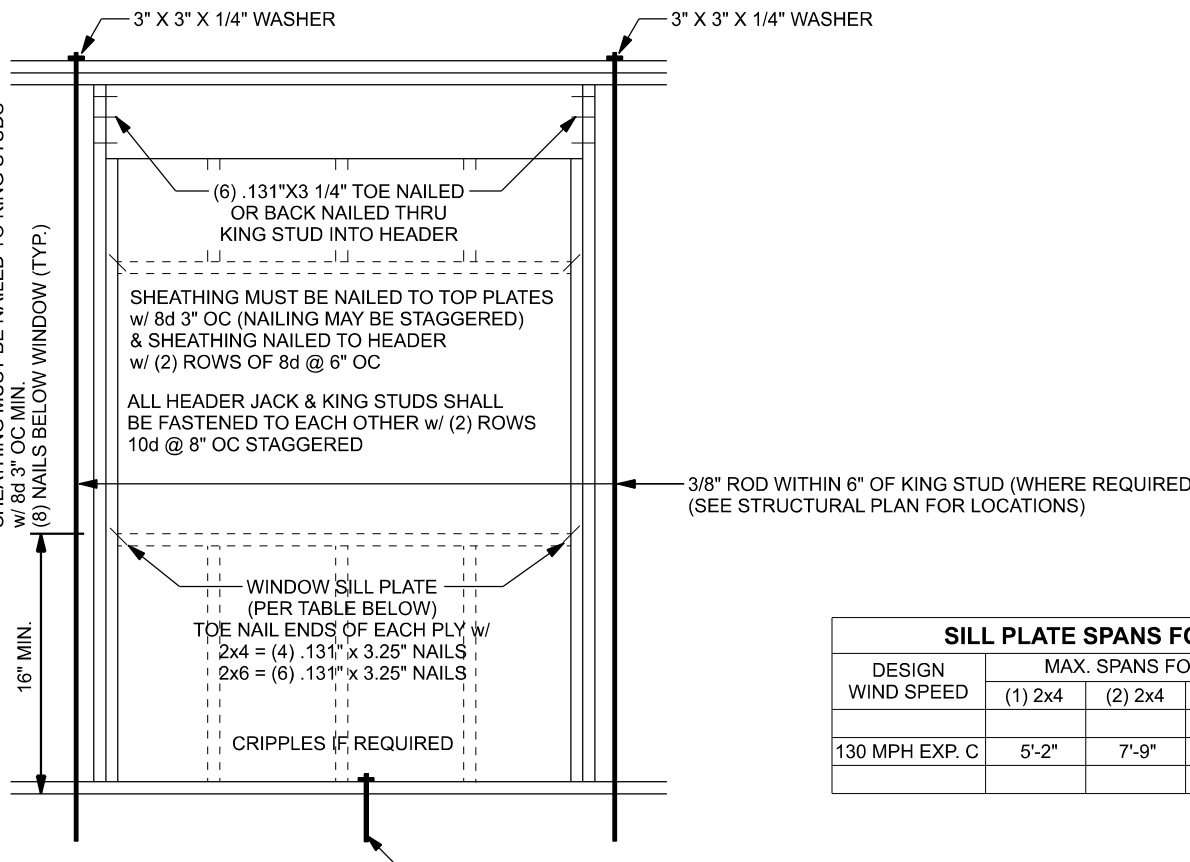


# ROOF SHEATHING FASTENING TABLE (RAFTER / TRUSS SG = 0.49)

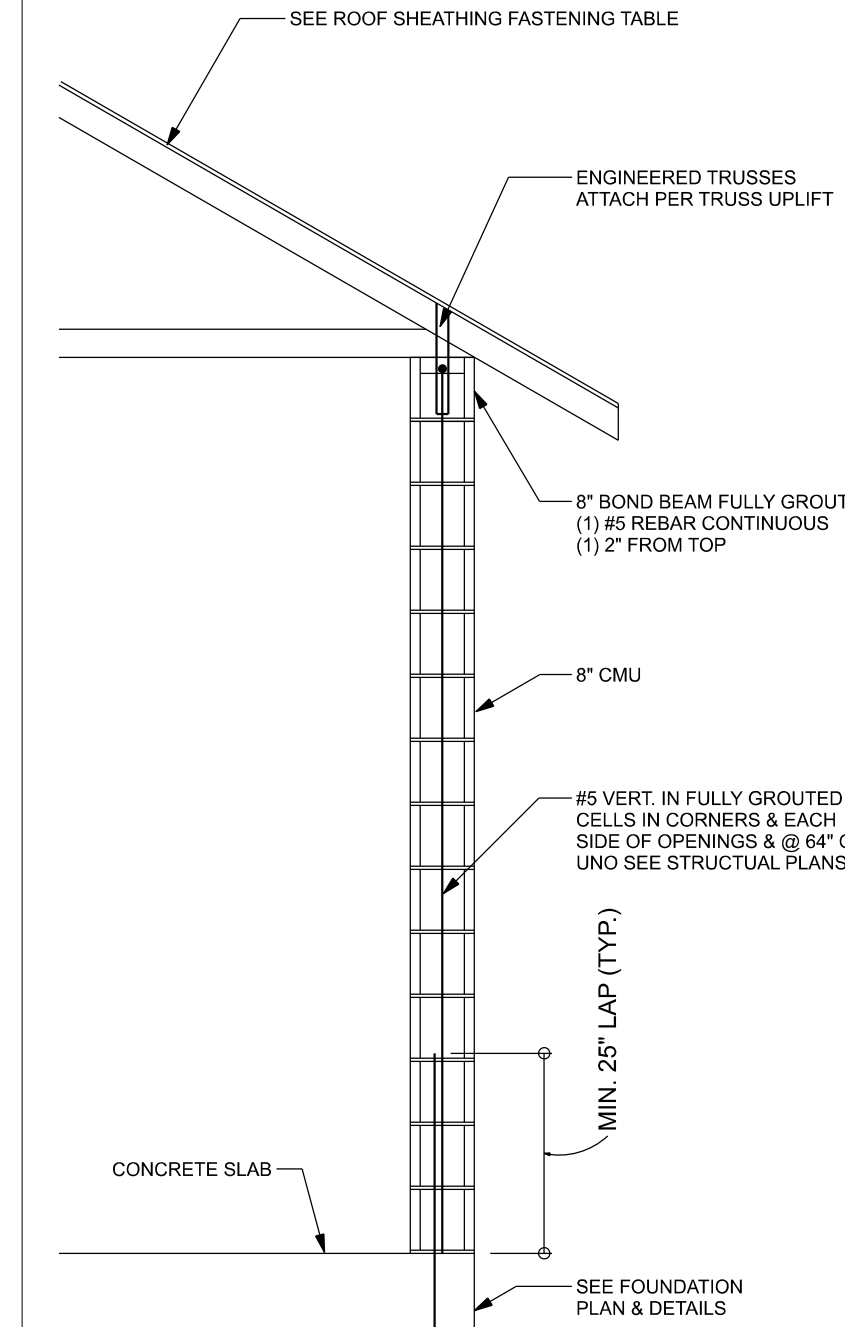
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	12" oc
120 mph Exp. C	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc
130 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RRS-01 (2 3/8" x 0.113")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RRS-03 (2 1/2" x 0.131") or ASTM F1667 RRS-04 (3" x 0.120")	6" oc	6" oc

Note:  
For sheathing located a minimum of 4 feet from the perimeter edge of the roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 6 inches on center along intermediate supports in the panel field.  
Note:  
This table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing may need to be increased based in the type of roofing material being used. See manufacturer Florida product approval.

