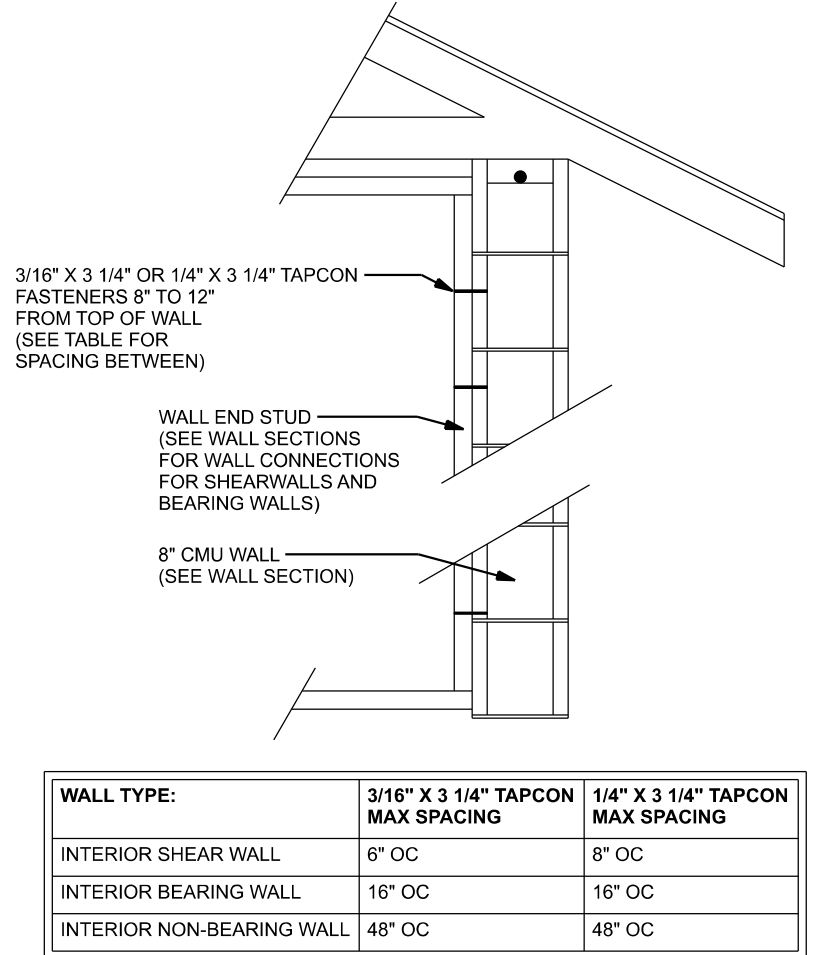
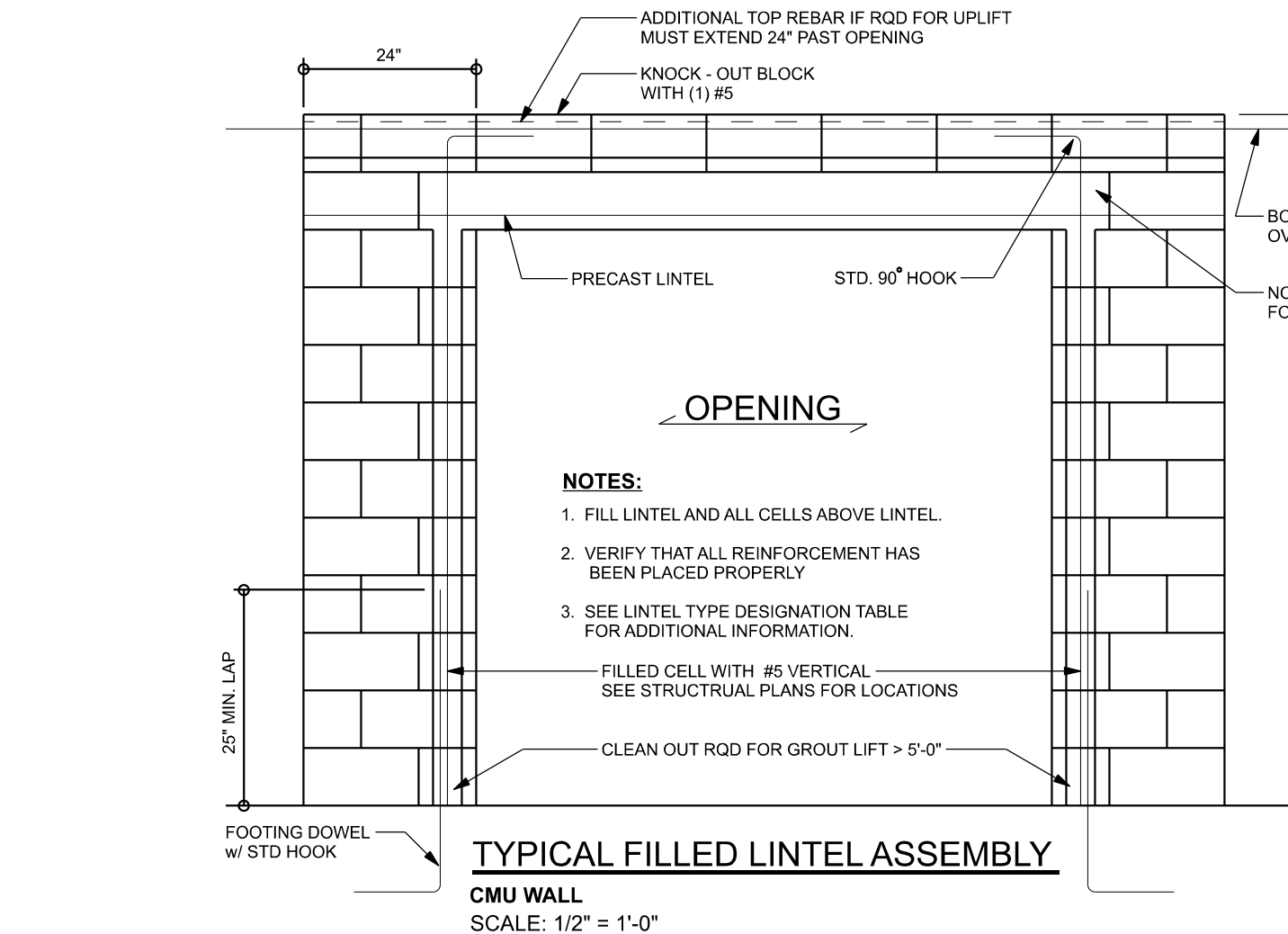
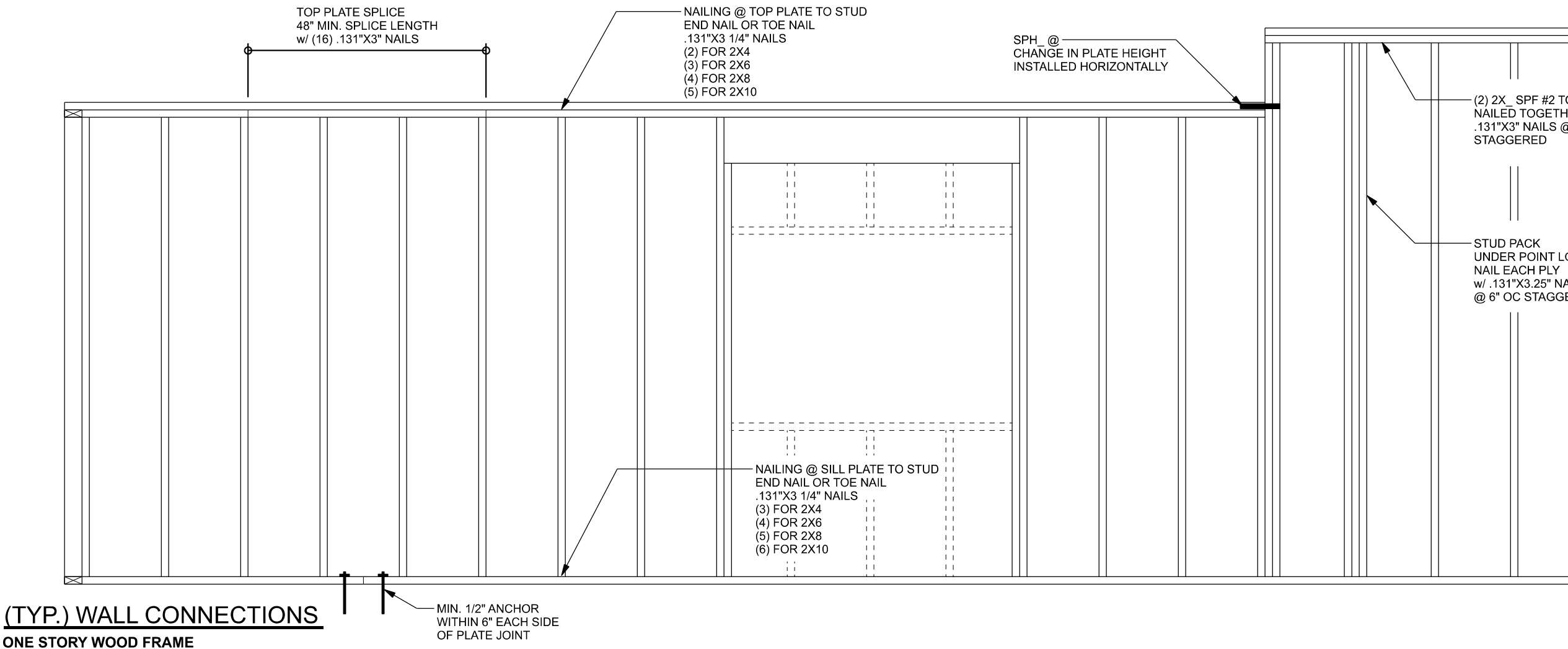


Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc
130 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc

Note:
For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field.
Note:
This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.



