DATE <u>07/3</u>	1/2008	Columbia County B This Permit Must Be Prominently Posted		etruction	PERMIT 000027224
APPLICANT	DATRICK	WILSON	PHONE	352 206-5459	00002/224
ADDRESS	6800	SOUTHPOINT PARKWAY	– JACKSONVILLE	332 200-3437	FL 32216
OWNER	·	DA HOMES	PHONE	904 296-1490	
ADDRESS	278	SW TIMBER RIDGE DRIVE	LAKE CITY	3.00.0000000000000000000000000000000000	FL 32024
CONTRACTO		EODORE BROCK	PHONE	407 227-3504	
LOCATION O	F PROPER	TY 90W, TL ON 247S, TR ON 252B	, TL ON TIMBER RIDGE	E, 5TH ON	
		LEFT			
TYPE DEVEL	OPMENT	SFD,UTILITY ES	TIMATED COST OF CO	NSTRUCTION	89950.00
HEATED FLO	OR AREA	1312.00 TOTAL ARI	EA 1799.00	HEIGHT	STORIES 1
FOUNDATIO	N CONC	C WALLS FRAMED I	ROOF PITCH 6/12	FLO	OOR SLAB
LAND USE &		RSF-2	4	. HEIGHT	
Minimum Set l	Back Requir	rments: STREET-FRONT 25.00	REAR	15.00	SIDE 10.00
NO. EX.D.U.	0	FLOOD ZONE X	DEVELOPMENT PERM	MIT NO.	- ×
PARCEL ID	10-4S-16-	-02856-120 SUBDIVISIO	ON TIMBERLANDS		
LOT 20	BLOCK	PHASE UNIT	-	AL ACRES 0.5	0
000001647		CD CUASCAMA		2/1/	
000001647 Culvert Permit	No.	Culvert Waiver Contractor's License Nur	mher Met	Applicant/Owner/O	Contractor
CULVERT	NO.	08-400 BK	,	Applicant/Owner/C VR	Y
Driveway Conr	nection			roved for Issuance	1
COMMENTS:	ELEVATI	ION CONFIRMATION LETTER REQUIRED	AT SLAB, MFE AT 96'	PER	
PLAT			*		
				Check # or Ca	sh 918688
		FOR BUILDING & ZONIN	NG DEPARTMENT	ONLY	(footer/Slab)
Temporary Pov	ver	Foundation		Monolithic	(Toolet/Stab)
		date/app. by	date/app. by	_	date/app. by
Under slab roug	gh-in plumb			Sheathing/N	Jailing
Framing		date/app. by	date/app. by	_	date/app. by
	date/ap	Rough-in plumbing all pp. by	bove slab and below wood	floor	date/app. by
Electrical roug		Heat & Air Duct	,	Doni hoom (Lintal)	
		date/app. by	date/app. by	Peri. beam (Lintel)	date/app. by
Permanent power		c.O. Final		Culvert	
M/H tie downs,		lectricity and plumbing	date/app. by	Pool	date/app. by
Reconnection		date/app Pump pole	o. by Utility Pol		date/app. by
		date/app. by date	/app. by	date/app. by	_
M/H Pole dat	e/app. by	Travel Trailer	late/app. by	Re-roof	date/app. by
BUILDING PEI	RMIT FFF	\$ 450.00 CERTIFICATION FE	E.\$ 9.00	SURCHARGE	FEE \$ 9.00
MISC. FEES \$					
,					
FLOOD DEVEL			CLERKS OFFICE		LFEE 568.00
	OFFICE <	Jae 18 Cole	OLEDWO OFFICE	/ / X	/

PERMIT

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0806-54 Date Received 477 By Dermit # 1647/27224
Zoning Official Date 14.07.08 Flood Zone FEMA Map # NA Zoning KSF, 7
Land Use Levation NA MFE River NA Plans Examiner WD Date 7/3/08
Comments Elevation Confirmation Latter at slab
SNOC LEH Deed or PA Site Plan State Road Info Parent Parcel #
□ Dev Permit # □ In Floodway □ Letter of Authorization from Contractor
□ Unincorporated area □ Incorporated area □ Town of Fort White □ Town of Fort White Compliance letter
Septic Permit No. <u>08-400</u> Fax <u>(904)-332-6367</u>
Name Authorized Person Signing Permit Theodore C. Brock, Pargick Wilson : Phone (904)-296-1490
Address 6800 Southpoint Pkwy. #300 Jacksonville, FL 32216
Owners Name Maronda Homes Inc. of Florida Phone (904)-296-1490
911 Address 278 SW Timber Ridge Drive Lake City, FZ, 32024
Contractors Name Theodore C. Brock Phone (407)-227-3504
Address6800 Southpoint Pkwy. #300 Jacksonville, FL 32216
Fee Simple Owner Name & AddressN/A
Bonding Co. Name & Address N/A
Architect/Engineer Name & Address Tomas Ponce 4005 Maronda Way Sanford, FL 32771
Mortgage Lenders Name & Address Bank of America 250 Park Ave. S. #400 Winter Park, FL 32789
Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progress Energy
Property ID Number 10-45-10-02850-120 Estimated Cost of Construction 50,745.00
Subdivision Name Timberlands Lot 20 Block 1 Unit Phase
Driving Directions Hwy 90, Left on 247 South; Right on 252B; Left on Timber Ridge Daive; 5th Lor
on left.
Number of Existing Dwellings on Property
Construction of Residential Single Family Dwelling Total Acreage .50 Lot Size NA
Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height NA
Actual Distance of Structure from Property Lines - Front 500 Side 450 (L) Side 45 (R) Rear 65
Number of Stories Heated Floor Area 1312 Total Floor Area 1799 Roof Pitch 12/6
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Page 1 of 2 (Both Pages must be submitted together.)

Revised 11-30-07

568.00

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

<u>OWNERS CERTIFICATION:</u> I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

	tand and agree that I have informed and provided this en responsibilities in Columbia County for obtaining
Contractor's Signature (Permitee) Theodore C. Brock	Contractor's License Number BC 1250382 Columbia County Competency Card Number
Affirmed under penalty of perjury to by the <u>Contractor</u> and	d subscribed before me this W day of May 20 <u>B</u> .
Personally known XXX or Produced Identification	
Melina i mycapie	SEAL:
State of Florida Notary Signature (For the Contractor)	Notary Public State of Florida

Melissa L McKague My Commission DD493647 Expires 11/22/2009 Columnia Country building carroit Application

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

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infrastructures and facilities has been correcte	ıd.	*	8	$_{\odot}$	
owners certification: I hereby certify the done in compliance with all applicable law the above written jesphrabilities in Column	s and regula	ting construc	tion and zo	oning. I further und	
			•		i e
Owners Signature Stiffe Hoogg	*	• * * * * * *		×	
CONTRACTORS AFFIDAVIT: By my signature written statement to the owner of all the at this Building Permit.					
Sec &	·			mber (BC)25	10382
Contractor's Signature (Permitee) Theodore C. Brock	# # * * *	Columbia Co Competency		er	
Affirmed under penalty of perjury to by the Con Personally known XXX or Produced Identificat		ibscribed befo	re me this <u>2</u>	24 day of JUNE	2008
Melina i Marague	SE	AL:			* g
State of Florida Notary Signature (For the Conti Melissa L. McKague	actor)		A N	otary Public State of Florida lelissa L McKague fy Commission DD493647 xpires 11/22/2009	

Page 2 of 2 (Both Pages must be submitted together.)

Revised 11-30-07

This Instrument Prepared by and Return to:

Amy Wesp SOUTHERN TITLE COMPANY, LLC.

HOLDING

3943 BAY MEADOWS ROAD JACKSONVILLE, Florida 32217

as a necessary incident to the fulfillment of conditions contained in a title insurance commitment issued by it.

Property Appraisers Parcel I.D. (Folio) Number(s): R02856-000

Grantee(s) I.D..#(s): File No:JX0812085

nst:200812010775 Date:6/6/2008 Time:1:05 PM Doc Sjamp-Deed:6293.00

7-Dc, P.DeWirt Cason, Columbia County Page 1 of 1 8:1151 P:2385

WARRANTY DEED (CORPORATION)

This Warranty Deed Made this 27th day of May, 2008, by RML HOLDINGS INC., A FLORIDA CORPORATION, and having its place of business at 703 NW BLACKBERRY CIRCLE, LAKE CITY, Florida 32055, hereinafter called the grantor,

to MARONDA HOMES, INC. OF FLORIDA, A FLORIDA CORPORATION, whose post office address is: 11200 ST. JOHNS INDUSTRIAL PARKWAY, JACKSONVILLE, FLORIDA 32246, hereinafter called the grantee,

\$899,000.00

WITNESSETH: That said grantor, for and in consideration of the sum of \$30.00 Dollars and other valuable considerations, receipt whereof is hereby acknowledged, by these presents grants, bargains, sells, aliens, remises, releases, conveys and confirms unto the grantee, all that certain land situate in Columbia County, Florida, viz: LOTS 1, 9, 10, 11, 12, 13, 14, 15, 16, 17, 18, 19, 20, 24, 27, 28, 29, 30, 31, 32, 33, 34, 35, 36, 37, 38, 39, 40, AND 41, OF TIMBERLANDS, PHASE 1, ACCORDING TO PLAT THEREOF AS RECORDED IN PLAT BOOK 9. PAGE 26 AND 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

TOGETHER with all the tenements, hereditaments and appurtenances thereto belonging or in anywise appertaining. To Have and to Hold, the same in fee simple forever

And the grantor hereby covenants with said grantee that the grantor is lawfully scized of said land in fee simple; that the grantor has good right and lawful authority to sell and convey said land; that the grantor hereby fully warrants the title to said land and will defend the same against the lawful claims of all persons whomsoever; and that said land is free of all encumbrances, except taxes accruing subsequent to December 31, 2007, reservations, restrictions and easements of record, if any.

(Wherever used herein the terms "grantor" and "grantee" included all the parties to this instrument, and the heirs, legal representatives and essigns of individuals, and the successors and assigns of corporation.)

in Witness Whereof, the Grantor has caused these presents to be executed in its name, and its corporate seal to be hereinto affixed, by its proper officers thereunto duly authorized, the day and year first above written.

Signed seafed and delivered in our presence:

ATTEST:		
Secretary	RML HOLDINGS INC.	
Witness Signature: Nover D. Marris Printed Name: World D. Morris	BY: A mit 47	Rail
Witness Signature: Jodym . Coble. AVP Printed Name: Jodym . Coble	ROBERT R. LARDIZABAL, PR	ESIDENT
STATE OF FLORIDA COUNTY OF DUVAL		
The foregoing instrument was acknowledged before me ROBERT R. LARDIZABAL as PRESIDENT of RML behalf of the corporation. He/she is personally known to me	HOLDINGS INC., A PLUKUDA CUK	PORATION, on as identification,
My Commission Expires:	· Stand D-7	nain;
WORTH D. MORRIS	Printed Name: WOEF Notary Public Serial Number	TO MORRIS



STATE OF FLORIDA DEPARTMENT OF HEALTH

08-400

Permit Application Number _____

APPLICATION FOR ONSITE SEWAGE DISPOSAL SYSTEM CONSTRUCTION PERMIT

		6 Kilo	ee :		-		-	-	-		-		-	_	6000	. [PA	Rī	וו	- (SIT	Έ	PĹ	A.	۱-	-	t time				_	(C.19	-	-			- .	·	-	·			_
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FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION

Florida Department of Community Affairs Residential Whole Building Performance Method A

Project Name: MONTGOMERY GAINESVILLE Address: 218 SW. TIMPER RUMP DY. City, State: LAK CHM FL 32195 Owner: ELECTRIC Climate Zone: North	Builder: MARONDA HOMES Permitting Office: COLUMNOM Permit Number: 27224 Jurisdiction Number: 221000
1. New construction or existing New	12. Cooling systems a. Central Unit b. N/A c. N/A 13. Heating systems a. Electric Heat Pump b. N/A c. N/A C. N/A C. N/A C. N/A 14. Hot water systems a. Electric Resistance b. N/A c. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) 15. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)
Glass/Floor Area: 0.09 Total as-built po	

I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code.

PREPARED BY:

DATE: Wayne Campbell

I hereby certify that this building, as designed, is in compliance with the Florida Energy Code.

OWNER/AGENT: MULMA MYLAPI

Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes.

BUILDING OFFICIAL:

DATE:

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub:, Plat:,,,
PERMIT #:

BASE				AS-B	JILT				
GLASS TYPES .18 X Conditioned X BSPM = Floor Area	= Points	The course water took		hang Len Ho	jt Area X	SPI	их	SOF	= Points
.18 1312.0 18.59	4390.0	1.Single, Clear	E	1.0 6.0	16.0	47.9	92	0.97	743.0
		2.Single, Clear	Е	1.0 8.0	40.0	47.9	92	0.99	1899.0
			W	3.0 6.0	11.0	43.8	34	0.73	351.0
			W	1.0 6.0		43.8		0.97	467.0
		5.Single, Clear	E	1.0 6.0		47.9		0.97	743.0
		6.Single, Clear	W	1.0 6.0	24.0	43.8	34	0.97	1020.0
		As-Built Total:			118.0				5223.0
WALL TYPES Area X BSP	M = Points	Туре		R-Va	ue Area	X	SPN	Л =	Points
Adjacent 231.0 0.70	161.7	1. Concrete, Int Insul, Exterior		4.	874.0		1.13		992.0
Exterior 874.0 1.70	1485.8	2. Frame, Steel, Adjacent		13.0	231.0		0.90		207.9
Base Total: 1105.0	1647.5	As-Built Total:			1105.0				1199.9
DOOR TYPES Area X BSP	M = Points	Туре			Area	Х	SPN	/I =	Points
Adjacent 17.0 2.40	40.8	1.Exterior Insulated			20.0		4.10		82.0
Exterior 20.0 6.10	122.0	2.Adjacent Wood			17.0		2.40		40.8
Base Total: 37.0	162.8	As-Built Total:			37.0				122.8
CEILING TYPES Area X BSP	M = Points	Туре	F	R-Value	Area X	SPM	X S	CM =	Points
Under Attic 1312.0 1.73	2269.8	1. Under Attic		19.0	1339.0	2.34 >	〈 1.00		3133.3
Base Total: 1312.0	2269.8	As-Built Total:			1339.0				3133.3
FLOOR TYPES Area X BSP	M = Points	Туре		R-Val	ue Area	Х	SPN	Л =	Points
Slab 149.0(p) -37.0	-5513.0	1. Slab-On-Grade Edge Insulation	on	0.0	149.0(p	_	41.20		-6138.8
Raised 0.0 0.00	0.0				N.				
Base Total:	-5513.0	As-Built Total:			149.0				-6138.8
INFILTRATION Area X BSP	M = Points				Area	Х	SPN	л =	Points
1312.0 10.2	1 13395.5		400 00 900		1312.	0	10.21		13395.5

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , , PERMIT #:

	BASE		AS-BUILT									
Summer Ba	se Points: 10	6352.6	Summer As-Built Points: 16935.7									
Total Summer Points	X System = Multiplier	Cooling Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)									
16352.6	0.3250	5314.6	(sys 1: Central Unit 30000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS) 16936 1.00 (1.08 x 1.147 x 0.86) 0.260 0.950 4483.9 16935.7 1.00 1.072 0.260 0.950 4483.9									

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub:, Plat:,,,
PERMIT #:

BASE	AS-BUILT
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area	Overhang Type/SC Ornt Len Hgt Area X WPM X WOF = Points
.18 1312.0 20.17 4763.0	1.Single, Clear E 1.0 6.0 16.0 26.41 1.02 429.0
,	2.Single, Clear E 1.0 8.0 40.0 26.41 1.01 1065.0
	3.Single, Clear W 3.0 6.0 11.0 28.84 1.08 343.0 4.Single, Clear W 1.0 6.0 11.0 28.84 1.01 319.0
	4.Single, Clear W 1.0 6.0 11.0 28.84 1.01 319.0 5.Single, Clear E 1.0 6.0 16.0 26.41 1.02 429.0
	6.Single, Clear W 1.0 6.0 24.0 28.84 1.01 697.0
	As-Built Total: 118.0 3282.0
WALL TYPES Area X BWPM = Points	Type R-Value Area X WPM = Points
Adjacent 231.0 3.60 831.6	1. Concrete, Int Insul, Exterior 4.1 874.0 6.42 5611.1
Exterior 874.0 3.70 3233.8	2. Frame, Steel, Adjacent 13.0 231.0 4.90 1131.9
Base Total: 1105.0 4065.4	As-Built Total: 1105.0 6743.0
DOOR TYPES Area X BWPM = Points	Type Area X WPM = Points
Adjacent 17.0 11.50 195.5	1.Exterior Insulated 20.0 8.40 168.0
Exterior 20.0 12.30 246.0	2.Adjacent Wood 17.0 11.50 195.5
Base Total: 37.0 441.5	As-Built Total: 37.0 363.5
CEILING TYPES Area X BWPM = Points	Type R-Value Area X WPM X WCM = Points
Under Attic 1312.0 2.05 2689.6	1. Under Attic 19.0 1339.0 2.70 X 1.00 3615.3
Base Total: 1312.0 2689.6	As-Built Total: 1339.0 3615.3
FLOOR TYPES Area X BWPM = Points	Type R-Value Area X WPM = Points
Slab 149.0(p) 8.9 1326.1 Raised 0.0 0.00 0.0	1. Slab-On-Grade Edge Insulation 0.0 149.0(p 18.80 2801.2
Base Total: 1326.1	As-Built Total: 149.0 2801.2
INFILTRATION Area X BWPM = Points	Area X WPM = Points
1312.0 -0.59 -774.1	1312.0 -0.59 -774.1

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub:, Plat:,,,
PERMIT #:

	BASE		AS-BUILT										
Winter Base	Points:	12511.5	Winter As-Built Points: 16030.9										
Total Winter X Points	System = Multiplier	Heating Points	Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)										
12511.5	0.5540	6931.4	(sys 1: Electric Heat Pump 30000 btuh ,EFF(8.0) Ducts:Unc(S),Con(R),Int(AH),R6.0 16030.9 1.000 (1.060 x 1.169 x 0.88) 0.426 0.950 7106.8 16030.9 1.00 1.095 0.426 0.950 7106.8										

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub:, Plat:,,,
PERMIT #:

	BASE		AS-BUILT										
WATER HEA Number of Bedrooms	TING	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	х	Tank X Ratio	Multiplier	X Credit Multiplie		
3		2635.00		7905.0	50.0	0.92	3		1.00	2635.00	1.00	7905.0	
					As-Built To	tal:						7905.0	

	CODE COMPLIANCE STATUS												
	BASE							AS	-BUILT				
Cooling Points	+	Heating Points	+	Hot Water Points	The same of	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
5315		6931		7905	State of all	20151	4484		7107		7905		19496

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub:, Plat:,,,	PERMIT #:
TESTALOG. Lot., Odb., Tlat.,,,	PERMIT #.

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	V
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall;	
		foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility	
		penetrations; between wall panels & top/bottom plates; between walls and floor.	V
		EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends	
		from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members.	1
	¥	EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed	V
		to the perimeter, penetrations and seams.	
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases,	/
		soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate;	./
		attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is	V
		installed that is sealed at the perimeter, at penetrations and seams.	
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a	
=	1	sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from	
		conditioned space, tested.	
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA,	/
		have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	V
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	/
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.2

The higher the score, the more efficient the home.

ELECTRIC, Lot:, Sub:, Plat:,,,

1.	New construction or existing	New	12.	Cooling systems	
2.	Single family or multi-family Sin	gle family		. Central Unit	Cap: 30.0 kBtu/hr
3.	Number of units, if multi-family	1	-		SEER: 13.00
4.	Number of Bedrooms	3	t	o. N/A	
5.	Is this a worst case?	Yes			_
6.	Conditioned floor area (ft²)	1312 ft²	_	. N/A	-
7.	Glass type 1 and area: (Label reqd. by 13-104.4.5 if no	t default)			-
a.	U-factor: Description		13.	Heating systems	(2.2.2
	(or Single or Double DEFAULT) 7a(Sngle Default)	118.0 ft²		. Electric Heat Pump	Cap: 30.0 kBtu/hr
b.	SHGC:				HSPF: 8.00
	(or Clear or Tint DEFAULT) 7b. (Clear)	118.0 ft²	t	o. N/A	
8.	Floor types			•	_
a.	Slab-On-Grade Edge Insulation R=0.0, 1	49.0(p) ft	_	. N/A	
b.	N/A		_		-
c.	N/A		14.	Hot water systems	
9.	Wall types		a	. Electric Resistance	Cap: 50.0 gallons
a.	Concrete, Int Insul, Exterior R=4.1	, 874.0 ft ²	_		EF: 0.92
b.	Frame, Steel, Adjacent R=13.0	, 231.0 ft ²	t	. N/A	
c.	N/A				
d.	N/A		_ 0	. Conservation credits	
e.	N/A			(HR-Heat recovery, Solar	
10.	Ceiling types			DHP-Dedicated heat pump)	
a.	Under Attic R=19.0,	1339.0 ft ²	15.	HVAC credits	PT,
b.	N/A			(CF-Ceiling fan, CV-Cross ventilation,	_
c.	N/A		_	HF-Whole house fan,	
11.	Ducts		_	PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Con. AH(Sealed):Interior Sup. R=6.0	0, 150.0 ft		MZ-C-Multizone cooling,	
	N/A	ľ	_	MZ-H-Multizone heating)	

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed based on installed Code compliant feature

Builder Signature:

TIMBER RUGE PORTY/FL Zip: Lake City, Fr 32615

stNOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is not a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar Mesignation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCSB v4.5)

Project Summary Entire House MARONDA HOMES

Job: MONTGOMERY

Date:

By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: 407-321-0064

Project Information

For:

MONTGOMERY

Notes:

Design Information

Weather: Gainesville, FL, US

Winter Design Conditions

Summer Design Conditions

Outside db Inside db Design TD	70 °	F F	Outside db Inside db Design TD	92 75 17	°F °F °F
			Daily range	M	
			Relative humidity	50	%
			Moisture difference	52	ar/lb

Heating Summary

Sensible Cooling Equipment Load Sizing

Structure Ducts Central vent (80 cfm) Humidification Piping	24549 2428 3238 0	Btuh Btuh Btuh Btuh Btuh	Structure Ducts Central vent (80 cfm) Blower	13023 5274 1488 0	Btuh Btuh Btuh Btuh
Equipment load	30215	Btuh	Use manufacturer's data	∍n	×
Infiltration			Rate/swing multiplier Equipment sensible load	0.97 19191	Btuh

Simplified

	1111111	ration	

Method

latent	Cooling	Equipment	Load	Sizina
POCCILL	Gooming	Edalbilletir	Loud	OIZILIS

Construction quality Average	Latent Cooling Equipment Load Si	ZII
Fireplaces	Structure 2160 Btu	h
	Ducts 1179 Btu	
Heating Cooling	Central vent (80 cfm) 2813 Btu	
Area (ft²) 1261 1261 Volume (ft³) 10088 10088	Equipment latent load 6151 Btu	h
Air changes/hour 0.45 0.23	Equipment total load 25342 Btu	h
Equiv. AVF (cfm) 76 39	Req. total capacity at 0.74 SHR 2.2 ton	

Heating Equipment Summary

Cooling Equipment Summary

	•	-				
Make TEMPS Trade HEAT P Model N4H330	UMP		Make Trade Cond Coil	TEMPSTAR HEAT PUMP N4H330AKA FSU4X3000A		
Efficiency Heating input Heating output Temperature rise Actual air flow Air flow factor Static pressure Space thermosta	0 0 0.000 0.60	cfm/Btuh	Efficien Sensibl Latent of Total co Actual a Air flow Static p	cy e cooling cooling poling air flow	22200 7800 30000 935 0.051	SEER Btuh Btuh Btuh cfm cfm/Btuh in H2O

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Building Analysis Entire House MARONDA HOMES

Job: MONTGOMERY

Date:

By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: 407-321-0064

Project Information

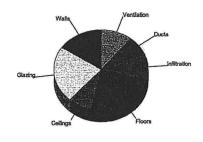
For:

MONTGOMERY

		Design Co	onditions		的复数形式的
Location: Gainesville, FL, US Elevation: 151 ft Latitude: 30°N Outdoor: Dry bulb (°F) Daily range (°F) Wet bulb (°F) Wind speed (mph)	Heating 33 - 15.0	Cooling 92 19 (M) 77 7.5	Indoor: Indoor temperature (°F) Design TD (°F) Relative humidity (%) Moisture difference (gr/lb) Infiltration: Method Construction quality Fireplaces	Heating 70 37 30 10.6 Simplified Average 0	Cooling 75 17 50 52.0

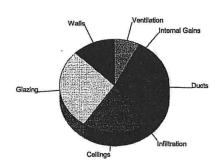
Heating

Component	Btuh/ft ²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Piping Humidification Ventilation Adjustments Total	4.4 47.0 0.0 1.8 19.9 2.5	4760 6861 0 2379 7487 3063 2428 0 3238 0	15.8 22.7 0.0 7.9 24.8 10.1 8.0 0.0 10.7



Cooling

Component	Btuh/ft²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Ventilation Internal gains Blower Adjustments	2.3 37.6 0.0 2.6 0.0 0.6	2507 5492 0 3385 0 719 5274 1488 920 0	12.7 27.8 0.0 17.1 0.0 3.6 26.7 7.5 4.7 0.0
Total	×	19784	100.0



Overall U-value = 0.199 Btuh/ft2-°F

ERROR: negative wall area in MASTER BDR - check windows.

Duct System Summary Entire House MARONDA HOMES

Job: MONTGOMERY

Date:

By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: 407-321-0064

Project Information

For

MONTGOMERY

•	He	ating			Co	ooling
External static pressure		in H2O				in H2O
Pressure losses	0.00	in H2O			0.00	in H2O
Available static pressure	0.60	in H2O			0.60	in H2O
Supply / return available pressure	0.43 / 0.17	in H2O			0.43 / 0.17	in H2O
Lowest friction rate	2.857	in/100ft	•		2.857	in/100ft
Actual air flow	935	cfm	:•:		935	cfm
Total effective length (TEL)			21	ft		

Supply Branch Detail Table

Name	1	Design (Btuh)	Htg (cfm)	Cig (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
MASTER BDR	h	4970	200	200	4.286	7	· x@	VIFx	10.0	0.0	st5
MASTER BATH	h	2370	50	50	8.571	5	хФ .	VIFx	5.0	0.0	st5
UTILITY ROOM	C	863	50	50	2.857	4	w x	VIFx	15.0	0.0	st1
FAMILY/DINING RM	C	3991	240	240	2.857	8	x@	VIFx	15.0	0.0	ST1
KITCHEN	C	2069	100	100	4.286	5	ωx	VIFx	10.0	0.0	ST1
BDR #2	h	3335	120	120	2.857	6	хØ	VIFx	15.0	0.0	ST3
BATHROOM	h	763	20	20	4.286	4	×@	VIFx	10.0	0.0	ST2
BDR #3	h	3880	120	120	4.286	6	x@	VIFx	10.0	0.0	ST4
MASTER TOILET	С	794	35	35	2.857	4	x@	VIFx	15.0	0.0	st5

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Clg (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	675	675	2.857	622	. 14	0 x 0	VinlFlx	
ST2	Peak AVF	20	20	4.286	369	4	0 x 0	VinlFlx	
ST3	Peak AVF	120	120	2.857	527	6	0 x 0	VinIFlx	
ST4	Peak AVF	120	120	4.286	613	6	0 x 0	VinIFlx	1
st5	Peak AVF	285	285	2.857	595	10	0 x 0	VinlFlx	st1

Bold/italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (cfm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Mati	Trunk
rb1	0x 0	935	935	6.0	2.857	670	16	x0 0		VIFx	

Columbia County Building Department Culvert Permit

Culvert Permit No.

000001647

DATE 07/31	1/2008 PARCEL ID #	10-45-16-02856-120	
APPLICANT	PATRICK WILSON	PHONE 352 206	-5459
ADDRESS 6	800 SOUTHPOINT PARKWAY	JACKSONVILLE	FL 32216
OWNER MA	ARONDA HOMES	PHONE 407 227	-3504
ADDRESS 27	78 SW TIMBER RIDGE DRIVE	LAKE CITY	FL 32024
CONTRACTOR	R THEODORE BROCK	PHONE 904 296	-1490
LOCATION OF	F PROPERTY 90W, TL ON 247S, TR ON	252B, TL ON TIMBER RIDGE, 5TH	
LOT ON LEFT	****		
SUBDIVISION/ SIGNATURE _ X	INSTALLATION REQUIREMENT Culvert size will be 18 inches in diamedriving surface. Both ends will be mitter thick reinforced concrete slab. INSTALLATION NOTE: Turnouts will a) a majority of the current and existing the driveway to be served will be purent to the current and existing paved or concrete or paved driveway, which current and existing paved or concrete installation shall conform to the department of Transportation Permit in the current and existing paved or concrete or paved driveway.	ter with a total lenght of 32 feet, length of 4 foot with a 4 : 1 slope and place be required as follows: ing driveway turnouts are paved, paved or formed with concrete. It is dealer to a minimum of 12 feet wide or the ever is greater. The width shall concreted turnouts.	eaving 24 feet of oured with a 4 inch or;
	Other		*

ALL PROPER SAFETY REQUIREMENTS SHOULD BE FOLLOWED DURING THE INSTALATION OF THE CULVERT.