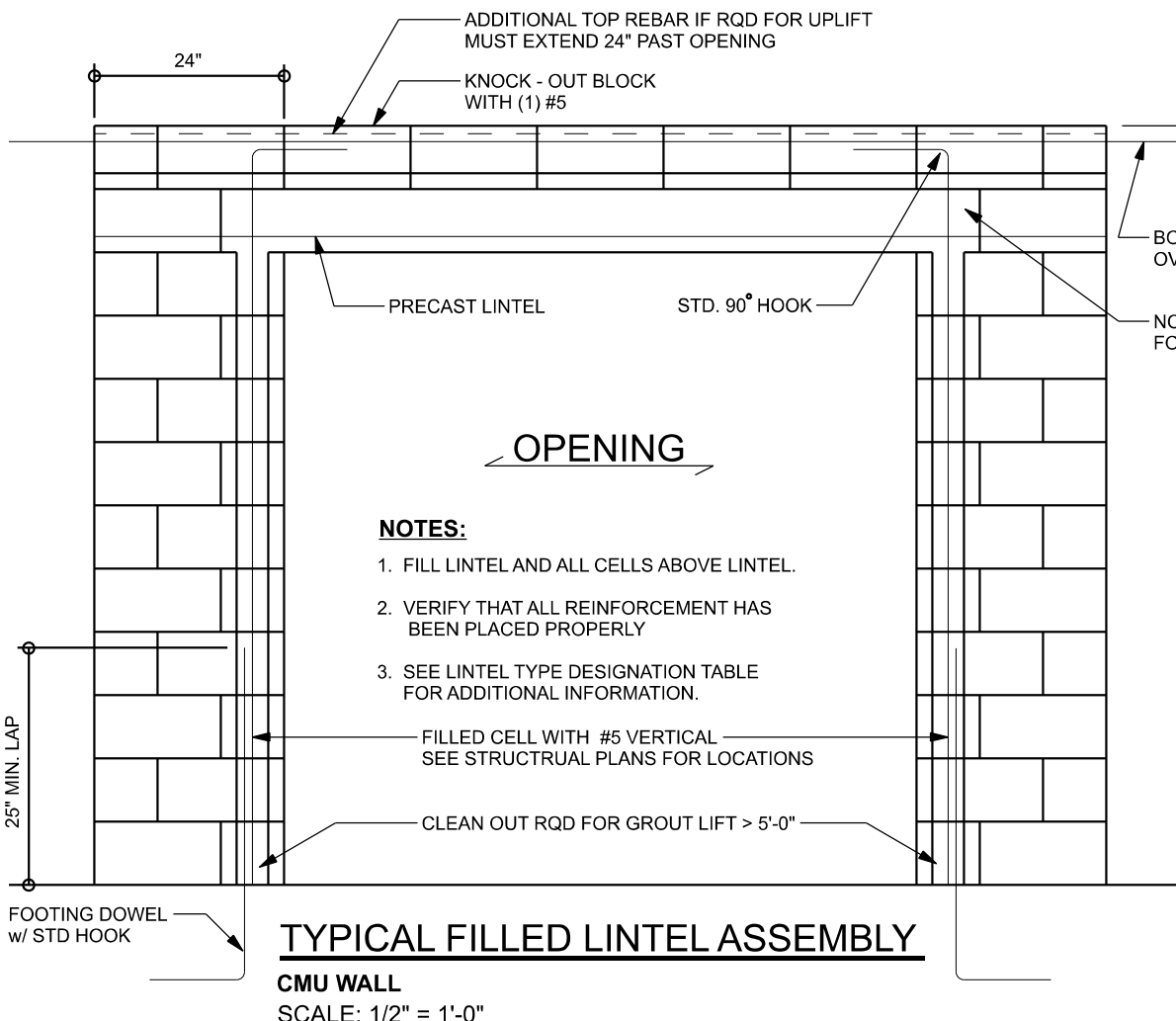


SILL PLATE SPANS FOR 10'-0" WALL HEIGHT					
DESIGN WIND SPEED	MAX. SPANS FOR SPF #2				BASED ON WFCM TABLE A-3.2(9)  FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)
	(1) 2x4	(2) 2x4	(1) 2x6	(2) 2x6	
130 MPH EXP. C	5'-2"	7'-9"	7'-7"	11'-3"	

FOR OTHER WALL HEIGHTS (H) SILL SPAN SHALL BE DIVIDED BY (H/10)

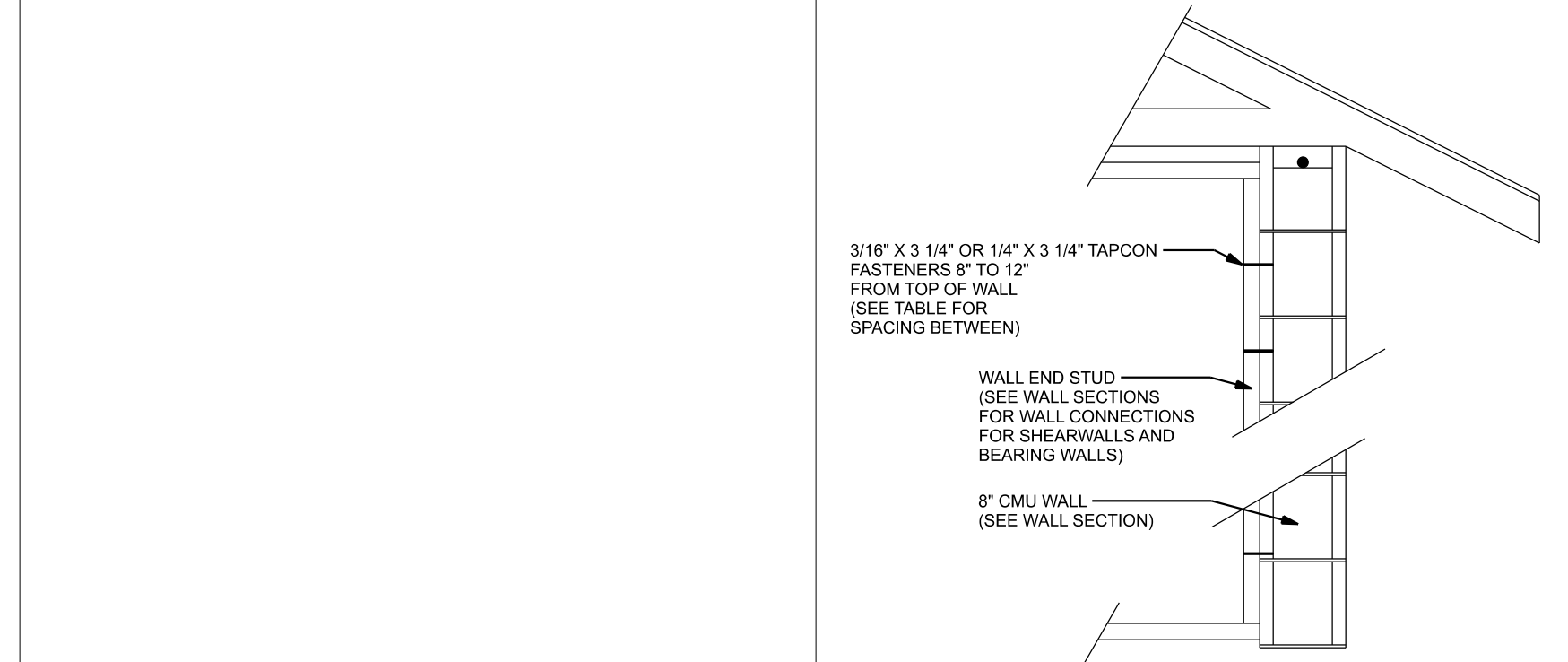


## (TYP.) EXTERIOR WALL ONE STORY CMU SCALE: 1/2" = 1'-0"



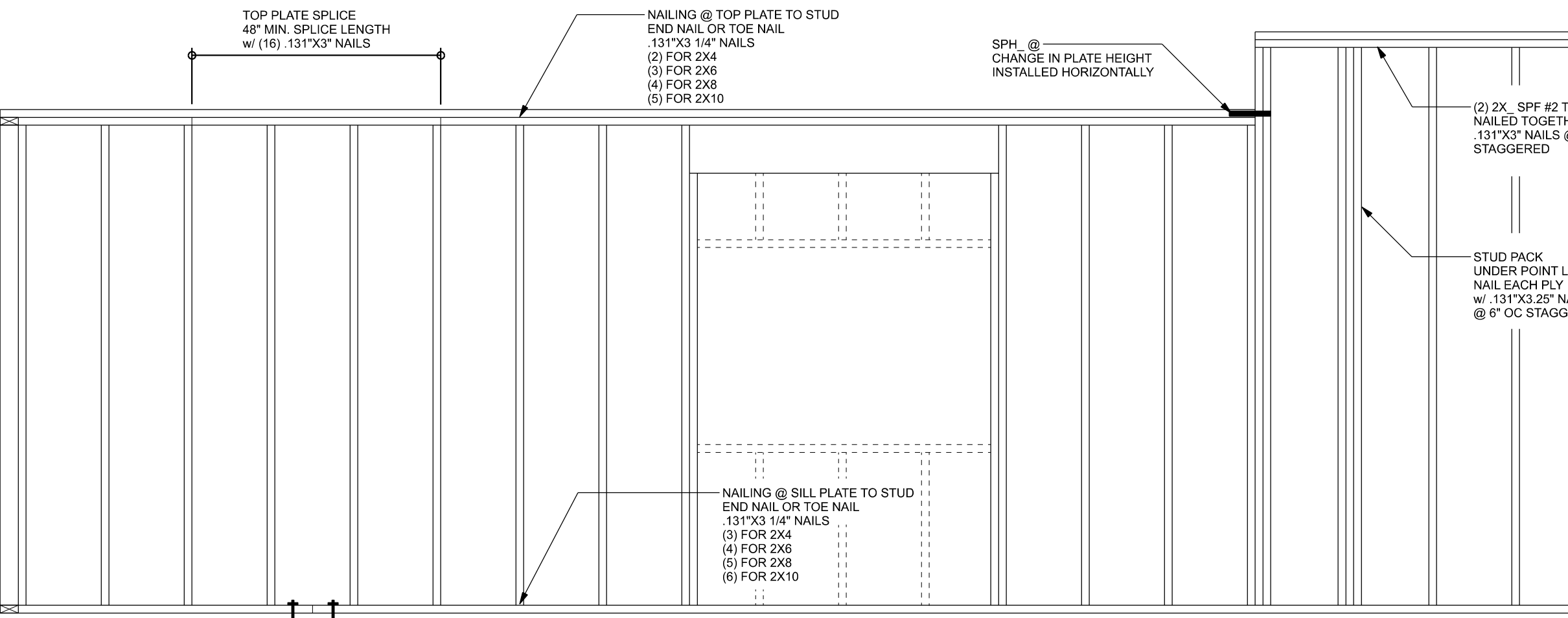
### NOTES:

1. FILL LINTEL AND ALL CELLS ABOVE LINTEL.
2. VERIFY THAT ALL REINFORCEMENT HAS BEEN PLACED PROPERLY.
3. SEE LINTEL TYPE DESIGNATION TABLE FOR ADDITIONAL INFORMATION.