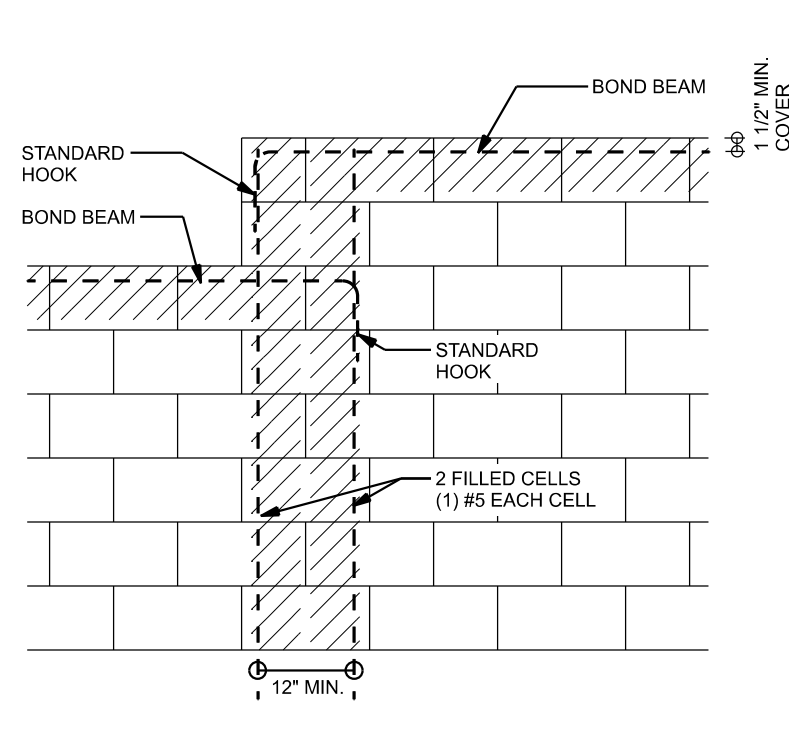
INT. FRAME WALL TO CMU CONNECTION



TYP. WALL CONNECTIONS
ONE STORY WOOD FRAME

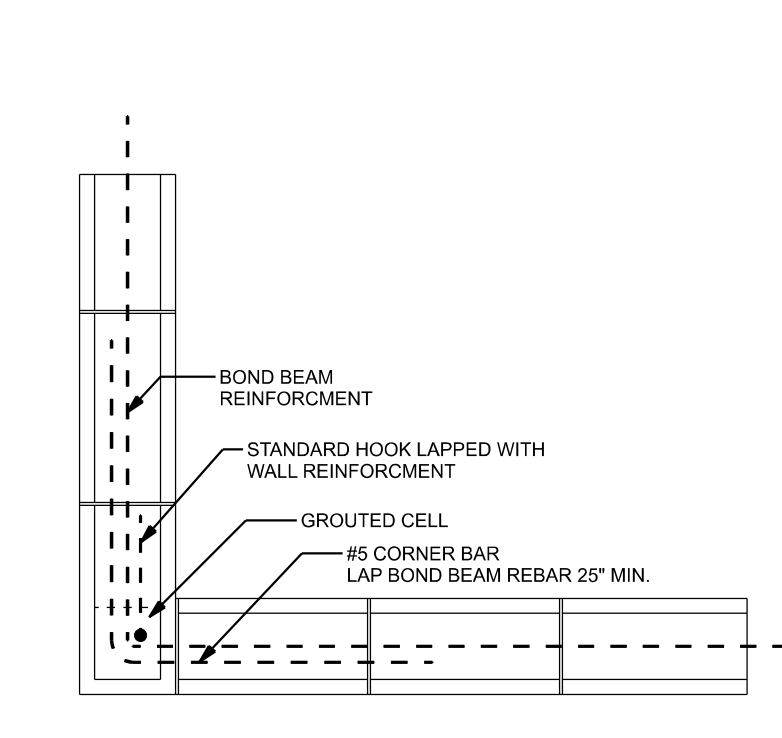
CHANGES IN BOND BEAM HEIGHT

(BASED ON FBC FIG. R609.2.5)
SCALE: 1/2" = 1'-0"



CORNER CONTINUITY OF BOND BEAM AND WALL REINFORCEMENT

(BASED ON FBC FIG. R609.2.4)
SCALE: 3/4" = 1'-0"



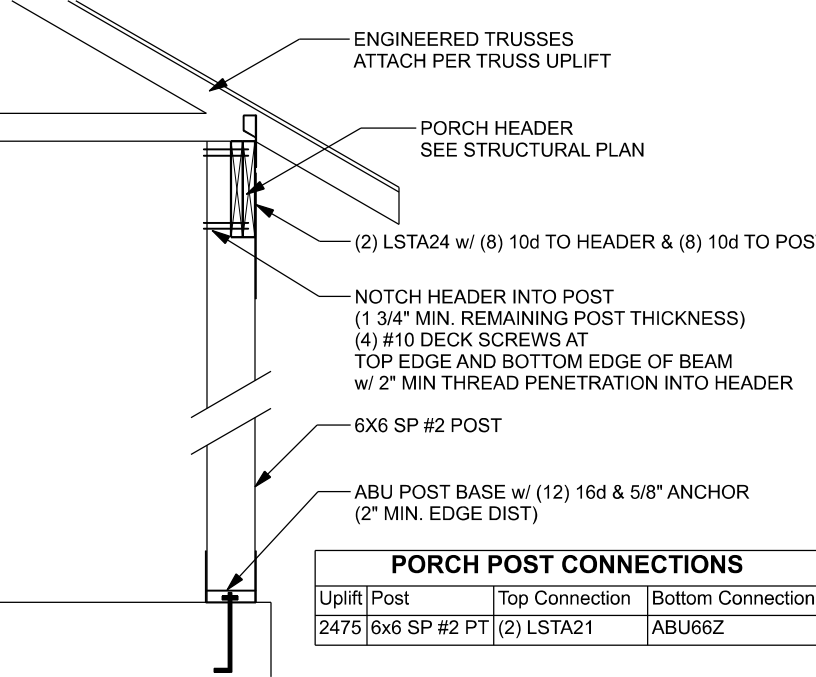
DESIGN	MAX. SPANS FOR SPF #2	BASED ON WFCM TABLE A-3.239
WIND SPEED	(1) 2x4 (2) 2x4 (1) 2x6 (2) 2x6	
130 MPH EXP. C	5'-2" 7'-9" 7'-7" 11'-3"	FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	4-8d@1 1/2"	4-8d@1 1/2"
415	280	H3	5-8d@1 1/2"	5-8d@1 1/2"
575	495	H2.5A	9-8d@1 1/2"	9-10d@1 1/2"
1340	1015	H10A	9-10d@1 1/2"	9-10d@1 1/2"
720	620	LTS12-20	6-10d@1 1/2"	6-10d@1 1/2"
1000	860	MTS12-30	7-10d@1 1/2"	7-10d@1 1/2"
1450	1345	MTS20-30	12-10d@1 1/2"	12-10d@1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	805	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stewall	To Stud / Post	Anchor
1825	1800	DT122	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HT14	18-16d@2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DT122	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HT14	18-16d@2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stewall	To Post	Anchor
2200	ABU44	ABU44	12-16d	5/8"x12" Drill & Epoxy
2300	ABU66	ABU66	12-16d	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200	ABU44	ABU44	12-16d	5/8"x12" Drill & Epoxy
2300	ABU66	ABU66	12-16d	5/8"x12" Drill & Epoxy

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WIND LOADS, 130 MPH EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH), STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)	
(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

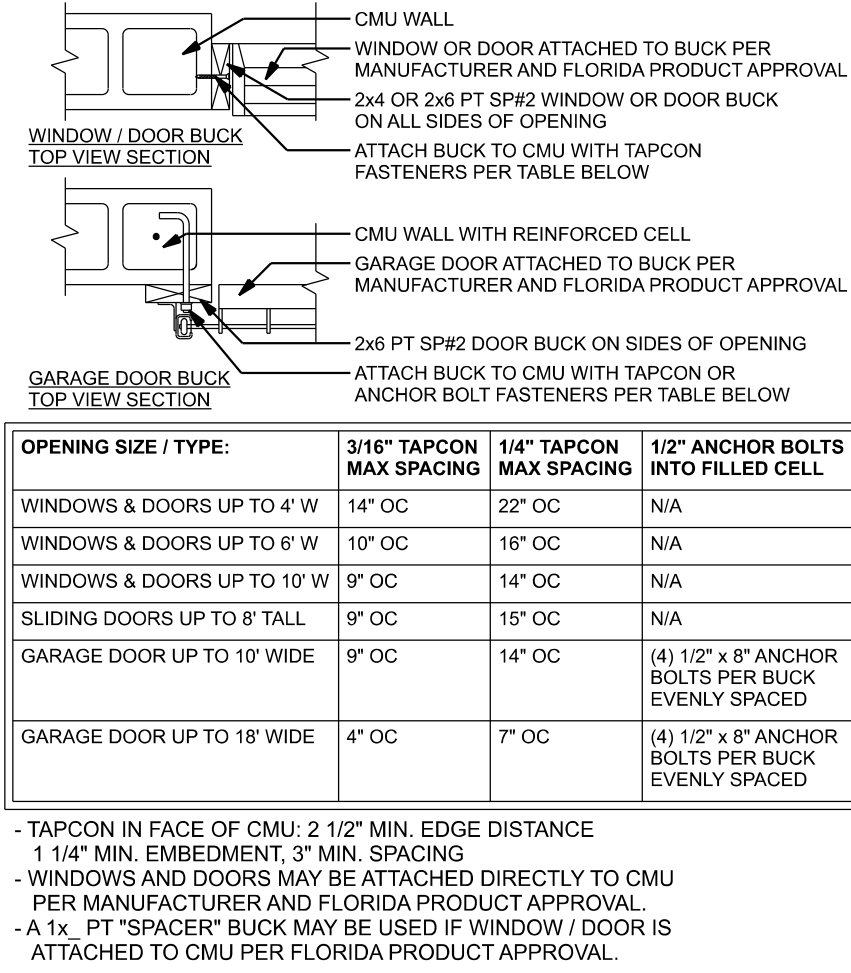
	SP #2	Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0



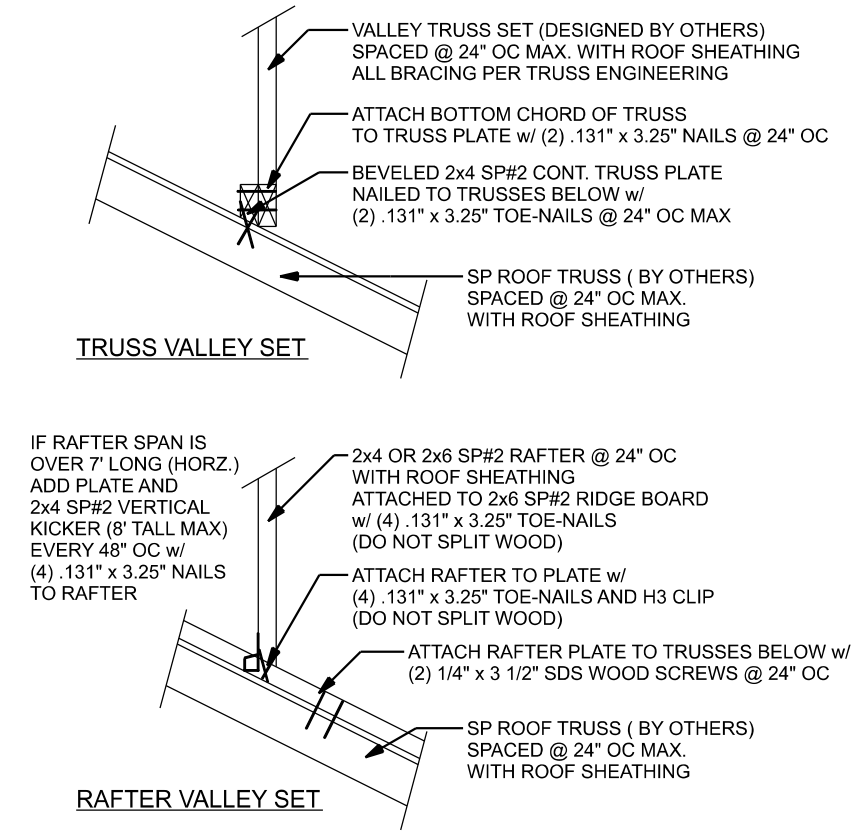
POSTS CUT FLUSH WITHOUT NOTCH - BRACE TO PREVENT ROTATION:
EAVE / HIP ROOF:
- STRAP TRUSS w/ H2.5A OR EQUAL TO EACH SIDE OF HEADER (FRONT & BACK)
- USE LONGER STRAP AS NEEDED IF TOP PLATES ARE INSTALLED
- (2) LSTA24 16-10d OR (2) MTS20 14-10d ON FRONT AND REAR OF POST TO HEADER
- ATTACH HEADER TO POST w/ (8) 131" x 3.25" TOE-NAIL
- SILE END
- EXTEND GABLE SHEATHING TO BOTTOM OF HEADER NAILED WITH 8d NAILS @ 6" MIN. INTO TRUSS BOTTOM CHORD AND INTO BEAM AT 1" MIN. FROM TOP & BOTTOM EDGE
- ATTACH HEADER TO POST w/ (8) 131" x 3.25" TOE-NAIL