135 NE Hernando Ave., Suite B-21 Lake City, FL 32055

Phone: 386-758-1008 Fax: 386-758-2160

Amount Paid 25.00



FLORIDA ENERGY EFFICIENCY CODE FOR BUILDING CONSTRUCTION Florida Department of Community Affairs

Residential Whole Building Performance Method A

Project Name: MONTGOMERY GAINESVILLE Address: 218 SW TIMPER RUME DY. City, State: LAKE CHY, 12 32145 Owner: ELECTRIC Climate Zone: North	Builder: MARONDA HOMES Permitting Office: COUMMING Permit Number: Jurisdiction Number:
1. New construction or existing 2. Single family or multi-family 3. Number of units, if multi-family 4. Number of Bedrooms 5. Is this a worst case? 6. Conditioned floor area (ft²) 7. Glass type¹ and area: (Label reqd. by 13-104.4.5 if not default) a. U-factor:	12. Cooling systems a. Central Unit b. N/A c. N/A 13. Heating systems a. Electric Heat Pump b. N/A c. N/A 14. Hot water systems a. Electric Resistance b. N/A 15. N/A 16. Conservation credits (HR-Heat recovery, Solar DHP-Dedicated heat pump) 16. HVAC credits (CF-Ceiling fan, CV-Cross ventilation, HF-Whole house fan, PT-Programmable Thermostat, MZ-C-Multizone cooling, MZ-H-Multizone heating)
Glass/Floor Area: 0.09 Total as-built p	
I hereby certify that the plans and specifications covered by this calculation are in compliance with the Florida Energy Code. PREPARED BY: DATE: WWY (Ampoli) 05 15 08 I hereby certify that this building, as designed, is in compliance with the Florida Energy Code. OWNER/AGENT: WWW AMAGEMED DATE: MISSA MYAGMA 05 15 08	Review of the plans and specifications covered by this calculation indicates compliance with the Florida Energy Code. Before construction is completed this building will be inspected for compliance with Section 553.908 Florida Statutes. BUILDING OFFICIAL: DATE:

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub:, Plat:,,,
PERMIT #:

				AS-B	UILT	5					
GLASS TYPES .18 X Condition Floor Are		PM =	Points	Type/SC	Ov Ornt	erhang Len H	gt Area X	SPN	ихз	SOF	= Points
.18 1312.	0	18.59	4390.0	1.Single, Clear 2.Single, Clear 3.Single, Clear 4.Single, Clear 5.Single, Clear 6.Single, Clear As-Built Total:	E W W E W	1.0 6. 1.0 8. 3.0 6. 1.0 6. 1.0 6. 1.0 6.	0 40.0 0 11.0 0 11.0 0 16.0	47.9 47.9 43.8 43.8 47.9 43.8	2 4 4 2	0.97 0.99 0.73 0.97 0.97	743.0 1899.0 351.0 467.0 743.0 1020.0
WALL TYPES	Area X	BSPM	= Points	Type		R-Va	lue Area	X	SPM	=	Points
Adjacent Exterior	231.0 874.0	0.70 1.70	161.7 1485.8	1. Concrete, Int Insul, I 2. Frame, Steel, Adjac		4. 13.			1.13 0.90		992.0 207.9
Base Total:	1105.0		1647.5	As-Built Total:			1105.0	×			1199.9
DOOR TYPES	Area X	BSPM	= Points	Туре			Area	X	SPM	=	Points
Adjacent Exterior	17.0 20.0	2.40 6.10	40.8 122.0	Exterior Insulated Adjacent Wood	н1 +	- 1+- 1-	20.0 17.0		4.10 2.40		82.0 40.8
Base Total:	37.0		162.8	As-Built Total:			37.0				122.8
CEILING TYPES	Area X	BSPM	= Points	Туре		R-Value	Area X	SPM	x sc	M =	Points
Under Attic	1312.0	1.73	2269.8	1. Under Attic	P.:	19.0	1339.0	2.34 X	1.00		3133.3
Base Total:	1312.0		2269.8	As-Built Total:			1339.0				3133.3
FLOOR TYPES	Area X	BSPM	= Points	Туре	194	R-Va	ue Area	Χ	SPM	=	Points
Slab 1 Raised	49.0(p) 0.0	-37.0 0.00	-5513.0 0.0	1. Slab-On-Grade Edge	e Insulation	0.0) 149.0(p	-4	41.20		-6138.8
Base Total:	Townson and		-5513.0	As-Built Total:			149.0				-6138.8
INFILTRATION	Area X	BSPM	= Points				Area	Χ	SPM	=	Points
	1312.0	10.21	13395.5		.0.		1312.0)	10.21		13395.5

SUMMER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot: , Sub: , Plat: , , , PERMIT #:

BASE			AS-BUILT					
Summer Ba	se Points: 1	6352.6	Summer As-Built Points: 16935.7					
Total Summer Points	X System = Multiplier	Cooling Points	Total X Cap X Duct X System X Credit = Cooling Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)					
16352.6	0.3250	5314.6	(sys 1: Central Unit 30000btuh ,SEER/EFF(13.0) Ducts:Unc(S),Con(R),Int(AH),R6.0(INS) 16936 1.00 (1.08 x 1.147 x 0.86) 0.260 0.950 4483.9 16935.7 1.00 1.072 0.260 0.950 4483.9					

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub:, Plat:,,,
PERMIT #:

BASE	AS-BUILT
GLASS TYPES .18 X Conditioned X BWPM = Points Floor Area	Overhang Type/SC Ornt Len Hgt Area X WPM X WOF = Points
.18 1312.0 20.17 4763.0	1.Single, Clear E 1.0 6.0 16.0 26.41 1.02 429.0
	2.Single, Clear E 1.0 8.0 40.0 26.41 1.01 1065.0
	3.Single, Clear W 3.0 6.0 11.0 28.84 1.08 343.0
	4.Single, Clear W 1.0 6.0 11.0 28.84 1.01 319.0 5.Single, Clear E 1.0 6.0 16.0 26.41 1.02 429.0
A Company	6.Single, Clear W 1.0 6.0 24.0 28.84 1.01 697.0
	As-Built Total: 118.0 3282.0
	AS-Built Total. 110.0 3202.0
WALL TYPES Area X BWPM = Points	Type R-Value Area X WPM = Points
Adjacent 231.0 3.60 831.6	1. Concrete, Int Insul, Exterior 4.1 874.0 6.42 5611.1
Exterior 874.0 3.70 3233.8	2. Frame, Steel, Adjacent 13.0 231.0 4.90 1131.9
Base Total: 1105.0 4065.4	As-Built Total: 1105.0 6743.0
DOOR TYPES Area X BWPM = Points	Type Area X WPM = Points
Adjacent 17.0 11.50 195.5	1.Exterior Insulated 20.0 8.40 168.0
Exterior 20.0 12.30 246.0	2.Adjacent Wood 17.0 11.50 195.5
Base Total: 37.0 441.5	As-Built Total: 37.0 363.5
CEILING TYPES Area X BWPM = Points	Type R-Value Area X WPM X WCM = Points
Under Attic 1312.0 2.05 2689.6	1. Under Attic 19.0 1339.0 2.70 X 1.00 3615.3
Base Total: 1312.0 2689.6	As-Built Total: 1339.0 3615.3
FLOOR TYPES Area X BWPM = Points	Type R-Value Area X WPM = Points
Slab 149.0(p) 8.9 1326.1 Raised 0.0 0.00 0.0	1. Slab-On-Grade Edge Insulation 0.0 149.0(p 18.80 2801.2
Raised 0.0 0.00 0.0	
Base Total: 1326.1	As-Built Total: 149.0 2801.2
INFILTRATION Area X BWPM = Points	Area X WPM = Points
1312.0 -0.59 -774.1	1312.0 -0.59 -774.1

WINTER CALCULATIONS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub:, Plat:,,,
PERMIT #:

	BASE		AS-BUILT
Winter Base	Points:	12511.5	Winter As-Built Points: 16030.9
Total Winter X Points			Total X Cap X Duct X System X Credit = Heating Component Ratio Multiplier Multiplier Multiplier Points (System - Points) (DM x DSM x AHU)
12511.5	0.5540	6931.4	(sys 1: Electric Heat Pump 30000 btuh ,EFF(8.0) Ducts:Unc(S),Con(R),Int(AH),R6.0 16030.9 1.000 (1.060 x 1.169 x 0.88) 0.426 0.950 7106.8 16030.9 1.00 1.095 0.426 0.950 7106.8

WATER HEATING & CODE COMPLIANCE STATUS

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub:, Plat:,,,
PERMIT #:

BASE						A	S-BU	ĻT				
WATER HEA Number of Bedrooms	TING X	Multiplier	=	Total	Tank Volume	EF	Number of Bedrooms	Х	Tank >	Multiplier	X Credit Multiplie	
3		2635.00		7905.0	50.0	0.92	3		1.00	2635.00	1.00	7905
					As-Built To	tal:						7905

	CODE COMPLIANCE STATUS												
	BASE									AS	-BUILT		
Cooling Points	+	Heating Points	+	Hot Water Points	Special section (SE)	Total Points	Cooling Points	+	Heating Points	+	Hot Water Points	=	Total Points
5315		6931		7905	Special Age	20151	4484		7107		7905		19496

PASS



Code Compliance Checklist

Residential Whole Building Performance Method A - Details

ADDRESS: Lot:, Sub:, Plat:,,,	PERMIT #:
	<u> </u>

6A-21 INFILTRATION REDUCTION COMPLIANCE CHECKLIST

COMPONENTS	SECTION	REQUIREMENTS FOR EACH PRACTICE	CHECK
Exterior Windows & Doors	606.1.ABC.1.1	Maximum:.3 cfm/sq.ft. window area; .5 cfm/sq.ft. door area.	V
Exterior & Adjacent Walls	606.1.ABC.1.2.1	Caulk, gasket, weatherstrip or seal between: windows/doors & frames, surrounding wall; foundation & wall sole or sill plate; joints between exterior wall panels at corners; utility penetrations; between wall panels & top/bottom plates; between walls and floor. EXCEPTION: Frame walls where a continuous infiltration barrier is installed that extends from, and is sealed to, the foundation to the top plate.	
Floors	606.1.ABC.1.2.2	Penetrations/openings >1/8" sealed unless backed by truss or joint members. EXCEPTION: Frame floors where a continuous infiltration barrier is installed that is sealed to the perimeter, penetrations and seams.	1
Ceilings	606.1.ABC.1.2.3	Between walls & ceilings; penetrations of ceiling plane of top floor; around shafts, chases, soffits, chimneys, cabinets sealed to continuous air barrier; gaps in gyp board & top plate; attic access. EXCEPTION: Frame ceilings where a continuous infiltration barrier is installed that is sealed at the perimeter, at penetrations and seams.	/
Recessed Lighting Fixtures	606.1.ABC.1.2.4	Type IC rated with no penetrations, sealed; or Type IC or non-IC rated, installed inside a sealed box with 1/2" clearance & 3" from insulation; or Type IC rated with < 2.0 cfm from conditioned space, tested.	1
Multi-story Houses	606.1.ABC.1.2.5	Air barrier on perimeter of floor cavity between floors.	
Additional Infiltration reqts	606.1.ABC.1.3	Exhaust fans vented to outdoors, dampers; combustion space heaters comply with NFPA, have combustion air.	

6A-22 OTHER PRESCRIPTIVE MEASURES (must be met or exceeded by all residences.)

COMPONENTS	SECTION	REQUIREMENTS	CHECK
Water Heaters	612.1	Comply with efficiency requirements in Table 612.1.ABC.3.2. Switch or clearly marked cir	/
produce and a strategy to the strategy of the		breaker (electric) or cutoff (gas) must be provided. External or built-in heat trap required.	
Swimming Pools & Spas	612.1	Spas & heated pools must have covers (except solar heated). Non-commercial pools must have a pump timer. Gas spa & pool heaters must have a minimum thermal efficiency of 78%.	
Shower heads	612.1	Water flow must be restricted to no more than 2.5 gallons per minute at 80 PSIG.	V
Air Distribution Systems	610.1	All ducts, fittings, mechanical equipment and plenum chambers shall be mechanically attached, sealed, insulated, and installed in accordance with the criteria of Section 610. Ducts in unconditioned attics: R-6 min. insulation.	1
HVAC Controls	607.1	Separate readily accessible manual or automatic thermostat for each system.	1
Insulation	604.1, 602.1	Ceilings-Min. R-19. Common walls-Frame R-11 or CBS R-3 both sides. Common ceiling & floors R-11.	

ENERGY PERFORMANCE LEVEL (EPL) DISPLAY CARD

ESTIMATED ENERGY PERFORMANCE SCORE* = 85.2

The higher the score, the more efficient the home.

ELECTRIC, Lot:, Sub:, Plat:,,,

1. 2.	New construction or existing Single family or multi-family	New Single family	_ 1	2. Cooling systems a. Central Unit	Cap: 30.0 kBtu/hr
3.	Number of units, if multi-family	1	_	di Constat Cint	SEER: 13.00
4.	Number of Bedrooms	3	_	b. N/A	SEEK. 13.00
5.	Is this a worst case?	Yes	_	o. IVA	-
6.	Conditioned floor area (ft²)	1312 ft ²	-	c. N/A	
7.	Glass type 1 and area: (Label reqd. by 1:	. A 1000 Date 200	-	C. N/A	
12020	U-factor:		,	2 IXti	-
a.		Description Area	1	3. Heating systems	0 20 0 170 /
b.	(or Single or Double DEFAULT) 7a(SSHGC:	Sngle Default) 118.0 ft²	_	a. Electric Heat Pump	Cap: 30.0 kBtu/hr HSPF: 8.00
	(or Clear or Tint DEFAULT) 7b.	(Clear) 118.0 ft ²		b. N/A	
8.	Floor types				-
	Slab-On-Grade Edge Insulation	R=0.0, 149.0(p) ft	_	c. N/A	
b.	N/A			i	
c.	N/A		_ 1	Hot water systems	
9.	Wall types	. ×		a. Electric Resistance	Cap: 50.0 gallons
a.	Concrete, Int Insul, Exterior	R=4.1, 874.0 ft ²			EF: 0.92
b.	Frame, Steel, Adjacent	R=13.0, 231.0 ft ²	_	b. N/A	
c.	N/A			* 2	
d.	N/A			c. Conservation credits	
e.	N/A			(HR-Heat recovery, Solar	e
10.	Ceiling types	j		DHP-Dedicated heat pump)	
a.	Under Attic	R=19.0, 1339.0 ft ²	. 1	5. HVAC credits	PT,
b.	N/A			(CF-Ceiling fan, CV-Cross ventilation,	
c.	N/A		t .	HF-Whole house fan,	
11.	Ducts		-	PT-Programmable Thermostat,	
a.	Sup: Unc. Ret: Con. AH(Sealed):Interi	ior Sup. R=6.0, 150.0 ft		MZ-C-Multizone cooling,	
	N/A			MZ-H-Multizone heating)	
			_		

I certify that this home has complied with the Florida Energy Efficiency Code For Building Construction through the above energy saving features which will be installed (or exceeded) in this home before final inspection. Otherwise, a new EPL Display Card will be completed

based on installed Code compliant features.
Builder Signature:

Date: 05 | 15 | 08

Address of New Home: 278 SW TIMber Rolge Party/FL Zip: Lake Cty, FL 32615

*NOTE: The home's estimated energy performance score is only available through the FLA/RES computer program. This is <u>not</u> a Building Energy Rating. If your score is 80 or greater (or 86 for a US EPA/DOE EnergyStar designation), your home may qualify for energy efficiency mortgage (EEM) incentives if you obtain a Florida Energy Gauge Rating. Contact the Energy Gauge Hotline at 321/638-1492 or see the Energy Gauge web site at www.fsec.ucf.edu for information and a list of certified Raters. For information about Florida's Energy Efficiency Code For Building Construction, contact the Department of Community Affairs at 850/487-1824.

1 Predominant glass type. For actual glass type and areas, see Summer & Winter Glass output on pages 2&4. EnergyGauge® (Version: FLRCSB v4.5)

Project Summary Entire House **MARONDA HOMES**

Job: MONTGOMERY

Date:

G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: 407-321-0064

Project Information

For:

MONTGOMERY

Notes:

Weather:	Gainesville,	FL,	US
----------	--------------	-----	----

winter Design C	onaitions	Summer Design	Conditions
Outside db Inside db Design TD	33 °F 70 °F 37 °F	Outside db Inside db Design TD Daily range Relative humidity Moisture difference	92 °F 75 °F 17 °F M 50 % 52 gr/lb
Heating Sur	nmary	Sensible Cooling Equip	ment Load Sizing

Structure Ducts	24549 2428	Btuh Btuh	Structure Ducts	13023 5274	Btuh Btuh
Central vent (80 cfm)	3238	Btuh	Central vent (80 cfm)	1488	Btuh
Humidification Piping	ő	Btuh Btuh	Blower	0	Btuh
Equipment load	30215	Btuh	Use manufacturer's data Rate/swing multiplier	.n 0.97	
Infiltration			Equipment sensible load	19191	Btuh

Method Construction quality		Simplified Average		Latent Cooling Equipme	nt Loac	Sizing
Fireplaces		O O		Structure Ducts	2160 1179	Btuh Btuh
Area (ft²) Volume (ft³)	Heating 1261 10088	Cooling 1261 10088		Central vent (80 cfm) Equipment latent load	2813 6151	Btuh Btuh
Air changes/hour Equiv. AVF (cfm)	0.45 76	0.23	*	Equipment total load Req. total capacity at 0.74 SHR		Btuh ton

Heating Equipment Summary

He	eating Equip	ment Summary	Cooling Equ	ipment Summary
Make Trade Model	TEMPSTAR HEAT PUMP N4H330AKA		Make TEMPSTAR Trade HEAT PUMP Cond N4H330AKA Coil FSU4X3000A	
Actual a Air flow Static p	input output ature rise air flow factor	8 HSPF 0 Btuh @ 47°F 0 °F 0 cfm 0.000 cfm/Btuh 0.60 in H2O	Efficiency Sensible cooling Latent cooling Total cooling Actual air flow Air flow factor Static pressure Load sensible heat ratio	13 SEER 22200 Btuh 7800 Btuh 30000 Btuh 935 cfm 0.051 cfm/Btuh 0.60 in H2O 0.76

Bold/italic values have been manually overridden

Printout certified by ACCA to meet all requirements of Manual J 8th Ed.

Building Analysis Entire House MARONDA HOMES

Job: MONTGOMERY

Date:

By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: 407-321-0064

Project Information

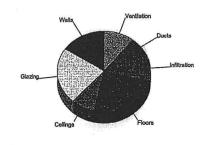
For:

MONTGOMERY

		Design Co	onditions		
Location: Gainesville, FL, US Elevation: 151 ft Latitude: 30°N Outdoor: Dry bulb (°F) Daily range (°F) Wet bulb (°F) Wind speed (mph)	Heating 33 - - 15.0	Cooling 92 19 (M) 77 7.5	Indoor: Indoor temperature (°F) Design TD (°F) Relative humidity (%) Moisture difference (gr/lb) Infiltration: Method Construction quality Fireplaces	Heating 70 37 30 10.6 Simplified Average 0	Cooling 75 17 50 52.0

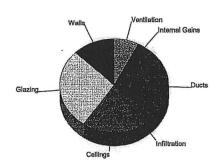
Heating

Component	Btuh/ft²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Piping Humidification Ventilation Adjustments	4.4 47.0 0.0 1.8 19.9 2.5	4760 6861 0 2379 7487 3063 2428 0 0 3238	15.8 22.7 0.0 7.9 24.8 10.1 8.0 0.0 0.0
Total		30215	100.0



Cooling

Component	Btuh/ft ²	Btuh	% of load
Walls Glazing Doors Ceilings Floors Infiltration Ducts Ventilation Internal gains Blower Adjustments	2.3 37.6 0.0 2.6 0.0 0.6	2507 5492 0 3385 0 719 5274 1488 920	12.7 27.8 0.0 17.1 0.0 3.6 26.7 7.5 4.7 0.0
Total		19784	100.0



Overall U-value = 0.199 Btuh/ft2-°F

ERROR: negative wall area in MASTER BDR - check windows.

Duct System Summary Entire House MARONDA HOMES

Job: MONTGOMERY

Date:

By: G. CARMACK

4005 MARONDA WAY, SANFORD, FL 32771 Phone: 407-321-0064

Project Information

For:

MONTGOMERY

pro-	Hea	iting			Co	ooling
External static pressure	0.60 ii	n H2O			0.60	in H2O
Pressure losses	0.00 ii	n H2O			0.00	in H2O
Available static pressure	0.60 ii	n H2O			0.60	in H2O
Supply / return available pressure	0.43 / 0.17 ii	n H2O			0.43 / 0.17	in H2O
Lowest friction rate	2.857 ii	n/100ft			2.857	in/100ft
Actual air flow	935 0	cfm	(*)		935	cfm
Total effective length (TEL)			21	ft		

Supply Branch Detail Table

Name		Design (Btuh)	Htg (cfm)	Clg (cfm)	Design FR	Diam (in)	Rect Size (in)	Duct Matl	Actual Ln (ft)	Ftg.Eqv Ln (ft)	Trunk
MASTER BDR	h	4970	200	200	4.286	7	- x@	VIFx	10.0	0.0	st5
MASTER BATH	h	2370	50	50	8.571	5	×00	VIFx	5.0	0.0	st5
UTILITY ROOM	c	863	50	50	2.857	4	x@	VIFx	15.0	0.0	st1
FAMILY/DINING RM	c	3991	240	240	2.857	8	x@	VIFx	15.0	0.0	ST1
KITCHEN	C	2069	100	100	4.286	5	ωx	VIFx	10.0	0.0	ST1
BDR #2	h	3335	120	120	2.857	6	хØ	VIFx	15.0	0.0	ST3
BATHROOM	h	763	20	20	4.286	4	χØ	VIFx	10.0	0.0	ST2
BDR #3	h	3880	120	120	4.286	6	χŒ	VIFx	10.0	0.0	ST4
MASTER TOILET	C	794	35	35	2.857	4	хØ	VIFx	15.0	0.0	st5

Supply Trunk Detail Table

Name	Trunk Type	Htg (cfm)	Cig (cfm)	Design FR	Veloc (fpm)	Diam (in)	Rect Duct Size (in)	Duct Material	Trunk
st1	Peak AVF	675	675	2.857	622	14	0 x 0	VinIFIx	st1
ST2	Peak AVF	20	20	4.286	369	4	0 x 0	VinIFIx	
ST3	Peak AVF	120	120	2.857	527	6	0 x 0	VinIFIx	
ST4	Peak AVF	120	120	4.286	613	6	0 x 0	VinIFIx	
st5	Peak AVF	285	285	2.857	595	10	0 x 0	VinIFIx	

Bold/Italic values have been manually overridden

Return Branch Detail Table

Name	Grill Size (in)	Htg (cfm)	Clg (¢fm)	TEL (ft)	Design FR	Veloc (fpm)	Diam (in)	RectSize (in)	Stud/Joist Opening (in)	Duct Matl	Trunk
rb1	0x 0	935	935	6.0	2.857	670	16	х0 0		VIFx	

Maronda Systems

Maronda Systems

4005 Maronda Way

Sanford FL 32771

(407) 321-0064

Fax (407) 321-3913

Engineer/Architect of Record:

Tomas Ponce, P.E.

367 Medallion Pl. Chuluota, Fl. 32766 FL PE # 50068,

Design Criteria: TPI

Design: Matrix Analysis

MiTek software

MONTGOMERY C

PLAN JOB # ADDRESS DIV/SUB MODEL LOT 278 SW TIMBER 9TM02001 20-1 JAX-9TM MGMC3 RIGHT RIDGE DR

Vault Standard

This structure was designed in accordance with, and meets the requirements of TPI standards and the FLORIDA 2004 BUILDING CODE for 125 M.P.H. Wind Zone. Truss loading is in accordance with ASCE 7-02. These trusses are designed for an enclosed building.

The Truss Engineering package for the above referenced site was generated by the Truss Designer/Architect/MiTek/Trenco.

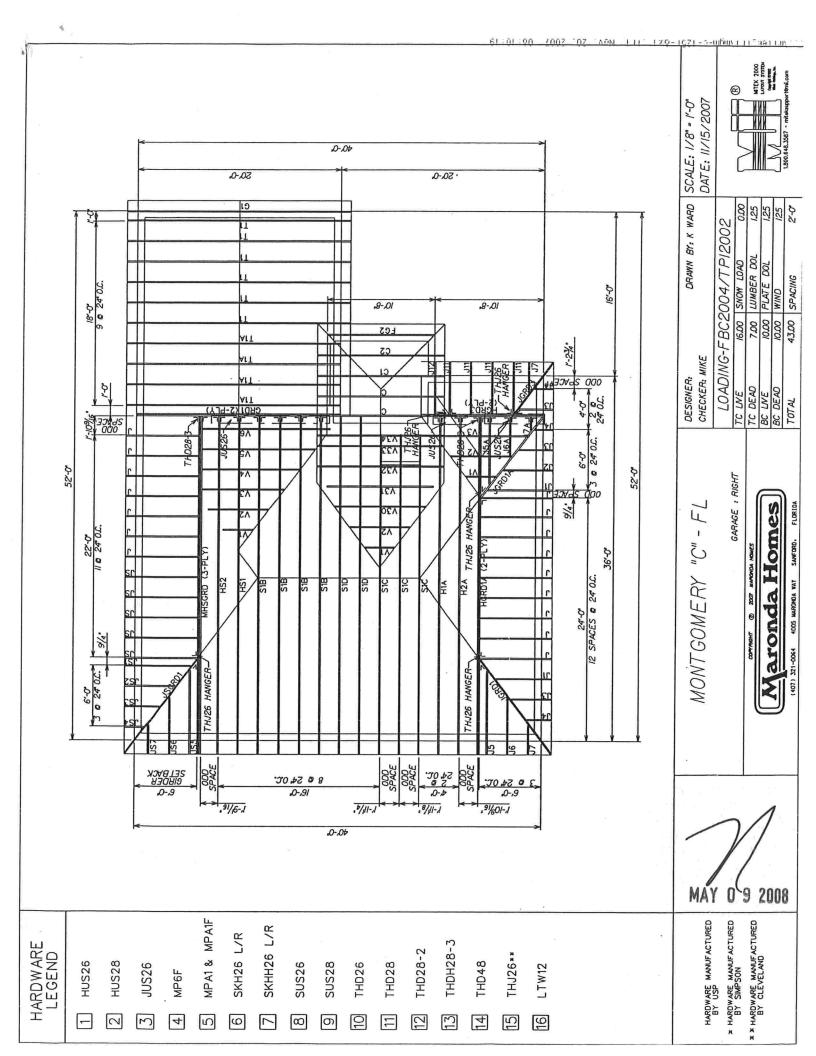
I, Tomas Ponce, P.E.

the Architect/Engineer of Record for the above referenced lot

Have reviewed the package and confirmed that it matches the physical and structural

Parameters found on the set of permit drawings.

Parameters fou	na on the set	of permit drawi	ngs.			·	
Truss ID	Run Date	Drawing	Truss ID	Run Date	Drawing	No. of Eng.	57
		Reviewed	Truss ID	Null Date	Reviewed	Dwgs:	
Layout	11/15/07	2/22/2008				Roof Loads-	
V	07/27/05	02/25/08				TC Live:	16.0 psf
HIP	11/02/06	02/25/08				TC Dead:	7.0 psf
						BC Live:	10.0 psf
С	01/23/08	2/22/2008	JS5	01/23/08	2/22/2008	BC Dead:	10.0 psf
C1	01/23/08	2/22/2008	JS6	01/23/08	2/22/2008	Total	43.0 psf
C2	01/23/08	2/22/2008	JS7	01/23/08	2/22/2008		
FG2	01/23/08	2/22/2008	JSGRD1	01/23/08	2/22/2008	DurFac- Lbr:	1.25
G1	01/23/08	2/22/2008	MHSGRD	01/23/08	2/22/2008	DurFac- Plt:	1.25
GRD1	01/23/08	2/22/2008	S1B	01/23/08	2/22/2008	O.C. Spacing:	24.0"
H1A	01/23/08	2/22/2008	S1C	01/23/08	2/22/2008		
H2A	01/23/08	2/22/2008	S1D	01/23/08	2/22/2008		
HGRD1A	01/23/08	2/22/2008	T1	01/23/08	2/22/2008		
HGRD3	01/23/08	2/22/2008	T1A	01/23/08	2/22/2008		
HS1	01/23/08	2/22/2008	V1	01/23/08	2/22/2008		
HS2	01/23/08	2/22/2008	V2	01/23/08	2/22/2008		
J	01/23/08	2/22/2008	V3	01/23/08	2/22/2008	,	
J1	01/23/08	2/22/2008	V30	01/23/08	2/22/2008		
J11	01/23/08	2/22/2008	V31	01/23/08	2/22/2008	1	
J12	01/23/08	2/22/2008	V32	01/23/08	2/22/2008	1	
J2	01/23/08	2/22/2008	V33	01/23/08	2/22/2008]	
J3	01/23/08	2/22/2008	V34	01/23/08	2/22/2008	1	
J4	01/23/08	2/22/2008	V4	01/23/08	2/22/2008	1	
J5	01/23/08	2/22/2008	V5	01/23/08	2/22/2008	1	
J5A	01/23/08	2/22/2008	V6	01/23/08	2/22/2008	1	
J6	01/23/08	2/22/2008				1	
J6A	01/23/08	2/22/2008				1	
J7	01/23/08	2/22/2008	INV#	DESC	QNTY		
J7A	01/23/08	2/22/2008	50060.0114	THD48			
JGRD1	01/23/08	2/22/2008	50060.0047	THD28		4	A
JGRD1A	01/23/08	2/22/2008	50060.0110	JUS26	11		
JGRD3	01/23/08	2/22/2008	50060.0058	THJ26	4		//
JS	01/23/08	2/22/2008	50060.0049	THD28-2	1		//
JS1	01/23/08	2/22/2008	50060.0050	THD28-3	1	1 /	
JS2	01/23/08	2/22/2008				1 /	. (
JS3	01/23/08	2/22/2008	SEAT PLAT	ES	111	1	
JS4	01/23/08	2/22/2008	F	r strawn		DATE: MAY	0 9 2008



Catesta and Minimus and Edit

GENERAL NOTES

Trusses are not marked in any way to Identify the frequency or location of temporary lateral restraint and diagonal bracing. Follow the recommendations for handling, installing and temporary restraining and bracing of trusses. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Brading of Metal Plate Connected Wood Trusses for more detailed information.

Truss Design Drawings may specify locations of permanent lateral restraint or reinforcement for individual truss members. Refer to the BCSI-B3 Summary Sheet - Permanent Restraint/Bracing of Chords & Web Members for more Information.

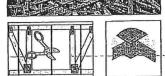
All other permanent bracing design is the responsibility of the Building Designer.

NOTAS GENERALES

Los trusses no están marcados de ningún modo que Identifique la frecuencia o localización de restricción lateral y arriostre diagonal temporales. Use las recomendaciones de manejo, Instalación, restricción y arriostre temporal de los trusses. Vea el folleto BCSI Guia de Buena Practica para el Manelo, Instalación, Restricción y Arriostre de los Trusses de Madera Conectados con Piacas de Metal para información más detallada.

Los dibujos de diseño de los trusses pueden especificar las localizaciones de restricción lateral permanente o refuerzo en los miembros individuales del truss. Vea la hoja resumen BC51-B3 - Restriction/Arriostre Permanente de Cuerdas y Miembros Secundarios para más Información. El resto de los diseños de arriostres permanentes son la responsabilidad del Diseñador del Edificio.





The consequences of improper handling, erecting, installing, restraining and bracing can result in a collapse of the structure, or worse, serious personal injury or death.

El resultado de un manejo, levantamiento, instalación, restricción y arrisotre incorrecto puede ser la caída de la estructura o aún peor, heridos o

Banding and truss plates have sharp edges. Wear gloves when handling and safety glasses when cutting banding.

Empaques y placas de metal tienen bordes afilados. Use quantes y lentes protectores cuando corte los empaques.

HOISTING RECOMMENDATIONS FOR TRUSS BUNDLES RECOMENDACIONES PARA LEVANTAR PAQUETES DE TRUSSES.

Marning! Don't overload the crane. iAdvertencial iNo sobrecarga la grúa!

Never use banding alone to lift a bundle. Do not lift a group of Individually banded bundles.

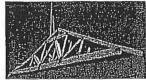
Nunca use sólo los empaques para levantar un paquete No levante un grupo de empaques individuales.

A single lift point may be used for bundles with trusses up to 45'. Two lift points may be used for bundles with

trusses up to 60'. Use at least 3 lift points for bundles with trusses greater than 60'.

Puede usar un solo lugar de levantar para paquetes de trusses hasta 45 pies. Puede usar dos puntos de levantar para paquetes más de 60 pies.

Use por lo menos tres puntos de levantar para paquetes más de 60 ples.