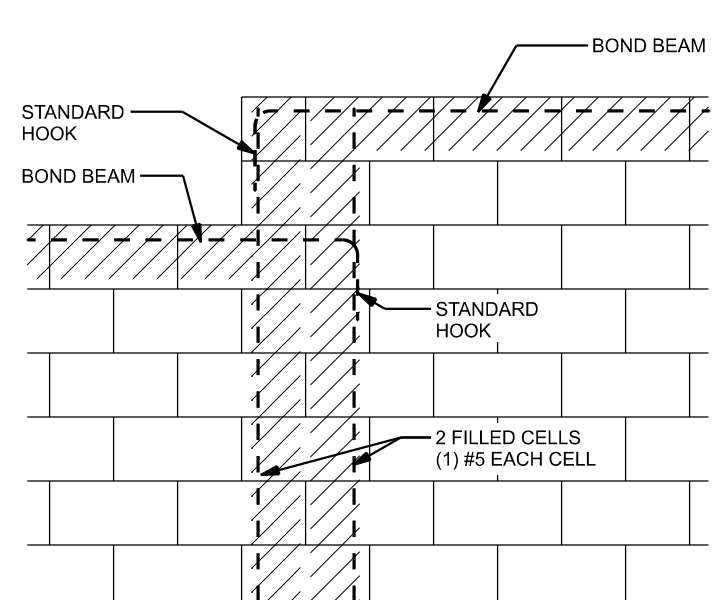
WALL TYPE:	3/16" X 3 1/4" TAPCON MAX SPACING	1/4" X 3 1/4" TAPCON MAX SPACING
INTERIOR SHEAR WALL	6" OC	8" OC
INTERIOR BEARING WALL	16" OC	16" OC
INTERIOR NON-BEARING WALL	48" OC	48" OC

## INT. FRAME WALL TO CMU CONNECTION

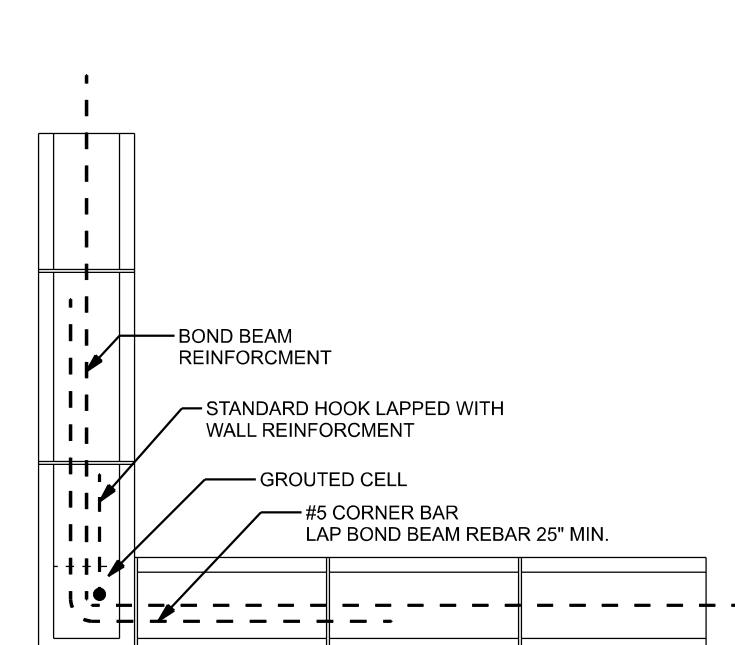


## (TYP.) WALL CONNECTIONS ONE STORY WOOD FRAME

## CHANGES IN BOND BEAM HEIGHT (BASED ON FBC FIG. R609.2.5) SCALE: 1/2" = 1'-0"



## CORNER CONTINUITY OF BOND BEAM AND WALL REINFORCEMENT (BASED ON FBC FIG. R609.2.4) SCALE: 3/4" = 1'-0"



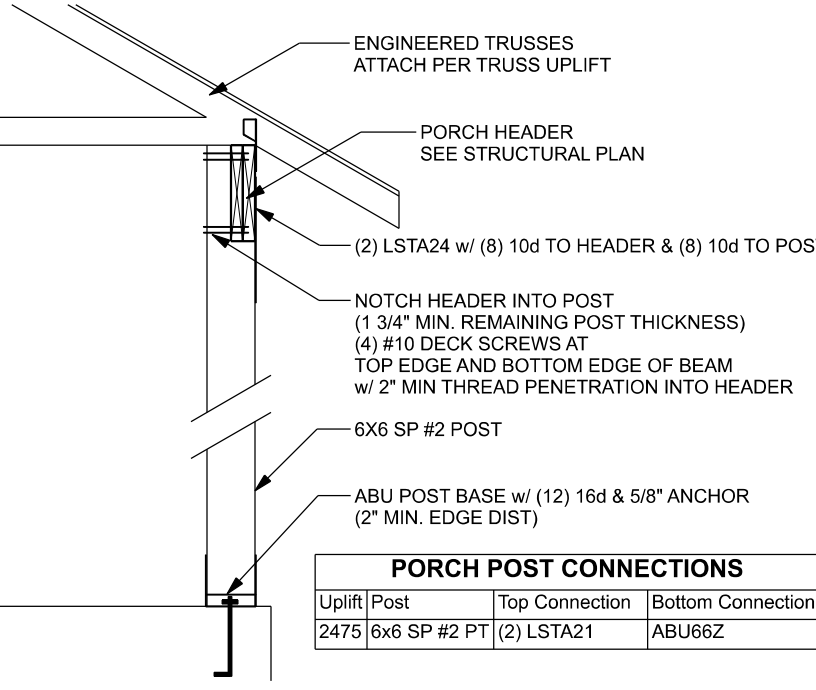
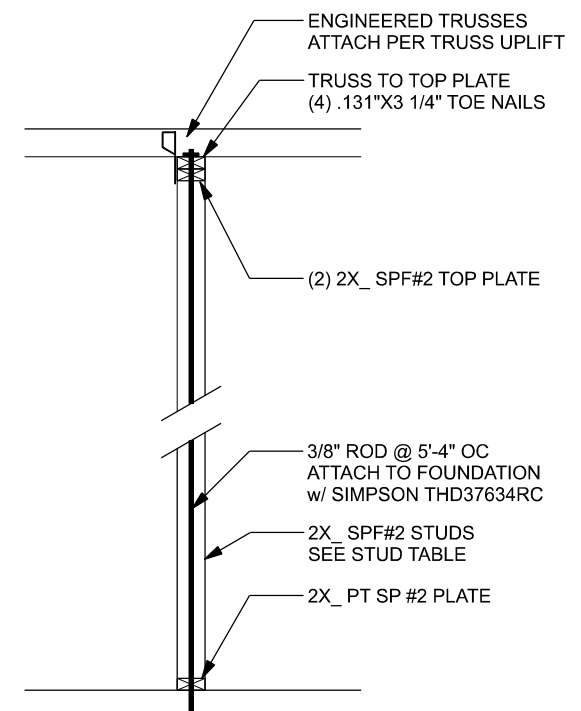
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
615	485	SDWC15600	4-8d@1 1/2"	4-8d@1 1/2"
415	280	H3	5-8d@1 1/2"	5-8d@1 1/2"
575	495	H2.5A	9-8d@1 1/2"	9-8d@1 1/2"
1340	1015	H10A	9-10d@1 1/2"	9-10d@1 1/2"
720	620	LTS12-20	6-10d@1 1/2"	6-10d@1 1/2"
1000	860	MTS12-30	7-10d@1 1/2"	7-10d@1 1/2"
1450	1345	MTS20-30	12-10d@1 1/2"	12-10d@1 1/2"
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member
1235	1235	LSTA21	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	805	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stemwall	To Stud / Post	Anchor
1825	1800	DT122	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HT14	18-16d@2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DT122	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HT14	18-16d@2 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stenwall	To Post	Anchor
2200	ABU44		12-16d	5/8"x12" Drill & Epoxy
2300	ABU66		12-16d	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor
2200	ABU44		12-16d	5/8"x7" Drill & Epoxy
2300	ABU66		12-16d	5/8"x7" Drill & Epoxy

## EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH), STUD SPACINGS SHALL BE MULTIPLIED BY 0.9 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNER OR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)	
(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

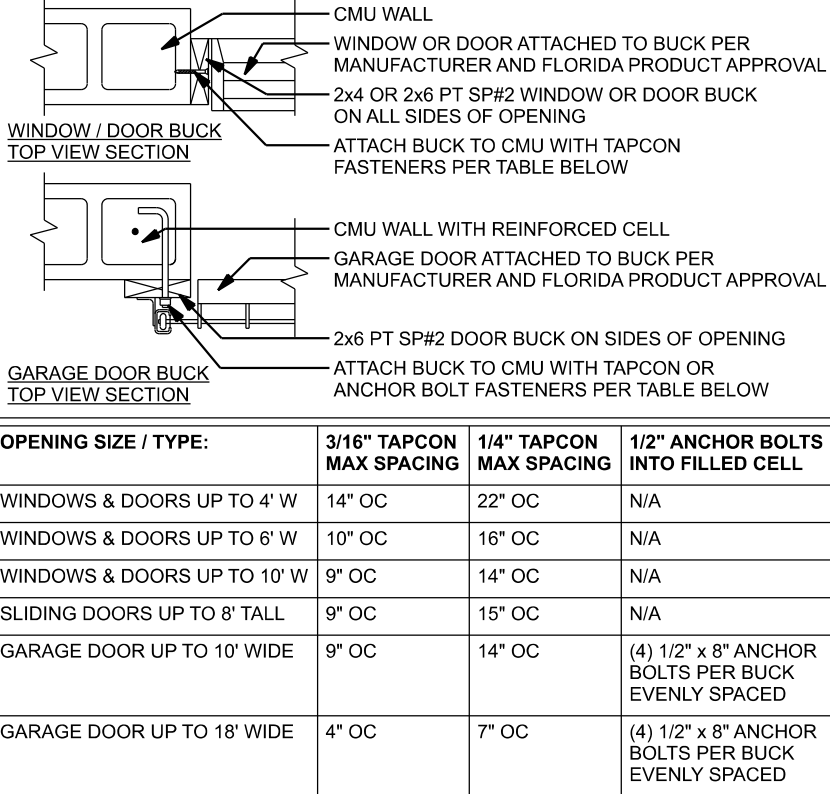
GRADE	SPECIES	TABLE	Fb	E
2x8	SP #2	925	1.4	
2x10	SP #2	800	1.4	
2x12	SP #2	750	1.4	
GLB	24F-V3 SP	2600	1.9	
LSL	TIMBERSTRAND	1700	1.7	
LVL	MICROLAM	2950	2.0	
PSL	PARALAM	2900	2.0	