TYP. PORCH POST

ONE STORY WOOD



DOOR & WINDOW BUCK ATTACHMENT



VALLEY SET FRAMING DETAIL

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER IS FULLY SATISFIED ALL THE REQUIRED REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. THE BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTION LOADS ON THE BUILDING STRUCTURE. STRAP 2x6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2x8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN
FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)
CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F_c = 2500 PSI.
WELDED WIRE REINFORCED SLAB: 6" x 6" W14 x W14, F_y = 88KSI, WELDED WIRE REINFORCEMENT FABRIC (W14) CONFORMING TO ASTM A193. LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT FIBER LENGTH 1/2 INCH TO 3 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS.
FIBER REINFORCED CONCRETE: FIBER REINFORCEMENT SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER IS FULLY SATISFIED ALL THE REQUIRED REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. THE BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTION LOADS ON THE BUILDING STRUCTURE. STRAP 2x6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2x8 RAFTERS 700 LB EACH END.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5:1 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT OUT CWT OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F_y = 40 KSI. ALL LAP SPACED 40" DB (25" FOR #5 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACES IN ACCORDANCE WITH ACI 318-16, UNO.
STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NOT LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

MASONRY NOTE:
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTION TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls F _m = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, hollow medium surface finish, 8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, F _y = 40 ksi, Lap splices min. 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class GR0, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

BUILDER'S RESPONSIBILITY:
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN CRITERIA & LOADS:		
BUILDING CODE	17TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)	
CODE FOR DESIGN LOADS	ASCE 7-16	
WINDLOADS		
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH	
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C	
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I	
RISK CATEGORY	II	
ENCLOSURE CLASSIFICATION	ENCLOSED	
INTERNAL PRESSURE COEFFICIENT	0.18	
ROOF ANGLE	7-45 DEGREES	
MEAN ROOF HEIGHT	30 FT	
C&D DESIGN PRESSURES SEE TABLE		
FLOOR LOADING		
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD	
SLEEPING ROOMS	30 PSF LIVE LOAD	
ROOF LOADING		
FLAT OR < 4:12	20 PSF LIVE LOAD	
4:12 TO < 12:12	16 PSF LIVE LOAD	
12:12 & GREATER	12 PSF LIVE LOAD	
SOIL BEARING CAPACITY 1500 PSF		
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE	
COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)		
EFFECTIVE WIND AREA (FT2)	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6(V/assd) -27.8(V/assd)	+25.6(V/assd) -34.2(V/assd)
0 - 20	+42.6(V/ult) -46.2(V/ult)	+42.6(V/ult) -57.0(V/ult)
GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)		
9x7 GARAGE DOOR	+22.6(V/assd) -25.5(V/assd)	
16x7 GARAGE DOOR	+21.7(V/assd) -24.1(V/assd)	

Blake Construction

Ronnie Shuman Ros

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 17th Edition Florida Building Code Residential (2020) to the best of my knowledge.

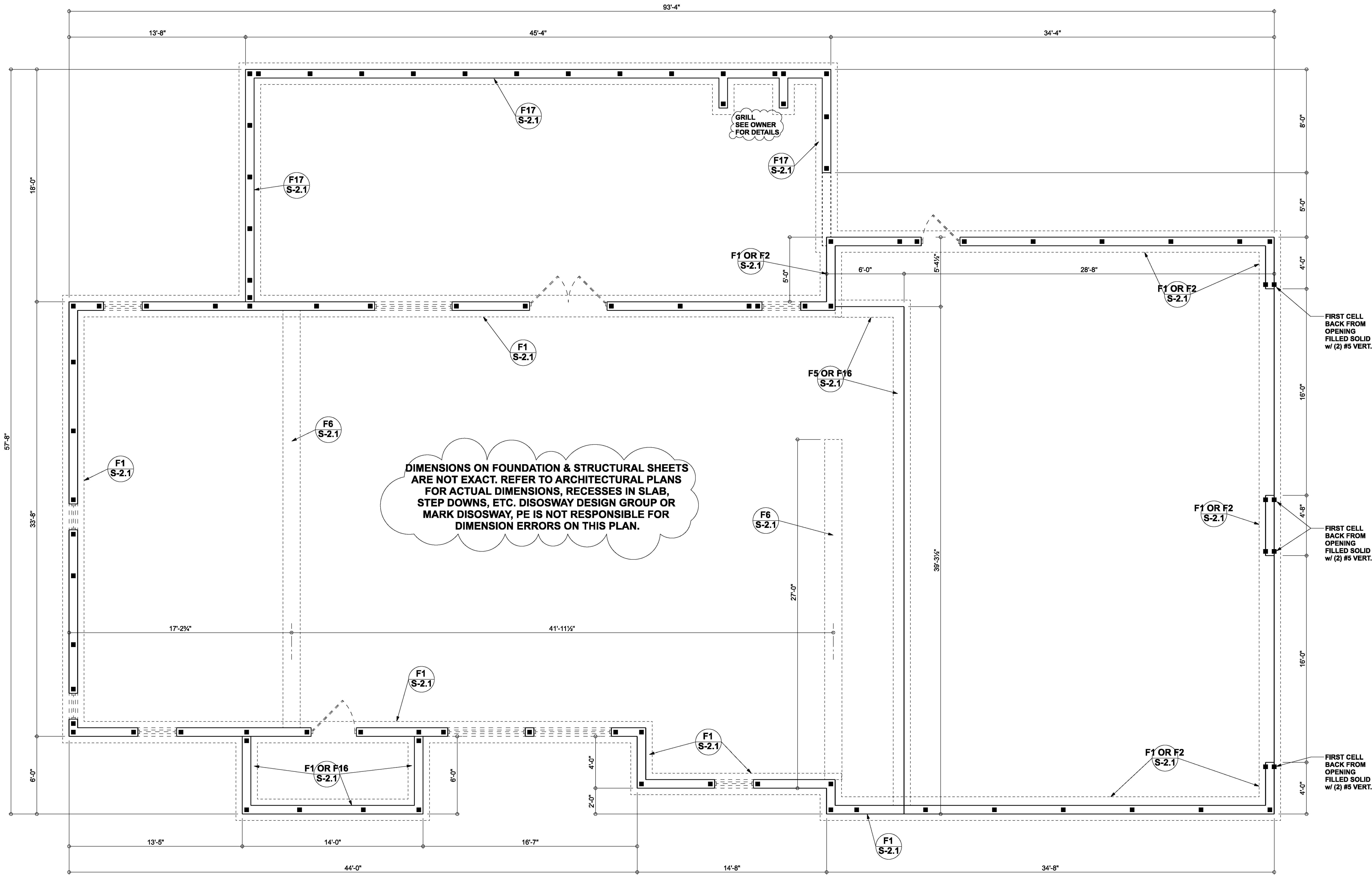
MARK DISOWAY P.E. 53915

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Thursday, August 26, 2021

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Lake City, Florida 32025
386.754.5419
disowaydesign@gmail.com

JOB NUMBER:
211179
S-1
OF 4 SHEETS



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

- FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING
- FN - 2 IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
- FN - 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL



Blake Construction

Ronnie Shuman Res

PROJECT ADDRESS:
205 SW Madison Court
Lake City, FL 32024

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53815

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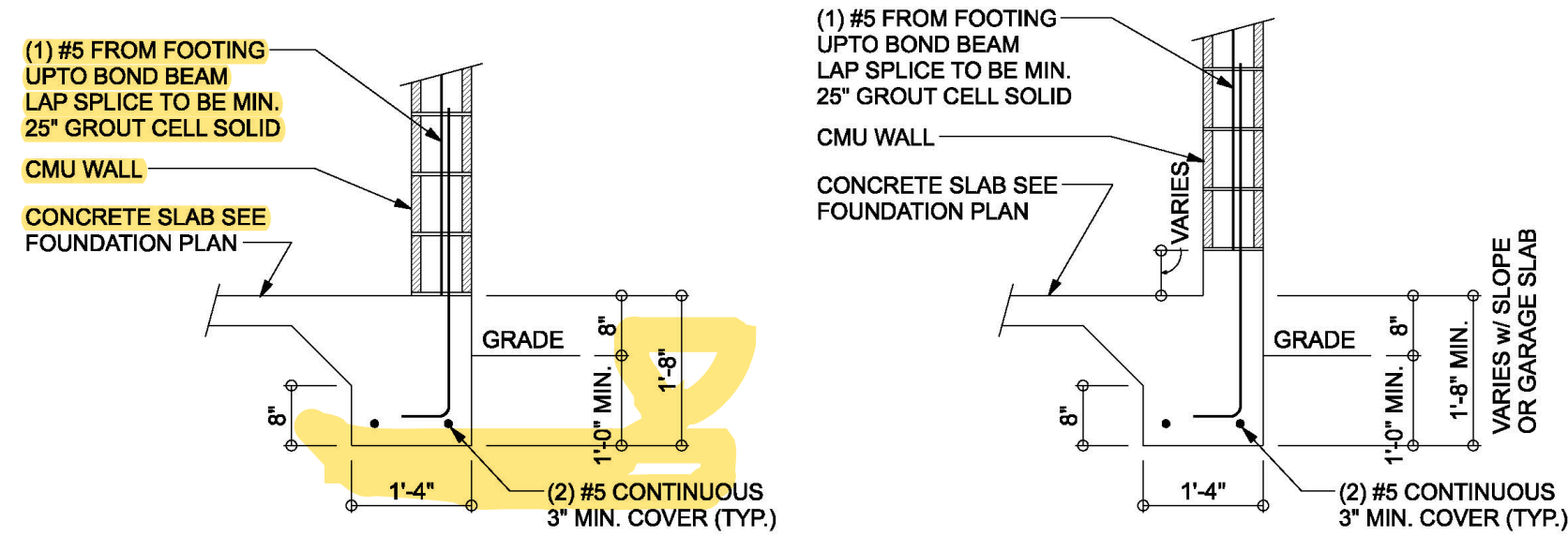