⚠ Warning! Do not over load supporting structure with truss bundle. IAdvertencial No sobrecargue la estructura

apoyada con el paquete de trusses. Place truss bundles in stable position

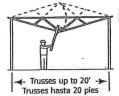
Puse paquetes de trusses en una posición

INSTALLATION OF SINGLE TRUSSES BY HAND INSTALACIÓN POR LA MANO DE TRUSSES INDIVIDUALES

Trusses 20' or less, support at peak. Levante

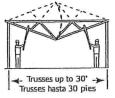
del pico los trusses de 20 pies o menos.

INDIVIDUALES



Trusses 30' or less, support at quarter points.

Levante de los cuartos de tramo los trusses de 30 pies o menos.



HANDLING — MANEJO

Avoid lateral bending. — Evite la flexión lateral.



The contractor is responsible for properly

mente los trusses en la obra

at the jobsite.

receiving, unloading and storing the trusses

El contratista tiene la responsabilidad de

recibir, descargar y almacenar adecuada-

stack of trusses at 8' to 10' on center.

allow for ventilation.

cover bundles to prevent moisture gain but

Refer to BCSI Guide to Good Practice for

of Metal Plate Connected Wood Trusses

handling and jobsite storage of trusses.

Si los trusses estarán guardados horizon-

suficiente detrás de la pila de los trusses.

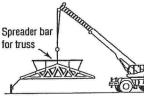
talmente, ponga bloqueando de altura

Handling, Installing, Restraining & Bracing.

for more detailed information pertaining to

Use special care in windy weather or near power lines and airports

Utilice cuidado especial en dias ventosos o cerca de cables eléctricos o de aeropuertos.



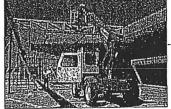
✓ Use proper rigging and hoisting equipment.

Do not store

upright.

unbraced bundles

Use equipo apropiado para levantar e improvisar



If trusses are to be stored horizontally, place blocking of sufficient height beneath the For trusses stored for more than one week,

No almacene verticalmente los trusses sueltos



Do not store on uneven around

tierra desigual.



HOISTING OF SINGLE TRUSSES — LEVANTAMIENTO DE TRUSSES INDIVIDUALI

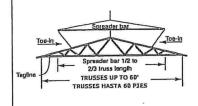
Hold each truss in position with the erection equipment until top chord temporary lateral restraint is installed and the truss is fastened to the bearing points.

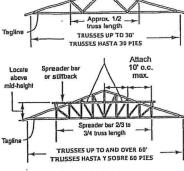
Sostenga cada truss en posición con equipo de grúa hasta que la restricción lateral temporal de la cuerda superior esté instalado y el truss está asegurado en los soportes.

Marning! Using a single pick-point at the peak can damage the truss.

¡Advertencia! El uso de un solo lugar para levantar en el pico puede hacer daño al truss.







60° or less

TEMPORARY RESTRAINT & BRACING RESTRICCIÓN Y ARRIOSTRE TEMPORAL

Refer to BCSI-BZ Summary Sheet - Truss Installation & Temporary Restraint/Bracing for more information. Vea el resumen BCSI B2 - Restricción/

Arriostre Temporal y Instalación de los Trusses para más información. Locate ground braces for first truss directly in

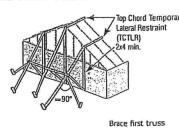
line with all rows of top chord temporary lateral restraint (see table in the next column).

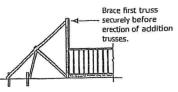
Coloque los arriostres de tierra para el primer truss directamente en linea con cada una de las filas de restricción lateral temporal de la cuerda superior (vea la table en la próxima columna).



Do not walk on unbraced

No camine en trusses





Para trusses guardados por más de una semana, cubra los paquetes para prevenir aumento de humedad pero permita ventilación.

Vea el folleto BCSI Guia de Buena Práctica para el Manejo, Instalación, Restricción y Arriostres de los Trusses de Madera Conectados con Plaças de Metal para información más detallada sobre el manejo y almacenado de los trusses en área de trabalo

BUILD WEST STREET OF STREET STREET

TEPS TO SETTING TRUSSES AS MEDIDAS DE LA INSTALLACIÓN DE LOS TRUSSES

- 1) Install ground bracing. 2) Set first truss and attach securely to ground bracing. 3) Set next 4 trusses with short member temporary lateral restraint (see below). 4) Install top chord diagonal bracing (see below). 5) Install web member plane diagonal bracing to stabilize the first five trusses (see below). 6) Install bottom chard temporary lateral restraint and diagonal bracing (see below). 7) Repeat process on groups of four trusses until all trusses are set.
 - 1) Instale los arriostres de tierra. 2) Instale el primero truss y ate seguramente al arriostre de tierra. 3) Instale los próximos cuatro trusses con restricción lateral temporal de miembro corto (vea abajo). 4) Instale el arriostre diagonal de la cuerda superior (vea abajo). 5) Instale arriostre diagonal para los planos de los miembros secundarios para estable los primeros cinco trusses (vea abajo). 6) Instale la restricción lateral temporal y amostre diagonal para la cuerda inferior (vea abajo). 7) Repita éste procedimiento en grupos de cuatro trusses hasta que todos los trusses estén instalados.
- Refer to BCSI-B2 Summary Sheet Truss Installation & Temporary Restraint/Bracing for more information.

Vea el resúmen BCSI-B2 - Instalación de Trusses y Arriostre Temporal para mayor información.

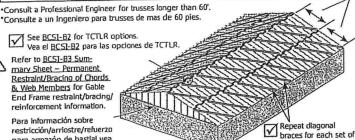
ESTRAINT/BRACING FOR ALL PLANES OF TRUSSES . RESTRICCIÓN/ARRIOSTRE EN TODOS PLANOS DE TRUSSES.

This restraint & bracing method is for all trusses except 3x2 and 4x2 parallel chord trusses.

Este método de restricción y arriostre es para todo trusses excepto trusses de cuerdas paralelas 3x2 v 4x2.

) TOP CHORD — CUERDA SUPERIOR

Truss Span	Top Chord Temporary Lateral Restraint (TCTLR) Spacing
Longitud de Tramo	Espaciamiento del Arriostre Temporal de la Cuerda Superior
Up to 30'	10' o.c. max.
Hasta 30 ples	10 pies máximo
30' to 45'	8' o.c. max.
30 a 45 pies	8 pies máximo
45' to 60'	6' o.c. max.
45 a 60 pies	6 pies máximo
60' to 80'*	4' o.c. max.
60 a 80 pies*	4 pies máximo

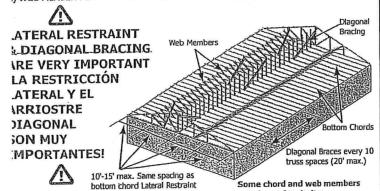


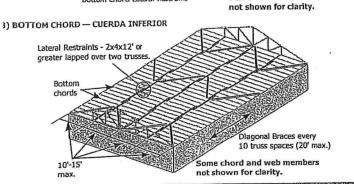
para armazón de hastial vea el resumen BCSI-B3 - Restricción/Arriostre Permanente de Cuerdas y Miembros Secundarios

Ground bracing not shown for clarity.

Repita los arrisotres diagonales para cada grupo de 4 trusses.

:) WEB MEMBER PLANE — PLANO DE LOS MIEMBROS SECUNDARIOS

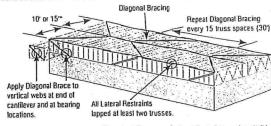




RESTRAINT & BRACING FOR 3x2 AND 4x2 PARALLEL CHORD TRUSSES LA RESTRICCIÓN Y EL ARRIOSTRE PARA TRUSSES DE CUERDAS PARALELAS 3X2 Y 4X2

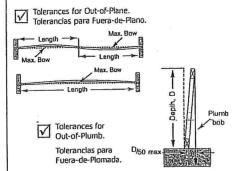
Refer to BCS1-B7 Summary Sheet - Temporary & Permanent Restraint/ Bracing for Parallel Chord Trusses for more information.

> Vea el resumen BCSI-B7 - Restricción y Arriostre Temporal y Permanente de Trusses de Cuerdas Paralelas para más información.



*Top chord Temporary Lateral Restraint spacing shall be 10' o.c. max, for 3x2 chords and 15' o.c. for 4x2 chords.

INSTALLING — INSTALACION



Outqi	
D/50	D (fL)
1/4-	1.
1/2*	2,
3/4"	3,
1-	4'
1-1/4"	5'
1-1/2"	6'
1-3/4	7'
2-	≥8'

はないない。	251251
Max. Bow	Truss Length
3/4*	12.5'
7/8*	14.6'
1*	16.7
1-1/8*	18.8
1-1/4*	20.8
-:1-3/8*	22.9
1-1/2*	25.0
1-3/4"	29.2
2-	≥33.3*

Height

12"

15*

CONSTRUCTION LOADING — CARGA DE CONSTRUCCION

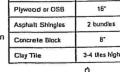
Do not proceed with construction until all lateral restraint and bracing is securely and properly in place.

No proceda con la construcción hasta que todas las restricciones laterales y los arriostres estén colocados en forma aproplada y segura.

Do not exceed maximum stack heights. Refer to BCS1-B4 Summary Sheet - Construction Loading for more information.

No exceda las máximas alturas recomendadas. Vea el resúmen BCSI-B4 Carga de Construcción para mayor información.





Material

Gypsum Board

Do not overload small groups or single trusses. No sobrecargue pequeños grupos o trusses individuales.

Never stack materials near a peak. Nunca amontone material cerca del pico.

Place loads over as many trusses as possible. Coloque las cargas sobre tantos trusses como sea posible

Position loads over load bearing walls. Coloque las cargas sobre las paredes soportantes.

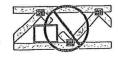


ALTERATIONS — ALTERACIONES

Refer to BCST-B5 Summary Sheet - Truss Damage, Jobsite Modifications & Installation Errors. Vea el resúmen BCSI-B5 Daños de trusses, Modificaciones en la Obra y Errores de Instalación.

Do not cut, alter, or drill any structural member of a truss unless specifically permitted by the Truss Design Drawing.

No corte, altere o perfore ningún miembro estructural de los trusses, a menos que esté específicamente permitido en el dibujo del diseño del truss.



Trusses that have been overloaded during construction or altered without the Truss Manufacturer's prior approval may render the Truss Manufacturer's limited warranty null and void.

Trusses que se han sobrecargado durante la construcción o han sido alterados sin una autorización previa del Fabricante de Trusses, pueden reducir o eliminar la garantia del Fabricante de Trusses.

NOTE: The Truss Manufacturer and Truss Designer rely on the presumption that the Contractor and crane operator (if applicable) are professionals with the capability to undertake the work they have agreed to do on any given project. If the Contractor believes it needs assistance in some aspect of the construction project, it should seek assistance from a competent party. The methods and procedures outlined in this document are intended to ensure that the overall construction techniques employed will put the trusses into place SAFELY. These recommendations for handling, installing, restraining and briding trusses are based upon the collective experience of leading personnel involved with truss design, manufacture and installation, that must, due to the nature of responsibilities involved, be presented only as a GUIDE for use by a qualified Building Designer or Contractor. It is not Intended that these recommendations be interpreted as superior to the Building Designer's design specification for handling, installing, restraining and bracing trusses and it does not precide the use of other equivalent methods for restraining/briding and providing stability for the walks, columns, floors, roots and all the Intervalent estimation of the properties of the prop



00 Enterprise Lane • Madison, WI 53719 608/274-4849 • www.sbcindustry.com



218 N. Lee St., Ste. 312 - Alexandira, VA 22314 703/683-1010 = w ww.tplnst.org

B1WARN11x17 20061115



MARONDA SYSTEMS

4005 Maronda Way

Sanford, FL 32771

(407) 321-0064

Fax (407) 321-3913

Date: November 1, 2006

To: Building Department

From: Maronda Systems

Tomas Ponce

Professional Engineer State of Florida #0050068

Subject: Valley Trusses

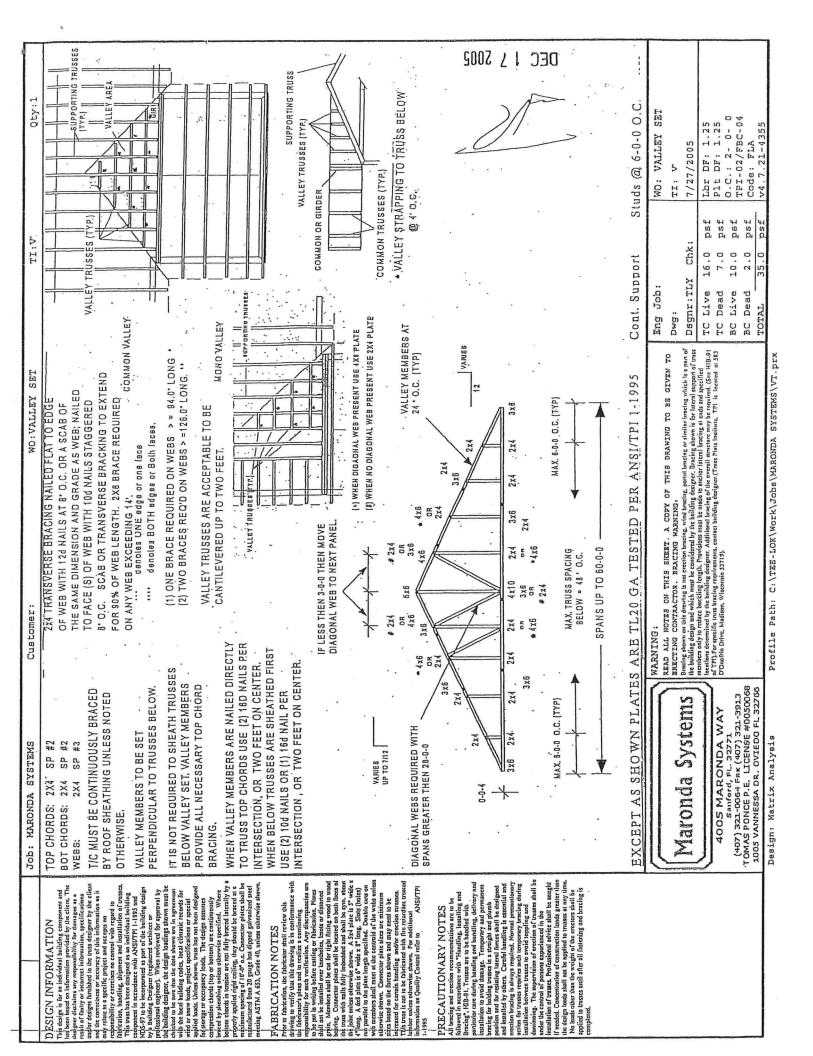
All valley trusses labeled V-1 through 100 are covered under the general valley sheet provided in the truss package signed and sealed by the engineer of record. The connections are noted on the structural info sheet of the plans. All criteria of the valley trusses are noted on the general sheet.

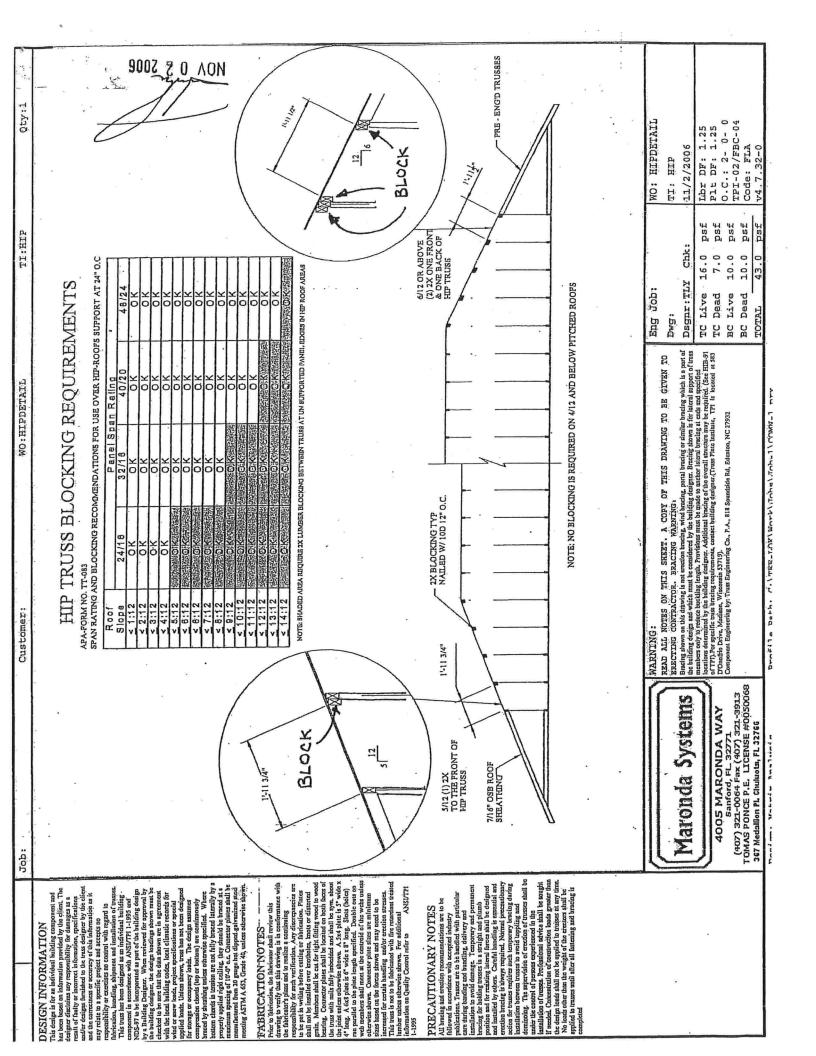
If you have any questions please feel free to call at 407-321-0064.

Sincerely,

Tomas Ponce, P.E.

Date: 11/1/06









Trenco 818 Soundslde Rd Edenton, NC 27932

Re: MONTGOMERY

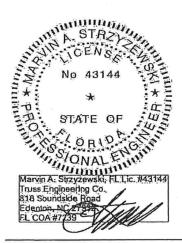
MONTGOMERY_FLORIDA_125

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Maronda Homes-Pittsburgh, PA.

Pages or sheets covered by this seal: E4632329

thru E4632392

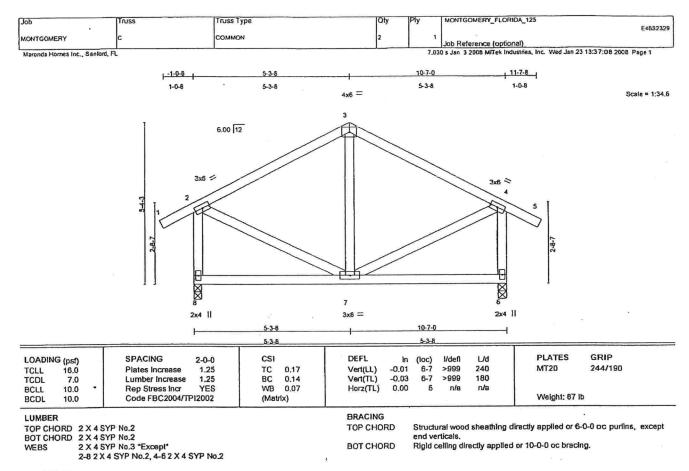
My license renewal date for the state of Florida is February 28, 2009.



January 23,2008

Strzyzewski, Marvin

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2. Engineering services provided by Truss Engineering Company.



REACTIONS (lb/size) 8=500/0-3-0, 6=500/0-3-8

Max Horz 8=69(LC 6)

Max Uplift8=143(LC 6), 6=143(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/26, 2-3=-340/160, 3-4=-340/160, 4-5=0/26, 2-8=-402/293, 4-6=-402/293 TOP CHORD

7-8=76/66, 6-7=12/56 BOT CHORD

3-7=0/96, 2-7=-38/230, 4-7=-38/230 WEBS

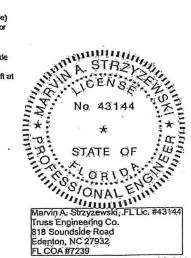
1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 143 lb uplift at joint 8 and 143 lb uplift at joint 6.

LOAD CASE(S) Standard



January 23,2008

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLIDED MITEE REFERENCE PAGE MII-7473 BEFORE USE. Design vold for use only with MiTek connectors. This design is based only upon parameters and middle building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trus designer. Brocing shown is for follers support of individual web members only. Additional temporary brocing to insure shirly during construction is the responsibility of the establish during construction is the responsibility of the establish during constructions is the responsibility of the establish during constructions are proposed in the supersisting of the building designer. For general guidance regarding tobrication, quality control, storage, delivery, srection and brocing, comult — MSI/IPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Trus Plate Institute, 583 D'Onotico Divie, Modison, WI 53719.



Edenton, NC 27932

Job Truss Truss Type MONTGOMERY_FLORIDA_125 F4632330 MONTGOMERY SPECIAL Job Reference (optional) 7.030 s Jan 3 2008 MITek Industries, Inc. Wed Jan 23 13:37:08 2008 Page 1 Maronda Homes Inc., Sanford, FL 1-0-8 3-11-8 11-7-8 1-0-8 3-11-8 2-8-0 3-11-8 1-0-8 Scale = 1:34.5 4x6 = 6.00 12 3x4 = 3x5 = 375 D-7-B 10 3x4 = 3x8 = 2x4 || 12 3x4 = 2x4 || 3-11-8 10-7-0 6-7-8 3-11-8 2-8-0 3-11-8 Plate Offsels (X.Y): [4:0-5-4.0-2-0] LOADING (psf) SPACING CSI DEFL in **Vdefl** PLATES (loc) L/d GRIP TCLL 16.0 Plates Increase 1.25 TC BC -0.01 0.23 Vert(LL) >999 240 244/190 MT20 Lumber Increase 1.25 7.0 0.12 Vert(TL) -0.02 8-9 >999 180 BCLL 10.0 Rep Stress Incr YES WB 0.10 Horz(TL) 0.00 n/a n/a BCDL Code FBC2004/TPI2002 10.0 (Matrix) Weight: 86 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 *Except* end verticals. BOT CHORD Rigld ceiling directly applied or 10-0-0 oc bracing, Except: 2-13 2 X 4 SYP No.2, 6-8 2 X 4 SYP No.2 6-0-0 oc bracing: 12-13. REACTIONS (lb/size) 8=500/0-3-8, 13=500/0-3-8 Max Horz 13=-59(LC 7) Max Uplift8=-223(LC 7), 13=-223(LC 6) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/26, 2-3=-189/218, 3-4=-302/355, 4-5=-256/378, 5-6=-329/339, 6-7=0/26, 2-13=-466/478, 6-8=-419/514

WEBS NOTES

BOT CHORD

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; partially; MWFRS (low-rise) and C-C Exterior(2) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

10-11=99/159, 9-10=126/247, 8-9=15/30, 11-12=0/20, 3-11=204/223, 12-13=45/9 4-10=9/58, 4-9=42/47, 5-9=34/138, 3-10=50/133, 6-9=144/266, 11-13=20/77, 2-11=193/309

- Provide adequate drainage to prevent water ponding.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wilde will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 223 lb uplift at joint 8 and 223 lb uplift at joint 13.

LOAD CASE(S) Standard



January 23,2008

🛕 WARNING - Verily design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design void for use only with Millex connectors. This design is based only upon pormettes shown, and is for an individual busking component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not fines designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing is insure statistically during construction is the negarostibility of the erector. Additional permanent bracing of the overal structure is the responsibility of the building designer. For general guidance regarding fabilities, adult of the control structure is the responsibility of the undifficult of the property bracing of the support of the property bracing designer. For general guidance regarding fabilities, and a NSI/TPII Quality Citleria, DSS-89 and BCSII Building Component Safety Information available from Truss Plate Institute, SSS D'Onatrio Drive, Mocfean, W153719.



russ Type MONTGOMERY FLORIDA 125 E4632331 PECIAL ONTGOMERY Job Reference (optional) 7.030 s Jan 3 2008 Mittek Industries, Inc. Wed Jan 23 13:37:09 2008 Page Maronda Homes Inc., Senford, FL 1-1-0-8 10-7-0 1 11-7-8 1-0-8 1-0-8 1-11-8 6-8-0 1-11-8 4x10 = Scale = 1:30,8 4x8 = 6.00 12 3x4 = 5 3x4 = 4x6 🗢 P-0-8 3x8 = 2x4 || 12 6x12 = = 2x4 II 10-7-0 1-11-8 8-7-8 Plate Offsets (X.Y): [4:0-6-8.0-1-12], [5:0-5-0.0-2-0], [6:0-2-15.0-2-0] LId DEFL LOADING (psf) SPACING 2-0-0 (loc) -0.03 9-10 >999 240 MT20 244/190 16.0 Plates Increase 1.25 TC 0.53 Vert(LL) TCLL TCDL Lumber Increase 1.25 BC 0.25 Vert(TL) -0.08 9-10 >999 180 WB BCLL 100 Rep Stress Incr YES 0.17 Horz(TL) 0.01 n/a n/a Code FBC2004/TP12002 Weight: 81 lb BCDL 10.0 (Matrix) BRACING LUMBER Structural wood sheathing directly applied or 6-0-0 oc purlins, except TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 TOP CHORD WEBS 2 X 4 SYP No.3 "Except" **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 4-9 2 X 4 SYP No.2, 2-13 2 X 4 SYP No.2, 6-8 2 X 4 SYP No.2 6-0-0 oc bracing: 8-9. REACTIONS (lb/size) 8=500/0-7-8, 13=500/0-3-8 Max Horz 13=44(LC 7) Max Uplift8=208(LC 7), 13=-208(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/26, 2-3=-157/214, 3-4=-236/202, 4-5=-267/323, 5-6=-286/276, 6-7=0/26, 2-13=-444/495, 6-8=-527/493

BOT CHORD

10-11=-60/109, 9-10=-182/276, 8-9=-23/35, 11-12=0/14, 3-11=-364/87, 12-13=-5/43 4-10=-233/537, 4-9=-21/29, 5-9=-132/331, 6-9=-351/470, 11-13=-43/26, 2-11=-141/251, 3-10=-455/544 WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; pertially; MWFRS (low-rise) and C-C Exterior(2) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide
- will fit between the bottom chord and any other members.
 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 208 lb uplift at joint 8 and 208 lb uplift at joint 13.

LOAD CASE(S) Standard



January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Miles connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not fuss designer. Bracing shown is for taleral support of individual web members only. Additional temporary bracing to insure slightly during construction is the responsibility of the service. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fobtaction, quality control, storage, defervey, erection and bracing, consult. AMSI/IPI Quality Criteria, DSB-89 and BCSII Building Component Safety Information.



Truss Truss Type MONTGOMERY_FLORIDA_125 MONTGOMERY G2 GABLE Job Reference (optional) 7.030 s Jan 3 2008 MiTek Industries, Inc. Wed Jan 23 13:37:10 2008 Page 1 Maronda Homes Inc., Senford, Fl

10-7-0 10-7-0 2x4 11 2x4 11 2x4 || 2x4 11 2x4 || 2x4 11 3x4 = 2 3 5 2x4 || 2x4 || 3x4 = 2x4 II 2x4 || 2x4 II 2x4 ||

0-7-0			10-7-0	 				
0-7-0			10-0-0				·	
LOADING (psf) TCLL 16.0 TCDL 7.0 BCLL 10.0 BCDL 10.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr NO Code FBC2004/TPI2002	CSI TC 0.05 BC 0.02 WB 0.05 (Matrix)	DEFL in Vert(LL) n/a Vert(TL) n/a Horz(TL) 0.00	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 53 lb	GRIP 244/190	

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 WEBS 2 X 4 SYP No.3 OTHERS 2 X 4 SYP No.3

BRACING

BOT CHORD

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=71/10-0-0, 9=175/10-0-0, 10=172/10-0-0, 11=170/10-0-0, 12=179/10-0-0, 13=118/10-0-0 Max Uplift8=47(LC 4), 9=-116(LC 4), 10=-114(LC 4), 11=-112(LC 4), 12=-118(LC 4), 13=-78(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-14=9/0, 1-2=-4/4, 2-3=-6/5, 3-4=-6/5, 4-5=-6/5, 5-6=-6/5, 6-7=-6/5, 7-8=-38/68

BOT CHORD 13-14=-4/4, 12-13=-4/4, 11-12=-5/6, 10-11=-5/6, 9-10=-5/6, 8-9=-5/8

WEBS 6-9=93/174, 5-10=92/169, 4-11=91/167, 3-12=98/177, 2-13=60/124, 2-12=1/3

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; partially; MWFRS (low-rise) gable end zone and C-C Exterior(2) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 6) Gable studs spaced at 2-0-0 oc.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 47 lb uplift at joint 8, 116 lb uplift at joint 9 114 lb uplift at joint 10, 112 lb uplift at joint 11, 118 lb uplift at joint 12 and 78 lb uplift at joint 13.
- 9) Non Standard bearing condition. Review required.

LOAD CASE(S) Standard



FL COA #7239

January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII.7473 BEFORE USE. Design vold for use only with Miles connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not iturs designer. Brocing show it for loteral support of individual web members only. Additional temporary brocing to insure slobility during construction is the responsibility of the erector. Additional permanent brocing at the overal structure is the responsibility of the building designer. For general guidance regarding fobiocition, qualify control, storage, delivery, erection and brocing, consult. AMSU/IPI1 Quality Citierla, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, SSB D'Onotrio Drive. Modison. WI SS719.



818 Soundside Road Edenton, NC 27932

MONTGOMERY FLORIDA 125 russ Type Job GABLE MONTGOMERY Job Reference (optional) 7.030 s Jan 3 2008 MITek Industries, Inc. Wed Jan 23 13:37:11 2008 Page 1 Maronda Homes Inc., Sanford, FL 21-0-0 20-0-0 10-0-0 1-1-0-0 1-0-0 10.00 10-0-0 1-0-0 Scale # 1:38 7 4x6 = 2x4 || 2x4 || 6.00 12 2x4 || 2x4 || 2x4 || 2x4 11 2x4 || 2x4 || 11 19 18 17 16 15 3x4 = 3x4 = 2x4 || 2x4 II 2x4 11 2x4 || 2x4 || 2x4 || 2x4 || 2x4 11 20-0-0 0-0-0 PLATES GRIP DEFL LOADING (psf) SPACING l/defl 2-0-0 (loc) Vert(LL) -0.00 13 n/r 120 MT20 244/190 TCLL Plates Increase 1.25 TÇ 0.13 16.0 Lumber Increase 1.25 BC 0.08 Vert(TL) -0.00 13 n/r 120 TCDL 7.0 0.00 WB 12 n/a n/a BCLL 10.0 Rep Stress Incr NO 0.04 Horz(TL) Weight: 101 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0 BRACING LUMBER Structural wood sheathing directly applied or 6-0-0 oc purlins. TOP CHORD TOP CHORD 2 X 4 SYP No.2 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD 2 X 4 SYP No.2 **OTHERS** REACTIONS (lb/size) 2=139/20-0-0, 12=139/20-0-0, 18=253/20-0-0, 19=210/20-0-0, 20=167/20-0-0, 21=173/20-0-0, 22=174/20-0-0, 17=210/20-0-0, 16=167/20-0-0, 15=173/20-0-0, 14=174/20-0-0 Max Horz 2=100(LC 6) Max Up#f2=57(LC 6), 12=77(LC 7), 19=61(LC 6), 20=63(LC 6), 21=68(LC 6), 22=41(LC 6), 17=59(LC 7), 16=63(LC 7), 15=68(LC 7), 14=41(LC 7)

Max Grav2=138(LC 1), 12=138(LC 1), 18=253(LC 1), 19=214(LC 10), 20=167(LC 1), 21=173(LC 10), 22=174(LC 10), 17=214(LC 11), 16=167(LC 1), 15=173(LC 11), 14=174(LC 11) FORCES (lb) - Maximum Compression/Maximum Tension 1-2=0/21, 2-3=93/43, 3-4=50/60, 4-5=32/89, 5-6=32/127, 6-7=32/176, 7-8=32/176, 8-9=32/127, 9-10=32/74, 10-11=-32/23, 11-12=51/14, 12-13=0/21 TOP CHORD 2-22=0/108, 21-22=0/108, 20-21=0/108, 19-20=0/108, 18-19=0/108, 17-18=0/108, 16-17=0/108, 15-16=0/108, BOT CHORD 14-15=0/108, 12-14=0/108

7-18=82/0, 6-19=96/92, 5-20=92/102, 4-21=92/104, 3-22=93/80, 8-17=96/92, 9-16=92/102, 10-15=92/104, WEBS

11-14=-93/80

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

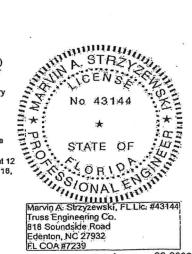
5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 2-0-0 oc.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 2, 77 lb uplift at joint 12, 61 lb uplift at joint 19, 63 lb uplift at joint 20, 69 lb uplift at joint 21, 41 lb uplift at joint 22, 59 lb uplift at joint 17, 63 lb uplift at joint 18, 68 lb uplift at joint 15 and 41 lb uplift at joint 14.