## (TYP.) INTERIOR BEARING WALL ONE STORY WOOD FRAME w/ RODS



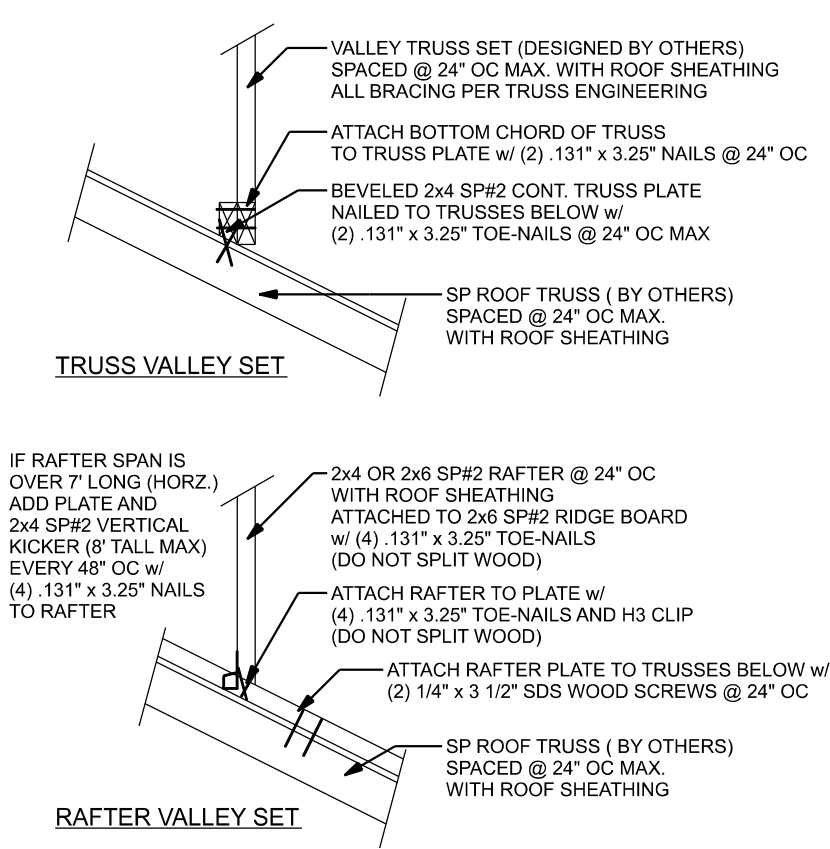
POSTS CUT FLUSH WITHOUT NOTCH - BRACE TO PREVENT ROTATION:  
EAVE / HIP ROOF:  
- STRAP TRUSS w/ H2.5A OR EQUAL TO EACH SIDE OF HEADER (FRONT & BACK)  
- USE LONGER STRAP AS NEEDED IF TOP PLATES ARE INSTALLED  
- (2) LSTA24 16-10d OR (2) MTS20 14-10d ON FRONT AND REAR OF POST TO HEADER  
- ATTACH HEADER TO POST w/ (8) 131" x 3.25" TOE-NAIL  
- SILE END  
- EXTEND GABLE SHEATHING TO BOTTOM OF HEADER NAILED WITH 8d NAILS @ 6" MIN. INTO TRUSS BOTTOM CHORD AND INTO BEAM AT 1" MIN. FROM TOP & BOTTOM EDGE  
- ATTACH HEADER TO POST w/ (8) 131" x 3.25" TOE-NAIL

## (TYP.) PORCH POST ONE STORY WOOD



- TAPCON IN FACE OF CMU: 2 1/2" MIN. EDGE DISTANCE 1 1/4" MIN. EMBEDMENT, 3" MIN. SPACING  
- WINDOWS AND DOORS MAY BE ATTACHED DIRECTLY TO CMU PER MANUFACTURER AND FLORIDA PRODUCT APPROVAL.  
- A 1x PT "SPACER" BUCK MAY BE USED IF WINDOW / DOOR IS ATTACHED TO CMU PER FLORIDA PRODUCT APPROVAL.

## DOOR & WINDOW BUCK ATTACHMENT



## VALLEY SET FRAMING DETAIL

## GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER IS FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. THE BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTION LOADS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN  
FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE)  
CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, Fc = 2500 PSI.  
WELDED WIRE REINFORCED SLAB: 6" x 6" W1 x W1.4, Fy = 88KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.R.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 3 INCHES, DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS.  
FIBER REINFORCED CONCRETE (FRC) SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5:1 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT OUT CUM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, Fy = 40 KSI. ALL LAP SPACES 40" DB (25" FOR #5 BARS). UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-08, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NOT LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

MASONRY NOTE:  
MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls Fm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow medium surface finish, 8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min. 40 bar dia. (25" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A525, Class GR6, 0.60 oz/lb or 304SS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/lb or 304SS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

BUILDER'S RESPONSIBILITY:  
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.  
CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.  
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.  
PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.  
VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

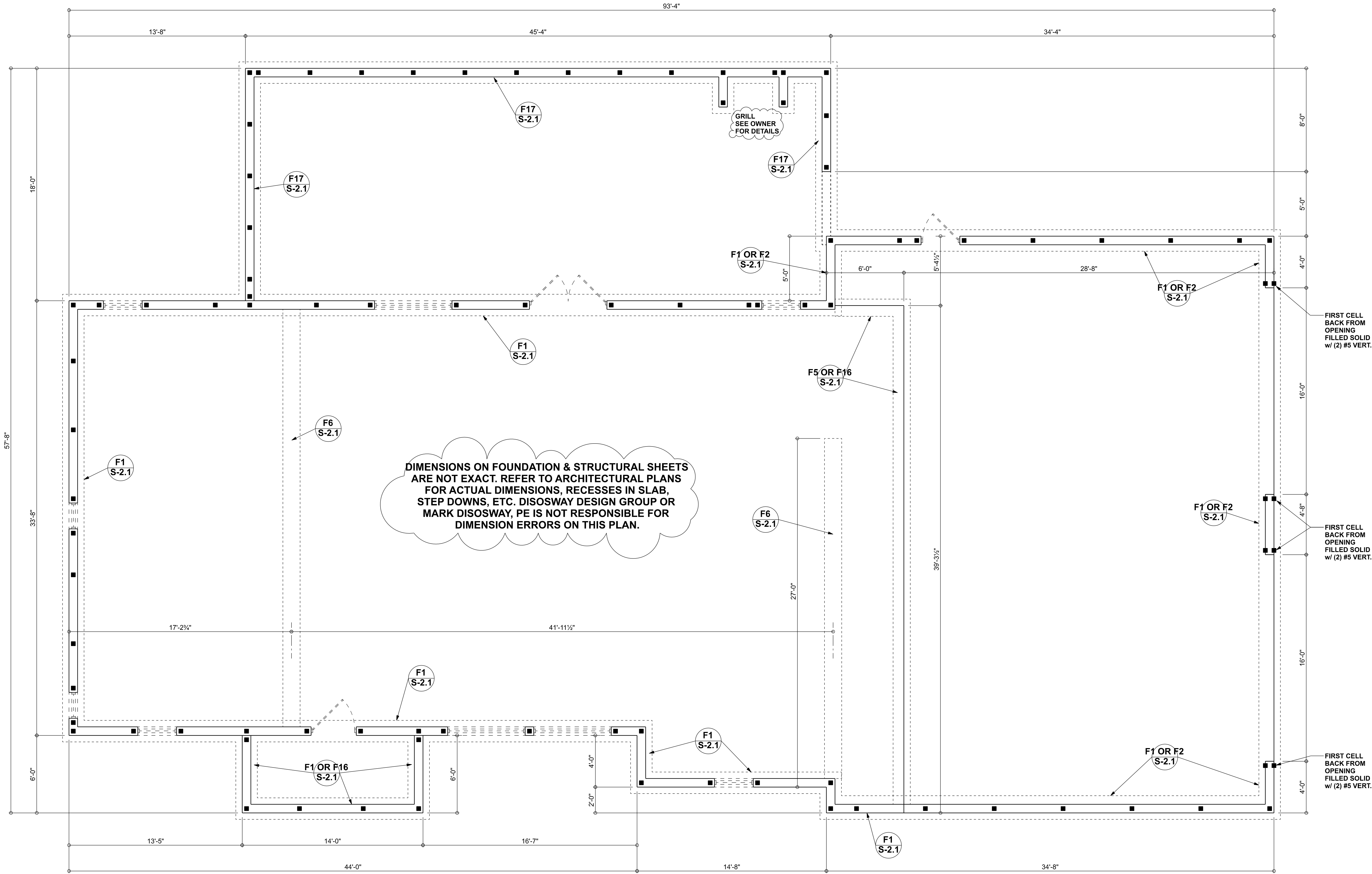
ROOF SYSTEM DESIGN:  
THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN CRITERIA & LOADS:	
BUILDING CODE	17TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2020)
CODE FOR DESIGN LOADS	ASCE 7-16
<b>WINDLOADS</b>	
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I
RISK CATEGORY	II
ENCLOSURE CLASSIFICATION	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	0.18
ROOF ANGLE	7-45 DEGREES
MEAN ROOF HEIGHT	30 FT
C&C DESIGN PRESSURES	SEE TABLE
<b>FLOOR LOADING</b>	
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD
SLEEPING ROOMS	30 PSF LIVE LOAD
<b>ROOF LOADING</b>	
FLAT OR < 4:12	20 PSF LIVE LOAD
4:12 TO < 12:12	16 PSF LIVE LOAD
12:12 & GREATER	12 PSF LIVE LOAD
SOIL BEARING CAPACITY	1500 PSF
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C)		
EFFECTIVE WIND AREA (FT2)	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0 - 20	+25.6(V/vasd) -27.8(V/vasd)	+25.6(V/vasd) -34.2(V/vasd)
0 - 20	+42.6(V/vasd) -46.2(V/vasd)	+42.6(V/vasd) -57.0(V/vasd)

GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C)		
9x7 GARAGE DOOR	+22.6(V/vasd) -25.5(V/vasd)	
16x7 GARAGE DOOR	+21.7(V/vasd) -24.1(V/vasd)	





FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES	
FN - 1	DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, PE IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
FN - 2	CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN
FN - 3	THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 6X6-1.4/1.4 WELDED WIRE MESH PLACED ON CHAIRS @ 1 1/2" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 6" LAPS SEALED w/ POLY TAPE OVER TERMITE-TREATED & COMPACTED FILL