Thursday, August 26, 2021

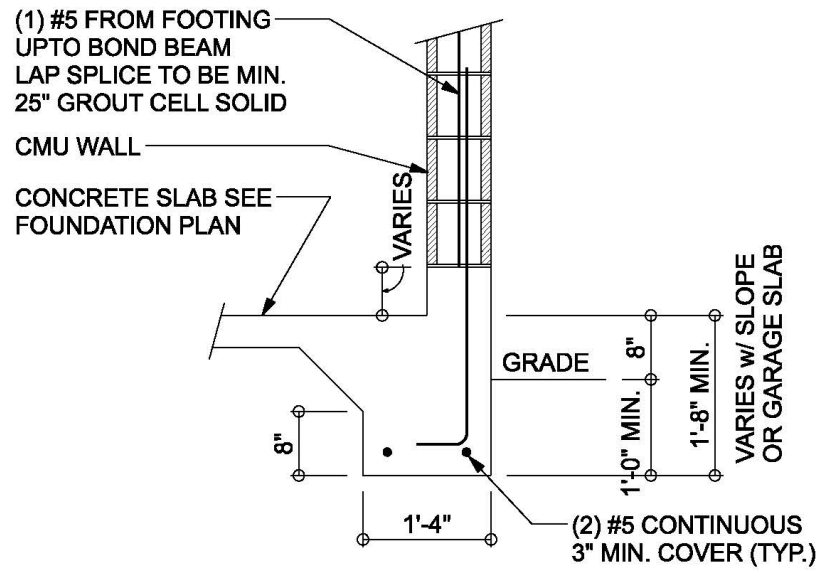
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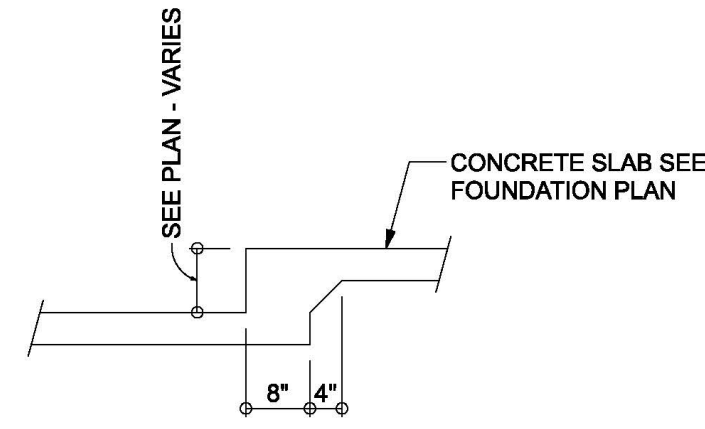
S-2
OF 4 SHEETS



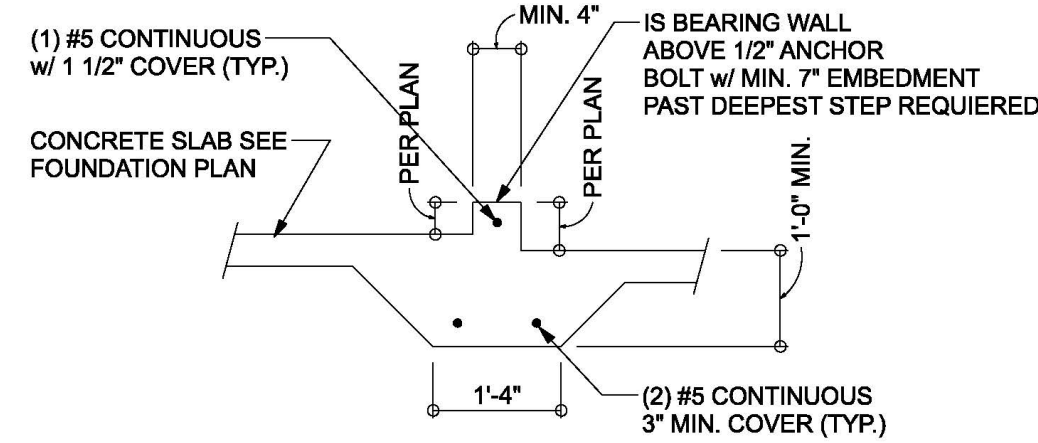
F1 MONOLITHIC FOOTING
SCALE: 1/2" = 1'-0"



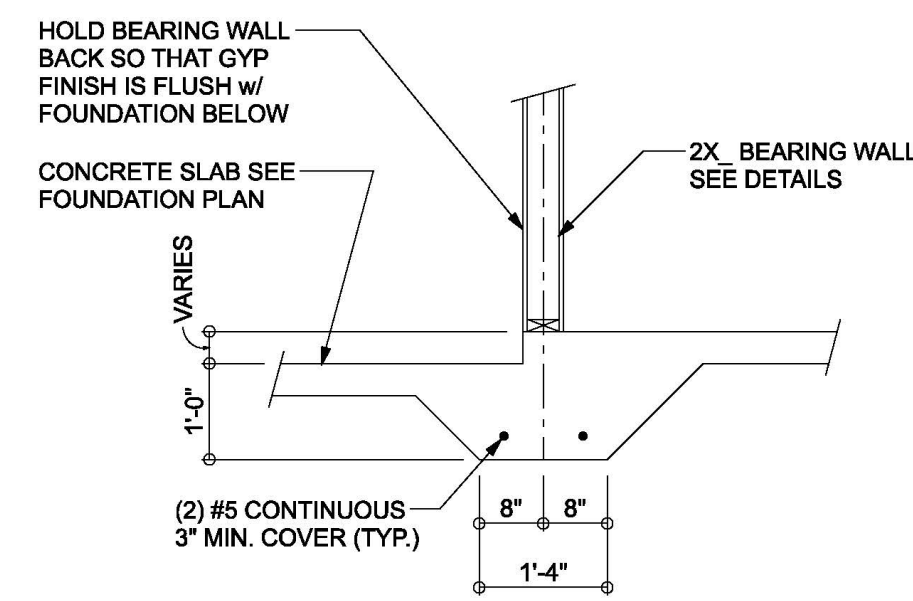
F2 GARAGE CURB FOOTING
SCALE: 1/2" = 1'-0"



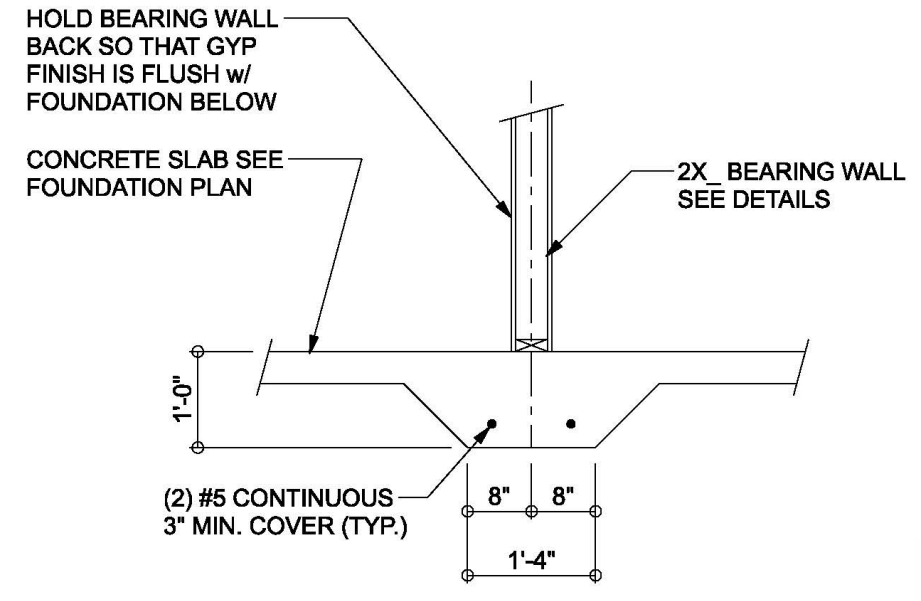
F3 NON-BEARING STEP DOWN
SCALE: 1/2" = 1'-0"



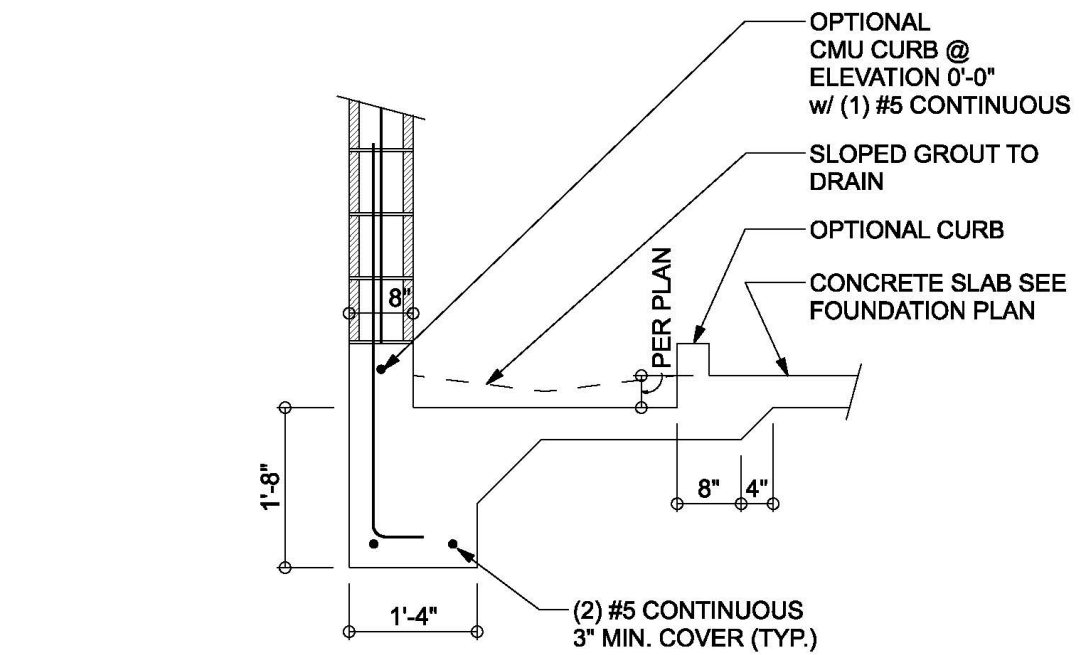
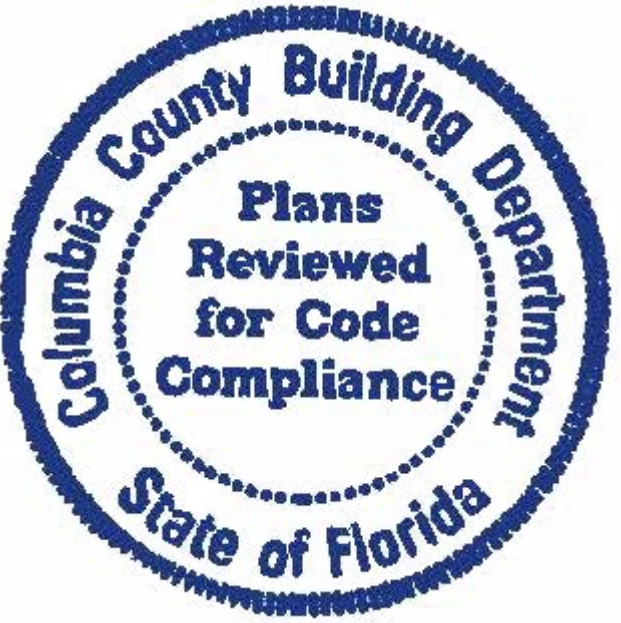
F4 DOUBLE STEP FOOTING
SCALE: 1/2" = 1'-0"



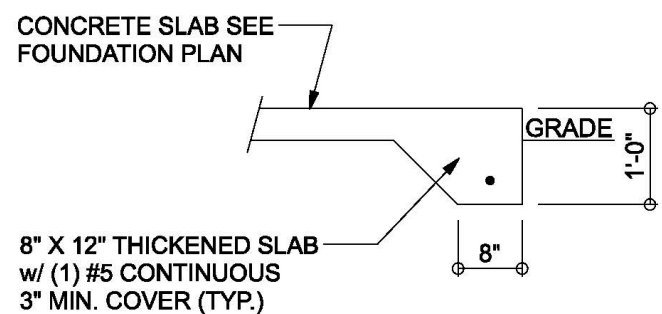
F5 STEP FOOTING BEARING
SCALE: 1/2" = 1'-0"