LOAD CASE(S) Standard



January 23,2008

neters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Miller connection. This design is based only upon porameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trus designer. Bracing sh for Interest support of individual web members only. Additional temporary bracing to Insue statisty during construction is the responsibility of ereclar. Additional permonent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fortheration, quadity control, strategic delivery, erection and bracing, consult. AMSI/IPTI Quality Criteria, DS3-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, S83 D'Onofrio Drive, Modison, WI S3719.





Job Truss Truss Type MONTGOMERY_FLORIDA_125 MONTGOMERY GRD1 COMMON Job Reference (optional) 7.030 s Jan 3 2008 MiTek Industries, Inc. Wed Jan 23 13:37:12 2008 Page 1 10-0-0 15-5-10 20-0-0 3-6-6 2-7-14 3-9-12 3-9-12 2-7-14 Scale = 1:37.2 4x6 || 6.00 12 3x6 = 3x6 > 3x4 > 12 11 13 10 14 9 15 8 16 6x12 = 6x8 = 3x6 || 10x10 = 6x8 = 3x6 11 10-0-0 13-9-12 16-5-10 20-0-0 3-6-6 2-7-14 3-9-12 3-9-12 2-7-14 3-6-6 Plate Offsets (X.Y): [1:0-47.Edge]. [2:0-0-0.0-0-0]. [3:0-0-0.0-0-0]. [5:0-0-0.0-0-0]. [6:0-0-0.0-0-0]. [7:0-4-7.Edge]. [9:0-3-8.0-4-8]. [10:0-5-0.0-6-0]. [11:0-3-8.0-4-8] LOADING (psf) SPACING 2-0-0 CSI DEFL in (loc) Vdefi L/d PLATES GRIP TCLL 16.0 1.25 TC 0.51 0.17 10-11 >999 Vert(LL) 240 MT20 244/190 BC WB TCDL 7.0 Lumber Increase 1.25 0.82 Vert(TL) 9-10 180 BCLL 10.0 Rep Stress Incr NO 0.66 Horz(TL) 0.07 n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 268 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-5-6 oc purlins. 2 X 8 SYP No.1D **BOT CHORD** Rigid ceiling directly applied or 9-7-12 oc bracing. 2 X 4 SYP No.3 "Except" WEBS 4-10 2 X 4 SYP No.2

REACTIONS (lb/size) 1=5315/0-8-0, 7=6787/0-8-0

Max Horz 1=81(LC 4) Max Uplift1=2359(LC 5), 7=2782(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=10451/4691, 2-3=10980/4991, 3-4=8298/3594, 4-5=8299/3595, 5-6=11067/4636, 6-7=12245/5056

1-12=4199/9254, 11-12=4199/9254, 11-13=4442/9805, 10-13=4442/9805, 10-14=4052/9888, 9-14=4052/9888, 9-15=4451/10854, 8-15=4451/10854, 8-16=4451/10854, 7-16=4451/10854 BOT CHORD

WEBS

2-12=571/311, 2-11=367/713, 3-11=1571/2956, 3-10=3047/1683, 4-10=3069/7142, 5-10=3152/1282,

5-9=1161/3048, 6-9=1148/471, 6-8=420/1168

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2 X 8 - 2 rows at 0-4-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Unbalanced roof live loads have been considered for this design.

4) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.33 plate grip DOL=1.33.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2359 lb uplift at joint 1 and 2782 lb uplift at Joint 7.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 2528 lb down and 1528 lb up at 6-2-4, 1317 lb down and 563 lb up at 8-0-13, 1317 lb down and 537 lb up at 10-0-13, 1320 lb down and 550 lb up at 12-0-13, 1320 lb down and 550 lb up at 14-0-13, and 1320 lb down and 550 lb up at 16-0-13, and 1320 lb down and 550 lb up at 18-0-13 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

Continued on page 2

No 43144 Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

January 23,2008

WARRING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-1473 BEPORE USE.

Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer- not inus designer, Bracing shown for fateral support of individual web members only. Additional temporary bracing to insure shifty during construction is the responsibility of the building designer, for general guidance regarding fabrication, qualify control, strange, delivery, excelled and bracing, consul
ANSUTRIT Quality Criteria, DSS-89 and SCS11 Sullding Component Safety Information available from Trus Plate Institute, SSS D'Onohifo Drive, Madison, WI SS719.



Job	Truss	Truss Type	Qhy	Ply	MONTGOMERY_FLORIDA_125	
MONTGOMERY	GRD1	COMMON	1	١.		E4632334
Mamada Hamas Inc. Sa					Job Reference (optional)	

7.030 s Jan 3 2008 MiTek Industries, Inc. Wed Jan 23 13:37:12 2008 Page 2

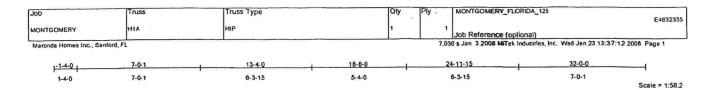
LOAD CASE(S) Standard Uniform Loads (pif)

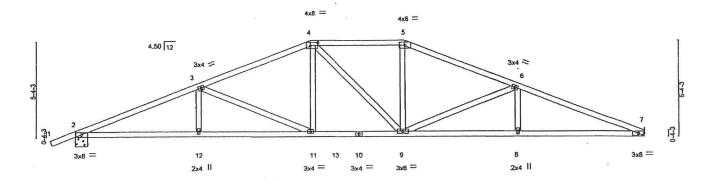
Vert: 1-4=-46, 4-7=-46, 1-7=-40

Concentrated Loads (lb)

Vert: 11=2528(B) 10=1317(B) 9=1320(B) 13=1317(B) 14=1320(B) 15=1320(B) 16=1320(B)







	7-0-1		1.	6-3-15	5-4-0			6-3-15			7-0-1		
Plate Of	isets (X.)	Y): [4:0-5-4.0-2-0]		,							T	
LOADIN TCLL TCDL BCLL	IG (psf) 16.0 7.0 10.0	•	SPACING Plates Increase Lumber Increase Rep Stress Incr	2-0-0 1.25 1.25 YES	CSI TC BC WB	0.46 0.83 0.62	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.22 -0.44 0.15	(loc) 8-9 8-9 7	l/defi >999 >865 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCDL	10.0		Code FBC2004/TI	P12002	(Mat	rlx)	100000000000000000000000000000000000000					Weight: 151 l	b

TOP CHORD

BOT CHORD

LUMBER

WEBS

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 "Except"

3-11 2 X 4 SYP No.2, 4-9 2 X 4 SYP No.2, 8-9 2 X 4 SYP No.2

REACTIONS (lb/size) 7=1378/Mechanical, 2=1467/0-8-0

Max Horz 2=108(LC 6)

Max Uplift7=277(LC 5), 2=373(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-3107/915, 3-4=-2355/784, 4-5=-2167/785, 5-6=-2353/791, 6-7=-3178/986 TOP CHORD

2-12=776/2837, 11-12=776/2837, 11-13=544/2171, 10-13=544/2171, 9-10=544/2171, 8-9=851/2931, BOT CHORD

7-8=-851/2931

3-12=0/288, 3-11=-744/251, 4-11=-33/552, 4-9=-149/133, 5-9=-50/551, 6-9=-845/328, 6-8=0/305

WEBS

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind; ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 277 lb uplift at joint 7 and 373 lb uplift at joint 2.

LOAD CASE(S) Standard



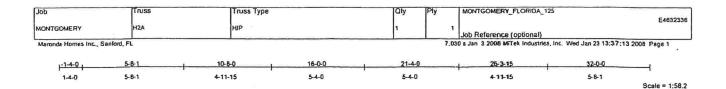
FL COA #7239

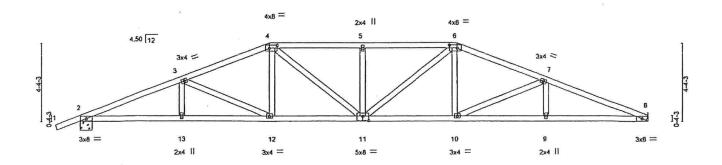
Structural wood sheathing directly applied or 3-1-4 oc purilins. Rigid ceiling directly applied or 6-6-12 oc bracing.

January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 BEFORE USE. Design valid for use only with Miles connectors. This design is based only upon parameters show, and is to rain individual building component. Applicability of design prometers and proper incorporation of component is responsibility of building designer- not thus designer. Bracing shows it for brieffold support of Individual web members only. Additional temporary bracing to insure slobity during construction is the responsibility of the service. Additional permanent bracing of the overal structure is the responsibility of the building designer. For general guidance regarding fobication, quality control, storage, delivery, execution and bracing, consult. AMSI/[FII] Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Trusi Piote Institute, SSB D'Onafrio Drive, Madison, WI 53719.







	-		5-8-1	10-8-0		16-0-0		21-4-0	1	26-3-15	32-0-	.0	
			5-8-1	4-11-15		5-4-0		5-4-0		4-11-15	5-8-	1	
Plate Of	isets (X.)	0:-[4:0-5-4.0-2-0]. [6:0-5-4.0	-2-0]. [11:0-4	-0.0-3-0]								
LOADIN	IG (psf)		SPACING	2-0-0	CSI		DEFL	in (loc)	Vdefl	Ľd	PLATES	GRIP	
TCLL	16.0		Plates Increase	1.25	TC	0.31	Vert(LL)	-0.23 10-11	>999	240	MT20	244/190	
TCDL	7.0		Lumber Increase	1.25	BC	0.74	Vert(TL)	-0.45 10-11	>841	180			
BCLL	10.0	•	Rep Stress Incr	YES	WB	0.34	Horz(TL)	0.15 8	n/a	n/a			
BCDL	10.0		Code FBC2004/TI	PI2002	(Mati	íx)					Weight: 156 II	b	

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 "Except" WEBS

4-11 2 X 4 SYP No.2, 6-11 2 X 4 SYP No.2

REACTIONS (lb/size) 8=1357/Mechanical, 2=1440/0-8-0

Max Horz 2=93(LC 6)
Max Uplift8=292(LC 5), 2=389(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-3076/945, 3-4=-2557/884, 4-5=-2680/982, 5-6=-2680/982, 6-7=-2594/906, 7-8=-3200/1042

1-z=0/21, z-3=-30/16/945, 3-4=-255/1684, 4-5=-2680982, 5-6=-2680992, 6-7=-2594900, 7-6=-3200/1042 2-13=812/2808, 12-13=-812/2808, 11-12=677/2372, 10-11=697/2407, 9-10=914/2954, 8-9=914/2954 3-13=0/213, 3-12=498/163, 4-12=-5/408, 4-11=-146/486, 5-11=-248/227, 6-11=-120/449, 6-10=-34/449, 7-10=-617/238 **BOT CHORD**

WEBS

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=5.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) Provide adequate drainage to prevent water ponding.4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 ib uplift at joint 8 and 389 ib uplift at

LOAD CASE(S) Standard



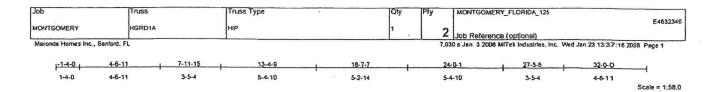
Structural wood sheathing directly applied or 3-3-5 oc purlins.

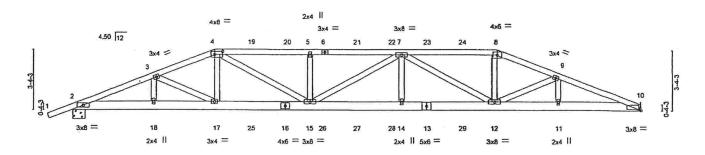
Rigid celling directly applied or 6-5-8 oc bracing.

January 23,2008

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Millex connection. This design is based only upon portunelers shown, and is for an individual bublishing component, Applicability of design parameters and proper incorporation of component is responsibility of building designer- not trus designer. Broche shown is for lateral support of individual web membes only. Additional temporary bracing to insure shiftly during construction is the responsibility of the erector. Additional permanent proching of these shortly during construction is the responsibility of the erector. Additional permanent proching of the everal structure is the responsibility of the building designer, for general guidance regarding clarication, quality control, storage, delivery, sercicin and traction, constit. AMSI/IPII Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information.







	-	9-0	-11 -15		13-4-8		18-7-7	1 2	4-0-1		2/-5-5	32-0-0	
		4-6	-11 3-5-4		5-4-10		5-2-14	. 5	4-10	•	3-5-4	4-6-11	
Plate Of	fsets (X.Y): [4:	0-5-8.0-2-0] [10:0-1-8.	Edge]									
LOADIN	NG (psf) 16.0		SPACING Plates Increase	2-0-0 1.25	CSI	0.40	DEFL Vert(LL)	in (loc) 0.33 14-15	I/defl · >999	L/d 240	PLATES MT20	S GRIP 244/190	
TCDL	7.0 10.0		Lumber Increase Rep Stress Incr	1.25 NO	BC WB	0.65 0.32	Vert(TL) Horz(TL)	-0.58 14-15 0.12 10	>676 n/a	180 n/a	WITZO	244/190	
BCDL	10.0		Code FBC2004/TF		(Mati		noiz(1L)	0.12 10	IVa	ma	Weight:	363 lb	

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.2

2 X 4 SYP No.3

REACTIONS (lb/size) 10=2663/Mechanical, 2=2724/0-8-0

Max Horz 2=81(LC 5)

Max Uplift10=1231(LC 8), 2=1293(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/24, 2-3=-6438/2939, 3-4=-6463/3120, 4-19=-7627/3687, 19-20=-7627/3687, 5-20=-7627/3687, 5-6=-7627/3687,

6-21=-7627/3687, 21-22=-7627/3687, 7-22=-7627/3687, 7-23=-6362/3199, 23-24=-6362/3199, 8-24=-6362/3199,

8-9-6898/3337, 9-10-6875/3273

BOT CHORD 2-18=-2731/5943, 17-18=-2731/5943, 17-25=-2895/6098, 16-25=-2895/6098, 15-16=-2895/6098, 15-26=-3680/7760,

26-27=3680/7760, 27-28=3680/7760, 14-28=3680/7760, 13-14=3680/7760, 13-29=3680/7760, 12-29=3680/7760, 11-12=3023/6402, 10-11=3023/6402

WEBS 3-18-62/168, 3-17-214/255, 4-17-448/1039, 4-15-884/1812, 5-15-414/270, 7-15-208/142, 7-14-129/434,

7-12-1677/758, 8-12-921/1975, 9-12-154/31, 9-11-17/136

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all piles, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.33 plate grlp DOL=1.33.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psr bottom chord live load nonconcurrent with any other live loads.
 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-8-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1231 lb uplift at joint 10 and 1293 lb uplift at joint 2.

Continued on page 2



Structural wood sheathing directly applied or 4-8-8 oc purlins.

Rigid ceiling directly applied or 8-11-2 oc bracing.

January 23,2008

🗥 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Mille k connections. This design is based only upon poromeles shown, and is for an individual bubliding component. Applicability of design poromenters and proper incorporation of component is responsibility of building designer - not thus designer, Brocing shown is for lateral support of Individual web members only. Additional temporary bracing to insure solidity during construction is the responsibility of the erector. Additional permanent brocing at the overal structure is the responsibility of the building designer, For general guidance regarding laboration, acustif — AMSI/(PII) Quality Criteria, DSS-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, S83 D'Onofrio Drive, Modison, Wt 53719.



Job	Truss	Truss Type	Oty	Ply	MONTGOMERY_FLORIDA_125	E4632340
MONTGOMERY	HGRD1A	HIP	1	2	Job Reference (optional)	L-1002010

Maronda Homes Inc., Sanford, FL

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NOTES

10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 69 lb down and 69 lb up at 7-11-15, 69 lb down and 69 lb up at 10-0-11,) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) by a to down and 69 ib up at 12-0-11, 69 ib down and 69 ib up at 14-0-11, 69 ib down and 69 ib up at 14-0-11, 69 ib down and 69 ib up at 18-0-11, 69 ib down and 69 ib up at 18-0-11, 69 ib down and 69 ib up at 18-0-11, 69 ib down and 69 ib up at 18-0-11, 69 ib down and 69 ib up at 18-0-11, 78 ib down and 69 ib up at 18-0-11, 78 ib down and 69 ib up at 18-0-11, 78 ib down and 69 ib up at 18-0-11, 78 ib down and 68 ib up at 18-0-11, 80 ib down and 88 ib up at 18-0-11, 80 ib down and 88 ib up at 18-0-1 is the responsibility of others.

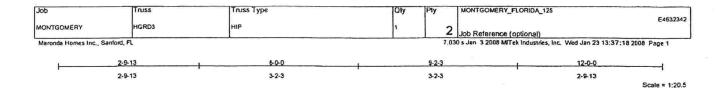
LOAD CASE(S) Standard

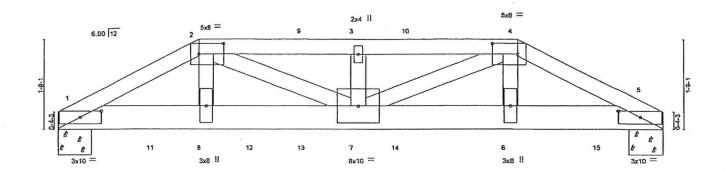
1) Regular. Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-4=-46, 4-8=-46, 8-10=-46, 2-10=-40

Concentrated Loads (lb)

Met: 4-69(B) 6-69(B) 8-69(B) 16-78(B) 13-78(B) 17-671(B) 12-753(B) 19-69(B) 20-69(B) 21-69(B) 22-69(B) 23-69(B) 24-69(B) 25-78(B) 26-78(B) 27-78(B) 28-78(B) 29-78(B)





	2-9-13			3-2-3	3-2-3				2-9-13				
Plate Of	fsets (X.)	n: [1:1	0-5-0.0-1-7]. [2:0-6-0.0	2-8]. [4:0-6-0	0.0-2-8]. [5:0	-5-0.0-1-7]							
LOADIN	IG (psf)		SPACING	2-0-0	CSI		DEFL	In	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	16.0	- 1	Plates Increase	1.25	TC	0.49	Vert(LL)	0.12	7	>999	240	MT20	244/190
TCDL	7.0	- 1	Lumber Increase	1.25	BC	0.87	Vert(TL)	-0.20	7-8	>673	180		
BCLL	10.0	•	Rep Stress Incr	NO	WB	0.73	Horz(TL)	0.04	5	n/a	n/a	1	
BCDL	10.0	- 1	Code FBC2004/TI	212002	(Mat	ix)						Weight: 121 II	b

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 6 SYP No.2

2 X 4 SYP No.3

BRACING TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-0-15 oc purlins. Rigid ceiling directly applied or 9-3-0 oc bracing.

REACTIONS (lb/size) 1=4170/0-8-0, 5=2848/0-8-0 Max Horz 1=23(LC 4)

Max Uplift1=1876(LC 7), 5=1366(LC 8)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-7702/3557, 2-9=-9293/4523, 3-9=-9293/4523, 3-10=-9293/4523, 4-10=-9293/4523, 4-5=-5762/2784 TOP CHORD

 $1-11 = -3109/6742, \ 8-11 = -3109/6742, \ 8-12 = -3213/7003, \ 12-13 = -3213/7003, \ 7-13 = -3213/7003, \ 7-14 = -2440/5113, \ 12-13 = -3213/7003, \ 12-$ **BOT CHORD**

6-14-2440/5113, 6-15-2411/5039, 5-15-2411/5039

2-8-979/2469, 2-7-1424/2522, 3-7=0/73, 4-7=-2245/4580, 4-6=-276/707

WEBS NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2 X 6 - 2 rows at 0-4-0 oc.

- Webs connected as follows: 2 X 4 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all piles, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf, BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.33 plate grip DOL=1.33.
- Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1876 lb uplift at joint 1 and 1366 lb uplift
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 11 lb down and 50 lb up at 2-9-13, 11 lb down and 50 lb up at 4-11-8, and 11 lb down and 50 lb up at 7-0-8, and 11 lb down and 50 lb up at 9-2-3 on top chord, and 1338 lb down and 537 lb up at 1-11-3, 140 lb down and 62 lb up at 2-9-12, 1317 lb down and 563 lb up at 3-11-3, 27 lb down at 4-11-8, 2623 lb down and 1473 lb up at 5-11-4, 243 lb down and 112 lb up at 6-10-0, 27 lb down at 7-0-8, 128 lb down and 70 lb up at 8-10-0, and 140 lb down and 62 lb up at 9-1-7, and 18 lb down and 31 lb up at 10-10-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard Continued on page 2



January 23,2008

🛕 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-1473 BEPORE USE. MARKING - Verity design parameters and READ INTIES ON THIS AND INCLUDED BILLER REFERENCE FAME BUTTED FOR SECURIO VIEW.

Design void for use only with Milek connection. This design is based only upon populate in shown, and is for on individual building component.

Applicability of design paramenters and proper incorporation of component is responsibility of building designer—not fixe designer, excelling the for local support of individual web members only. Additional temporoly backing to insure stability during construction is the responsibility of the building designer, For general guidance regarding fobication, qualify control, stronge, delivery, exection and broading, consult—AMSI/IPII Quality Criteria, DSB-89 and BCSII Building Component Safety Information—ovalidable from Truss Plate Institute, SSB D'Onohio Drive, Modison, WI 53719.



Job	Truss	Truss Type	Qly	Ply	MONTGOMERY_FLORIDA_125	
MONTGOMERY	HGRD3	HIP	1,			E4632342
				2	Job Reference (optional)	

Maronda Homes Inc., Sanford, F

7.030 s Jan 3 2008 MiTek Industries, Inc. Wed Jan 23 13;37:18 2008 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-2=-46, 2-4=-46, 4-5=-46, 1-5=-40
Concentrated Loads (lb)

Vert: 2=-11(F) 4=-11(F) 8=-140(F) 7=-2623(B) 6=-268(F=-140, B=-128) 9=-11(F) 10=-11(F) 11=-1338(B) 12=-1317(B) 13=-27(F) 14=-270(F=-27, B=-243) 15=-18(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-1473 BEFORE USE.

Design valid for use only with Miles connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not thus designer, Broche shown to included support of Individual web members only. Additional demporary broching to insure stability during construction is the responsibility of the erection. Additional permanent broching of the overall structure is the responsibility of the building designer. For general guidance regarding totarication, qualify continel, strange, delivery, erection and broching consult. AMSI/IPII Qualify Criteria, DSB-89 and BCSII Building Component Safety Information available from Truss Plate Institute, SS3 D'Onohilo Drive, Modison, WI 53719.



818 Soundside Road

russ Type MONTGOMERY FLORIDA 125 E4632343 PECIAL Job Reference (optional)

7.030 s Jan 3 2008 MiTek Industries, Inc. Wed Jan 23 13:37:18 2008 Page 1 Maronda Homes Inc., Sanford, FL 1-4-0 14-10-5 13-4-0 17-0-5 : 18-8-0 32-0-0 1-4-0 7-0-4 8-3-12 2-2-1 1-7-10 1-8-5 B-3-15 Scale = 1:59 3 3x4 || 4x6 = 6x12 = 3x4 = 4.50 12 3x4 = 12x12 2x4 || 3x10 2×4 12 10 3YR = 2.00 12 3x4 || 2x4 || 8210 = 18-8-0 7-0-14-10-5 17-0-6 24-11-15 7-0-4 6-3-12 1-6-5 2-2-1 1-7-10 6-3-15 7-0-1 Plate Offsets (X.Y): [2:0-0-7.Edge], [4:0-5-4.0-2-0], [7:0-10-0.0-2-12], [17:0-0-1.0-0-0] in (loc) LOADING (psf) SPACING 2-0-0 CSI DEFL **Udefl** L/d PLATES GRIP TCLL 16.0 Plates Increase 1.25 TC 0.72 Vert(LL) -0.48 15-16 >787 240 MT20 244/190 TCDL Lumber Increase 1.25 BC 0.83 -0.97 15-16 180 Vert(TL) BCLL 10.0 Rep Stress Incr YES WB 0.70 Horz(TL) 0.49 n/a BCDL 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 176 lb LUMBER BRACING Structural wood sheathing directly applied or 2-2-12 oc purilns. Rigid ceiling directly applied or 5-10-0 oc bracing. Except TOP CHORD 2 X 4 SYP No.2 TOP CHORD 2 X 4 SYP No.1D *Except* BOT CHORD **BOT CHORD** 6-12 2 X 4 SYP No.3, 17-18 2 X 4 SYP No.2 5-10-0 oc bracing: 2-16 WEBS 2 X 4 SYP No.3 *Except* WEBS 7-11 1 Row at midpt 3-15 2 X 4 SYP No.2, 11-13 2 X 4 SYP No.2, 7-13 2 X 4 SYP No.2 8-112 X 4 SYP No.2 REACTIONS (lb/size) 9=1357/Mechanical, 2=1440/0-8-0 Max Horz 2=107(LC 6) Max Uplift9=-277(LC 5), 2=-373(LC 4) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/19, 2-3=-4920/1443, 3-4=-3887/1203, 4-5=-3935/1257, 5-6=-3910/1254, 6-7=-3833/1228, 7-8=-2295/791, 8-9=3122/987 **BOT CHORD** 2-16=1285/4588, 15-16=1287/4587, 14-15=955/3655, 13-14=1021/3935, 12-13=47/20, 6-13=51/77, 11-12=47/140, 10-11=852/2879, 9-10=852/2879 3-16=0/269, 3-15=951/326, 4-15=28/492, 4-14=168/718, 5-14=97/141, 5-13=134/67, 11-13=966/3798, WEBS 7-13=963/3750, 7-11=2824/816, 8-11=852/328, 8-10=0/303 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-02; 125rnph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified. 3) Provide adequate drainage to prevent water ponding.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-8-0 tall by 1-0-0 wide

- will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 277 lb uplift at joint 9 and 373 lb uplift at joint 2.

LOAD CASE(S) Standard

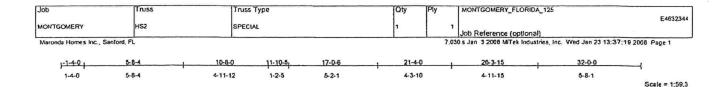


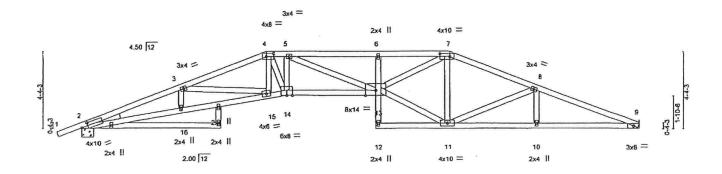
January 23,2008

eters and READ NOTES ON THIS AND INCLUDED MITER REPERENCE PAGE MII-7473 BEFORE USE Design volid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building compone Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trus designer. Roccing its for lateral support of Individual web members only. Additional temporary broating to insure shiply during construction is the responsibility erector. Additional permanent bracing of the overal structure is the responsibility of the building designer, for general guidance regarding flabrication, qualify control, stragge, defivery, erection and bracing, consult. AMS/I/PIT Qualify Citleria, DSB-89 and BCSIT Building Composaries and a consultation ovalidates from Truss Plate Institute. 583 D'Onofrio Drive, Madison, WI 53719.



Edenlon, NC 27932





			5-8-4	10-8-0	11-10-5	17-0-6		21-4-0		26-3-15	32-0-0	
			5-8-4	4-11-12	1-2-5	5-2-1		4-3-10		4-11-15	5-8-1	·
Plate Of	sets (X.Y): [2:	0-4-4.0-2-0]. [4:0-5-4.0	-2-0]. [7:0-7-	0.0-2-0]. [17:	0-0-1.0-0-0]				4		
OADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in (loc)	Vdefi	L/d	PLATES	GRIP
TCLL	16.0		Plates Increase	1.25	TC	0.57	Vert(LL)	-0.53 13-14	>721	240	MT20	244/190
CDL	7.0	- 1	Lumber Increase	1.25	BC	0.88	Vert(TL)	-1.05 13-14	>362	180		
BCLL	10.0	•	Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.44 9	n/a	n/a		
BCDL	10.0	- 1	Code FBC2004/TF	P12002	(Mat	ix)					Weight: 174 lb).

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.1D *Except*

6-12 2 X 4 SYP No.3, 17-18 2 X 4 SYP No.2

WEBS 2 X 4 SYP No.3

WEDGE

Left: 2 X 4 SYP No.3

REACTIONS (lb/size) 9=1357/Mechanical, 2=1440/0-8-0

Max Horz 2=91(LC 6)

Max Uplift9=292(LC 5), 2=388(LC 4)

FORCES (Ib) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/19, 2-3=-4883/1469, 3-4=-4303/1388, 4-5=-4572/1510, 5-6=-4896/1640, 6-7=-4779/1608, 7-8=-2589/903, 8-9=-3203/1045

BOT CHORD 2-16=-1312/4535, 15-16=-1318/4542, 14-15=-1162/4074, 13-14=-1320/4572, 12-13=0/85, 6-13=-204/196,

11-12=-50/133, 10-11=-917/2956, 9-10=-917/2956 WEBS 3-16=0/199, 3-15=-490/203, 4-15=-38/290, 4-14=-357/1193, 5-14=-333/178, 5-13=-140/482, 11-13=-697/2461,

7-13=832/2729, 7-11=622/264, 8-11=624/245, 8-10=0/236

NOTES

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) Provide adequate drainage to prevent water ponding.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

- 7) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 292 ib uplift at joint 9 and 388 lb uplift at

LOAD CASE(S) Standard



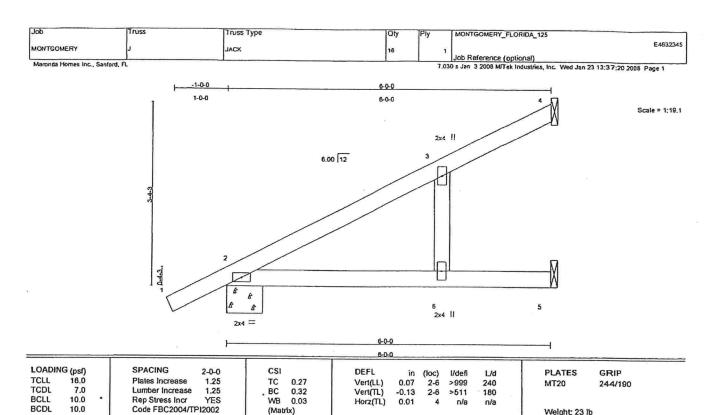
Structural wood sheathing directly applied or 2-4-8 oc purlins.