Blake Construction

Ronnie Stuman Res

PROJECT ADDRESS:  
205 SW Madison Court  
Lake City, FL 32024

DIMENSIONS:  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915

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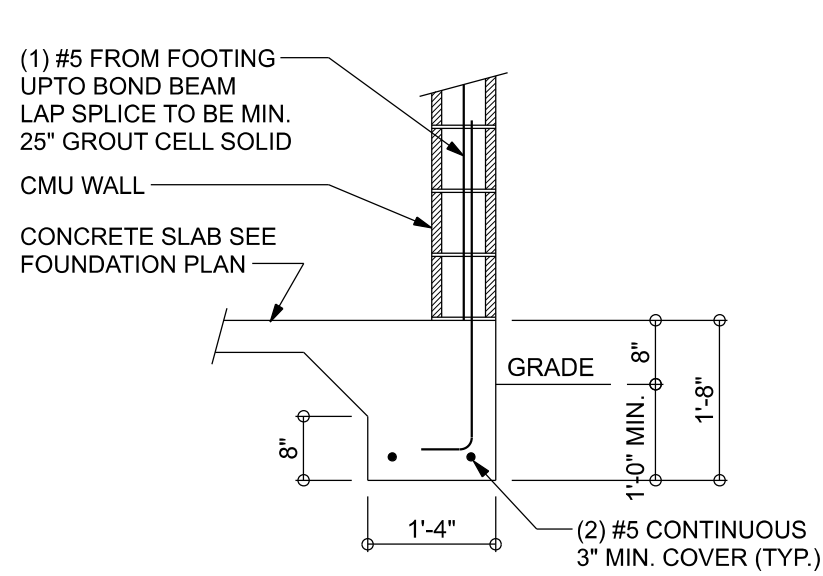


Thursday, August 26, 2021

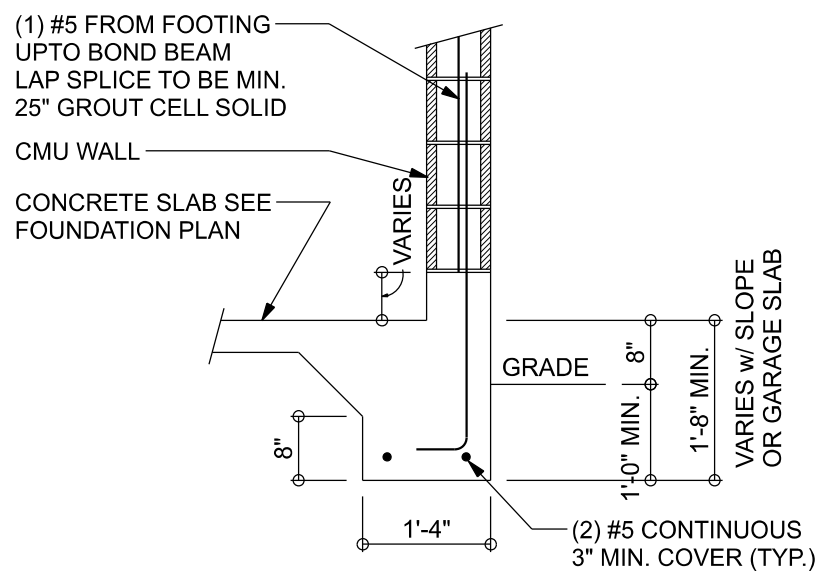
Mark Disosway P.E.  
163 SW Midtown Place  
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JOB NUMBER:  
211179

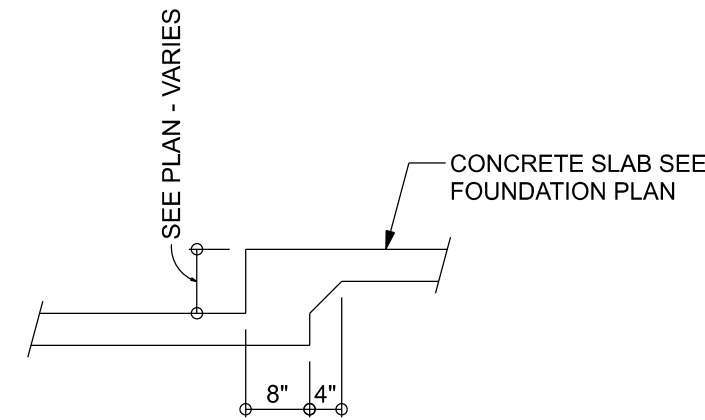
S-2  
OF 4 SHEETS



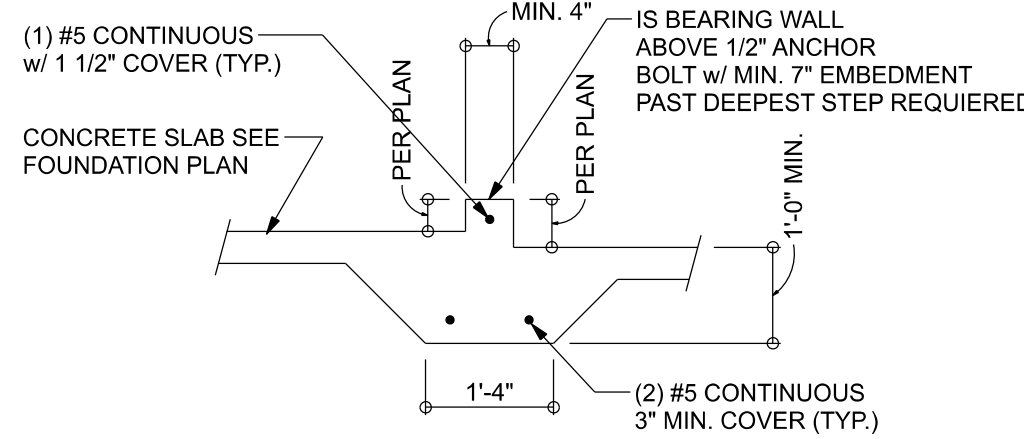
**F1 MONOLITHIC FOOTING**  
SCALE: 1/2" = 1'-0"



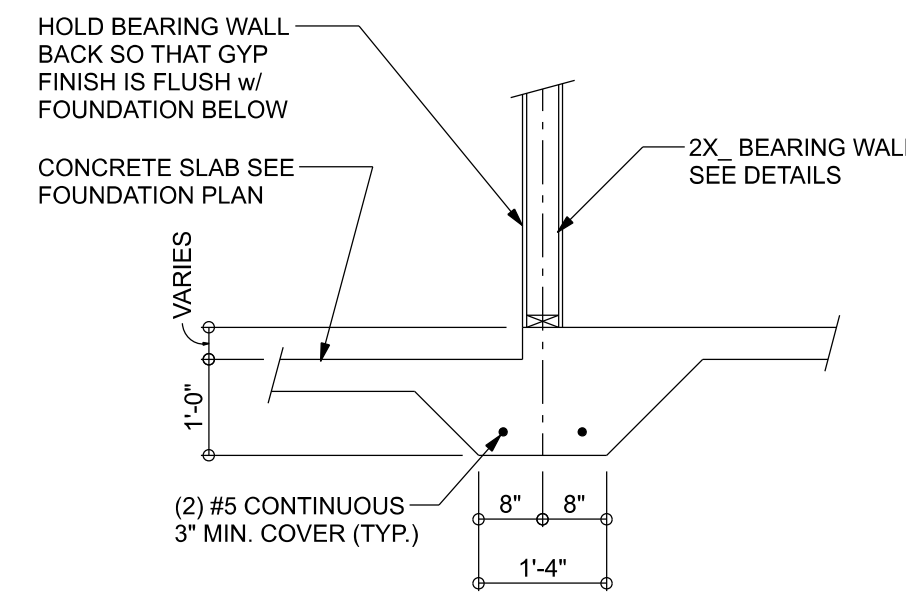
**F2 GARAGE CURB FOOTING**  
SCALE: 1/2" = 1'-0"



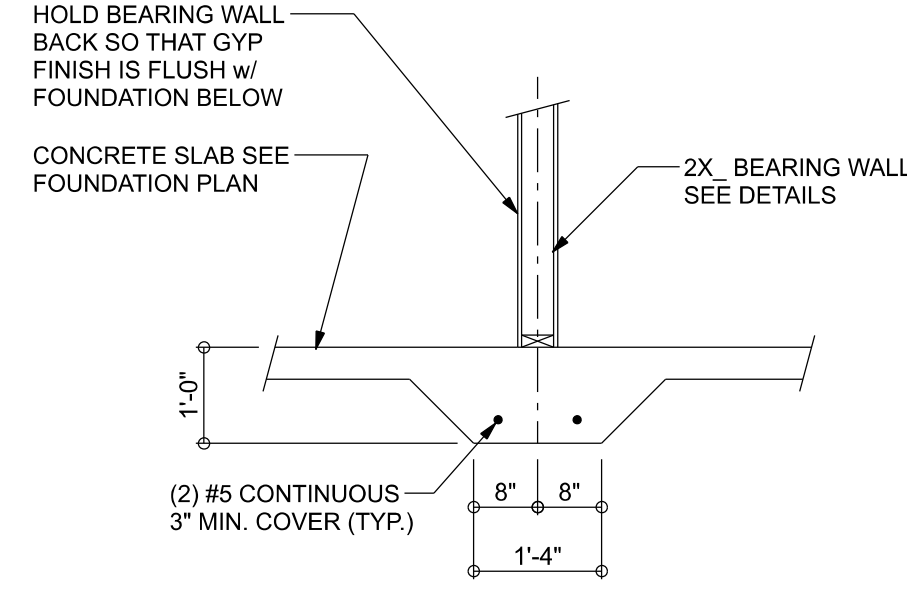
**F3 NON-BEARING STEP DOWN**  
SCALE: 1/2" = 1'-0"