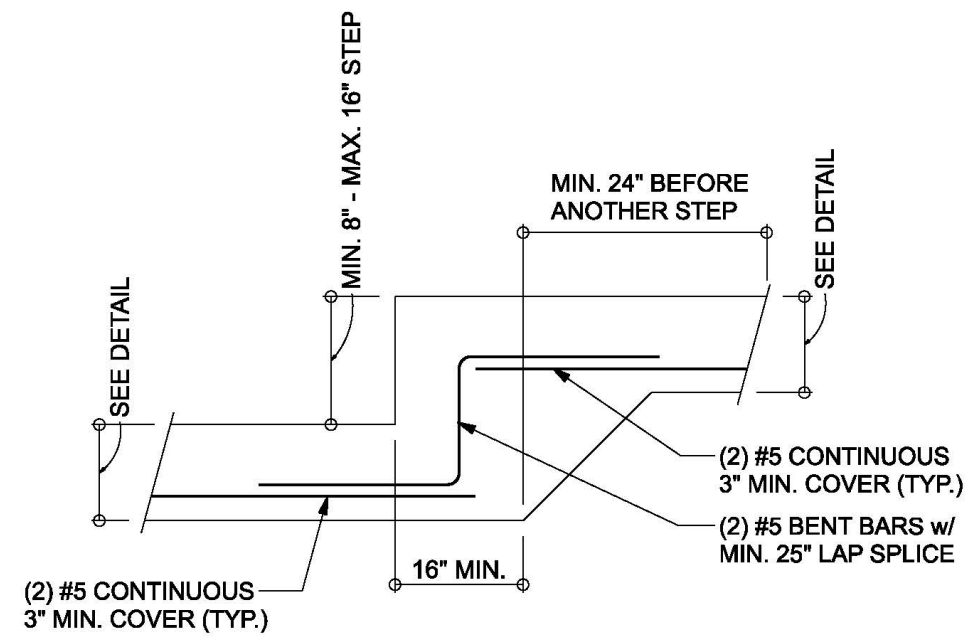
F6 INTERIOR BEARING FOOTING
SCALE: 1/2" = 1'-0"



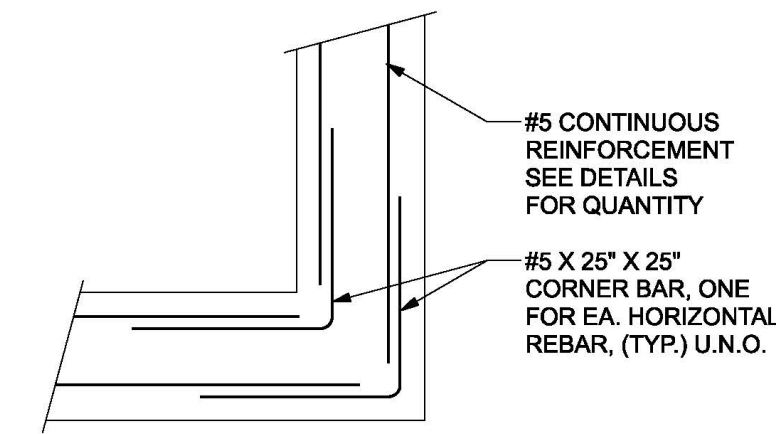
F7 FOOTING @ SHOWER @ MASONRY
SCALE: 1/2" = 1'-0"



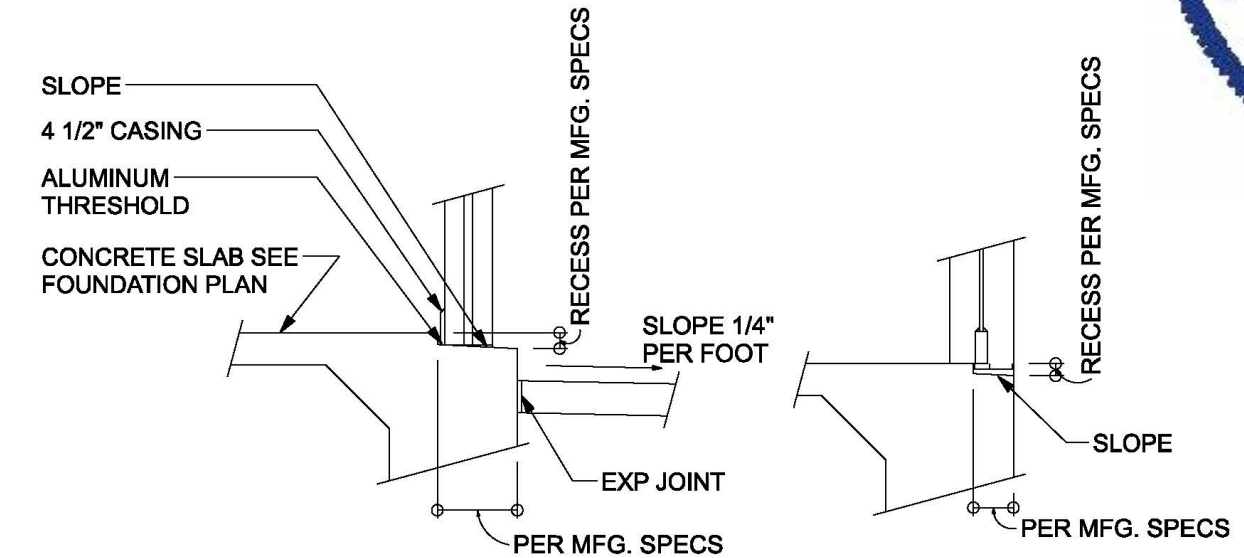
F8 THICKEND EDGE
SCALE: 1/2" = 1'-0"



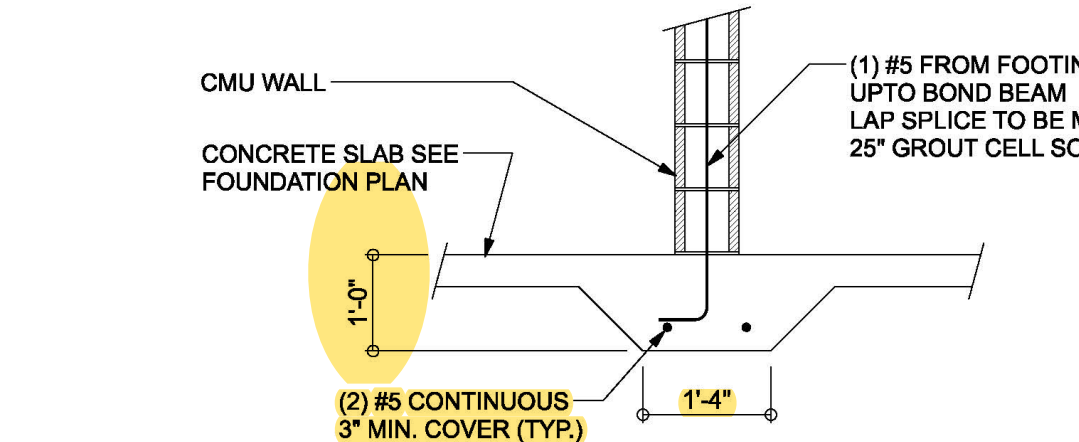
F9 (TYP.) STEP FOOTING DETAIL
SCALE: 1/2" = 1'-0"



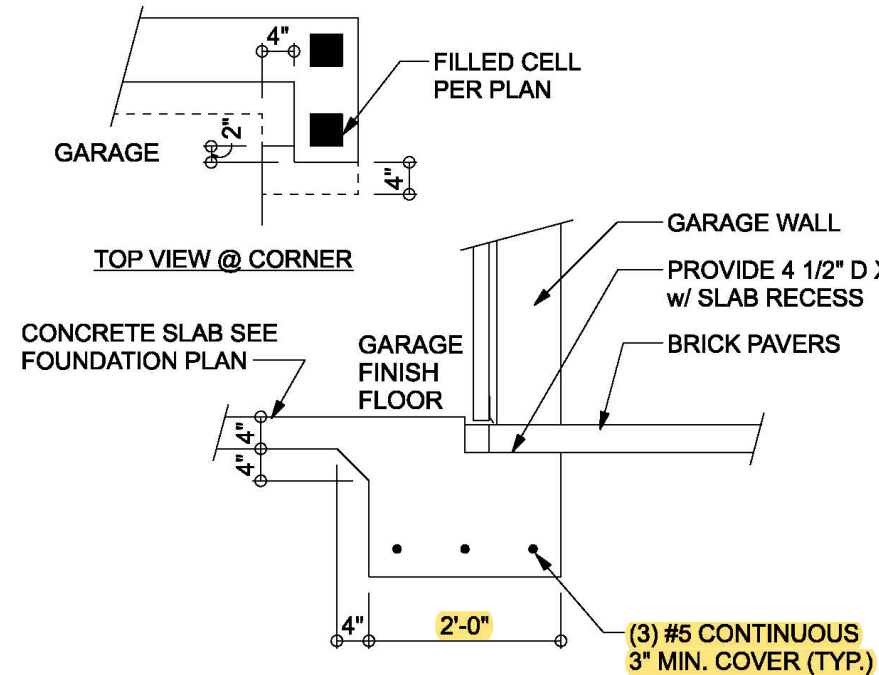
F10 (TYP.) CORNER BAR DETAIL
SCALE: 1/2" = 1'-0"



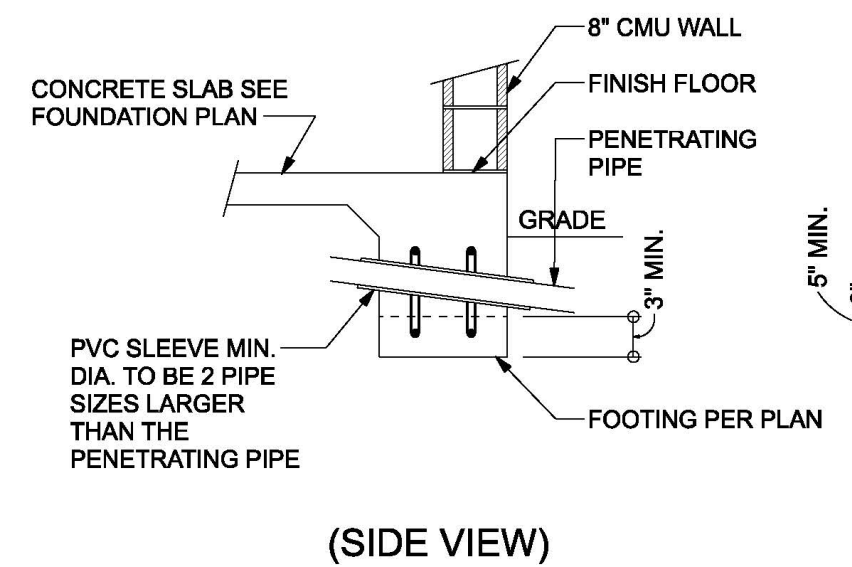
F11 EXTERIOR DOOR POURED SILLS
SCALE: 1/2" = 1'-0"