Rigid ceiling directly applied or 5-9-1 oc bracing.

January 23,2008

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design vold for use only with Milex connectors. This design is based only upon parameters und reference PAGE MIL-7473 BEFORE USE. Design vold for use only with Milex connectors. This design is parameters than individual bubbling component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer, floring at its for lartest support of individual web members only. Additional temporary bracing to insure shiftly during construction is the responsibility of erector. Additional permonent bracing of the overal structure is the responsibility of the building designer, for general guidance regarding fobraction, quality control, storage, delivery, erection and bracing, consult. AMSI/IPI1 Quelity Criteria, DSB-89 and BCSI1 Building Compone Safety Information. available from Truss Plate Institute, S83 D'Onolrio Drive, Madson, WI S3719.





TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3 WEBS

REACTIONS (lb/size) 4=115/Mechanical, 2=313/0-8-0, 5=118/Mechanical

Max Horz 2=157(LC 6) Max Uplift4=-52(LC 6), 2=-116(LC 6), 5=-29(LC 6)

FORCES (ib) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-105/9, 3-4=-38/47 2-6=0/0, 5-6=0/0 TOP CHORD

BOT CHORD

WEBS 3-6=-9/109

- NOTES (6)

 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified,
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 4, 116 lb uplift at joint 2 and 29 lb uplift at joint 5.
- 6) Atlach with (2) 16d Common Toe-Nails (0.162"x 3.5") at joints 4 and 5.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

January 23,2008

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-9473 BEFORE USE. Design and for use only with Mifex connectors. This design is based only upon portuneters shown, and is for an individual building component. Design and for use only with Mifex connection. This design is based only upon portuneters shown, and is for an individual building component. Applicability of design portuneties and proper incorporation of component is responsibility of building designer-not has designer. Strocking shows is for interest support of individual web membes only. Additional temporary broking to insure shorting construction is the responsibility of the exector. Additional permanent brocking of the overall structure is the responsibility of the building designer. For general guidance regarding fobiocition, quelity control, storage, designer, see an executive statement of the building designer. For general guidance regarding fobiocition, quelity control, storage, designer, see an executive statement of the building designer. For general guidance regarding statement of the building designer for general guidance regarding statement of the building designer. For general guidance regarding statement of the building designer for general guidance regarding the properties of the building designer. For general guidance regarding the properties of the building designer for general guidance regarding the properties of the building designer. For general guidance and general guidance regarding the properties of the building designer. For general guidance and general guidance are greatly as a properties of the building designer. For general guidance are greatly as a properties of the building designer for general guidance and guidance are greatly as a properties of the building designer. For general guidance are greatly as a properties of the building designer for general guidance and guidance are greatly as a properties of the building designer. For general guidance are greatly as a properties of the guidance and guidance are greatly as a properties of the guidance and guidance and guidance are guidance an



MONTGOMERY_FLORIDA_125 Oly Truss Truss Type E4632346 MONTGOMERY JACK Job Reference (optional)
7.030 s Jan 3 2008 MTek Industries, Inc. Wed Jan 23 13:37:20 2008 Page 1 Maronda Homes Inc., Sanford, FL 1-0-0 4-5-1 1-0-0 Scale = 1:14.0 6.00 12 0-4-3 Ų. δ 4-5-1 PLATES GRIP DEFL L/d SPACING CSI in (loc) Vdef LOADING (psf) MT20 244/190 1,25 1,25 TC BC Vert(LL) -0.01 2-4 >999 240 Plates Increase 0.18 16.0 TCLL

LUMBER

TCDL

BCLL

BCDL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

7.0

10.0

10.0

BRACING

Vert(TL)

Horz(TL)

-0.03

-0.00

>999

n/a

180

n/a

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 4-5-1 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 16 lb

REACTIONS (lb/size) 3=81/Mechanical, 2=249/0-8-0, 4=81/Mechanical Max Horz 2=124(LC 6)

Code FBC2004/TP12002

Lumber Increase

Rep Stress Incr

Max Uplift3=76(LC 6), 2=-112(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/21, 2-3=-60/30 TOP CHORD

2-4=0/0 **BOT CHORD**

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

0.15

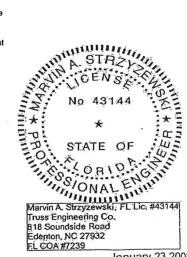
WB 0.00

(Matrix)

YES

- 2) This truss has been designed for a 10.0 psf bottom chord five load nonconcurrent with any other five loads,
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 3 and 112 lb uplift at joint 2.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x 3.5") at joints 3 and 4.

LOAD CASE(S) Standard

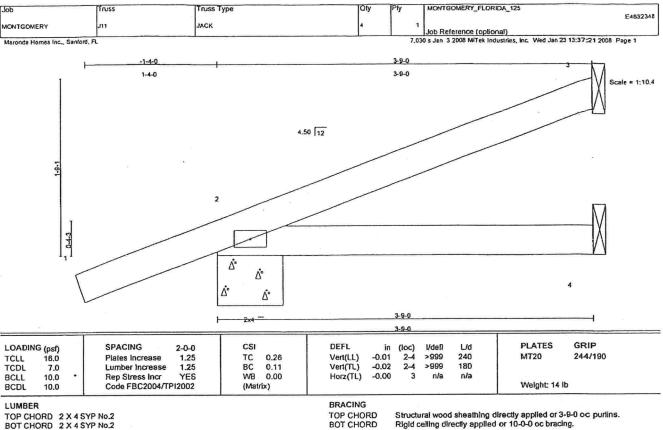


January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Milex connectors. This design is based only upon porometers shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer—not frust designer, Bracing shown is for falled support of holdwidual web members only. Additional temporary bracing to Insure 1995 by during construction is the responsibility of the erector. Additional permanent bracing of the overal structure is the responsibility of the designer, for general guidance regarding tablecular, quality control, stronge, delivery, exection and bracing, consult. AMSI/IPII Quality Criteria, DS8-89 and 8CSII Building Component Safely Information available from Trust Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Edenton, NC 27932



BOT CHORD

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=57/Mechanical, 2=243/0-8-0, 4=67/Mechanical

Max Horz 2=90(LC 6)

Max Uplifi3=41(LC 5), 2=-148(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=-38/16

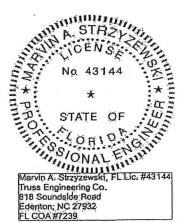
BOT CHORD 2-4=0/0

NOTES

- 1) Which ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grlp DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 41 lb uplift at joint 3 and 148 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x 3.5") at joints 3 and 4.

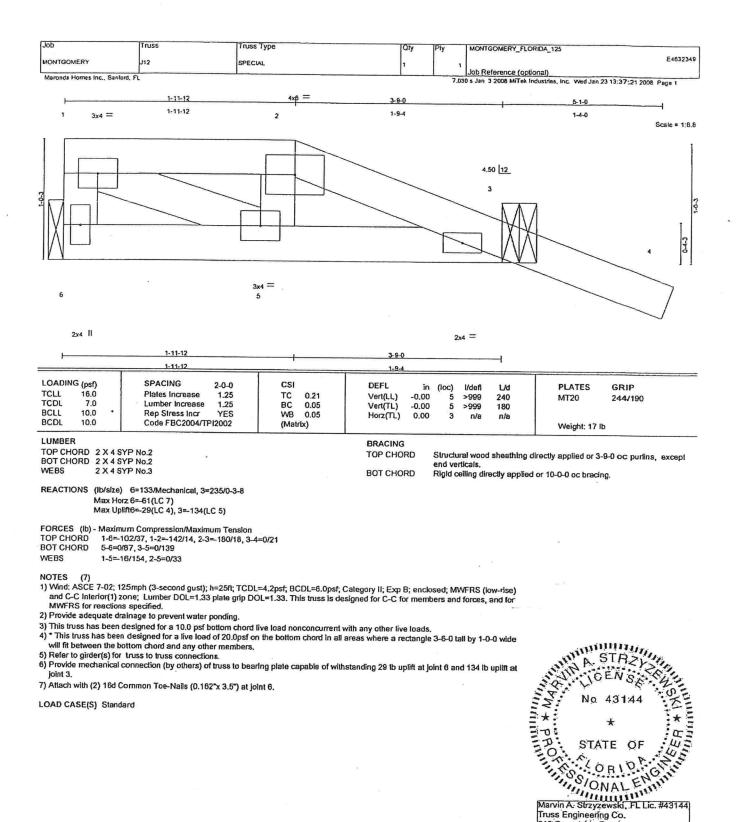
LOAD CASE(S) Standard



January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE PAGE MII-7473 BEFORE USE. Design vold for use only with Miles connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer- not truss designer. Bracing shown to related support of Individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of building designer- not truss designer. Bracing shown to related support of Individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer. For general guidance regarding fobrication, quotify control, storage, delivery, serection and bracing, consult. AMSI/IPIT Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information. evaluable from Truss Plate Institute, 583 D'Onofrio Drive, Modson, WI S3719.





WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE PAGE MIT-1473 BEFORE USE.

Design void for use only with Millek connectors. This design is based only upon parameters shown, and is far on individual building component.

Applicability of design parameters and proper incorporation of component is responsibility of building designer, not fuss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stoletify during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. AMSI/ITII Quality Criteria, DSB-89 and BCSII Building Component Safety Information available from Truss Piole Institute, 583 D'Onofrio Drive, Madison, Wi 537 19.



January 23,2008

Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 FL COA #7239

Truss Truss Type MONTGOMERY FLORIDA 125 E4632350 MONTGOMERY JACK Job Reference (optional)
7.030 s Jan 3 2008 MITek Industries, Inc. Wed Jan 23 13:37:21 2008 Page 1 Maronda Homes Inc., Sanford, FL 3-10-2 1-0-0 3-10-2 Scale = 1:12.7 6.00 12 0-4-3 Δ Δ. SPACING CSI DEFL PLATES GRIP LOADING (psf) 2-0-0 16.0 Plates Increase 1.25 0.18 Vert(LL) -0.01 2-4 >999 240 MT20 244/190 TCLL 180 BC TCDL 7.0 Lumber Increase 1.25 0 11 Vert(TL) -0.02>999 WB -0.00 BCLL 10.0 Rep Stress Incr YES 0.00 Horz(TL) Code FBC2004/TPI2002 (Matrix) Weight: 14 lb BCDL 10.0 BRACING LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 3-10-2 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD

REACTIONS (Ib/size) 3=66/Mechanical, 2=226/0-8-0, 4=69/Mechanical Max Horz 2=112(LC 6) Max Uplift3=61(LC 6), 2=-112(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=-49/25

BOT CHORD

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C_C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 61 lb uplift at joint 3 and 112 lb uplift at ioint 2.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x 3.5") at joints 3 and 4.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 REPORE USE. ARRIUNG - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTER REFERENCE PAGE MIL-7473 REPUNE USE. Design void for use only with Mifels connectors. This design is based only upon parameters show, and is for an individual budding component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Brocking shows to related support of individual web members only. Additional temporary brocking to insure solution parameters brocking of the overal shockine is the responsibility of the selection. Additional permanent brocking of the overal shockine is the responsibility of the shallong designer. For general guidance regarding labrication, quality control, storage, delivery, erection and brocking, consult. ANSI/TP11 Quality Criteria, BSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, S83 D'Onnotio Drive, Modelson, W1 53719.





Job	Truss	Truss Type	Oty Pfy MONTGOMERY_FLORIDA_125 E4632351
			Job Reference (optional) 7,030 s Jan 3 2008 MTek Industries, Inc. Wed Jan 23 13:37:22 2008 Page 1
Maronda Homes Inc., S.	anlord, FL		1,000 3 0811 0 2000 Hall Ok 110000103, III.
		-1-0-0 1-0-0	2-11-1 2-11-1 Scale = 1:10.5
	1,8-11	2	6.00 12

2-11-1 GRIP PLATES SPACING CSI DEFL Vdefl Lld LOADING (psf) 1.25 1.25 TC BC 240 MT20 244/190 16.0 Plates Increase 0.18 Vert(LL) -0.00 >999 TCLL 180 -0.01 >999 Vert(TL) TCDL 7.0 Lumber Increase 0.06 YES WB 0.00 Horz(TL) -0.00 Rep Stress Incr BCLL 10.0 BCDL Code FBC2004/TPI2002 Weight: 11 lb 10.0

BRACING

TOP CHORD **BOT CHORD**

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

Δ°

Q.

Ó°

Δ°

REACTIONS (Ib/size) 3=40/Mechanical, 2=191/0-8-0, 4=51/Mechanical Max Horz 2=93(LC 6) Max Uplift3=34(LC 6), 2=114(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=-38/14

04-3

BOT CHORD

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 34 lb uplift at joint 3 and 114 lb uplift at joint 2.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x 3.5") at joints 3 and 4.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 2-11-1 oc purilns. Rigid ceiling directly applied or 10-0-0 oc bracing.

January 23,2008

FIARRING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 BEFORE USE.

Design volid for use only with Millek connectors. This design is based only upon parameters shown, and is for an incividual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trus designer. Brocing shown is to intered support of individual web members only. Additional temporary brocing to insure stability during construction is the responsibility of the erector. Additional permanent brocing of the overal structure is the responsibility of the building designer. For general guidance regarding flobication, quality control, storage, designer, designer, for general guidance regarding flobication, quality control, storage, designer, and additional conditions are stability of the building Component of the property of the parameters of the property of the building Component of the property of the parameters of the property of the building Component of the property of the parameters of the property of the building Component of the property of the parameters of the parameters of the property of the parameters of the parameters



Edenton, NC 27932

MONTGOMERY FLORIDA 125 Toiss Truss Type E4632352 MONTGOMERY JACK Job Reference (optional) Maronda Homes Inc., Sanford, FL 7,030 s Jan 3 2008 MTek Industries, Inc. Wed Jan 23 13:37:22 2008 Page 1 1-0-0 Scale = 1:7.2 6.00 12 Δ° Δ° Å٩ 2x4 = PLATES SPACING DEFL Vdefl L/d LOADING (psf) 2-0-0 TC BC 240 MT20 244/190 16.0 Plates Increase 1.25 0.13 Vert(LL) -0.00 >999 TCLL -0.00 180 >999 1.25 Vert(TL) TCDL 7.0 Lumber Increase 0.02 WB 0.00 Horz(TL) -0.00 YES BCLL 10.0 Rep Stress Incr Code FBC2004/TPI2002 Weight: 6 lb BCDL 10.0 BRACING LUMBER Structural wood sheathing directly applied or 1-5-1 oc purlins. TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 TOP CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. **BOT CHORD** REACTIONS (lb/size) 2=132/0-8-0, 4=27/Mechanical, 3=9/Mechanical

Max Horz 2=63(LC 6)

Max Uplift2=103(LC 6), 3=12(LC 5)

Max Grav2=132(LC 1), 4=27(LC 1), 3=11(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-26/7

BOT CHORD 2-4=0/0

NOTES (6)

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- MWFRS for reactions specified.

 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 4) report to grider(s) for a use to use connections.

 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 103 lb uplift at joint 2 and 12 lb uplift at joint 3.
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x 3.5") at joints 3 and 4.

LOAD CASE(S) Standard



January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 BEFORE USE.

Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer and thus designer. Bracing shows for lateral support of buckfular when members only. Additional emprorary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overal structure is the responsibility of the building designer. For general guidance regarding tobaccolor, quality control, storage, delivery, erection and bracing, consul. ANSI/TRI Quality Criteria, DSS-89 and BCSI1 Suilding Component Safety Information available from Truss Plate Institute, SS3 D'Onotifo Drive, Madison, WI S3719.



818 Soundside Road

Truss Truss Type MONTGOMERY_FLORIDA_125 E4632353 MONTGOMERY JACK Job Reference (optional) 7.030 s Jan 3 2008 MiTek Industries, Inc. Wed Jan 23 13:37:22 2006 Page 1 Maronda Homes Inc., Sanford, FL 1-4-0 6-10-0 Scale = 1:16.9 2x4 || 4.50 12 0-4-3 6 Ų. 2x4 || 2x4 = 6-10-0 SPACING DEFL PLATES GRIP LOADING (psf) 2-0-0 In (loc) 240 180 Plates Increase 1.25 TC BC 0.37 Vert(LL) -0.11 2-6 >709 MT20 244/190 TCLL 18.0 TCDL 7.0 Lumber Increase 1.25 0.40 Vert(TL) -0.22 2-8 >348 WB 0.01 BCLL 10.0 Rep Stress Incr YES 0.03 Horz(TL) n/a Code FBC2004/TPI2002 (Matrix) Weight: 26 lb BCDL 10.0

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3

REACTIONS (lb/size) 4=135/Mechanical, 2=366/0-8-0, 5=131/Mechanical

Max Horz 2=138(LC 6) Max Uplift4=53(LC 6), 2=158(LC 6), 5=-22(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension

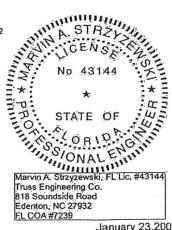
TOP CHORD 1-2=0/21, 2-3=83/6, 3-4=31/44

2-6=0/0, 5-6=0/0 **BOT CHORD**

WEBS 3-6=-4/102

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 4, 158 lb uplift at joint 2 and 22 lb uplift at joint 5.
- Attach with (2) 16d Common Toe-Nails (0.162"x 3.5") at joints 4 and 5.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REPERENCE PAGE MII-7473 BEFORE USE. Design vold for use only with Miles connector. This design is based only upon parameters that authors see that authors a service total.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not have designed to the properties of the service of t



MONTGOMERY_FLORIDA_125 Job ้าบรร Truss Type E4632354 MONTGOMERY Job Reference (optional) 7.030 s Jan 3 2008 MiTek Industries, Inc. Wed Jan 23 13:37:23 2008 Page 1 Maronda Homes Inc., Sanford, FL 3 Scale = 1:16.3 2x4 11 4.50 12 5-8-6 Plate Offsets (X.Y): [1:0-1-5.0-0-4] LOADING (psf) SPACING 2-0-0 CSI DEFL **Udefi** I /d PLATES GRIP 1.25 TC BC TCLL 16.0 Plates Increase 0.41 Vert(LL) 0.14 1-5 >568 240 MT20 244/190 0.45 180 TCDL 7.0 Vert(TL) -0.26 >304 Lumber Increase 1-5

0.00

BRACING

BOT CHORD

BCDL LUMBER

BCLL

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

WEBS

10.0

10.0

2 X 4 SYP No.3

REACTIONS (lb/size) 1=283/Mechanical, 3=137/Mechanical, 4=145/Mechanical

YES

Max Horz 1=105(LC 6) Max Uplifi1 = 49(LC 6), 3= 48(LC 6), 4=-41(LC 6)

Rep Stress Incr

Code FBC2004/TP12002

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=-94/14, 2-3=-29/45 BOT CHORD 1-5=0/0, 4-5=0/0

WEBS

2-5=-21/144

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

WB 0.04

(Matrix)

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 1, 48 lb uplift at joint 3 and 41 lb uplift at joint 4.

LOAD CASE(S) Standard



Weight: 23 lb

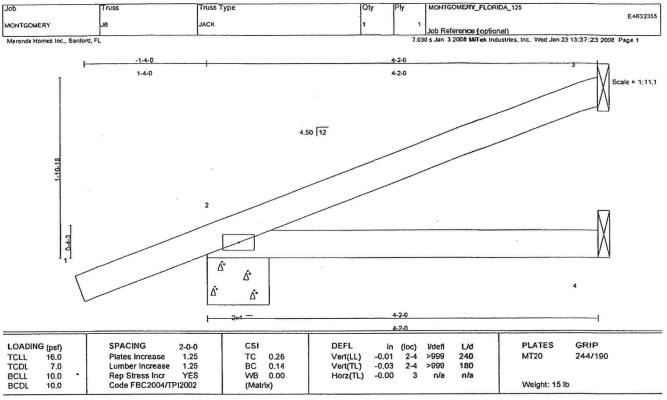
Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid celling directly applied or 10-0-0 oc bracing.

January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design votal for use only with Milek connectors. This design is based only upon porometers shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure shifty during construction is the responsibility of the erector. Additional permanent bracing of the overal structure is the responsibility of the designer. For general guidance regarding labrication, qualify control, storage, delivery, erection and bracing, consult. AMSI/IPTI Quality Criteria, DSB-89 and BCSI1 Building Component Salely Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madison, WI 53719.



Edenion, NC 27932



LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-2-0 oc purlins. Rigid celling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/stze) 3=69/Mechanical, 2=259/0-8-0, 4=75/Mechanical Max Horz 2=97(LC 6) Max Uplift3=-52(LC 6), 2=-148(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

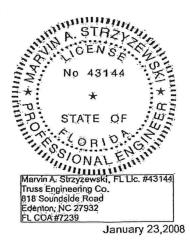
TOP CHORD 1-2=0/21, 2-3=-41/20

BOT CHORD 2-4=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at Joint 3 and 148 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x 3.5") at joints 3 and 4.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 REFORE USE. Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer- not truss designer. Reacing shown is for lateral support of Individual web membes only. Additional temporary brocing to insure shiftly during construction is the responsibility of the erector. Additional permanent bracking of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, qualify control, stronge, defevery, erection and bracking, consult. AMSI/IPII Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Maddon, Wi 53719.



4.50 12

4-0-6 - 4-0-6

Plate Off	sets (X.)	0: [1	0-1-5.0-0-4]										
LOADIN TCLL TCDL BCLL BCDL	G (psf) 16.0 7.0 10.0 10.0	*	SPACING Plates Increase Lumber Increase Rep Stress Incr Code FBC2004/TF	2-0-0 1.25 1.25 YES PI2002	CSI TC BC WB (Mate	0.17 0.14 0.00 fx)	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.01 -0.03 -0.00	(loc) 1-3 1-3 2	Vdefl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Welght: 13 lb	GRIP 244/190

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 4–0-6 oc purlins. Rigid celling directly applied or 10-0-0 oc bracing.

3

REACTIONS (Ib/size) 1=168/Mechanical, 2=90/Mechanical, 3=78/Mechanical Max Horz 1=64(LC 6)

Max Upliff1=29(LC 6), 2=-77(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-47/26 BOT CHORD 1-3=0/0

NOTES

- Wind: ASCE 7-02; 125mph (3-second gust): h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 29 lb uplift at joint 1 and 77 lb uplift at joint 2.

LOAD CASE(S) Standard



January 23,2008

Scale = 1:10.8

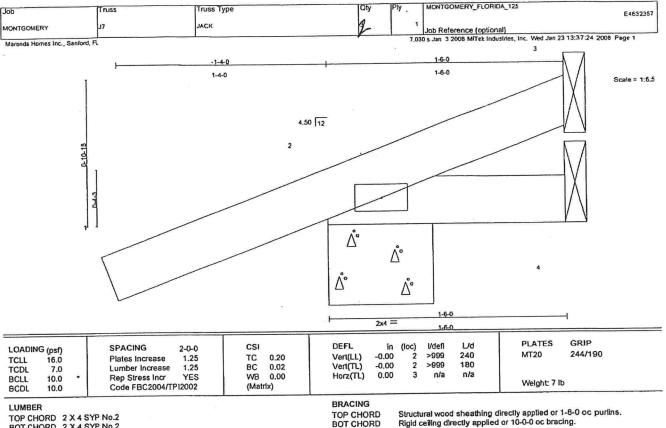
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEX REFERENCE PAGE MIL-7473 BEFORE USS.

Design void for use only with Mitek connection. This design is based only upon parameters shown, and is for an included building component.

Applicability of design parameters and proper incorparation of component is responsibility of building designer- not trus designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional temporary bracing to insure stability during construction is the responsibility of the building designer. For general guidance reparating tobalcation, quality control, storage, delivery, responsibility on and bracing, consult. AMSI/IPII Quality Criteria, DS8-89 and BCSII Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madson, WI 53719.



818 Soundside Road Edenton, NC 27932



BOT CHORD

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 2=162/0-8-0, 4=28/Mechanical, 3=-1/Mechanical

Max Horz 2=56(LC 6)

Max Uplift2=139(LC 6), 3=8(LC 5)

Max Grav2=162(LC 1), 4=28(LC 1), 3=18(LC 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-26/16

BOT CHORD 2-4=0/0

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=8.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 139 lb uplift at joint 2 and 8 lb uplift at
- 6) Attach with (2) 16d Common Toe-Nails (0.162"x 3.5") at joints 3 and 4.

LOAD CASE(S) Standard



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 BEFORE USE. Design void for use only with MTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design promenters and proper incorporation of component is responsibility of building designer- not finus designer. Bracing show is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the forecast, Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding the objection, qualify conflict, storage, defivery, erection and bracing, consult. ANSI/TPIT Quality Criteria, DSB-89 and BCSIT Building Component Safety Intermation. available from Truss Plate Institute, SS3 D'Onofrio Drive, Maddison, WI SS719.

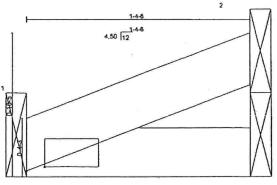


Edemon, NC 27932

Job Truss Truss Type Qty MONTGOMERY_FLORIDA_125 MONTGOMERY J7A JACK Job Reference (optional) Is Jan 3 2008 MTek Industries, Inc. Wed Jan 23 13:37:24 2008 Page

Structural wood sheathing directly applied or 1-4-6 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



Scale = 1:6.2

Plate Offsets (X,Y): [1:0-1-5.0-0-4] GRIP LOADING (psf) SPACING DEFL in (loc) I/defl L/d PLATES TCLL 16.0 Plates Increase 1.25 TC BC 0.02 Vert(LL) -0.00 >999 240 MT20 244/190 1.25 180 TCDI 7.0 Lumber Increase 0.02 Vert(TL) -0.00 >999 WB 0.00 -0.00 BCLL YES 10.0 Rep Stress Incr Horz(TL) n/a n/a Code FBC2004/TP12002 Weight: 4 lb BCDL 10.0 (Matrix)

BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=58/Mechanical, 3=27/Mechanical, 2=31/Mechanical

Max Horz 1=23(LC 6)

Max Uplift1=9(LC 6), 2=-27(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-17/9

BOT CHORD 1-3=0/0

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grlp DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at Joint 1 and 27 lb uplift at Joint

LOAD CASE(S) Standard

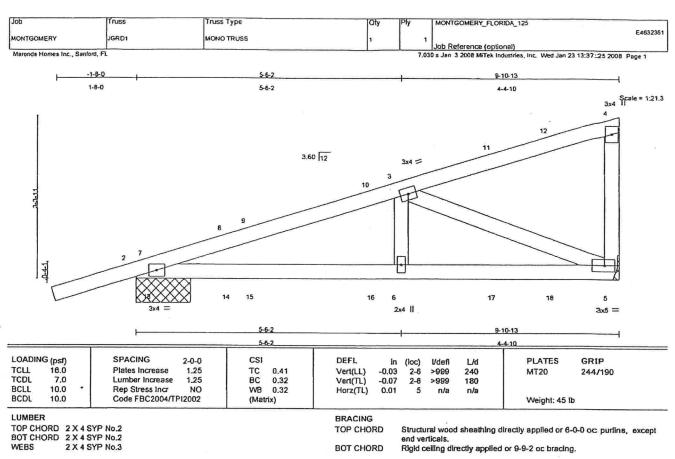


January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIJ-7473 BEFORE USE. Design volid for use only with Milek connectors. This design is based only upon parameters and individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer- not have designer. Bracing shown for falled support of individual web membes only. Additional temporary bracing to insure shiftly during construction is the responsibility of the erector. Additional permanent bracing of the everall structure is the responsibility of the designer. For general guidance regarding fobication, quotify continol, storage, delivery, erection and bracing, consult. AMSUFFIT Quality Criteria, DSB-89 and BCSIT Building Component Safety Information available from Truss Plate Institute, S83 D'Onotrio Drive, ModSon, W153719.



Edenion, NC 27932



REACTIONS (lb/size) 5=651/Mechanical, 2=507/1-1-11

Max Horz 2=153(LC 3)

Max Uplift5=-338(LC 7), 2=-244(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-7=-927/324, 7-8=-923/326, 8-9

1-2=0/21, 2-7=-927/324, 7-8=-923/326, 8-9=-903/336, 9-10=-903/345, 3-10=-869/337, 3-11=-115/47, 11-12=-76/18,

4-12=38/4, 4-5=161/154

BOT CHORD 2-13=409/863, 13-14=409/863, 14-15=409/863, 15-16=409/863, 6-16=409/863, 6-17=409/863, 17-18=409/863,

5-18=-409/863

WEBS 3-6=0/283, 3-5=-851/389

NOTES

- I) Wind: ASCE 7-02; 125mph (3-second gust); h=25fl; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.33 plate grip DOL=1.33.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 338 lb uplift at joint 5 and 244 lb uplift at joint 2.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 35 lb down and 37 lb up at 2-5-12, 39 lb up at 4-11-12, 23 lb down and 67 lb up at 5-3-12, 35 lb down and 101 lb up at 7-5-12, and 89 lb down and 72 lb up at 8-7-12, and 43 lb down at 1-11-12 on top chord, and 12 lb up at 1-11-12, 13 lb up at 2-5-12, 11 lb down at 4-11-12, 35 lb down at 5-3-12, and 41 lb down at 7-5-12, and 91 lb down and 60 lb up at 8-7-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=48, 2-7=-14, 4-7=46, 2-13=-20, 5-13=-40

Concentrated Loads (lb)

Vert: 3=-23(F) 6=-35(F) 9=37(B) 10=6(B) 11=-35(B) 12=-89(F) 14=12(F) 15=13(B) 16=-11(B) 17=-41(B) 18=-91(F)



January 23,2008

WARRING - Verify deetign parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-1473 BEFORE USE.

Design void for use only with Mifel connectors. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorporation of component is responsibility of building designer - not has designer, Brocing shown is for toterd support of individual web members only. Additional temporary brocing to insure stobility during constitutions is the responsibility of the erector. Additional permanent broching of the overal structure is the responsibility of the building designer, for general guidance regarding lobrication, quality control, storage, delivery, erection and brocking, consult. AMSI/IFIT Quality Criteria, DSB-89 and BCSI Building Component Safety Information ovaliable from Trus Plate Institute, 583 D'Onatria Drive, Modison, W. 53719.