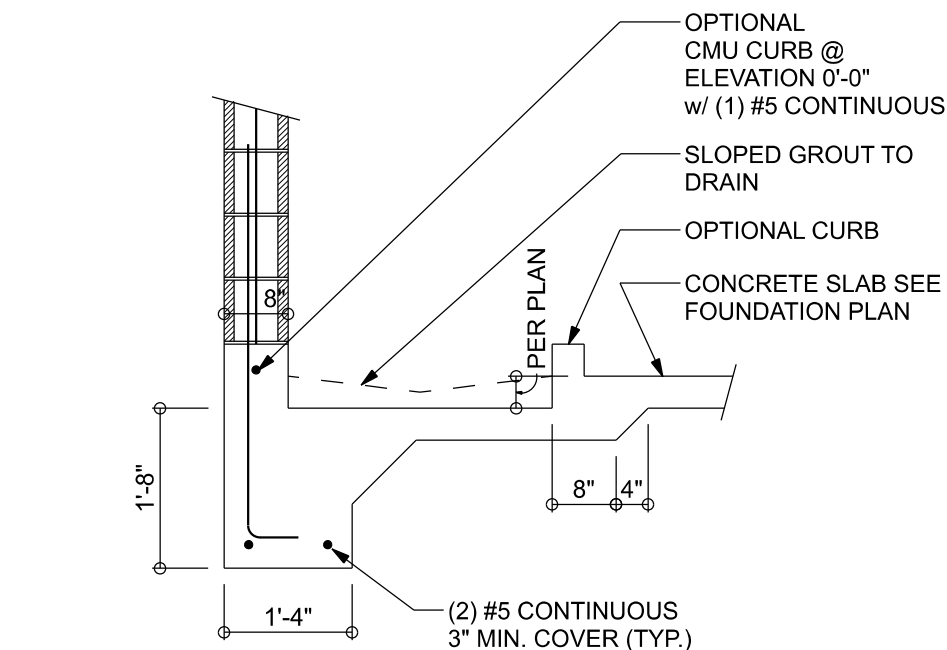
**F4 DOUBLE STEP FOOTING**  
SCALE: 1/2" = 1'-0"



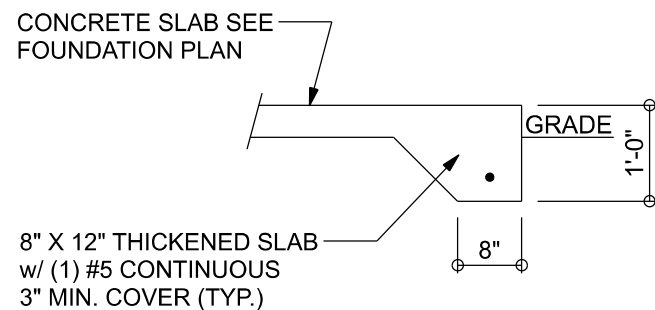
**F5 STEP FOOTING BEARING**  
SCALE: 1/2" = 1'-0"



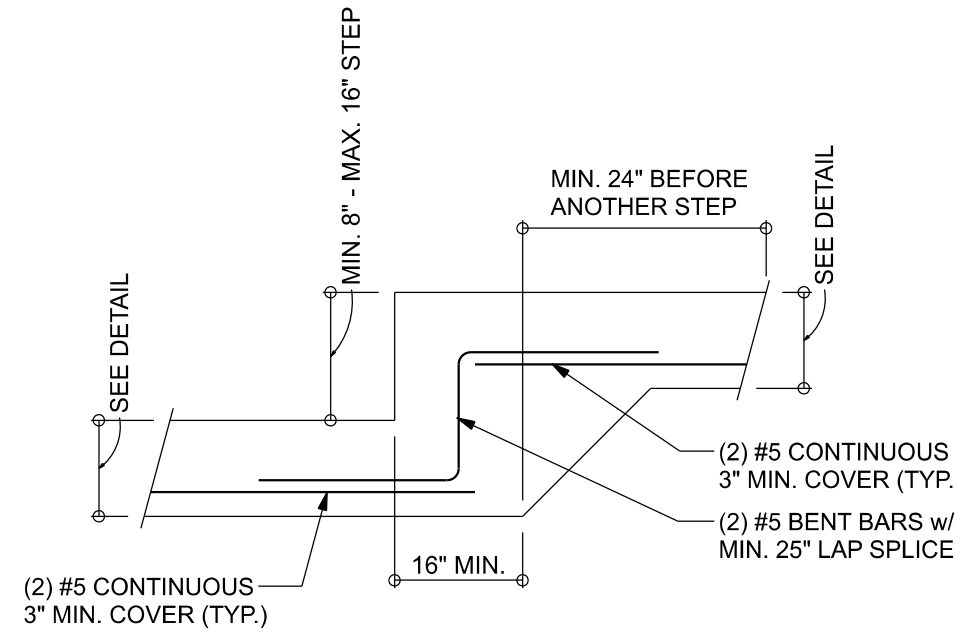
**F6 INTERIOR BEARING FOOTING**  
SCALE: 1/2" = 1'-0"



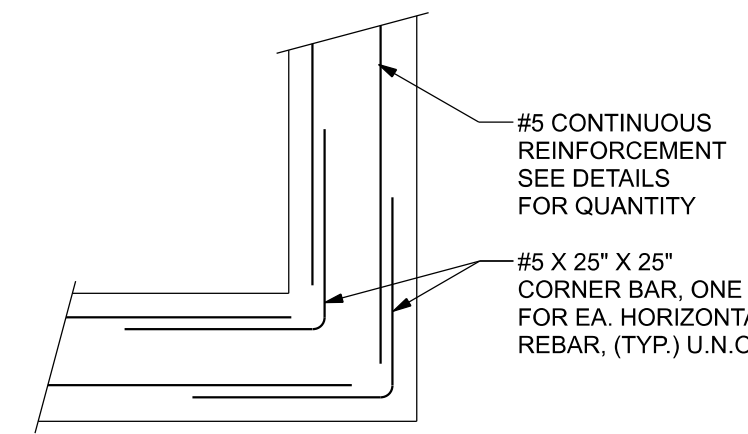
**F7 FOOTING @ SHOWER @ MASONRY**  
SCALE: 1/2" = 1'-0"



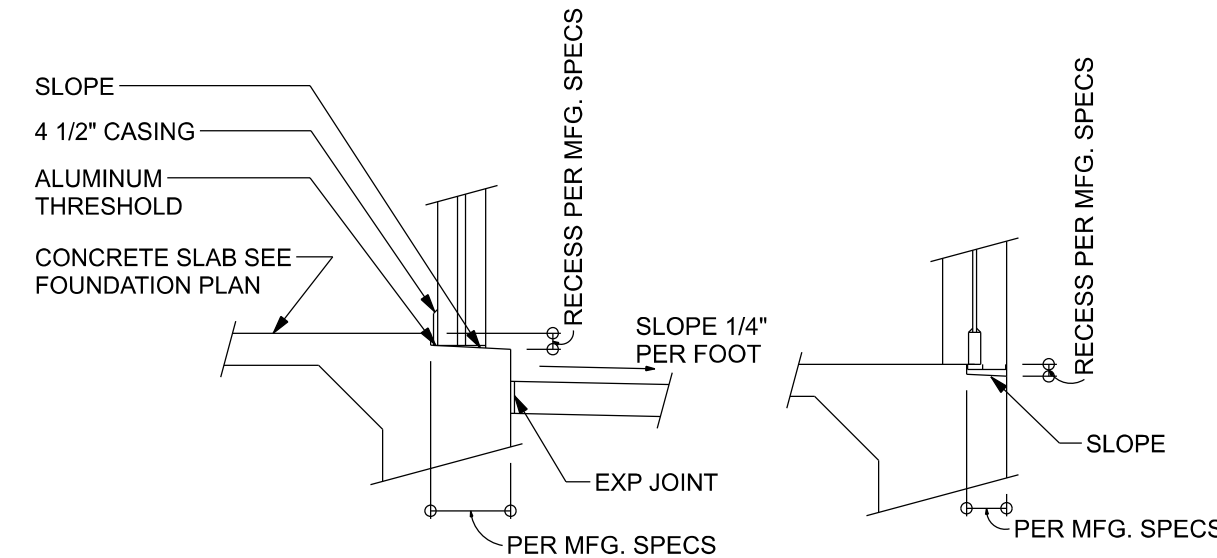
**F8 THICKEND EDGE**  
SCALE: 1/2" = 1'-0"



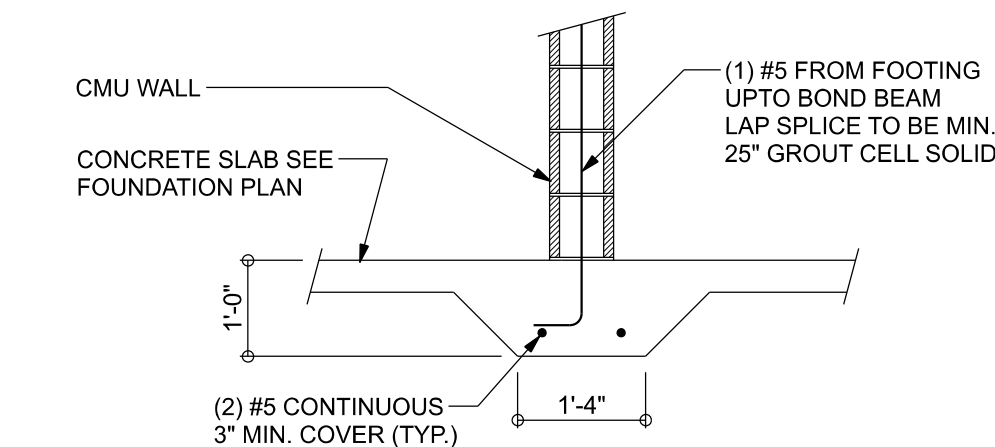
**F9 (TYP.) STEP FOOTING DETAIL**  
SCALE: 1/2" = 1'-0"