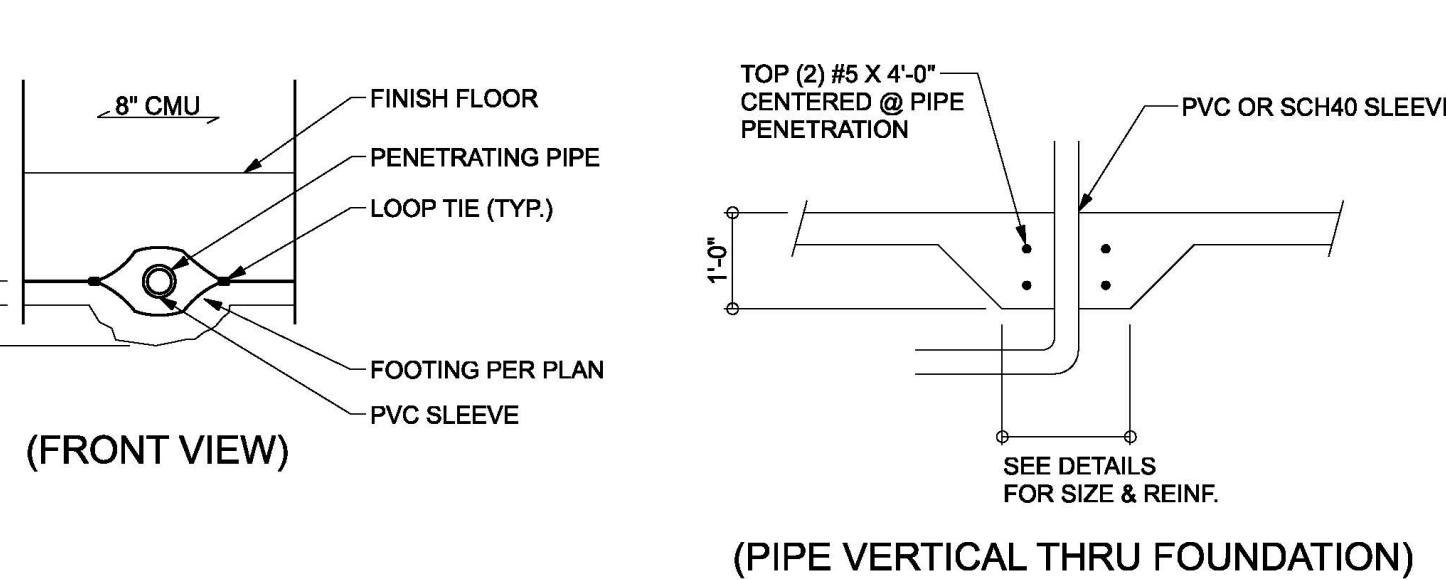
F12 INTERIOR BEARING FOOTING w/ CMU WALL
SCALE: 1/2" = 1'-0"



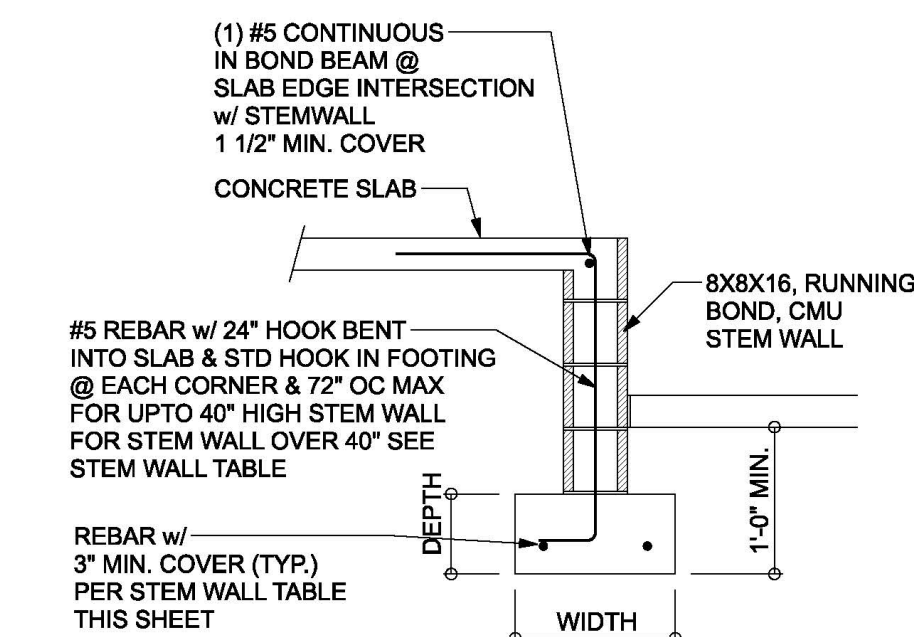
F13 THICKENED EDGE @ GARAGE DOOR
SCALE: 1/2" = 1'-0"



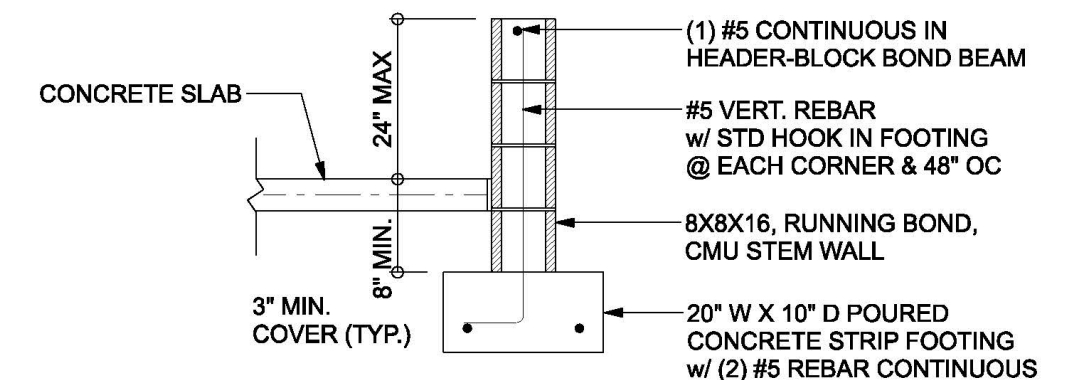
F14 TYPICAL FOUNDATION PENETRATIONS
SCALE: 1/2" = 1'-0"



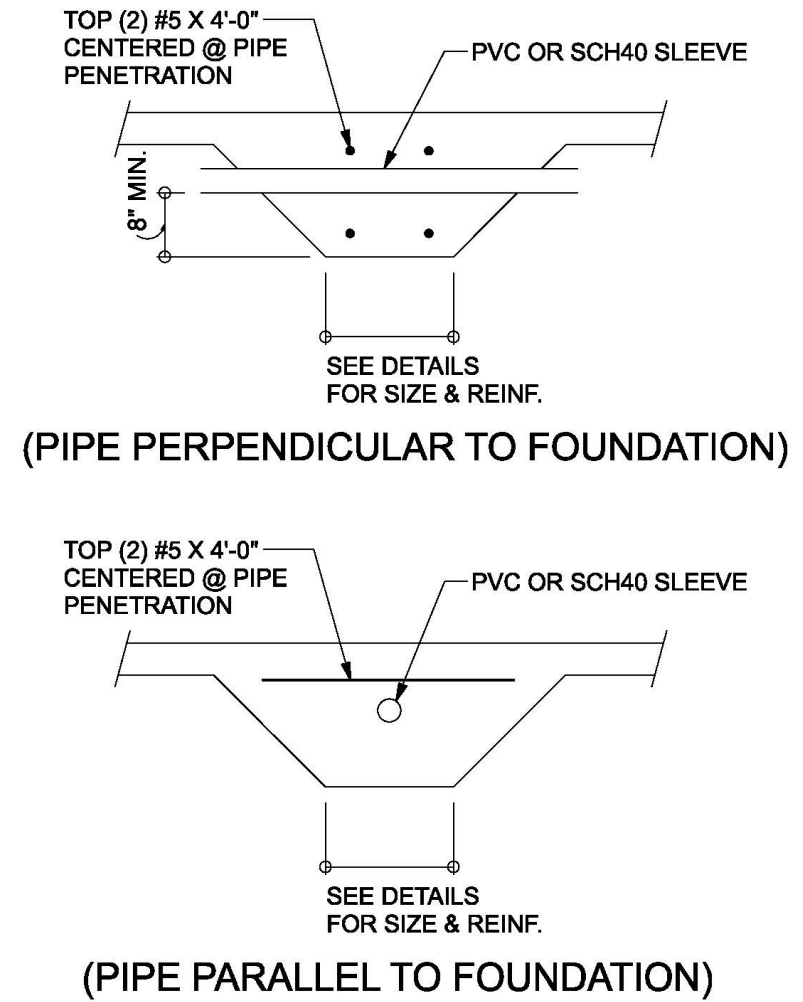
F15 STEM WALL FOOTING @ PORCH
SCALE: 1/2" = 1'-0"



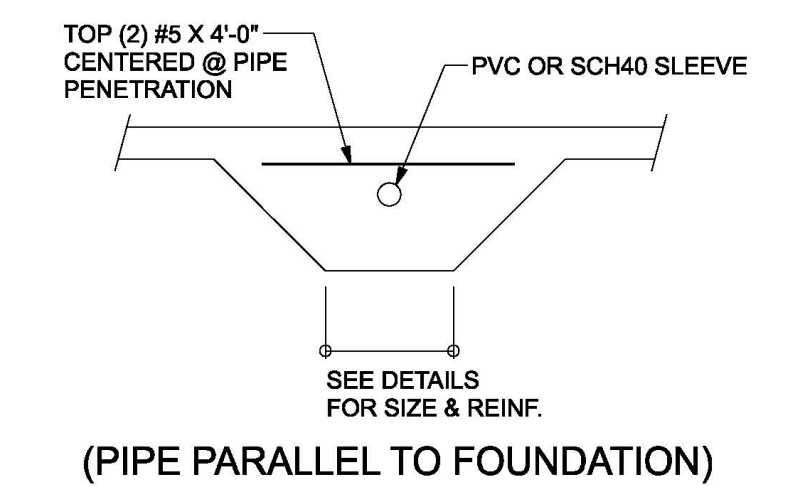
F16 STEM WALL @ GARAGE STEP DOWN
SCALE: 1/2" = 1'-0"



F17 STEM WALL CURB FOOTING @ SCREENED LANAI
SCALE: 1/2" = 1'-0"



(PIPE PERPENDICULAR TO FOUNDATION)



(PIPE PARALLEL TO FOUNDATION)

Blake Construction

Ronnie Shuman Res

PROJECT ADDRESS:
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DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915

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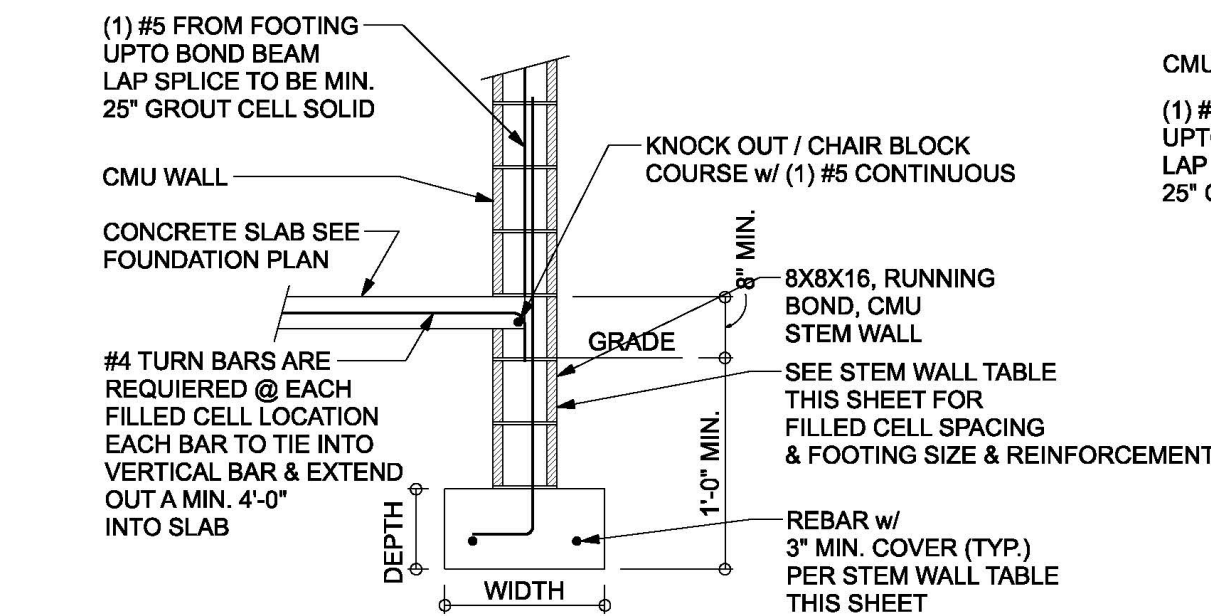