818 Soundside Road Edenton, NC 27932

MONTGOMERY FLORIDA 125 Truss Truss Type E4632362 WONTGOMERY JGRD1A MONO TRUSS Job Reference (optional) Maronda Homes Inc., Sanford, Fl 7.030 s Jan 3 2008 MiTek Industries, Inc. Wed Jan 23 13:37:26 2008 Page 1 9-10-7 4-4-1D 5-5-13 Scale = 1-20.9 3x4 II 3 3.60 12 14 15 16 5 17 18 19 2x4 | 4-10 LOADING (psf) GRIP SPACING CSI DEFI PLATES TC BC 244/190 TCLL 16.0 Plates Increase 1.25 0.41 Vert(LL) -0.04 1-5 >999 240 MT20 -0.09 >999 180 Vert(TL) 1-5 TCDI 7.0 Lumber Increase 1.25 0.44 WB 0.37 0.02 Rep Stress Incr BCL1 10.0 Horz(TL) 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 43 lb BRACING LUMBER TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 5-11-9 oc purlins, except BOT CHORD 2 X 4 SYP No.2 Rigid ceiling directly applied or 8-2-2 oc bracing. BOT CHORD 2 X 4 SYP No.3 REACTIONS (lb/size) 1=468/1-1-5, 4=731/Mechanical Max Horz 1=119(LC 7) Max Uplift1=195(LC 7), 4=444(LC 7) FORCES (lb) - Maximum Compression/Maximum Tension 1-6=1060/501, 6-7=1057/505, 7-8=1036/503, 8-9=1017/500, 2-9=984/473, 2-10=120/56, 10-11=-77/40, 11-12-49/21, 3-12-27/0, 3-4-169/184 1-13=-570/990, 13-14=-570/990, 14-15=-570/990, 15-16=-570/990, 5-16=-570/990, 5-17=-570/990, 17-18=-570/990, BOT CHORD 18-19=570/990, 4-19=570/990 WEBS 2-5=0/320, 2-4=-987/562 NOTES 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.33 plate grip DOL=1.33. A STAZY 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
) Refer to girder(s) for truss to truss connections.
) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 195 lb uplift at joint 1 and 444 lb uplift at joint 2 lb up at 1-9-5, 39 lb up at 3-11-13, 44 lb down and 100 lb up at 5-1-5, 20 lb down and 78 lb up at 6-5-13, and 91 lb down and 66 lb up at 8-5-5, and 35 lb down and 101 lb up at 8-11-13 on top chord, and 13 lb up at 1-9-5, 11 lb down at 3-11-13, 38 lb down at 5-1-5, 29 lb down at 6-5-13, and 105 lb down and 86 lb up at 8-5-5, and 41 lb down at 8-11-13 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
) In the LOAD CASE(S) Standard
) Regular: Lumber increase=1.25, Plate increase=1.25
Uniform Loads (pii)

Vert 1-6=14, 3-6=46, 1-13=20, 4-13=40

Marvin A Stzyzewski, FL Lic, #43144 [Truss Engineering Co. 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide 4) Refer to girder(s) for truss to truss connections. 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 195 lb uplift at joint 1 and 444 lb uplift at 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 10 lb down and 23 lb up at 1-9-5, 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). LOAD CASE(S) Standard 1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 Continued on page 2 FL COA #7239 January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 REFORE USE.

Design void for use only with MiTek connection. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorparation of component is responsibility of building designer not inus designer. Bracing shows to related support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding lobications, questly control, storage, delivery, erection and bracing, consult. ANSI/IPTI Quality Criteria, DSS-89 and BCSI I Suilding Component Safety Information available from Truss Plate Institute, SS3 D'Onotrio Drive, Madson, WI 53719.



818 Soundside Road Edenton, NC 27932

Job Truss Type	Qty	Ply	MONTGOMERY_FLORIDA_125 E4632382
MONTGOMERY JGRD1A MONO TRUSS	1	1	Job Reference (optional)

Maronda Homes Inc., Sanford, FL

030 s Jan 3 2008 M/Tek Industries, Inc. Wed Jan 23 13:37:26 2008 Page 2

LOAD CASE(S) Standard Concentrated Loads (lb)

Verl: 7=15(B) 8=6(F) 9=-44(B) 10=-20(F) 11=-91(B) 12=-35(F) 14=13(B) 15=-11(F) 16=-38(B) 17=-29(F) 18=-105(B) 19=-41(F)

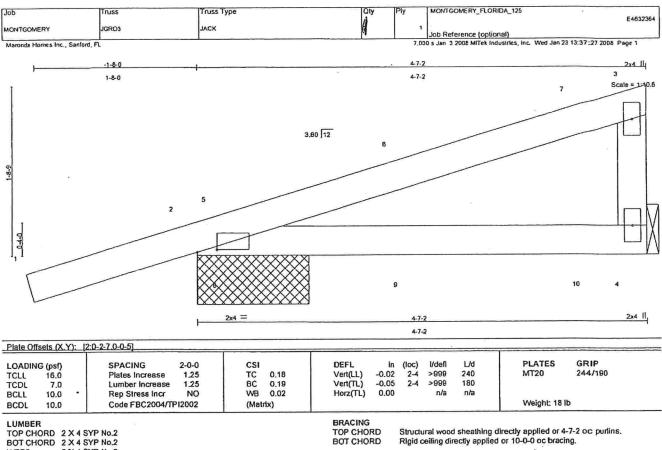
WARNING - Verify design parameters and READ NOTES ON TRIS AND INCLUDED MITER REFERENCE PAGE MIL-1473 REFORE USE.

Design voild for use only with Milek connectors. This design is based only upon parameters shown, and is far on individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not trus designer, Bracing shown is for tolered support of individual web members only. Additional temporary bracing to insure stability of this constitution is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding flabrication, qualify control, storage, defivery, erection and bracing, consult.

AMSI/TPI Quelify Critical, DSB-89 and 8CSII Building Component Safety Information available from Truss Plate truttule, S83 D'Onatrio Drive, Madison, WI 53719.



Edenton, NC 27932



2 X 4 SYP No.3 WEBS

REACTIONS (lb/size) 2=263/1-1-12, 4=174/Mechanical Max Horz 2=87(LC 3) Max Uplift2=147(LC 7), 4=-55(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-5=-36/0, 5-6=-35/0, 6-7=-18/20, 3-7=-25/18

BOT CHORD 2-8=0/0, 8-9=0/0, 9-10=0/0, 4-10=0/0

WEBS 3-4=-82/82

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.33 plate grip DOL=1.33.
- 2) This truss has been designed for a 10,0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 147 lb uplift at joint 2 and 55 lb uplift at joint 4.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 39 lb up at 4-0-4, and 43 lb down at 2-2-4 on top chord, and 12 lb up at 2-2-4, and 11 lb down at 4-0-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Regular. Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=46, 2-5=14, 3-5=46, 2-8=-20, 4-8=-40 Concentrated Loads (lb)

Vert: 7=6(F) 9=12(B) 10=-11(F)



January 23,2008

WARKING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIL-7473 BEFORE USE. Design volid for use only with Miles connectors. This design is based only upon porameters shown, and is for on individual bullding component. Applicability of design parametries and proper incorporation of component is responsibility of design error individual web members only. Additional temporary broating to insulability during construction is the responsibility of the construction is the responsibility of the error designer. For general guidance regarding clothical for upon the control property of the building designer. For general guidance regarding tobication, quotity control, stronge, delivery, erection and broating, consult. AMSI/IPIT Quality Criteria, DSB-89 and BCSIT Building Component Safety Internation available from Trust Plate Institute, S83 D'Onotrio Dilve, Modison, Wi S3719.



MONTGOMERY_FLORIDA_125 Truss Type Joh Truss E4632365 SPECIAL MONTGOMERY Job Reference (optional) 7.030 s Jan 3 2006 MiTek Industries, Inc. Wed Jan 23 13:37:27 2006 Page 1 Maronda Homes Inc., Sanford, FL -1-0-0 1-0-0 6-0-0 Scale = 1:19.0 6.00 12 3.00 12 Ġ 3x4 = PLATES GRIP LOADING (psf) CSI SPACING 1/def Ľď 2-0-0 (loc) -0.05 >999 240 MT20 244/190 Vert(LL) TC 0,31 TCLL 16.0 Plates Increase 1.25 1.25 BC 0.30 Vert(TL) -0.13 >504 180 Lumber Increase TCDL Rep Stress Incr YES WB 0.00 Horz(TL) -0.00 n/a n/a BCLL 10.0 Weight: 21 lb Code FBC2004/TPI2002 BCDL 10.0 (Matrix) LUMBER BRACING Structural wood sheathing directly applied or 6-0-0 oc purios. TOP CHORD TOP CHORD 2 X 4 SYP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD 2 X 4 SYP No.2 BOT CHORD

REACTIONS (lb/size) 3=121/Mechanical, 2=313/0-8-0, 4=112/Mechanical

Max Horz 2=155(LC 6)

Max Uplift3=-117(LC 6), 2=-114(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/18, 2-3=-92/46

BOT CHORD 2-4=-22/22

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
 Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 117 lb uplift at joint 3 and 114 lb uplift at
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x 3.5") at joints 3 and 4.

LOAD CASE(S) Standard



January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Mifex connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not thus designer. Roccing shows it for talenal support of individual web members only. Additional temporary bracing to insure stolibility during construction is the responsibility of the erector. Additional permanent brocking of the overall shucture is the responsibility of the building designer. For general guidance regarding fobication, quolify control, storage, delivery, erection and brocking, consult. ANSI/IPI Quality Citleda, DSB-89 and BCS11 Building Component Safety Information civalable from Times Plate Institute, SSB D'Onotrio Drive, Modison, Wt 53719.



Truss Type MONTGOMERY_FLORIDA_125 E4632366 SPECIAL ONTGOMERY Job Reference (optional) Maronda Homes Inc., Sanford, FL 7.030 s Jan 3 2008 MITek Industries, Inc. Wed Jan 23 13:37:27 2008 Page 1 -1-0-0 1-0-0 5-4-2 3 Scale = 1:17,4 6.00 12 0-4-3, 3.00 12 3x4 = LOADING (psf) SPACING CS DEFL in -0.03 PLATES GRIP 2-0-0 Vdeft. Ld 1.25 TCLL 16.0 Plates Increase TC 0.23 Vert(LL) 240 MT20 244/190 >999 TCDL 7.0 Lumber Increase 1.25 0.23 -0.08 180 Vert(TL) >732

Horz(TL)

BRACING

TOP CHORD **BOT CHORD**

-0.00

n/a

-Weight: 19 lb

Structural wood sheathing directly applied or 5-4-2 oc purlins. Rigid cailing directly applied or 10-0-0 oc bracing.

LUMBER

BCII

BCDL

10.0

10.0

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=105/Mechanical, 2=286/0-8-0, 4=99/Mechanical Max Horz 2=141(LC 6)

YES

Max Uplift3=101(LC 6), 2=-112(LC 6)

Rep Stress Incr

Code FBC2004/TPI2002

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/18, 2-3=-81/40

1-2=0/18, 2-3=81/40

2-4=-19/19 BOT CHORD

NOTES

1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2pst; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

WB 0.00

(Matrix)

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 3 and 112 lb uplift at
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x 3.5") at joints 3 and 4.

LOAD CASE(S) Standard



January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design vold for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component applicability of design parameters and proper incorporation of component is responsibility of building designer - not thus designer. Braching is for lateral support of individual web members only. Additional temporary bracing to insure building designer - not thus designer. Braching is for lateral support of individual web members only. Additional temporary braching to insure building designer, for general guidance reporting fabrication, qualify control, starage, delivery, exection and braching, consult — AMSUTPIT Quality Criteria, DSB-89 and BCS11 Building Compone Safety Information—ovaloable from Truss Plate Institute. \$83 D'Onatria Drive, Madison, WI 53719.



Edemon, NC 27932

MONTGOMERY_FLORIDA_125 Qty Truss Truss Type E4632367 MONTGOMERY JS2 SPECIAL Job Reference (optional) 7.030 s Jan 3 2008 MiTek Industries, Inc. Wed Jan 23 13:37:28 2005 Page 1 1-0-0 3-10-2 3-10-2 1-0-0 Scale = 1:12.7 6.00 12 0-4-3 Ų, 3.00 12 Δ٠ Ġ Á٠ 3x4 = GRIP LOADING (psf) PLATES SPACING CSI DEFL **Vdefl** L/d 1.25 TC BC Verl(LL) >999 MT20 244/190 TCLL 16.0 Plates Increase 0.18 -0.01 2-4 240 -0.02 >999 180 0.11 Vert(TL) TCDL 7.0 Lumber Increase WB -0.00

Horz(TL)

BRACING

TOP CHORD

BOT CHORD

LUMBER

BCLL

TOP CHORD 2 X 4 SYP No.2

10.0 10.0

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 3=66/Mechanical, 2=226/0-8-0, 4=69/Mechanical

YES

Max Horz 2=110(LC 6)

Max Uplift3=-63(LC 6), 2=-109(LC 6)

Rep Stress Incr

Code FBC2004/TPI2002

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/18, 2-3=-52/25

BOT CHORD 2-4=-13/13

NOTES

NOTES (7)

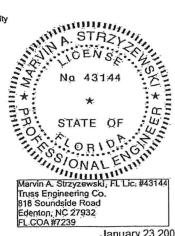
1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

0.00

(Matrix)

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 63 lb uplift at joint 3 and 109 lb uplift at
- 7) Attach with (2) 16d Common Toe-Nalls (0.162"x 3.5") at joints 3 and 4.

LOAD CASE(S) Standard



Weight: 14 lb

Structural wood sheathing directly applied or 3-10-2 oc puriins. Rigid ceiling directly applied or 10-0-0 oc bracing.

January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MI-7473 BEFORE USE. Design valid for use only with Milek connectors. This design is based only upon parameters shown, and is far on individual building component Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not has designer. Bracing it for toleral support of individual web members only. Additional temporary bracing to insure boility during construction is the responsibility of erector, Additional permanent bracing of the overal shucture is the responsibility of building designer. For general guidance regarding lobbication, qualify conflux, strategy, delivery, execution, consult — ANSI/IPI1 Quality Criteria, DSE-89 and BCS11 Building Components and provided the provided of the confluence of the confluence



MONTGOMERY FLORIDA 125 Truss Type E4632368 JS3 SPECIAL Job Reference (optional) 7,030 s Jan 3 2008 MITek industries, Inc. Wed Jan 23 13:37:28 2008 Page 1 Maronda Homes Inc., Sanford, FL 1-0-0 Scale = 1:9.3 6.00 12 Δ Δ° 3.00 12 Ď٥ Δ̈́ 3x4 = 2-4-2 PLATES GRIP LOADING (psf) SPACING CSI DEFL Vdeft 1/d 2-0-0 244/190 Plates Increase 1.25 TC BC 0.18 Vert(LL) -0.00 >999 240 MT20 16.0 TCLL >999 180 -0.00 TCDL 7.0 Lumber Increase 1.25 0.04 Vert(TL) WB 0.00 Rep Stress Incr YES Horz(TL) BCLL 100 Weight: 9 lb (Malrix) Code FBC2004/TPI2002 10.0 BCDL BRACING LUMBER Structural wood sheathing directly applied or 2-4-2 oc purlins. TOP CHORD TOP CHORD 2 X 4 SYP No.2 Rigid ceiling directly applied or 10-0-0 oc bracing. BOT CHORD 2 X 4 SYP No.2 BOT CHORD

REACTIONS (Ib/size) 3=22/Mechanical, 2=171/0-8-0, 4=39/Mechanical Max Horz 2=79(LC 6)

Max Uplift3=26(LC 5), 2=115(LC 6)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/18, 2-3=-37/7

BOT CHORD

NOTES

- (7) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 3 and 115 lb uplift at
- 7) Attach with (2) 16d Common Toe-Nalls (0.162"x 3.5") at joints 3 and 4.

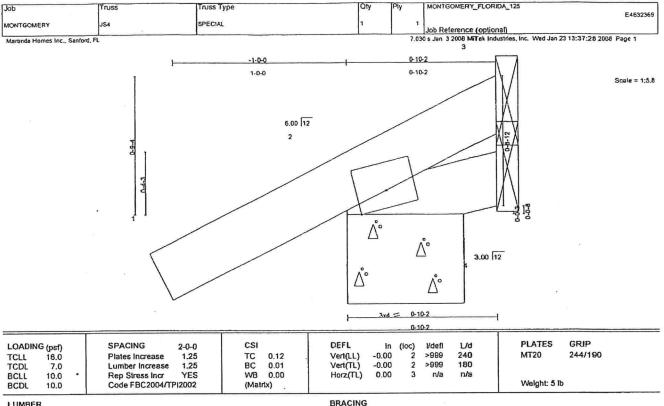
LOAD CASE(S) Standard



January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer, not it has designer. Broting shown is for toleral support of individual web members only. Additional temporary broting to insure stability during construction is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and broting, cansult. ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information. available from Trust Plate Institute. SSB D'Onotifo Drive, Modison, WI S37 19.





TOP CHORD **BOT CHORD**

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 2=123/0-8-0, 4=15/Mechanical, 3=-21/Mechanical

Max Horz 2=48(LC 6)

Max Uplift2=-115(LC 6), 3=-21(LC 1) Max Grav2=123(LC 1), 4=15(LC 1), 3=30(LC 6)

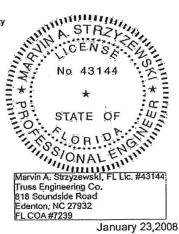
FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/15, 2-3=-35/37

BOT CHORD 2-4-3/3

NOTES (7)

- 1) Which ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANS/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 115 lb uplift at joint 2 and 21 lb uplift at joint 3.
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x 3.5") at joints 3 and 4.

LOAD CASE(S) Standard

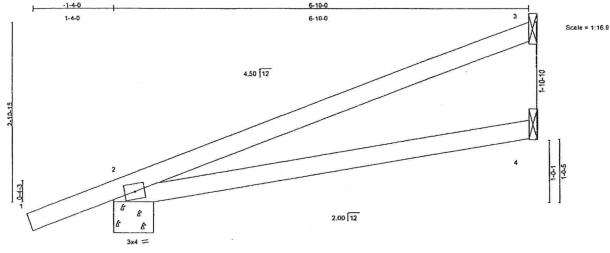


Structural wood sheathing directly applied or 0-10-2 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 REPORE USE. Design volid for use only with Miles connection, This design is based only upon parameters shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shown is for toleral support of individual web members only. Additional temporary bracing to insue slobikly during construction is the responsibility of the building designer. For general guidance regarding foblacion, quality control, storage, desiryer, see facility of the building designer. For general guidance regarding foblacion, quality control, storage, desiryer, see facility. Consult AMSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Trust Plate Institute. S83 D'Onotrio Drive, Modison, WI 53719.



Job	Truss	Truss Type	Oty	Ply	MONTGOMERY_FLORIDA_125	*
MONTGOMERY	JS5	SPECIAL	1		1 Job Reference (optional)	E4632370
Maronda Homes Inc., Sa	inford, FL			7.	.030 s Jan 3 2008 MiTek Industries, Inc. Wed Jan 2	3 13:37:29 2008 Page 1
	-1-4-0	1	6-10-0			



							6-10	-0-						
LOADIN TCLL TCDL BCLL	IG (psf) 16.0 7.0 10.0	•	SPACING Plates Increase Lumber Increase Rep Stress Incr	2-0-0 1.25 1.25 YES	CSI TC BC WB	0.37 0.39 0.00	DEFL Vert(LL) Vert(TL) Horz(TL)	in -0.09 -0.23 -0.00	(loc) 2-4 2-4 3	l/defl >845 >338 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190	
BCDL	10.0		Code FBC2004/TI	PI2002	(Mat	rix)						Weight: 23 lb	י	

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=138/Mechanical, 2=366/0-8-0, 4=129/Mechanical Max Horz 2=137(LC 6) Max Uplift3=115(LC 6), 2=157(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/19, 2-3=-73/41

BOT CHORD 2-4=-17/17

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
 Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 115 lb uplift at joint 3 and 157 lb uplift at joint 2.
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x 3.5") at joints 3 and 4.

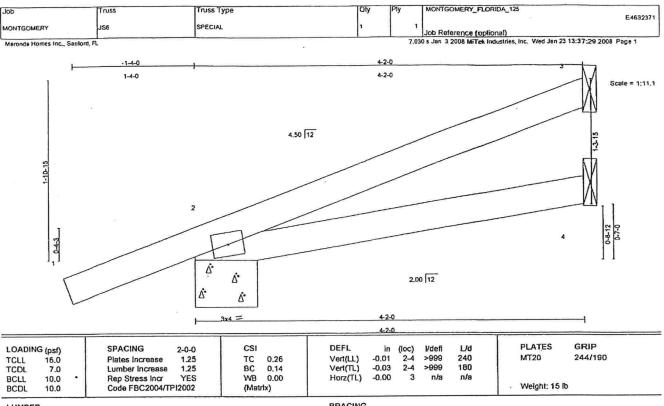
LOAD CASE(S) Standard



January 23,2008

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design votal for use only with Millex connection. This design is based only upon parameters than including a mixed in mixed by the Design votal for use only with Millex connection. This design is based only upon parameters shown, and is for on individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trus designer. Roccing shown is for toleral support of individual web members only. Additional temporary bracing to insure shifty during consultaction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the designer, for general guidance regarding fobication, quosity control, stravage, delivery, erection and bracing, consult. AMSI/TPI1 Quality Criteria, DS8-89 and BCSI1 Building Component Safety Information. available from Truss Plate Institute, S83 D'Onotrio Drive, Madison, WI 53719.





LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

BRACING

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-2-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=69/Mechanical, 2=259/0-8-0, 4=75/Mechanical Max Horz 2=95(LC 6) Max Uplift3=53(LC 6), 2=146(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/19, 2-3=-42/21

BOT CHORD 2-4=-10/10

NOTES

- NOTES (7)

 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 3 and 146 lb uplift at
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x 3.5") at joints 3 and 4.

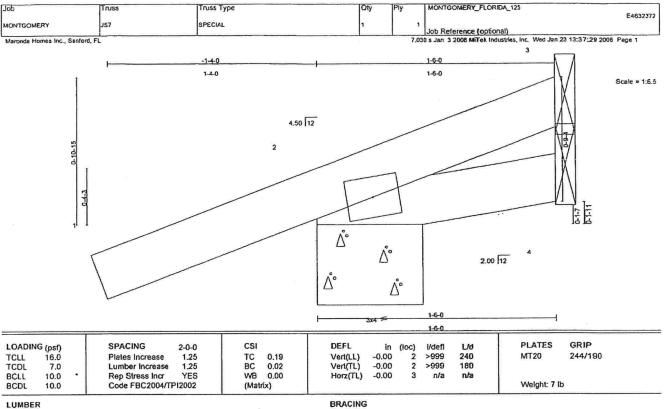
LOAD CASE(S) Standard



January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 REFORE USE. Design void for use only with Miles connectors this design is based only upon porometes show, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trus designer, forcing shows it for lateral support of Individual web members only. Additional temporary brocking to insure shoulding designer - not trus designer, forcing shows it for lateral support of Individual web members only. Additional temporary brocking to insure shoulding designer for general guidance regarding to the creation. Additional permonent brocking of the overal structure is the responsibility of the building designer. For general guidance regarding labrication, quotify control, storage, delivery, exection and brocking, consult. AMSI/IPII Quality Criteria, DSB-89 and BCSII Building Component Safety Intermation.





TOP CHORD

BOT CHORD

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 2=162/0-8-0, 4=28/Mechanical, 3=-1/Mechanical

Max Horz 2=54(LC 6)

Max Uplift2=136(LC 6), 3=10(LC 5)

Max Grav2=162(LC 1), 4=28(LC 1), 3=15(LC 4)

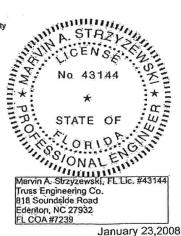
FORCES (lb) - Maximum Compression/Maximum Tension 1-2=0/17, 2-3=-30/16 2-4=-4/4 TOP CHORD

BOT CHORD

NOTES

- 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 3) *This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-8-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 136 lb uplift at joint 2 and 10 lb uplift at joint 3.
- 7) Attach with (2) 16d Common Toe-Nails (0.162"x 3.5") at Joints 3 and 4.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 1-6-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 BEFORE USE. Design vold for use only with Millek connectors. This design is based only upon porometers shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for talerd support of individual web membes only. Additional temporary bracing to insure stiffy during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fobication, quality control, storage, desivery, erection and bracing, consult. AMSI/IPTI Quality Criteria, DSB-89 and BCSI1 Building Component Solely Information available from Truss Plate Institute, S83 D'Onatio Drive, Modison, WI 53719.



MONTGOMERY_FLORIDA_125 Truss Type E4832373 MONTGOMERY JSGRD1 SPECIAL Job Reference (optional) s Jan 3 2008 MITek Industries, Inc. Wed Jan 23 13:37:30 2008 Page 1 Maronda Homes Inc., Sanford, FL 10-4-5 9-10-13 1-8-0 5-8-9 4-2-4 0-5-8 Scale = 1:21.8 6x8 = 3,60 12 20 19 6x12 = 04-3 3x4 = 15 1.80 12 3x4 = 9-10-13 5-8-9 4-2-4 5-8-9 Plate Offsets (X.Y): [2:0-10-5.0-1-8] LOADING (psf) DEFL Ud PLATES GRIP SPACING CSI 1/defi 2-0-0 in (loc) 1.25 0.61 0.07 >999 240 MT20 244/190 16.0 Plates Increase TC Vert(LL) TCLL TCDL 7.0 Lumber Increase 1.25 BC 041 Vert(TL) -0.09 2-7 >999 180 WB NO 0.26 0.02 nta BCLL 10.0 Rep Stress Incr Horz(TL) n/a Code FBC2004/TPI2002 Weight: 51 lb **BCDI** 100 (Matrix) LUMBER BRACING Structural wood sheathing directly applied or 5-8-7 oc purlins, except TOP CHORD 2 X 4 SYP No.2 TOP CHORD BOT CHORD 2 X 4 SYP No.2 2 X 4 SYP No.3 "Except" **BOT CHORD** Rigid celling directly applied or 6-3-14 oc bracing. WEBS 5-62 X 6 SYP No.2 Left 2 X 4 SYP No.2 2-6-8 SLIDER REACTIONS (Ib/size) 2=540/0-11-11, 8=573/Mechanical Max Horz 2=151(LC 3) Max Uplift2=375(LC 3), 8=505(LC 3) FORCES . (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/17, 2-9=-1260/917, 3-9=-1241/916, 3-10=-1224/920, 10-11=-1214/913, 4-11=-1214/890, 4-12=-357/331, 1-2-17, 2-9-1200917, 3-9-1241910, 3-19-1224920, 10-11-1214913, 4-11-1214090, 4-12-30// 12-13-3447306, 13-14-310/256, 5-14-293/214, 5-6-201472 2-15-958/1163, 15-16-957/1176, 16-17-951/1187, 7-17-950/1195, 7-18-958/1161, 18-19-957/1171, BOT CHORD 19-20-954/1185, 6-20-953/1197 WEBS 4-7=0/287, 4-6=842/637, 5-8=-610/562, 6-8=-269/215 STRZY TOEN A STRZY ICENS 1) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.33 plate grip DOL=1.33. 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide No 43144

* * *

ORIONAL

Marvin A Strzyzewski, FL Lic. #43144

Truss Engineering Co. will fit between the bottom chord and any other members. 5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. Refer to girder(s) for truss to truss connections. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 375 lb uplift at joint 2 and 505 lb uplift at ioint 8. 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 56 to up at 4-0-3, 7 lb down and 86 lb up at 5-3-12, 6 lb down and 99 lb up at 6-6-3, 60 lb down and 178 lb up at 8-7-12, and 27 lb down and 156 lb up at 9-0-3, and 11 lb up at 1-11-12 on top chord, and 12 lb up at 1-11-12, 1 lb up at 4-0-3, 35 lb down at 5-3-12, 29 lb down at 6-8-3, and 89 lb down at 8-7-12, and 59 to down at 9-0-3 on bottom chord. The design/selection of such connection device(s) is the responsibility of 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). LOAD CASE(S) Standard Truss Engineering Co. 818 Soundside Road Edenton, NC 27932 Continued on page 2 FL COA #7239 January 23,2008

WARNING - Verify design parameters and SEAD NOTES ON THIS AND INCLUDED NITEE REFERENCE PAGE MIL-7473 BEPORE U.S..

Design volid for use only with Milet connectors. This design is based only upon parameters shown, and is for an individual building component,
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not inus designer. Bracing shown
is for lateral support of individual web members only. Additional temporary bracing to insure stobility during constituction is the responsibility of the
erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, For general guidance regarding
tobication, quality control, strategic, devery, erection and bracing, consult. AMS/IPTI Quality Citteria, DSB-89 and BCSII Building Component
Safety Information available from Truss Plate Institute, 583 D'Onotrio Drive, Madkon, WI S3719.



Job	Truss	Truss Type	Qly	Ply	MONTGOMERY_FLORIDA_125
MONTGOMERY	JSGRD1	SPECIAL	1	1	Job Reference (optional)

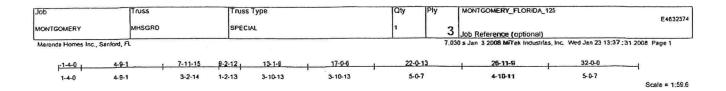
Maronda Hornes Inc., Sanford, FL

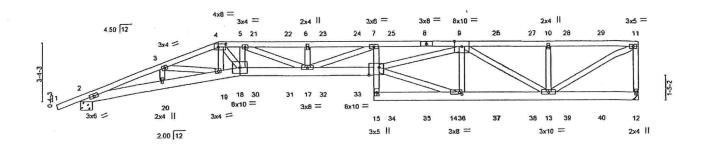
7.030 s Jan 3 2008 MiTek Industries, Inc. Wed Jan 23 13:37:30 2008 Page 2

LOAD CASE(S) Standard 1) Regular: Lumber Increase=1.25, Plate Increase=1.25 Uniform Loads (pif) Veri: 1-5=46, 2-6=40

Concentrated Loads (lb)

Vert: 10=56(F) 11=9(B) 12=12(F) 13=-60(B) 14=-27(F) 15=12(B) 16=1(F) 17=-35(B) 18=-29(F) 19=-89(B) 20=-59(F)





	1	4-9-1	7-11-15	p-2-12	13-1-9		17-0-6		22-0-13	_	26-11-9	32-0-0		
	,-	4-9-1	3-2-14	1-2-13	3-10-13	•	3-10-13	•	5-0-7		4-10-11	5-0-7		
Plate Off	sets (X.Y): [4:0-	5-4.0-2-0]. [14:0-1-12	.0-1-8]_[16:0	-7-0.0-5-4]									_
LOADIN	G (psf)		SPACING	2-0-0	CSI			DEFL	in (loc)	Vdefl	L∕d	PLATES	GRIP	
TCLL	16.0		Plates Increase	1.25	TC	0.52		Vert(LL)	0.62 16-17	>605	240	MT20	244/190	
TCDL	7.0		Lumber Increase	1.25	BC	0.93		Vert(TL)	-1.04 16-17	>363	180			
BCLL	10.0	•	Rep Stress Incr	NO	WB	88.0		Horz(TL)	0.34 12	n/a	n/a			
BCDL	10.0		Code FBC2004/TI	12002	(Mat	rix)						Welght: 588 lb	i	

BRACING

TOP CHORD

BOT CHORD

except end verticals.