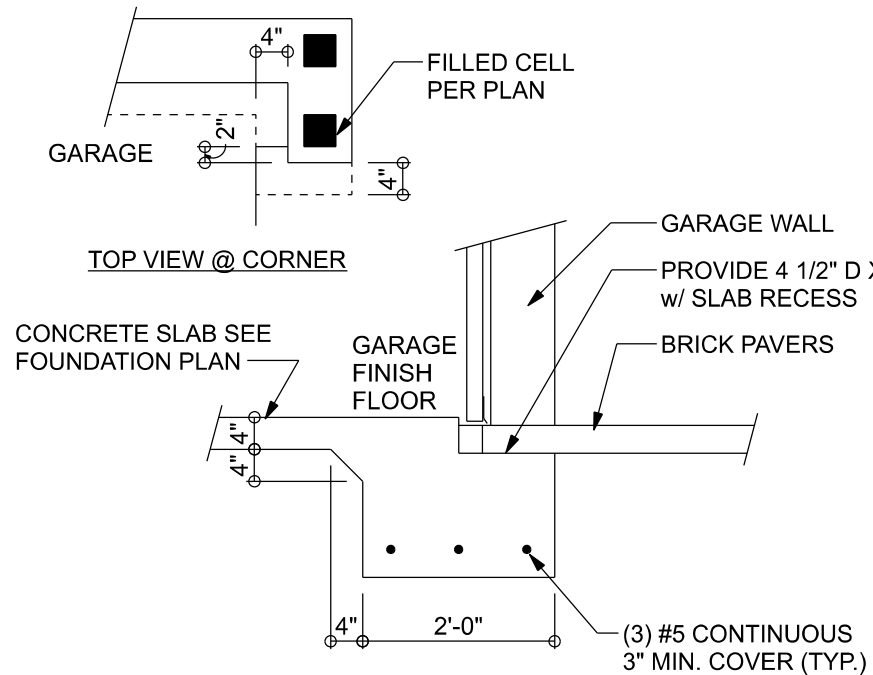
**F10 (TYP.) CORNER BAR DETAIL**  
SCALE: 1/2" = 1'-0"



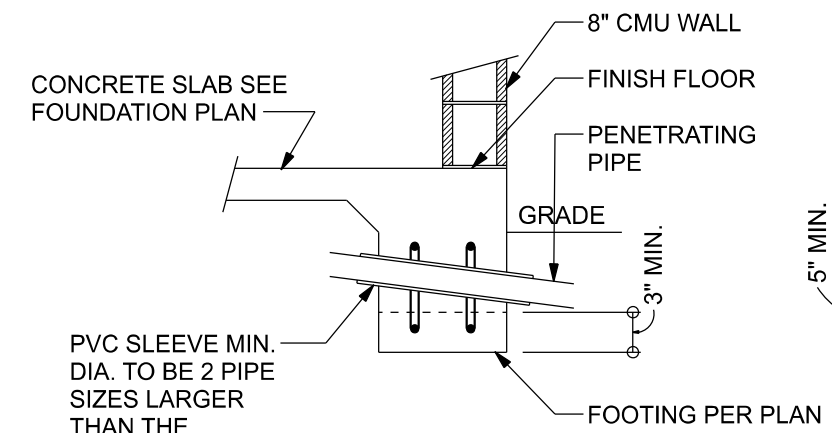
**F11 EXTERIOR DOOR POURED SILLS**  
SCALE: 1/2" = 1'-0"



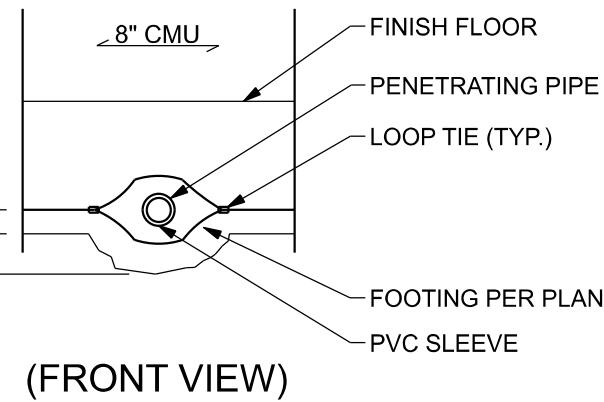
**F12 INTERIOR BEARING FOOTING w/ CMU WALL**  
SCALE: 1/2" = 1'-0"



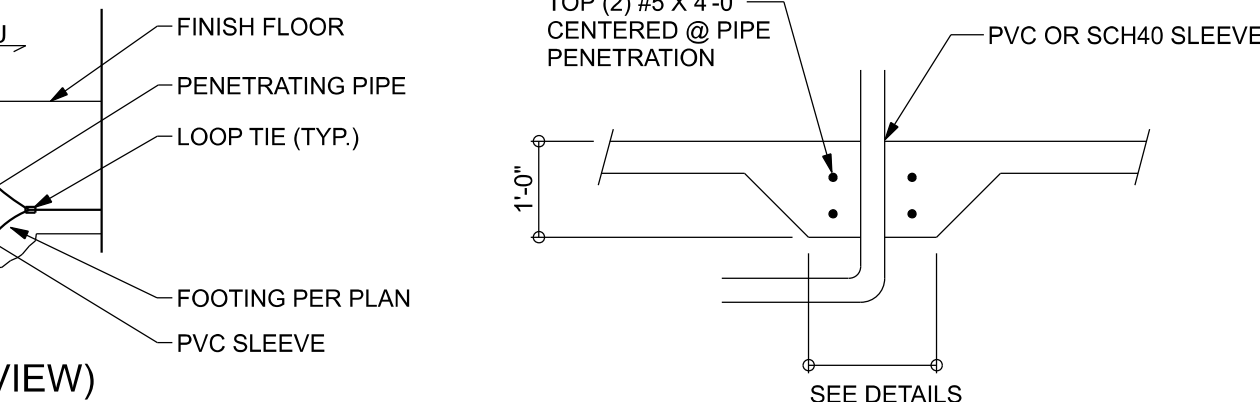
**F13 THICKENED EDGE @ GARAGE DOOR**  
SCALE: 1/2" = 1'-0"



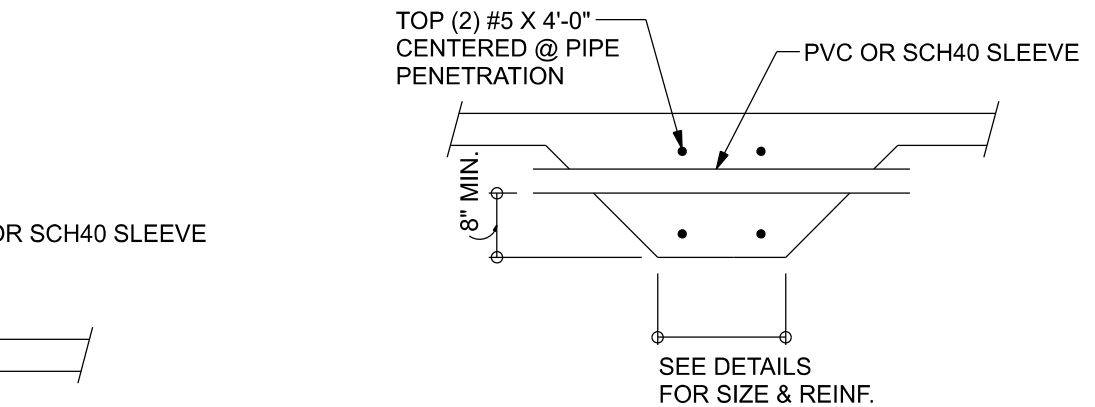
**F14 TYPICAL FOUNDATION PENETRATIONS**  
SCALE: 1/2" = 1'-0"



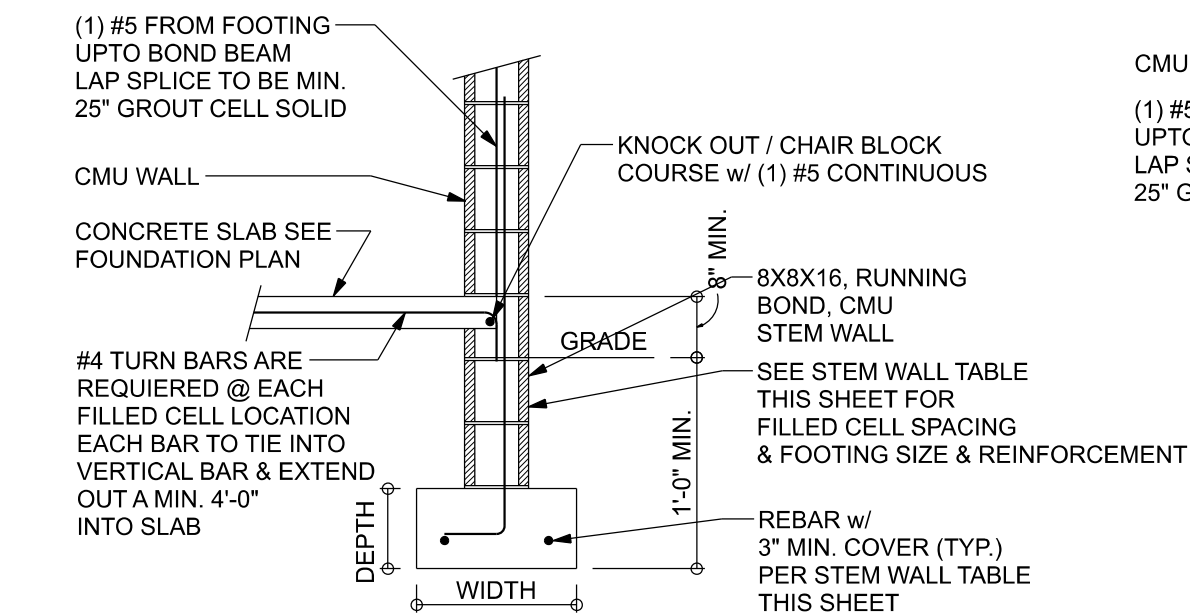
**F15 STEM WALL FOOTING @ PORCH**  
SCALE: 1/2" = 1'-0"