Thursday, August 26, 2021

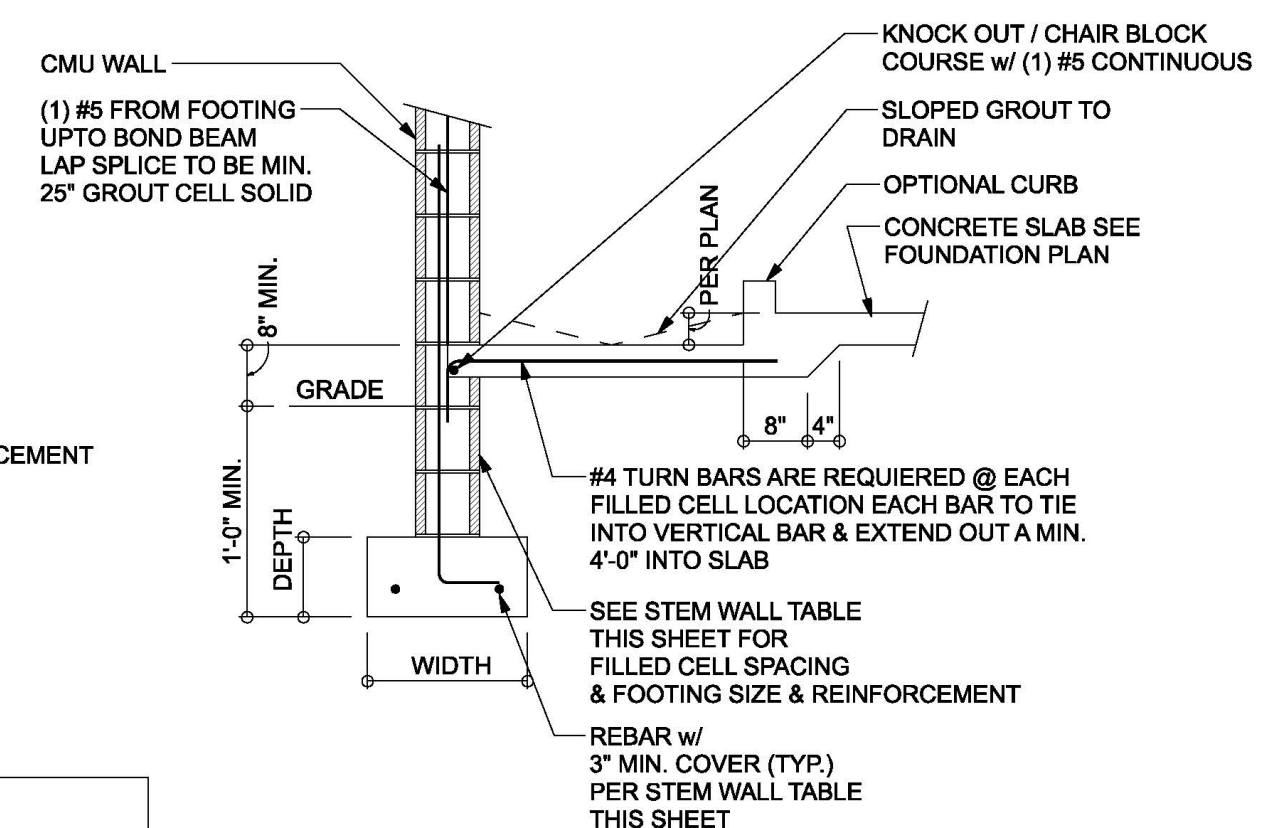
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JOB NUMBER:
211179

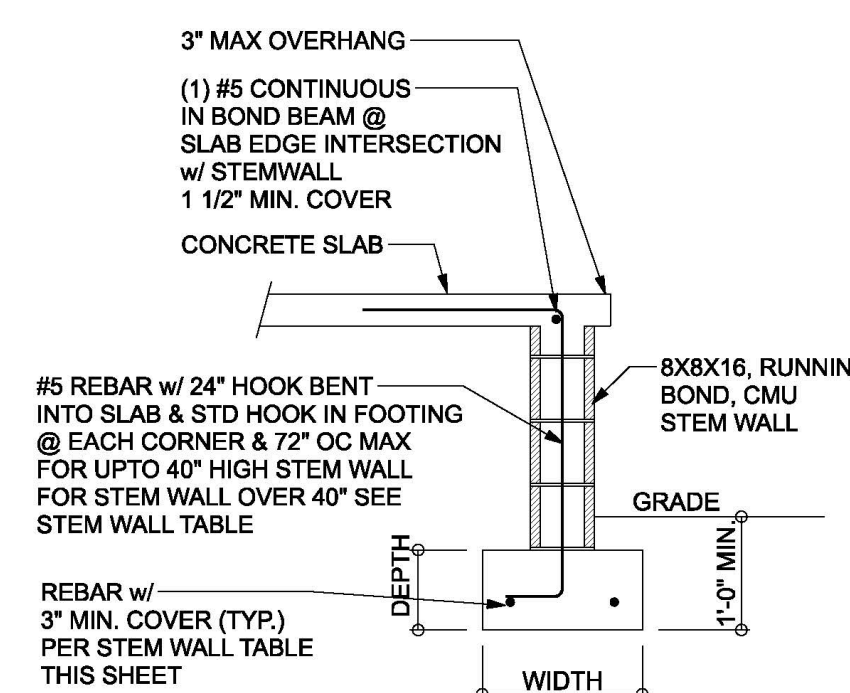
S-2.1
OF 4 SHEETS



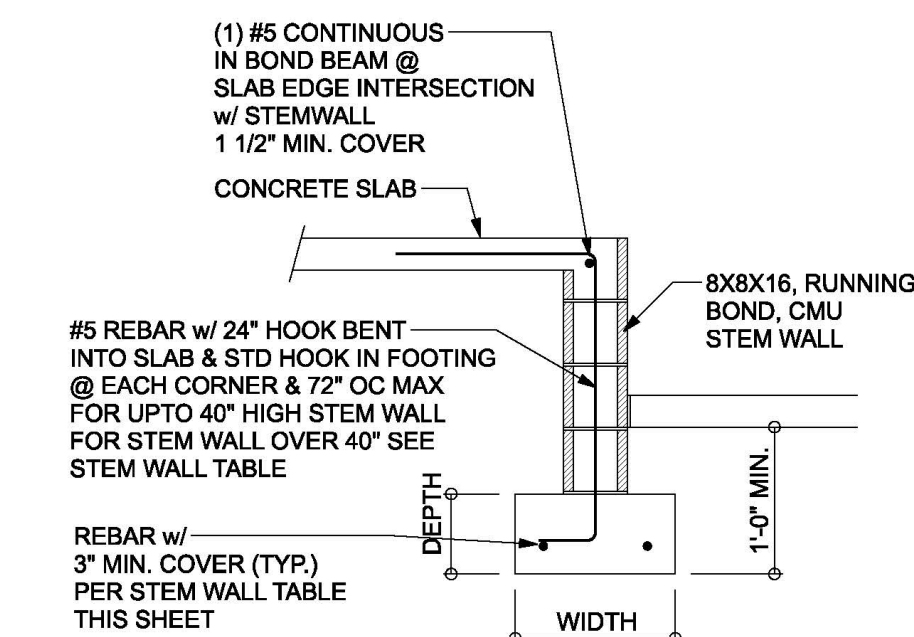
F1 STEM WALL FOOTING
SCALE: 1/2" = 1'-0"



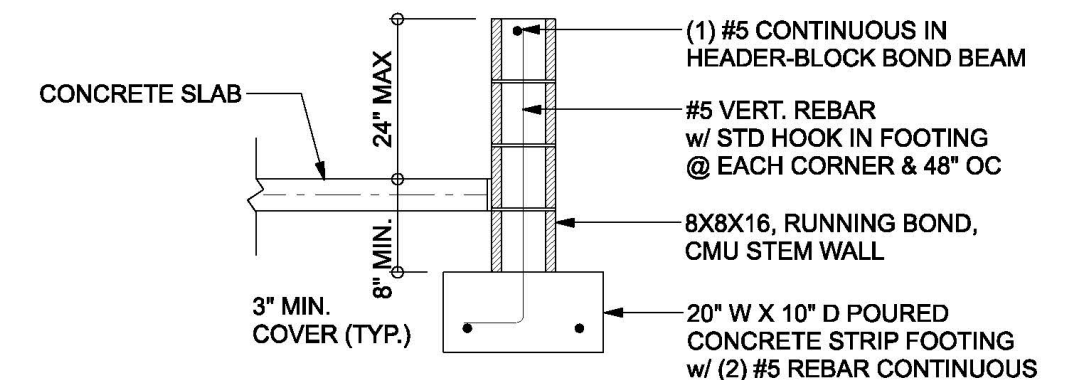
F7 FOOTING @ SHOWER @ MASONRY
SCALE: 1/2" = 1'-0"



F15 STEM WALL FOOTING @ PORCH
SCALE: 1/2" = 1'-0"



F16 STEM WALL @ GARAGE STEP DOWN
SCALE: 1/2" = 1'-0"



F17 STEM WALL CURB FOOTING @ SCREENED LANAI
SCALE: 1/2" = 1'-0"

STEM WALL TABLE

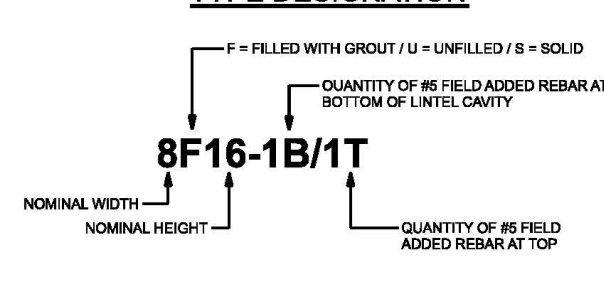
STEM WALL HEIGHT	FOOTING DIMENSION				NUMBER / SIZE OF REBAR IN FOOTING	MAX FILLED CELL SPACING (O.C.) IN STEM WALL
	1-STORY DEPTH	1-STORY WIDTH	2-STORY DEPTH	2-STORY WIDTH		
8" - 40"	10"	20"	10"	20"	(2) #5 REBARS FOR 1-STORY OR (3) #5 REBARS FOR 2-STORY	MATCH FILLED CELL SPACING PER PLAN
48" - 64"	10"	20"	10"	20"	(2) #5 REBARS FOR 1-STORY OR (3) #5 REBARS FOR 2-STORY	40"
72" - 80"	10"	30"	10"	30"	(3) #5 REBARS FOR 1-STORY & 2-STORY	32"

NOTE:
ALL STEM WALL FOUNDATIONS
OVER 3'-0" IN HEIGHT TO BE POURED SOLID

OPTIONAL STEM WALL FOUNDATION



TYPE DESIGNATION



MATERIALS

1. Fc precast lintel = 3500 psi
2. Fc prestressed lintel = 6000 psi
3. Grout per ASTM C476 Fc = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump
4. Concrete Masonry Units (CMU) per ASTM C90 minimum net area compressive strength = 1900 psi
5. Rebar per ASTM A615 grade 60
6. Prestressing strand per ASTM A416 grade 270 low relaxation
7. Mortar per ASTM C270 type M or S

GENERAL NOTES

1. Provide full mortar bed and head joints.
2. Shore filled lintels as required.
3. Installation of lintel must comply with the architectural and/or structural documents.
4. Lintels are manufactured with 5 1/2" long notches at the ends to accommodate vertical del reinforcement and grouting.
5. All lintels meet or exceed L/250 deflection, except lintels 17'-4" and longer with a nominal height of 8' meet or exceed L/180 deflection.
6. Bottom field added rebar to be located at the bottom of the lintel cavity.
7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage.
8. Cast-in-place concrete may be provided in composite lintel in lieu of concrete masonry units.
9. Safe load rating based on national design analysis per ACI 318 and ACI 308.
10. Precast Approvals: Miami-Dade County, Florida No. 03-0006-05
11. The exterior surface of lintels installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-208 or other approved coating.
12. Lintels loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:
Applied vertical load + Applied horizontal load
Safe vertical load + Safe horizontal load ≤ 1.0
13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the lintel. See detail at right.