LUMBER

TOP CHORD 2 X 4 SYP No.2

BOT CHORD 2 X 6 SYP No.2 *Except*

7-15 2 X 4 SYP No.1D

WEBS

2 X 4 SYP No.3

REACTIONS (lb/size) 12=2568/Mechanical, 2=2659/0-8-0

Max Horz 2=158(LC 7) Max Uplift12=-1244(LC 7), 2=-1376(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD

1-2=0/22, 2-3=-9729/4973, 3-4=-10520/5640, 4-5=-11872/6355, 5-21=-13502/7111, 21-22=-13502/7111, 6-22=-13502/7111, 23-24=-13502/7111, 7-24=-13502/7111, 7-25=-13819/7150, 8-25

8-9=-13819/7150, 9-26=-3723/1851, 26-27=-3723/1851, 10-27=-3723/1851, 10-28=-3723/1851, 28-29=-3723/1851,

11-29-3723/1851, 11-12-2377/1194

2-20-4729/9067, 19-20-4768/9131, 18-19-5400/10032, 18-30-6404/11948, 30-31-6404/11948, 17-31-6404/11947, 17-32-7496/14435, 32-33-7495/14439, 16-33-7495/14442, 15-16-121/298, 7-16-0/131. **BOT CHORD**

15-34=-685/1310, 34-35=-685/1310, 14-35=-685/1310, 14-36=-3115/6152, 36-37=-3115/6152, 37-38=-3115/6152,

13-38=3115/6152, 13-39=61/118, 39-40=61/118, 12-40=61/118

3-20-269/291, 3-19-686/892, 4-19-497/1001, 4-18-1612/3107, 5-18-801/543, 5-17-799/1681, 6-17-296/383, 7-17-999/437, 14-16-2441/4895, 9-16-4335/8219, 9-14-1372/777, 9-13-2841/1478, 10-13-342/233,

11-13=-2095/4218

NOTES

WEBS

1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc, 2 X 4 - 1 row at 0-9-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.

3) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise); Lumber DOL=1.33 plate grip DOL=1.33.

4) Provide adequate drainage to prevent water ponding.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Refer to glrder(s) for truss to truss connections.

8) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity Continued on page 2

A STRZ

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No 43144

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Marvin A Strzyzewski, FL Lic. #43144

Truss Engineering Co.

V 818 Soundalds Decided Truss Engineering Co. **B18 Soundside Road**

Edenton, NC 27932 FL COA #7239

Structural wood sheathing directly applied or 5-10-10 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REPERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Miller connectors. This design is based only upon proameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer- not fuss designer. Bracing shows it for larged support of individual web members only. Additional temporary bracing to laws the prostruction is the responsibility of the execut. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, qualify control, storage, delivery, erection and bracing, consult. AMSI/[FII] Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Piole Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Oty	Ply	MONTGOMERY_FLORIDA_125 E463	2374
MONTGOMERY	MHSGRD	SPECIAL	1	3	Job Reference (optional)	

Maronda Homes Inc., Sanford, FL

NOTES
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1244 lb uplift at joint 12 and 1376 lb uplift at joint 2.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 75 lb down and 162 lb up at 7-11-15, 75 lb down and 162 lb up at 12-0-11, 75 lb down and 162 lb up at 12-0-11, 75 lb down and 162 lb up at 12-0-11, 75 lb down and 162 lb up at 12-0-11, 69 lb down and 89 lb up at 18-0-11, 69 lb down and 69 lb up at 20-0-11, 69 lb down and 69 lb up at 22-0-11, 69 lb down and 69 lb up at 28-0-11, and 69 lb down and 69 lb up at 30-0-11 on top chord, and 745 lb down and 454 lb up at 7-11-15, 72 lb down at 10-0-11, 78 lb down at 16-0-11, 78 lb down and 68 lb up at 18-0-11, 78 lb down and 68 lb up at 22-0-11, and 78 lb down and 68 lb up at 23-0-11 on bottom chord. The design/selection of 24-0-11, 78 lb down and 68 lb up at 26-0-11, and 78 lb down and 68 lb up at 28-0-11, and 78 lb down and 68 lb up at 30-0-11 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Regular. Lumber Increase=1.25, Plate Increase=1.25

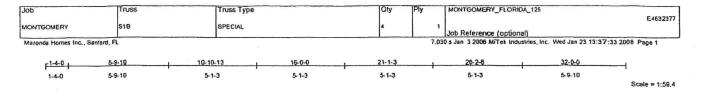
Uniform Loads (plf)

Vert: 1-4-46, 4-11-46, 2-18-40, 16-18-40, 12-15-40

Concentrated Loads (lb)

Vert: 4=75(F) 8=69(F) 19=.745(F) 9=69(F) 21=.75(F) 22=.75(F) 23=.75(F) 24=.75(F) 25=.69(F) 26=.69(F) 27=.69(F) 28=.69(F) 29=.69(F) 30=.72(F) 31=.72(F) 32=.72(F) 33=.72(F) 34=.78(F) 35=.78(F) 36=.78(F) 37=.78(F) 38=.78(F) 39=.78(F) 40=.78(F)





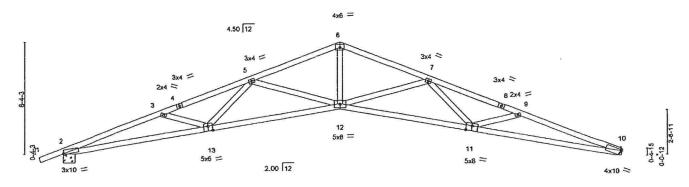


Plate Off	sels (X.	Y): [2	:0-0-7.Edge]. [10:Edge.	0-0-11]. [11:0	4-0.0-3-4].	13:0-3-0.0-3	3-41		-				
LOADIN	G (psf)		SPACING	2-0-0	CSI		DEFL	in (loc)	Vdefl	L/d	PLATES	GRIP	
TCLL	16.0	- 1	Plates Increase	1.25	TC	0.69	Vert(LL)	-0.57 11-12	>661	240	- MT20	244/190	
TCDL	7.0	- 1	Lumber Increase	1.25	BC	0.93	Vert(TL)	-1.15 11-12	>330	180			
BCLL	10.0	*	Rep Stress Incr	YES	WB	0.68	Horz(TL)	0.59 10	n/a	n/a			
BCDL	10.0	- 1	Code FBC2004/TI	PI2002	(Mati	ix)	1				Weight: 141 lb):	

BRACING

TOP CHORD

BOT CHORD

7-7-12

LUMBER

TOP CHORD 2 X 4 SYP No.2 **BOT CHORD**

2 X 4 SYP No.2 *Except*

2-132 X 4 SYP No.1D, 10-11 2 X 4 SYP No.1D

WEBS 2 X 4 SYP No.3

REACTIONS (lb/size) 2=1443/0-8-0, 10=1360/Mechanical

Max Horz 2=119(LC 6) Max Uplift2=363(LC 6), 10=265(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/19, 2-3=-4787/1438, 3-4=-4633/1254, 4-5=-4592/1266, 5-6=-3414/995, 6-7=-3414/995, 7-8=-4745/1358.

7-7-12

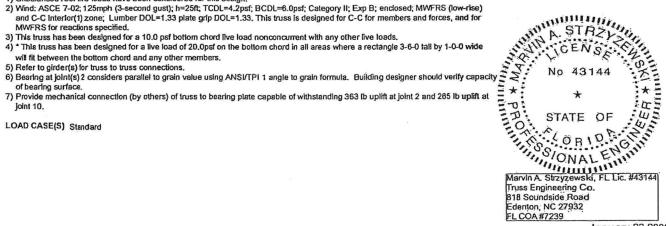
8-9-4787/1344, 9-10-5042/1587 **BOT CHORD** 2-13=-1279/4475, 12-13=-1056/4004, 11-12=-1095/4076, 10-11=-1437/4742

3-13=127/236, 5-13=1/594, 5-12=837/368, 6-12=500/2123, 7-12=905/409, 7-11=63/694, 9-11=239/309

NOTES

1) Unbalanced roof live loads have been considered for this design,

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Calegory II; Exp B; enclosed; MWFRS (low-rise)



8-4-4

Structural wood sheathing directly applied or 2-2-0 oc purlins.

Rigid ceiling directly applied or 2-2-0 oc bracing.

January 23,2008

⚠ WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design vold for use only with MITex connectors. This design is bosed only upon porcemeters shown, and is for an individual bublishing component. Applicability of design parameteris and proper incorporation of component is responsibility of building designer. For this designer, Roccing show is for larlend support of individual web members only. Additional temporary bracing to insure shiply during construction is the responsibility of the bublishing designer. For general guidance responsibility of the bublishing designer, For general guidance regarding fabrication, qualify control, storage, delivery, erection and broating, consulf. AMSI/IPIT Quality Ciferia, DSB-89 and BCSII Building Component Sofety Intermedian evalidate from Truss Prote Institute. S83 D'Onofito Drive. Modison, WI S3719.



MONTGOMERY_FLORIDA_125 Truss Type Truss E4632378 Job SIC Job Reference (optional)
7.030 s Jan 3 2008 MITek Industries, Inc. Wed Jan 23 13:37:34 2008 Page 1 Maronda Homes Inc., Sa 22-11-11 -1-4-0 7-3-3 6-11-11 7-7-12 1-4-0 Scale = 1:56.1 4x6 = 4.50 12 2x4 = 5x6 > 4 12 5x8 = 13 4x6 = 2.00 12 5x6 = 27-6-8 30-2-14 27-4-12 16-0-0 0-1-12 2-8-6 5-8-6 7-7-12 8-4-4 Plate Offsets (X.Y): [13:0-3-0.0-3-4] GRIP PLATES I/defi DEFL 2-0-0 CSI LOADING (psf) >999 240 MT20 244/190 Vert(LL) -0.32 -0.67 2-13 TC Plates Increase 1.25 16.0 TCLL 2-13 >485 180 1.25 BC 0.89 Vert(TL) TCDL Lumber Increase 0.28 10 n/a n/a Horz(TL) Rep Stress Incr YES WB 0.67 BCLL 100 Weight: 138 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0 BRACING Structural wood sheathing directly applied or 2-7-9 oc purlins. LUMBER TOP CHORD TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 Rigid celling directly applied or 6-0-0 oc bracing. BOT CHORD 2 X 4 SYP No.3 REACTIONS (lb/size) 10=1426/0-3-8, 2=1225/0-8-0 Max Horz 2=139(LC 6) Max Uplift10=-269(LC 7), 2=-337(LC 6) FORCES (lb) - Maximum Compression/Maximum Tension 1-2=0/19, 2-3=-3854/1173, 3-4=-3619/985, 4-5=-3577/998, 5-6=-2359/702, 6-7=-2362/705, 7-8=-1963/531, 8-9=0/187 TOP CHORD 2-13=1067/3608, 12-13=802/3009, 11-12=604/2205, 10-11=300/58, 9-10=130/0 6-12=317/1409, 7-12=88/201, 3-13=209/255, 5-13=36/648, 5-12=836/363, 7-11=550/317, 8-10=-1252/454, BOT CHORD WEBS 8-11=-433/2084

NOTES

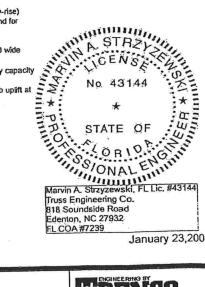
Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02, 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Bearing at Joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 269 lb uplift at joint 10 and 337 lb uplift at

LOAD CASE(S) Standard

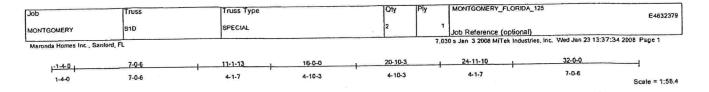


January 23,2008

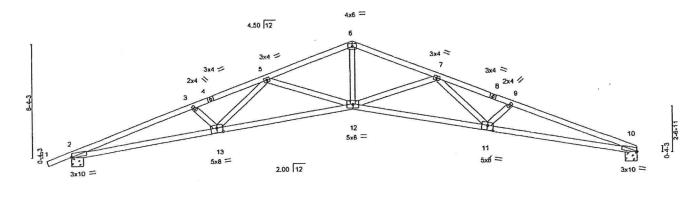
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-1473 BEFORE USE. WARNING - verify design parameters and NEAD NOTES ON THIS AND INCLUDED BLIEF NET PAGE BLIFTED BLIFTED AND AUTOR VOIC.

Design volid for use only with Miller connection. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer, not have designer. Bracing shown
for taleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the
for taleral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the
area of the properties of the propert





RATE ARE ATTRACT.



eta Officia (V V): 13	:0-0-7,Edge]. [8:0-0-0.0-0-0]. [10:0-0	-7 Fdgel [11:0-4-0.0-3-0]	[13:0-4-0.0-3-0]	
OADING (psf) CLL 16.0 CDL 7.0 CLL 10.0 CDL 10.0	SPACING 2-0-0 Plates Increase 1.25 Lumber Increase 1.25 Rep Stress Incr YES Code FBC2004/TPI2002	CSI TC 0.92 BC 0.90 WB 0.66 (Matrix)	DEFL in (loc) Vdefl Vert(LL) -0.55 12-13 >681	L/d PLATES GRIP 240 MT20 244/190 180 Welght: 138 lb

BRACING

TOP CHORD

BOT CHORD

23-8-12

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 *Except*

2-13 2 X 4 SYP No.1D, 10-11 2 X 4 SYP No.1D

2 X 4 SYP No.3

WEBS

REACTIONS (lb/size) 10=1345/0-8-0, 2=1429/0-8-0

Max Horz 2=120(LC 8)

Max Upliff10=260(LC 7), 2=362(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=0/19, 2-3=-4776/1382, 3-4=-4624/1303, 4-5=-4586/1314, 5-6=-3344/975, 6-7=-3343/975, 7-8=-4612/1376, TOP CHORD

8-9-4652/1364, 9-10-4807/1451

2-13=1224/4470, 12-13=1012/3880, 11-12=1033/3889, 10-11=1295/4502 BOT CHORD 3-13=140/198, 5-13=100/736, 5-12=779/342, 6-12=476/2068, 7-12=789/364, 7-11=150/758, 9-11=149/218 WEBS

NOTES

Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.

16-0-0

- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide
- will fit between the bottom chord and any other members.

 5) Bearing at joint(s) 10, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 260 lb uplift at joint 10 and 362 lb uplift at Joint 2.

LOAD CASE(S) Standard



B-3-4

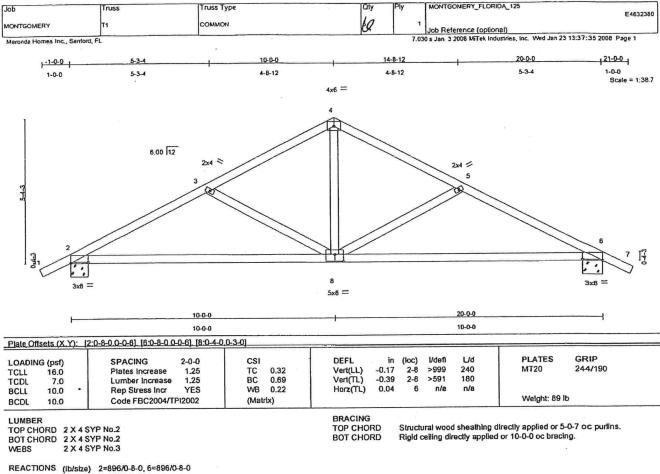
Structural wood sheathing directly applied.

Rigid ceiling directly applied or 5-8-8 oc bracing.

January 23,2008

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEE REFERENCE PAGE MII-9473 BEFORE USE. WANTING - veryy sesign parameters and READ NOTES ON THIS AND INCLUDED BITTLE NOT EXECUTED BITTLE NOT EXECUTED BITTLE NOT THIS AND INCLUDED BITTLE NOT EXECUTED BITTLE NOT ON THE BITTLE NOT ON T





Max Horz 2=100(LC 6)

Max Uplift2=-239(LC 6), 6=-239(LC 7)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/21, 2-3=-1279/429, 3-4=-1036/322, 4-5=-1036/322, 5-6=-1279/429, 6-7=0/21

BOT CHORD

2-8=-249/1109, 6-8=-249/1109 3-8=-283/222, 4-8=-80/676, 5-8=-283/222 WEBS

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 239 lb uplift at joint 2 and 239 lb uplift at joint 6.

LOAD CASE(S) Standard



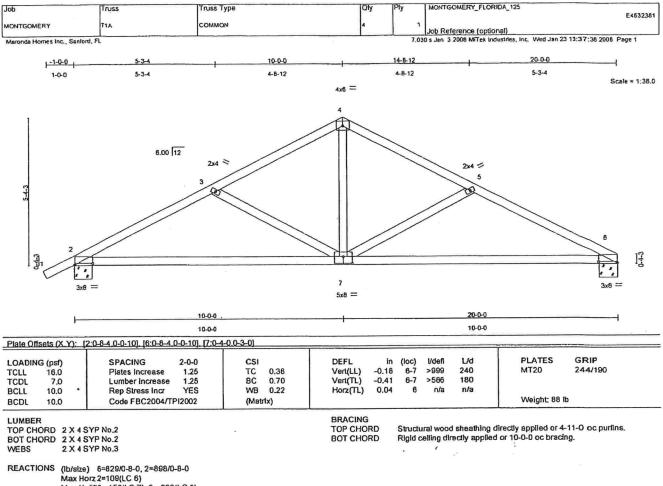
January 23,2008

WARNING - Verify deeting parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-173 BEFORE USE.

Design valid for use only with Millek connectors. This design is based only upon porometers shown, and is for an individual building component.

Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not it was designer. Procing shown for taleral support of individual web members only. Additional temporary bracing to insure slowly during construction is the responsibility of the exector. Additional permanent bracing of the overal shucture is the responsibility of the building designer. For general guidance regarding fablication, quality control, strategy, delivery, exercision and bracing, consult — ANSI/IPI Quality Criteria, DSB-89 and BCSI1 Building Component Solety Information available from Truss Plate Institute, SSB D'Onofilo Drive, Modison, WI 53719.





Max Uplift6=156(LC 7), 2=239(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/21, 2-3=-1284/443, 3-4=-1041/335, 4-5=-1042/337, 5-6=-1290/457

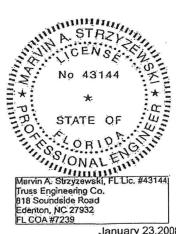
2-7=-312/1113, 6-7=-332/1122 **BOT CHORD**

3-7=282/222, 4-7=97/682, 5-7=-292/245 WEBS

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 156 lb uplift at joint 6 and 239 lb uplift at joint 2.

LOAD CASE(S) Standard

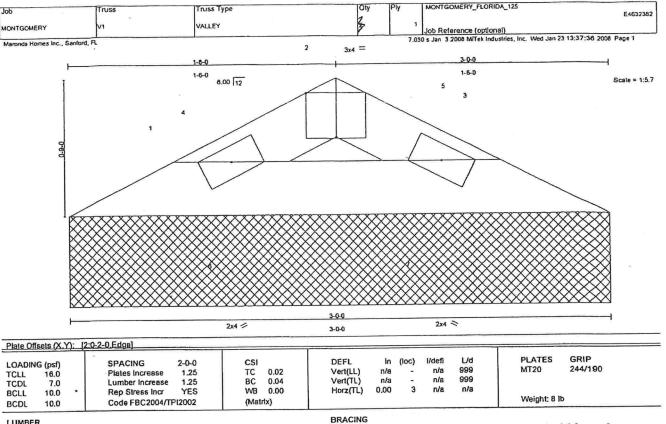


January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-1473 REFORE USE.

Design void for use only with Miller connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer- not thus designer. Reading this for lateral support of Individual web members only. Additional temporary brocing to insure stability during construction is the responsibility of the cultivation of promoter tracing of the overall structure is the responsibility of the building designer. For general guidance regarding fability of upon construction is the designer, for general guidance regarding fability of the building designer. For general guidance regarding fability of the building designer. For general guidance regarding fability of the building designer. For general guidance regarding fability of the building designer. For general guidance regarding fability of the building designer. For general guidance regarding fability of the building designer. For general guidance regarding fability of the building designer. For general guidance regarding fability of the building designer. For general guidance regarding fability of the building designer. For general guidance regarding fability of the building designer.





TOP CHORD BOT CHORD

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=76/3-0-0, 3=76/3-0-0

Max Horz 1=-7(LC 4)

Max Upliff1=-14(LC 6), 3=-14(LC 7)

FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-4=-53/42, 2-4=-45/44, 2-5=-45/44, 3-5=-53/42

1-6=27/41, 6-7=27/41, 3-7=27/41 BOT CHORD

NOTES

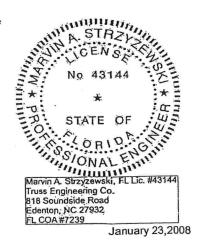
1) Unbalanced roof live loads have been considered for this design.

- 2) Wnd: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2pst; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 1 and 14 lb uplift at joint 3.

LOAD CASE(S) Standard

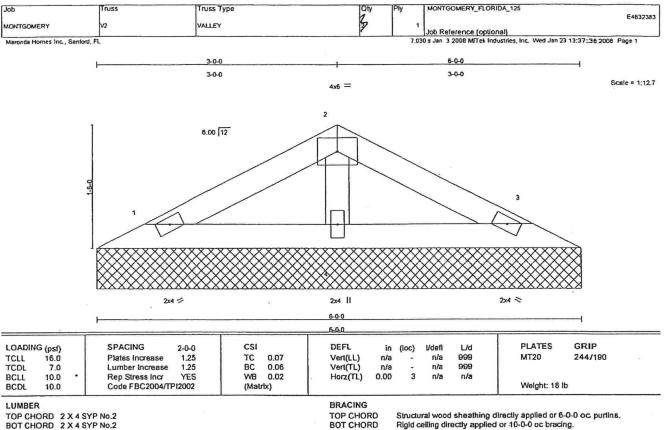


Structural wood sheathing directly applied or 3-0-0 oc purins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Millex connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer- not have designer. Bracing shows to full designer and truss designer, Bracing shows to full designer and truss designer. Bracing shows to full designer and truss designer, Bracing shows to full designer and truss designer. Bracing shows to full designer and truss designer, Bracing shows to full designer and truss designer. Additional permanent bracing of the overal structure is the responsibility of the building designer. For general guidance regarding tobaction, quality control, storage, designer, and a RASI/[FII] Quality Criteria, DS8-89 and BCSI1 Building Component Safety Information ovaliable from Truss Piote Institute. S83 D'Onofrio Drive, Modison, Wt S3719.





TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

OTHERS 2 X 4 SYP No.3

REACTIONS (lb/size) 1=96/6-0-0, 3=96/6-0-0, 4=217/6-0-0

Max Horz 1=20(LC 4) Max Uplift1=32(LC 6), 3=-35(LC 7), 4=-14(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=33/28, 2-3=-33/28

BOT CHORD 1-4=-1/12, 3-4=-1/12

2-4=-98/79

WEBS

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02: 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Gable requires continuous bottom chord bearing.
- 5) "This truss has been designed for a five load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 32 lb uplift at joint 1, 35 lb uplift at joint 3 and 14 lb uplift at joint 4.

LOAD CASE(S) Standard

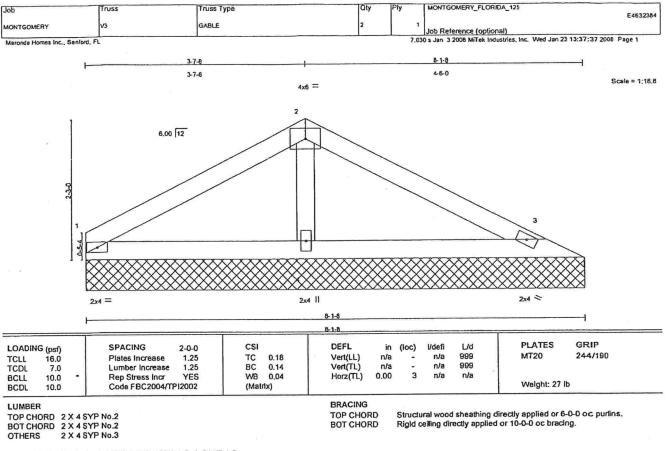


January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Millek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not have designer. Procing shown is for lateral support of individual web members only. Additional temporary bracing to insure slowling during construction is the responsibility of the suiting designer. For general guidance regarding fobication, qualify control, storage, delivery, excellion and bracing, consult. AMSI/(PI) Qualify Criteria, DSB-89 and BCS11 Building Component Safety Information ovaliable from Truss Plate Institute, S83 D'Onotiro Drive, Madison. WI 53719.



Edemon, NC 27932



REACTIONS (lb/size) 1=147/8-1-8, 3=158/8-1-8, 4=341/8-1-8

Max Horz 1=-32(LC 4)

Max Uplift1=49(LC 6), 3=57(LC 7), 4=21(LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=53/46, 2-3=-53/45

1-4-2/20, 3-4-2/20 **BOT CHORD**

WEBS

2-4=153/119

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Whid: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Gable requires continuous bottom chord bearing.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 1, 57 lb uplift at joint 3 and 21 lb uplift at joint 4.

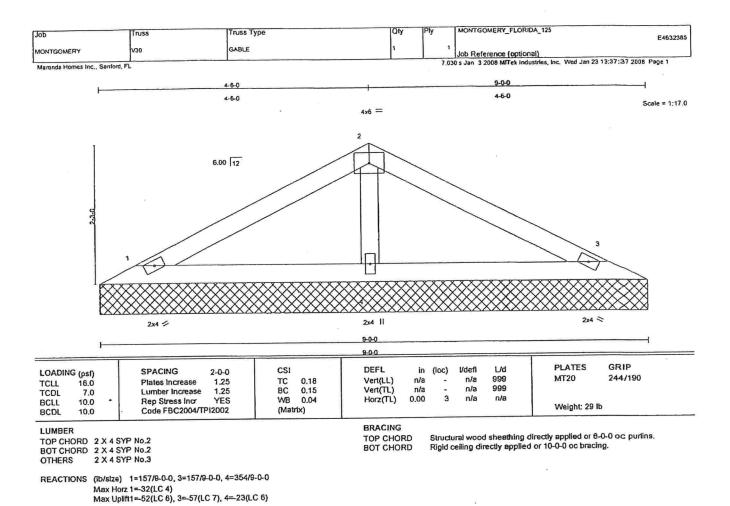
LOAD CASE(S) Standard



January 23,2008

umsters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design volid for use only with MiTek connection. This design is based only upon parameters shown, and is for an individual building component, Applicability of design paramenters and proper incorporation of component is responsibility of building designer, not has designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the building designer, for general guidance regarding fobication, quality carried, storage, delivery, erection and bracing, consult. ANSI/IPII Quality Criteria, DSS-89 and BCSI1 Building Component Society Intermedian available from trus Piote institute, SSI D'Onotifo Drive, Maddon, Wt 53719.





WEBS

TOP CHORD

BOT CHORD

FORCES (lb) - Maximum Compression/Maximum Tension

1-2=-53/46, 2-3=-53/45 1-4=-2/19, 3-4=-2/19

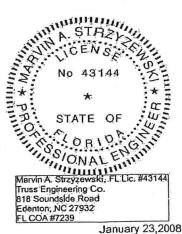
2-4=-160/124

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 1, 57 lb uplift at joint 3 and 23 lb uplift at joint 4.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Delign void for use only with Miles connection. This design is based only upon parameters shown, and is for an individual building component.

Applicability of design parameters and proper incorporation of component is responsibility of building designer- not inus designer. Bracing shown is for falled support of individual web membes only. Additional temporary bracing to insure shifting for specialisty of the state of the property bracing to insure shifting designer, not prostruction is the responsibility of the state of the responsibi



Truss Type MONTGOMERY_FLORIDA_125 E4632386 MONTGOMERY V31 GABLE Job Reference (optional) 7.030 s Jan 3 2008 MTck Industries, Inc. Wed Jan 23 13:37:38 2008 Page 1 Maronda Homes Inc., Sanford, FL 10-7-D 5-3-8 Scale = 1:20.9 4x6 = 6.00 12 10-7-0 10-7-0 Plate Offsets (X.Y): [1:0-2-5.0-1-8] [3:0-2-5.0-1-8] LOADING (psf) SPACING 2-0-0 CSI DEFL in Vdefi L/d PLATES GRIP 16.0 Plates Increase 1.25 TC TCLL 0.20 Vert(LL) 999 n/a n/a MT20 244/190 TCDL 7.0 Lumber Increase 1.25 BC 0.28 Vert(TL) n/a 999 n/a BCLL 10.0 Rep Stress Incr YES WB 0.06 Horz(TL) 0.00 n/a BCDI 10.0 Code FBC2004/TPI2002 (Matrix) Weight: 37 lb LUMBER BRACING TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

BOT CHORD 2 X 4 SYP No.2

REACTIONS (lb/size) 1=190/10-7-0, 3=190/10-7-0, 4=530/10-7-0 Max Horz 1=-45(LC 4)
Max Uplift1=-49(LC 6), 3=-57(LC 7), 4=-73(LC 6)
Max Grav1=195(LC 10), 3=195(LC 11), 4=530(LC 1)

FORCES (Ib) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-75/64, 2-3=-75/64 **BOT CHORD** 1-4-4/38, 3-4-4/38 WEBS 2-4=-258/185

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 49 lb uplift at joint 1, 57 lb uplift at joint 3 and 73 lb uplift at joint 4.

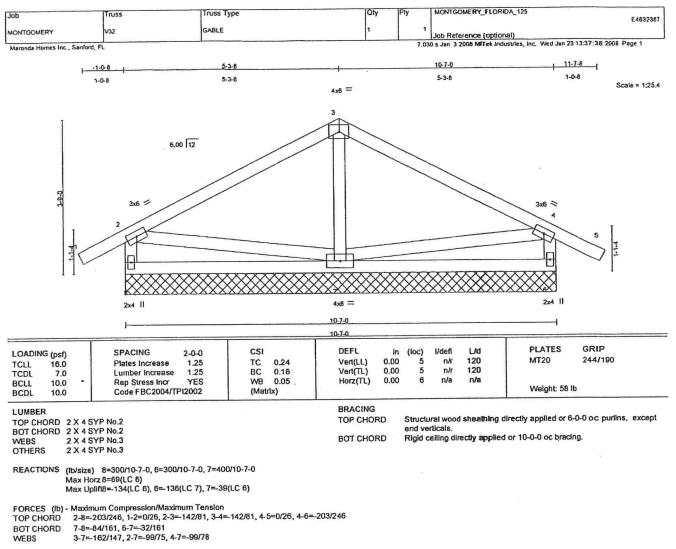
LOAD CASE(S) Standard



January 23,2008

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design vold for use only with Millek connectors. This design is based only upon pormeltes shown, and is for an individual building component. Applicability of design paramenters and proper incorporation of component is responsibility of building designer - not trust designer. Bracing shows it for lateral support of individual web members only. Additional temporary bracing to insure stripting during construction is the responsibility of the erector. Additional permanent bracing of the event structure is the responsibility of the building designer. For general guidance regarding labitations qualify control, stronge, delivery, erection and bracing, consult. ANSI/TRI Quality Criteria, DS8-89 and BCSI1 Building Component Safety Information available from Truss Plate trafficies, 583 D'Onotifio Dave, Madison, Wi 53719.





