2" x 5" x 0.062"	17'-3"	b	14'-10"	b	13'-2"	b	11'-11"	b	11'-0"	b	10'-3"	b	9'-7"	b

Snap Sections	Tributary Load Width 'W'= Upright Spacing											
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" or Span "L" / bending (b), deflection (d)													
2" x 2" x 0.044"	6'-7"	d	5'-11"	d	5'-7"	d	5'-3"	d	4'-10"	b	4'-5"	b	4'-1"	b

B. Sections As Horizontals Fastened To Posts Through Side Into Screw Bosses

Hollow Sections	Tributary Load Width 'W' = Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" or Span "L" / bending (b), deflection (d)													
3" x 2" x 0.045"	9'-7"	b	8'-3"	b	7'-3"	b	6'-6"	b	5'-11"	b	5'-6"	b	5'-1"	b
3" x 2" x 0.070"	11'-5"	b	9'-10"	b	8'-8"	b	7'-10"	b	7'-2"	b	6'-8"	b	6'-3"	b
2" x 3" x 0.045"	11'-2"	d	9'-9"	b	8'-8"	b	7'-10"	b	7'-2"	b	6'-8"	b	6'-2"	b
2" x 4" x 0.050"	12'-6"	b	10'-9"	b	9'-6"	b	8'-7"	b	7'-11"	b	7'-4"	b	6'-10"	b
2" x 5" x 0.062"	19'-3"	b	16'-7"	b	14'-9"	b	13'-5"	b	12'-4"	b	11'-6"	b	10'-9"	b

Snap Sections	Tributary Load Width 'W'= Upright Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Height "H" or Span "L" / bending (b), deflection (d)													
2" x 2" x 0.044"	8'-10"	d	7'-8"	b	6'-9"	b	6'-0"	b	5'-5"	b	4'-11"	b	4'-7"	b

Note:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Using screen panel width 'W' select girt lengths.
3. Site specific engineering required for pool enclosures over 30' in mean roof height.
4. Span/height is to be measured from center of beam and upright connection to fascia or wall connection.
5. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height are designed to be residential gardrails provided they are attached with min. (3) #10 x 1-1/2" s.m.s. into the screw bosses and do not exceed 8'-0" o.c.
6. Girt spacing shall not exceed 6'-8".
7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"
8. 2" x 4" & 2" x 5" hollow girts shall be connected w/ an internal or external 1-1/2" x 1-1/2" x 0.044" angle.
9. Spans/heights may be interpolated.
10. To convert spans/heights to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

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Coastal Craftsmen Aluminum, Inc.

1406 SW 15th Avenue, Ocala, Florida 34474
Phone No. (352) 369-1444 or Fax No. (352) 369-1988

October 9, 2008

To Whom It May Concern:

Re: Power of Attorney

To Whom It May Concern:

I, William Woodard, President of Coastal Craftsmen Aluminum, Inc. hereby authorize Marion County Building Department to include *Andrew Turner* on the list of employees to sign any and all papers or documents necessary to obtain licenses and permits for jobs contracted by *Coastal Craftsmen Aluminum, Inc.*

If you have any questions please call our office at (352) 369-1444.

Sincerely,

William Woodard
President
Coastal Craftsmen Aluminum
CGC047465

The foregoing instrument was acknowledged before me this 9th day of October 2008 by William Woodard who is personally known to me or who has produced _____ as identification.

Notary Public

Seal:

JENNIFER I. JONES
Comm# DD0762241
Expires 2/25/2012
Florida Notary Assn., Inc