SAFE LOAD TABLE NOTES

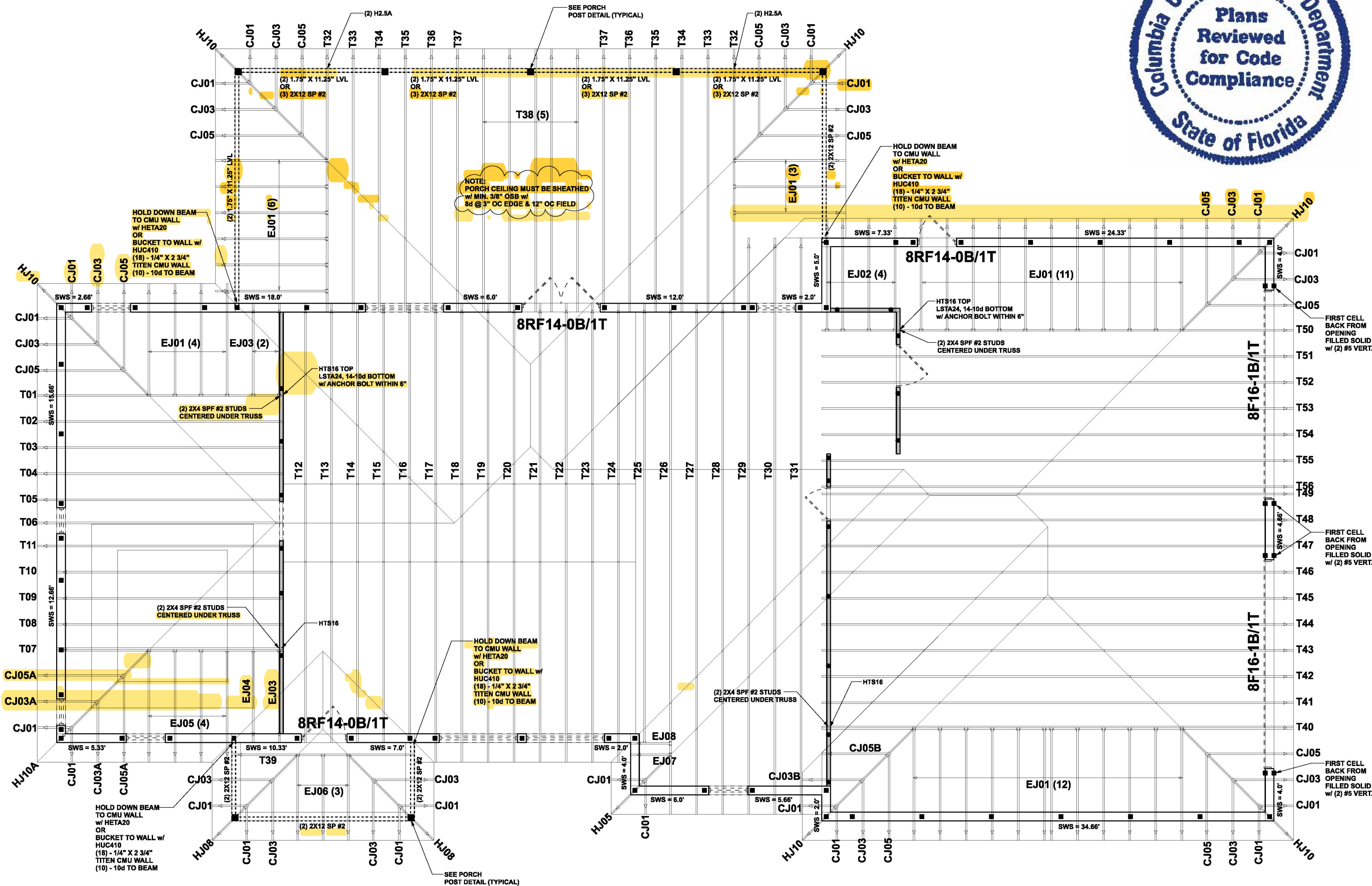
1. All values based on minimum 4 inch nominal bearing.
2. N.R. = Not Rated
3. Safe loads are superimposed allowable loads.
4. Safe loads based on grade 40 or grade 60 field rebar.
5. One #7 rebar may be substituted for two #5 rebars in 8" lintel only.
6. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at c-d away from face of support.
7. For composite lintel heights not shown, use safe load from next lower height shown.
8. For lintel lengths not shown, use safe load from next longest length shown.
9. All safe loads in units of pounds per linear foot.
10. All safe loads based on simply supported spans.
11. The number in the parenthesis indicates the percent reduction for grade 40 field added rebar.
Example 7'-6" lintel type 8F32-1B safe gravity load = 6472(0.0462) (15)(40.071); w/ 15% reduction 6472 * (.85) = 5501 plf

SAFE GRAVITY LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

LENGTH	TYPE	RUL	SAFE LOAD - POUNDS PER LINEAR FOOT									
			8F8-03	8F12-03	8F16-03	8F20-03	8F24-03	8F28-03	8F32-03	8F36-03	8F40-03	8F44-03
2'-10" (34")	PRECAST	2231	3069	4055	5113	7547	8874	10384	11809			
3'-4" (42")	PRECAST	2231	3069	4055	5113	7547	8874	10384	11809			
4'-0" (48")	PRECAST	1966	2567	3715	4820	6867	8054	9602	10861			
4'-4" (54")	PRECAST	1590	2093	3069	4055	5113	7547	8874	10384	11809		
5'-0" (60")	PRECAST	1217	1590	2193	2831	3703	4376	5400	6254			
5'-10" (70")	PRECAST	1062	1438	1999	2690	3123	3696	4349				
6'-4" (78")	PRECAST	908	1238	1717	2340	2777	3127	3538				
7'-4" (88")	PRECAST	743	1011	1728	2332	2705	2888	3191	3552			
8'-4" (102")	PRECAST	584	699	1160	1625	2064	2436	2818	3302			
10'-0" (120")	PRECAST	478	535	880	1247	2033	2777	3183	3538			
11'-4" (136")	PRECAST	362	392	545	795	1294	1884	2193	2602			
12'-0" (144")	PRECAST	337	347	547	795	1294	1884	2193	2602			
13'-4" (160")	PRECAST	296	471	755	1075	1428	1838	2316	2883			
14'-0" (168")	PRECAST	279	424	706	1002	1326	1687	2127	2630			
14'-4" (172")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
15'-4" (184")	PRESTRESSED	N.R.	412	710	1020	1733	2058	2330	2613			
17'-4" (208")	PRESTRESSED	N.R.	302	538	850	1326	1809	1949	2047			
19'-4" (232")	PRESTRESSED	N.R.	225	418	730	1207	1582	1515	1518			
21'-4" (256")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
22'-0" (264")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
24'-0" (288")	PRESTRESSED	N.R.	165	315	550	784	1047	1285	1399			
			N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.			
			128	250	450	654	864	1092	1222			

SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-LINTELS

LENGTH	TYPE	RUL	SAFE LOAD - POUNDS PER LINEAR FOOT									
			8F8-03	8F12-03	8F16-03	8F20-03	8F24-03	8F28-03	8F32-03	8F36-03	8F40-03	8F44-03
4'-4" (52")	PRECAST	1635	1581	2699	3226	3636	4060	4479	4893			
4'-8" (56")	PRECAST	1494	1398	2463	2992	3398	3826	4246	4661			
5'-4" (68")	PRECAST	868	626	1770	1716	2277	2639	3452	3896			
5'-10" (70")	PRECAST	810	1107	2481	4067	4360	4900	5471	6011			
6'-4" (76")	PRECAST	797	609	1693	1902	2124	2448	2714	3010			
7'-4" (88")	PRECAST	669	735	1400	2469	2778	3143	3523	3923			
8'-4" (100")	PRECAST	411	486	969	1968	2253	2729	3201	3748			



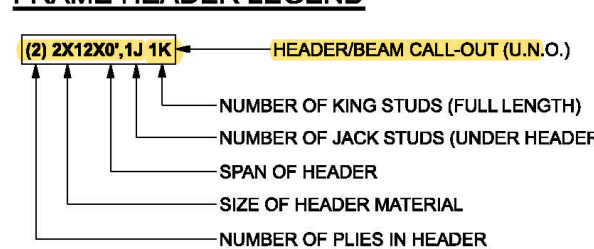
STRUCTURAL PLAN

SCALE: 3/16" = 1'-0"

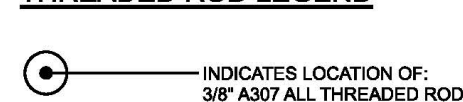
STRUCTURAL PLAN NOTES

- SN-1 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS.
- SN-2 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BC51-103, BC51-81, BC51-82, & BC51-83. BC51-81, BC51-82, & BC51-83 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE.

FRAME HEADER LEGEND



THREADED ROD LEGEND



ACTUAL vs REQUIRED SHEARWALL

	TRANSVERSE	LONGITUDINAL
ACTUAL	51.9'	143.3'
REQUIRED	36.0'	28.0'

UNLESS NOTED OTHERWISE ON STRUCTURAL PLANS

1. USE HETA16 CMU TO TRUSS
2. USE H2.5A FRAME TO TRUSS
3. ALL LENTELS TO BE: 8F16-0B/1T
4. ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X12 SP #2
5. ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE

ENGINEERED TRUSSES

- ATTACH PER TRUSS UPLIFT TO CMU WALLS:
- HETA16 FOR UP TO 1350 LB UPLIFT
- (2) HETA16 FOR UP TO 2035 LB UPLIFT
- MGT FOR UP TO 3965 LB UPLIFT TO FRAME WALLS:
- (1) H2.5A FOR UP TO 495 LB UPLIFT OR
- (2) H2.5A FOR UP TO 990 LB UPLIFT
- SEE CONNECTOR TABLE OR SIMPSON BOOK FOR ADDITIONAL OPTIONS

Blake Construction

Ronnie Shuman Ros

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205 SW Madison Court
Lake City, FL 32024

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Thursday, August 26, 2021

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JOB NUMBER:
211179

S-3
OF 4 SHEETS

CONNECTIONS, WALL & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER. BUILDERS FIRST SOURCE JOB #2802469