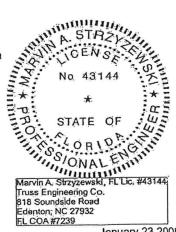
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interfor(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Gable requires continuous bottom chord bearing.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 134 lb uplift at joint 8, 136 lb uplift at joint 6 and 39 lb uplift at joint 7.

LOAD CASE(S) Standard

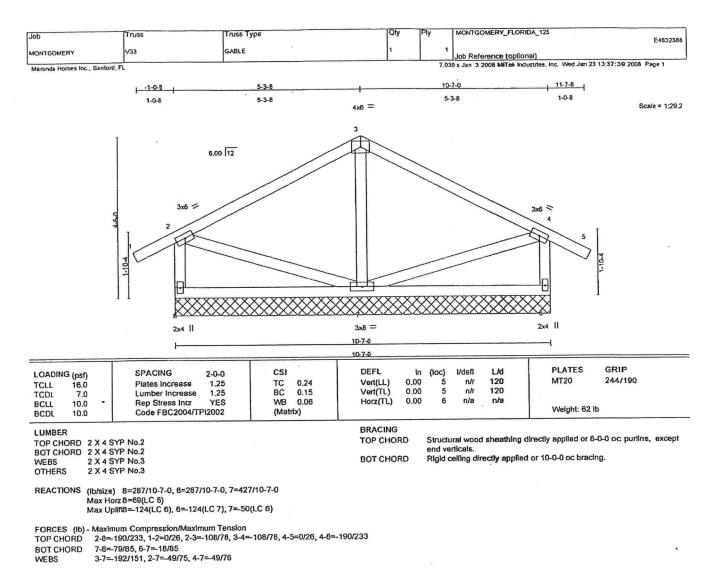


January 23,2008

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-1473 BEFORE USB. Design volid for use only with Milek connection. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer-not inus designer. Broching shown is for lateral support of individual web members only. Additional temporary broching to insure stability during construction is the responsibility of the certain parameters and procing of the overall structure is the responsibility of the building designer. For general guidance regarding of the overall structure is the responsibility of the building designer. For general guidance regarding obstitution, quality control, storage, delivery, erection and broching, consult. ANSI/IP11 Quality Criteria, DSB-89 and BCS11 Building Component Safety Intermation. available from Truss Piole Institute, SS3 D'Onofila Diffee, Madison, WI 537 19.



Edemon, NC 27932



NOTES

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Gable requires continuous bottom chord bearing.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 124 lb uplift at joint 8, 124 lb uplift at joint 6 and 50 lb uplift at Joint 7.

LOAD CASE(S) Standard



January 23,2008

MARMING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REPERENCE PAGE MII-1473 BEFORE USE. Design void for use only with Milek connections. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer-not inus designer. Broading shown is for falleral support of individual web members only. Additional temporary bracing to insure stobility of turing construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building construction is the responsibility of the building control general guidance regarding foliocation, quality control, storage, delivery, erection and bracing, consult. ANSI/TPII Quality Criteria, DSB-89 and BCSII Building Component Safety information available from Truss Flate Institute, 583 D'Onofrio Drive, Modson, WI 53719.



MONTGOMERY_FLORIDA_125 Truss Type E4632389 MONTGOMERY GABLE Job Reference (optional)
7.030 s Jan 3.2008 MiTek Industries, Inc. Wed Jan 23.13:37:39.2008 Page Maronda Homes Inc., Senford, FL -1-0-8 11-7-8 1-0-8 5-3-8 5-3-8 1-0-8 4x6 = Scale: 3/8"=1" 6.00 12 3x6 = 3x6 ≥ 2x4 || 2x4 | 3x8 = 10-7-0 10-7-0 LOADING (psf) PLATES GRIP SPACING CSI DEFL Vdefl L/d 2-0-0 in 0.00 120 MT20 244/190 Plates Increase 1.25 TC 0.25 Vert(LL) n/r TCLL 16.0 Lumber Increase 1.25 BC 0.15 Vert(TL) 0.00 n/r 120 TCDL BCLL 100 Rep Stress Incr YES WB 0.08 Horz(TL) 0.00 n/a n/a Weight: 64 lb Code FBC2004/TPI2002 (Matrix) BCDL 10.0 BRACING LUMBER TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2 Structural wood sheathing directly applied or 6-0-0 oc purlins, except TOP CHORD 2 X 4 SYP No.3 **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS OTHERS 2 X 4 SYP No.3 REACTIONS (lb/size) 8=282/10-7-0, 6=282/10-7-0, 7=436/10-7-0 Max Horz 8=69(LC 6) Max Uplift8=118(LC 6), 6=-118(LC 7), 7=-56(LC 6) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 2-8-186/226, 1-2-0/26, 2-3-97/75, 3-4-97/75, 4-5-0/26, 4-6-186/226 BOT CHORD 7-8=-77/67, 6-7=-15/67 WEBS 3-7=203/154, 2-7=38/76, 4-7=38/77

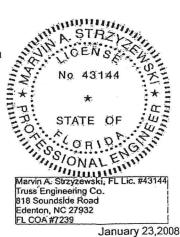
NOTES

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 118 lb uplift at joint 8, 118 lb uplift at joint 6 and 56 lb uplift at Joint 7.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design voted for use only with Milex connectors. This design is based only upon parameters shown, and is for on individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer- not ituss designer. Bracing shown is for lateral support of Individual web members only. Additional temporary bracing to insure stollarly during construction is the responsibility of the exector. Additional permonent bracing of the overal shucture is the responsibility of the building designer, For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult. AMSIVIPII Qualify Criteria, DS8-89 and BCS11 Building Component Safety Information. available from Truss Plate Institute, S83 D'Onotifo Drive, Madison. WI S3719.



MONTGOMERY_FLORIDA_125 Truss Truss Type E4632390 MONTGOMERY GABLE Job Reference (optional) 0 s Jan 3 2006 MFTek Industries, Inc. Wed Jan 23 13:37:40 2008 Page 1 Maronda Homes Inc., Sanford, FL 6-0-0 3-7-8 Scale = 1:20.7 4x6 = 6.00 12 2x4 | GRIP LOADING (psf) SPACING DEFL PLATES Vdefl Ld 2-0-0 Plates Increase 1.25 TC BC Vert(LL) n/a n/a 999 MT20 244/190 16.0 0.22 TCLL 999 TCDL 7.0 Lumber Increase 1.25 0.22 Vert(TL) n/a n/a WB 0.00 n/a BCLL 10.0 Rep Stress Incr YES 0.05 Horz(TL) Code FBC2004/TPI2002 (Matrix) Weight: 34 lb BCDL 10.0 BRACING

TOP CHORD

BOT CHORD

LUMBER

TOP CHORD 2 X 4 SYP No.2 BOT CHORD 2 X 4 SYP No.2

2 X 4 SYP No.3

OTHERS 2 X 4 SYP No.3

REACTIONS (Ib/size) 5=95/9-7-8, 3=196/9-7-8, 4=471/9-7-8

Max Horz 5=66(LC 7)
Max Uplift5=56(LC 6), 3=35(LC 7), 4=-103(LC 7) Max Grav5=134(LC 10), 3=196(LC 1), 4=471(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-5=90/74, 1-2=-76/62, 2-3=-55/79

4-5=31/71, 3-4=-31/71 **BOT CHORD** 2-4=-238/172

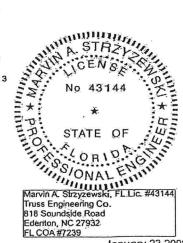
WEBS

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 56 lb uplift at joint 5, 35 lb uplift at joint 3 and 103 lb uplift al joint 4.

LOAD CASE(S) Standard



Structural wood sheathing directly applied or 6-0-0 oc purlins, except

Rigid ceiling directly applied or 6-0-0 oc bracing.

January 23,2008

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MII-7473 BEFORE USE. Design votid for use only with Miles connectors. This design is based only upon porameters shown, and is for on individual bubbing component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not trust designer. Brocing shown is for falled support of individual web members only. Additional temporary brocing to Insure shifty during construction is the responsibility of the building designer. For general guidance regarding for the continuous provided with the responsibility of the building designer. For general guidance regarding fobrication, quality continuit, storage, delivery, erection and brocing, consult. AMS/IPTI Quality Criteria, DSB-89 and BCSI Building Component Safety Intornation available from Trust Plate Institute, 583 D'Onofrio Drive, Modson, VI 53719.



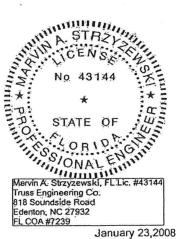
Truss Truss Type MONTGOMERY FLORIDA 125 E4632391 MONTGOMERY GABLE Job Reference (optional)
7.030 s Jan 3 2008 MiTek Industries, Inc. Wed Jan 23 13:37:40 2008 Page 1 Maronda Homes Inc., Sanford, FL 7-6-0 3-7-8 4x6 = Scale = 1:24.6 6.00 12 2x4 || 2x4 || 2 2x4 || 2x4 || 2x4 11 2x4 11 2x4 || GRIP SPACING CSI DEFL PLATES LOADING (psf) 2-0-0 in 16.0 Plates Increase 1.25 TC 0.06 Vert(LL) n/a n/a 999 MT20 244/190 TCLL 999 BC. TCDL 7.0 Lumber Increase 1.25 0.06 Vert(TL) n/a n/a WB 0.00 n/a nta 0.04 BCLL 10.0 Rep Stress Incr YES Horz(TL) Code FBC2004/TPI2002 Weight: 49 lb BCDL 10.0 BRACING LUMBER TOP CHORD 2 X 4 SYP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except 2 X 4 SYP No.2 end verticals. **BOT CHORD** BOT CHORD 2 X 4 SYP No.3 Rigid ceiling directly applied or 10-0-0 oc bracing. OTHERS 2 X 4 SYP No.3 REACTIONS (Ib/size) 11=58/11-1-8, 6=104/11-1-8, 9=177/11-1-8, 10=154/11-1-8, 8=142/11-1-8, 7=257/11-1-8 (ID/SIZE) 11-30; THE STATE OF T FORCES (lb) - Maximum Compression/Maximum Tension 1-11=-32/38, 1-2=-20/28, 2-3=-27/76, 3-4=-26/76, 4-5=-32/43, 5-6=-73/30 10-11=-5/90, 9-10=-5/90, 8-9=-5/90, 7-8=-5/90, 6-7=-5/90 TOP CHORD BOT CHORD 3-9=-91/23, 2-10=-88/90, 4-8=-82/90, 5-7=-134/140 WEBS NOTES 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-02; 125mph (3-second gust); h=25ft; TCDL=4.2psf; BCDL=6.0psf; Category II; Exp B; enclosed; MWFRS (low-rise) and C-C Interior(1) zone; Lumber DOL=1.33 plate grip DOL=1.33. This truss is designed for C-C for members and forces, and for

- MWFRS for reactions specified.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 11, 6 lb uplift at joint 6, 59 lb uplift at joint 10, 53 lb uplift at joint 8 and 94 lb uplift at Joint 7.

LOAD CASE(S) Standard



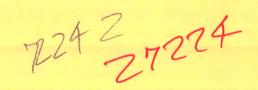
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIL-7473 REFORE USE. Design vold for use only with Miles connectors. This design is based only upon parameters have a main-value and a main-value of the design is based only upon parameters have not is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not fuss designers. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overal structure is the responsibility of the building designer, for general guidance regarding fobrication, quality control, stronge, defivery, erection and bracing, consult. AMSIVIPII Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onotifo Drive, Modson, WI 53719.



818 Soundside Road enton. NC 27932

GEO-TECH, INC.

Engineering Consultants in Geotechnical • Environmental • Construction Materials Testing



FIELD DENSITY WORKSHEET

CLIENT MATOHDA	1.10M	nl=S			DATE .	0	8July	108
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TOTAL ON-SITE TIME							FICE	
□ Limerock □ Subgrade □ Pipe Backfill □	Building	Pad 🖨	Building	g Footing	Oth	ner		
	LAB PR	OCTOR	TEST	PROBE	%	WET DENSITY	DRY DENSITY	%
TEST LOCATION	DENS.	OMC	DEPTH	DEPTH	MOIST.	(PCF)	(PCF)	COMP.
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OCCUPANO

COLUMBIA COUNTY, FLORIDA

Department of Building and Zoning Inspection
This Certificate of Occupancy is issued to the below named permit holder for the building and premises at the below named location, and certifies that the work has been completed in accordance with the Columbia County Building Code.

Parcel Number 10-4S-16-02856-120

Building permit No. 000027224

Use Classification SFD, UTILITY

Permit Holder THEODORE BROCK

Waste: 184.25

Fire:

70.62

Owner of Building MARONDA HOMES

Date: 11/14/2008

Location:

278 SW TIMBERRIDGE DRIVE, LAKE CITY, FL

254.87

Building Inspector

POST IN A CONSPICUOUS PLACE (Business Places Only)

27224

Inst:200812014516 Date:8/5/2008 Time:8:11 AM ________DC,P DeWitt Cason,Columbia County Page 1 of 1 B:1155 P:2456

NOTICE OF COMMENCEMENT

County Clerk's Office Stamp or Seal
Tax l'arcel Identification Number 10-45-16-02856-120
THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this NOTICE OF COMMENCEMENT.
1. Description of property (legal description): 20 Timber and
a) Street (job) Address: 2. General description of improvements: On Street (job) Address:
a) Name and address: Marona Homes Inca R Labor Scuthpoint Pkyy #300 Lax Ft 32216 b) Name and address of fee simple titleholder (if other than owner) c) Interest in property
4. Contractor Information a) Name and address: MAYONA HOME INC G. FL. USOD SOUTHDOUT PLANY #300. Mx FL 32210 b) Telephone No. (904) 2910 - 1490 Fax No. (Opt.) (904) 232-03-15
5. Suicty information
n) Name and address: b) Amount of Bond: c) Telephone No.: Fax No. (Opt.)
c) Telephone No.:Fax No. (Opt.) 6. Lender
a) Name and address:
7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served: a) Name and address: SWTHEYN THE HOWING W BUY 3945 BUYMEWWY RU MX FL 3201 b) Telephone No.: (104) 739-2205 Fax No. (Opt.)
8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(I)(b),
Florida Statutes:
u) Name and address: b) Telephone No.: Fax No. (Opt.)
9. Expiration date of Notice of Commencement (the expiration date is one year from the date of recording unless a different date is specified):
WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.
STATE OF FLORIDA COUNTY OF COLUMBIA 10.
Signature of Owner or Owner's Authorized Office/Director/Partner/Manager Theodore C- Brock
Print Name The foregoing instrument was acknowledged before me, a Plorida Notary, this 31 day of 30%, by:
fact) for MAYONGA HOMES INC OF PLOYIGA (name of party on behalf of whom instrument was executed).
Personully Known OR Produced Identification Type
Notary Signature MUV M L MUGHLe Notary Stamp or Scal: Notary Signature Notary Public State of Florida Melissa L McKague My Commission DD493647 Expires 11/22/2009
11. Verification pursuant to Section 92.525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

Signature of Natural Person Signing (in line #10 above.)

LEGALDESCRIPTION:

LOT TWENTY (20) OF "TIMBERLANDS, PHASE 1" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGES 26-27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.



Z <u>ე</u> EAST, <u>1</u>0, COLUMBIA COUNTY, FLORIDA TOWNSHIP RANGE

0

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60

90

TIMBER RID 60' RIGHT-OF-WAY ±20' ASPHALI ROAD RIDGEDRIVE

PAVEMENT

1) MARONDA HOMES

CERTIFIED TO:

N 89"43'36"W 260.00' N 89"44'25"W 259.99' ET 20.0' P.U.E 50.0' zz BEARING BASIS V 89*43'36"W 520.00" V 89*43'36"W 520.06" 89"43"36"W 130.00" 89"42"52"W 129.99" EE T 25.0 89°43'36"W 130.00' 89°42'41"W 130.08'

BUILDING SETBACK NOTE:

BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" FRONT 25', REAR 15', SIDE 10' ឆ AS FOLLOWS:

10.0

66.0

10.0

45.0

12.4

9.3'7'

45.0

E1 101

(9) '02.781 W"SS'81'00 S S 00'16'53'W 187.55' (M)

52.1

FORMBOARDS
FOR CONC.
FOUNDATION
ELEV.= 96.53'

(9) '05.781 W"S2'81'00 S (M) '52.781 W"02'81'00 S

LOT 21

TELEPHONE PEDESTAL

CATV RISER

WOOD POWER POLE

10.0 18.0

10.0

45.0

ELEVATIONS SHOWN HEREON ARE BASED UPON A BENCHMARK SET IN A 8" PINE AT THE FRONT OF LOT 2, WITH AN ELEVATION OF 98.76'. THIS INFORMATION WAS PROVIDED TO THIS SURVEYOR BY BRITT SURVEYING (PLATTING SURVEYOR) DATUM UNKNOWN. BENCHMARK NOTE:

SURVEYOR NOTES:

- TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.
- ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.
- 3) IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.
- 4) BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.
- THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE
- AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

FLOOD NOTE:

15.0

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED

TILE NOTE:

N 89"43"40"W 130.00"

E

THIS SURVEY IS SUBJECT TO ANY FACTS THAT MAY BE DISCLOSED BY A FULL AND ACCURATE TITLE SEARCH. THIS SURVEYOR HAS NOT PERFORMED A SEARCH OF THE PUBLIC RECORDS ON THIS PARCEL FOR ANY CLAIMS OF TITLE, EASEMENTS, OR RESTRICTIONS THAT MAY EFFECT THIS PARCEL THE PRESENCE OR ABSENCE OF ANY SUCH CLAIMS ARE NOT CERTIFIED HEREON.

LEGEND:

FOUND 1/2" REBAR NO IDENTIFICATION

ABBREVIATIONS:

A/C = AIR CONDITIONER
ASPH = ASPHALT
C = CALCULATED FROM MEASURED
CATV = CABLE TELEVISION
C/B = CONCRETE BLOCK
CLF = CHAIN LINK FENCE
CM = CONCRETE MONUMENT

- SET 1/2" REBAR & CAP LB. 6894
- FOUND 3/4" IRON PIPE FOUND 4" X 4" CONC. MON. P.S.M 5757

0

II

- SET 4" X 4" CONC. MON. P.S.M. 5582 SET NAIL & DISK P.S.M. 5582
- = FOUND NAIL & DISK = FOUND 6" X 6" S.R.D. R/W MON.

|

E

- 0 O = FOUND 1/2" REBAR & CAP LB. 6894

- CONC = CONCRETE
 ELEC = ELECTRIC
 ELEV = ELEVATION
 FND = FOUND
 FNC = FENCE
 LB = LICENSED SURVEYOR BUSINESS
 (M) = FIELD MEASURED
 MFF = MINIMUM FINISHED FLOOR ELEVATION
 MH = MANHOLE
 O.U. = OVERHEAD UTILITIES
 P = PLAT
- PLAT BOOK
 PUBLIC UTILITIES EASEMENT
 TRANSFORMER
 TYPICAL
 WATER METER
 WATER VALVE

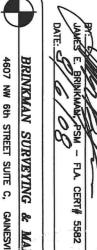
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CERTIFICATE OF SURVEYOR:

NOT VALID WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER, ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

I HEREBY CERTIFY THAT THE SURVEY DATA SHOWN HEREON, IS A TRUE AND CORRECT REPRESENTATION OF A SURVEY PERFORMED UNDER MY SUPERVISION OF THE HEREON DESCRIBED PROPERTY, AND IT MEETS THE MINIMUM TECHNICAL STANDARDS, AS SET FORTH BY THE FLORIDA BOARD OF LANDER AT SURVEY PERFORMED CODE.

STATUTES, AND CHAPTER AT STATUTES.



BRINKMAN SURVEYING & MAPPING INC.

PHONE: (352) 374-7707 4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609 FAX: (352) 374-8757

DATE: 8/6/08 SCALE: 1" = 30'"THE BENCHMARK IN QUALITY SERVICE" DRAWN BY: ZL CHECKED BY: J.B.

PREPARED FIELD WORK COMPLETED ON 7/31/08 FOR: MARONDA FIELDBOOK 98, PAGE 25 DRAWING NUMBER 103-08

DESCRIPTION:

LOT TWENTY (20) OF "TIMBERLANDS" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGE 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

1) MARONDA HOMES

CERTIFIED TO:



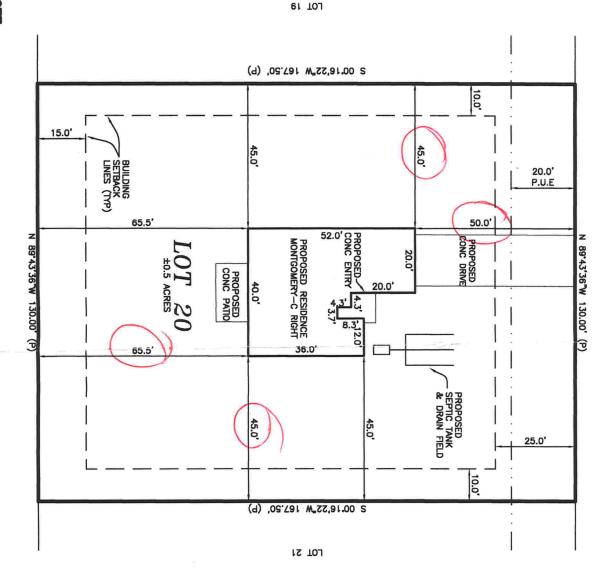
POSED

SECTION 10, 16 EAST, COI EAST, COLUMBIA COUNTY, FLORIDA TOWNSHIP 4 SOUTH, RANGE

1" = 30'

90

TIMBER RIDGE DRIVE



BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25', REAR 15', SIDE 10' BUILDING SETBACK NOTE:

FLOOD NOTE:

IN THE OPINION OF THIS SURVEYOR, ACCORDING TO THE NATIONAL FLOOD INSURANCE PROGRAM, FLOOD INSURANCE RATE MAP COMMUNITY PANEL NO. 120070-0175-B, DATED 1-6-88, THIS PROPERTY IS IN FLOOD ZONE "X" WHICH IS AN AREA DETERMINED TO BE OUTSIDE 500-YEAR FLOOD PLAIN, AS SCALED FROM SAID MAP. INFORMATION FROM THE FEDERAL EMERGENCY MANAGEMENT AGENCY (FEMA) FLOOD INSURANCE RATE MAPS, SHOWN ON THIS MAP, WAS CURRENT AS OF THE REFERENCED DATE. MAP REVISIONS AND AMENDMENTS ARE PERIODICALLY MADE BY LETTER AND MAY NOT BE REFLECTED ON THE MOST CURRENT MAP.

5)

THIS MAP OF SURVEY REFLECTS CONDITIONS LOCATED AS OF THE DATE OF FIELD WORK COMPLETION (SEE

6)

AREAS OF ENVIRONMENTAL CONCERN HAVE NOT BEEN LOCATED BY THIS SURVEYOR, UNLESS OTHERWISE DEPICTED HEREON.

£

BUILDING SETBACK LINES DEPICTED HEREON ARE SHOWN AS PER THE RECORD PLAT, BUT ARE SUBJECT TO CHANGE. PRIOR TO ANY NEW CONSTRUCTION, THE APPROPRIATE GOVERNING AUTHORITY SHOULD BE CONTACTED FOR THE CURRENT SETBACK REQUIREMENTS.

3) 2)

IN THE OPINION OF THIS SURVEYOR THE BOUNDARY SHOWN HEREON BEST REPRESENTS THE LOCATION OF THE SUBJECT PROPERTY IN RELATION TO THE DESCRIPTION AND THOSE PROPERTY CORNERS FOUND TO BE ACCEPTABLE TO THIS SURVEYOR.

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SURVEYOR NOTES:

TO THE BEST OF MY KNOWLEDGE, THERE ARE NO ENCROACHMENTS, BOUNDARY LINE DISPUTES, EASEMENTS, OR CLAIMS OF EASEMENTS, OTHER THAN ARE DEPICTED ON THIS DRAWING.

ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.

TITLE NOTE:

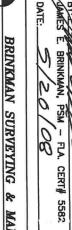
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ABBREVIATIONS: A/C = AIR CONDITIONER ASPH ASSPHALITED FROM MEASURED C = CALCULATED FROM MEASURED C = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE MONUMENT C = CONCRETTE MONUMENT C = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE C M = CONCRETTE MONUMENT C = CHAIN LINK FENCE C M = CONCRETTE C © 🖸 🗆 e = FOUND 3/4" IRON PIPE ■ = FOUND 4" X 4" CONC. MON. NO IDENTIFICATION © = FOUND 1/2" REBAR & CAP LB. 6894 O = SET 1/2" REBAR & CAP LB. 6894 ■ = FOUND 1/2" REBAR NO IDENTIFICATION

CERTIFICATE OF SURVEYOR: 'HIS IS NOT A BOUNDARY SURVEY

NOT VALLD WITHOUT THE SIGNATURE AND THE ORIGINAL RAISED SEAL OF A FLORIDA LICENSED SURVEYOR AND MAPPER. ADDITIONS OR DELETIONS TO THIS MAP BY ANYONE OTHER THAN THIS SURVEYOR IS PROHIBITED.

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BRINKMAN
SURVEYING
%
MAPPING
INC.
1.0

4607 NW 6th STREET SUITE C, GAINESVILLE, FL 32609

1" = 30' PHONE: (352) 374-7707 "THE BENCHMARK IN QUALITY SERVICE" FAX: (352) 374-8757 DRAWN BY: ZL

FIELD WORK COMPLETED ON **** FIELDBOOK **, PAGE **

DATE: 5, SCALE:

/19/08

CHECKED BY: J.B.

PREPARED FOR: MARONDA DRAWING NUMBER 103-08

LEGALDESCRIPTION:

LOT TWENTY (20) OF "TIMBERLANDS" AS PER PLAT THEREOF, AS RECORDED IN PLAT BOOK '9', PAGE 27 OF THE PUBLIC RECORDS OF COLUMBIA COUNTY, FLORIDA.

1) MARONDA HOMES

CERTIFIED TO:



 \equiv SECTION 10, TOWNSHIP 4 SOUTH, KAN 16 EAST, COLUMBIA COUNTY, FLORIDA TOWNSHIP 4 SOUTH, RANGE

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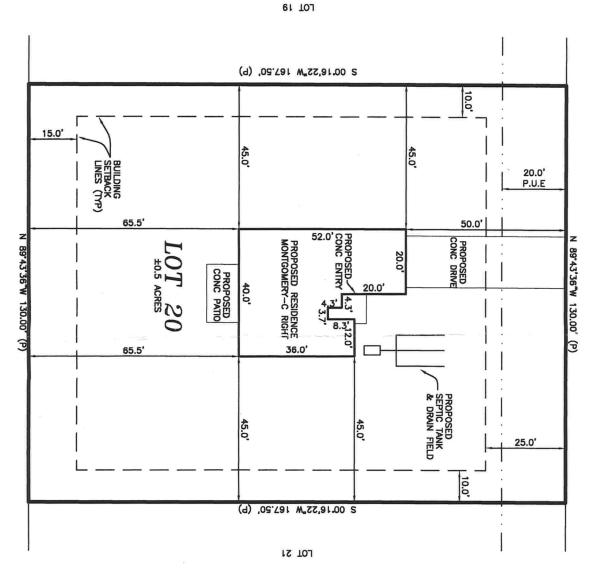
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S.W. TIMBER RIDGE DRIVE



BUILDING SETBACK INFORMATION FOR "TIMBERLANDS" IS AS FOLLOWS: FRONT 25", REAR 15", SIDE 10" BUILDING SETBACK NOTE:

FLOOD NOTE:

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5)

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6)

TITLE BLOCK).

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2)

ALL UTILITIES AND OR IMPROVEMENTS, IF ANY, MAY NOT BE SHOWN ON THIS DRAWING.

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SURVEYOR NOTES:

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LEGEND:

© = FOUND 1/2" REBAR & CAP L.B. 6894 ● = FOUND 1/2" REBAR NO IDENTIFICATION ABBREVIATIONS:

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CLF = CHAIN LINK FENCE
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LB. 6894

ELEC = ELECTRIC
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FOUND 4" X 4" CONC. MON. FIND = FOUND

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FOUND 5582

FOUND NAIL & DISK P.S.M. 5582

FOUND NAIL & DISK P.U.E. = PUBLIC UTILITI

FOUND 6" X 6" S.R.D. TRANS = TRANSFORMER

FOUND 6" X 6" S.R.D. TRANS = TRANSFORMER

FOUND 6" X 6" S.R.D. TRANSFORMER

FOUND 6" X C = ELECTRIC
V = ELEVATION
V = ELEVATION
D = FOUND
C = FENCE
B = LICENSED SURVEYOR BUSINESS
d) = FIELD MEASURED
J. = ANANHOLE
L. = OVERHEAD UTILITIES
P = PLAT

0

O = SET 1/2" REBAR & CAP LB. 6894

X = SET NAIL & DISK P.S.I X = FOUND NAIL & DISK BE = FOUND 6" X 6" S.R.D. PLAT BOOK PUBLIC UTILITIES EASEMENT WATER METER WATER VALVE TRANSFORMER TYPICAL

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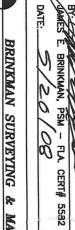
TELEPHONE PEDESTAL

WOOD POWER POLE

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BRINKMAN SURVEYING & MAPPING INC.

PHONE: (352) 374-7707 4607 NW 6th STREET SUITE C, GAINESVILLE, FI. 32609 FAX: (352) 374-8757

DATE: 5, SCALE: 1" = 30' /19/08 "THE BENCHMARK IN QUALITY SERVICE" CHECKED BY: J.B. DRAWN BY: ZL

PREPARED FOR: MARONDA FIELD WORK COMPLETED ON *** FIELDBOOK **, PAGE **

DRAWING NUMBER 103-08



HOMETEAM

TREATMENT WORKORDER 27224

☐ Termite Baiting System w/Tubes-under-the slab ☐ Tubes-under-the slab and Treat ☐ Bora-Care ☐ Treat Only DATE CALLED IN: DATE OF SCHEDULE: TIME CALLED IN: TIME SCHEDULE: SUBDIVISION JOB NAME: JOB ADDRESS: Shi Timber Ridge De: BILLING PHONE: BILLING NAME: BILLING ADDRESS: CALLED IN BY: PHONE: PERMIT NUMBER: LOT & MODEL NUMBER: DATE & TIME COMPLETED: 8/10 SOUARE FOOT: 1797 LINEAR FOOT: BLOCKVOIDS: SLAB TYPE: TYPE OF FILL: APPROX. DEPTH OF FOOTING: Outside: Inside: ☐ Addition ☐ Spot Treat ☐ Pool Addition ☐ Driveway ☐ Other ☐ Final/Completion PESTICIDE USED: + MOXA F TOTAL APPLIED: PERCENT (%) USED: _STICKER POSTED: __ TOTAL FOR P.T. PRICE PER SQ. FT. = ADDITIONAL TAX: TOTAL AMOUNT X TECHNICIAN:

SCHOOL IMPACT FEE 00100003632900 TOTAL FEES CHARGED CORRECTIONS IMPACT FEE 00100003632200 FIRE PROTECTION IMPACT FEE 10200003632220 ROAD IMPACT FEE 10100003632400 EMS IMPACT FEE 10300003632210 FEES: 046.00 CODE 2, CHECK NUMBER