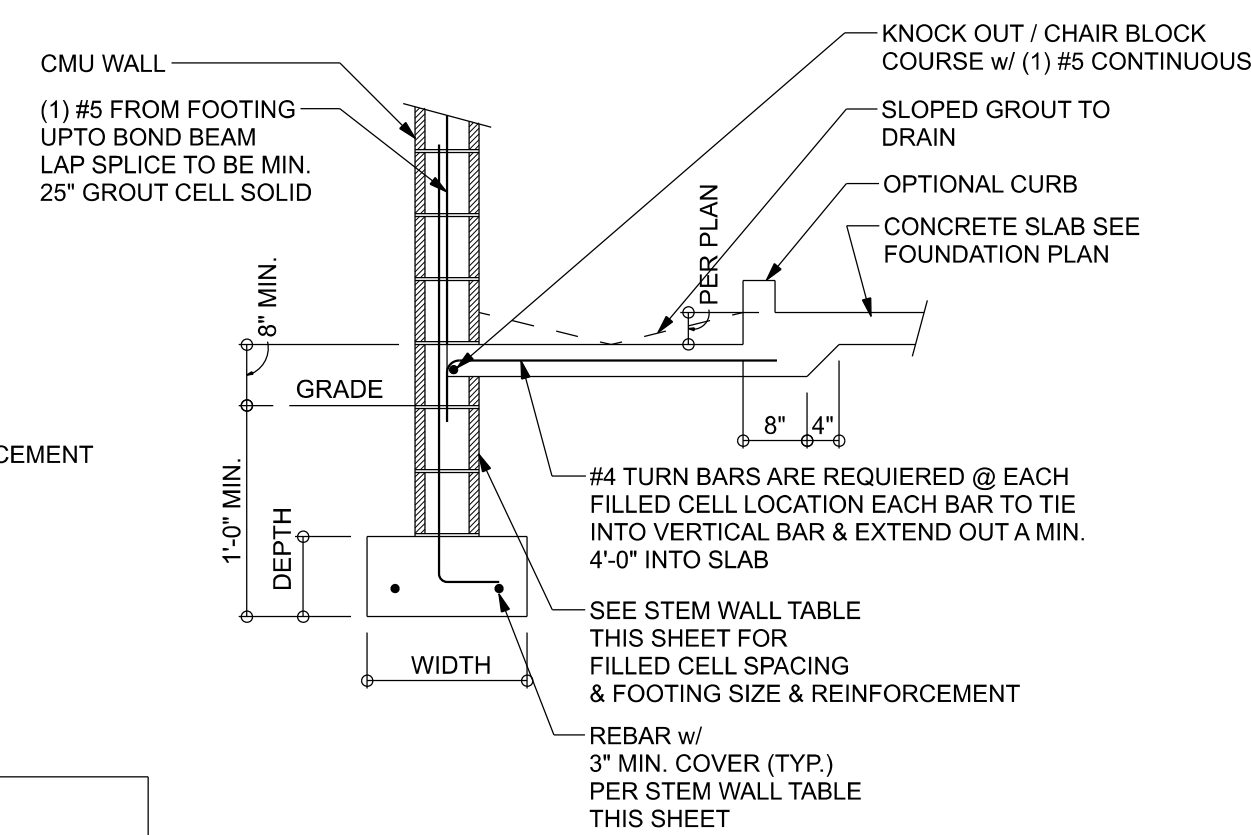
**F16 STEM WALL @ GARAGE STEP DOWN**  
SCALE: 1/2" = 1'-0"



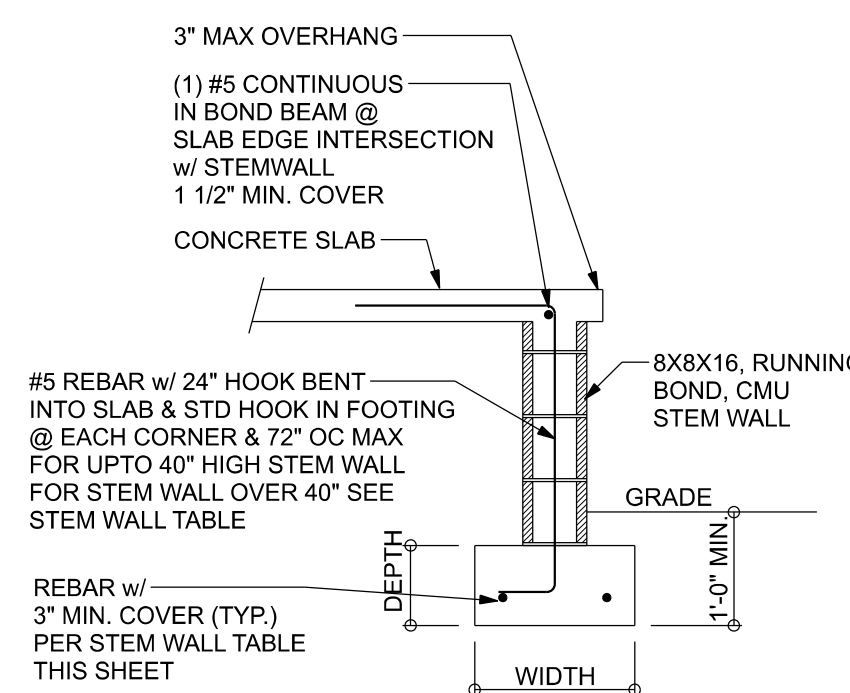
**F17 STEM WALL CURB FOOTING @ SCREENED LANAI**  
SCALE: 1/2" = 1'-0"



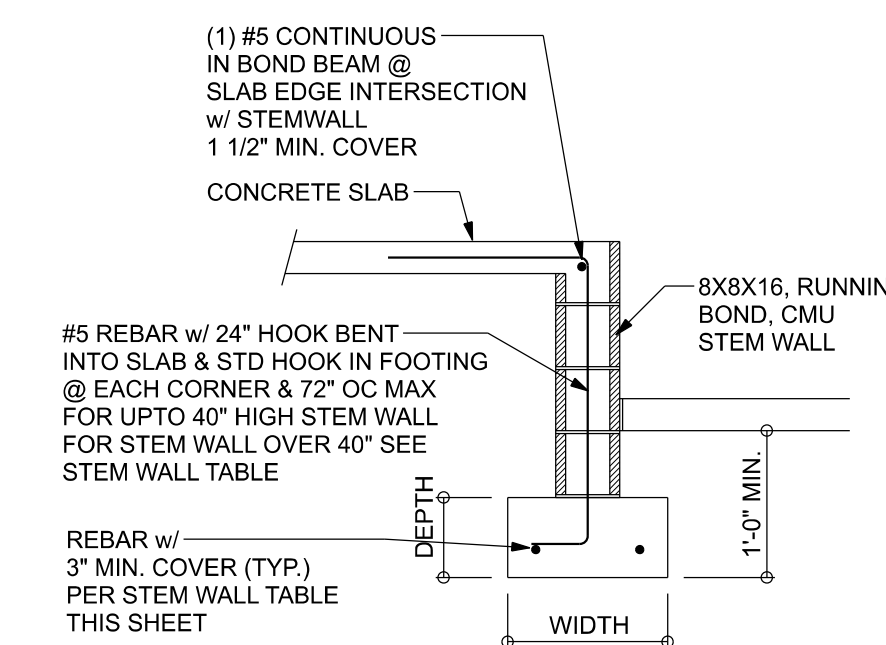
**F1 STEM WALL FOOTING**  
SCALE: 1/2" = 1'-0"



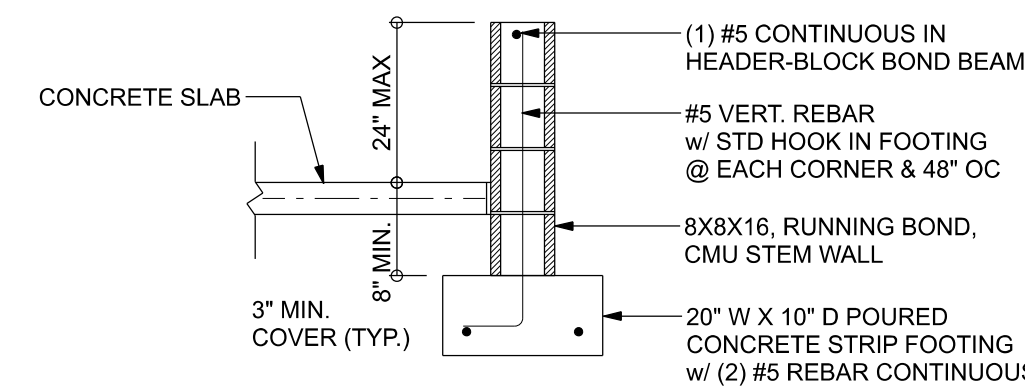
**F7 FOOTING @ SHOWER @ MASONRY**  
SCALE: 1/2" = 1'-0"



**F15 STEM WALL FOOTING @ PORCH**  
SCALE: 1/2" = 1'-0"



**F16 STEM WALL @ GARAGE STEP DOWN**  
SCALE: 1/2" = 1'-0"



**F17 STEM WALL CURB FOOTING @ SCREENED LANAI**  
SCALE: 1/2" = 1'-0"

STEM WALL TABLE						
STEMWALL HEIGHT	FOOTING DIMENSION				NUMBER / SIZE OF REBAR IN FOOTING	MAX FILLED CELL SPACING (O.C.) IN STEM WALL
	1-STORY		2-STORY			
	DEPTH	WIDTH	DEPTH	WIDTH		
8" - 40"	10"	20"	10"	20"	(2) #5 REBARS FOR 1-STORY OR (3) #5 REBARS FOR 2-STORY	MATCH FILLED CELL SPACING PER PLAN
48" - 64"	10"	20"	10"	20"	(2) #5 REBARS FOR 1-STORY OR (3) #5 REBARS FOR 2-STORY	40"
72" - 80"	10"	30"	10"	30"	(3) #5 REBARS FOR 1-STORY & 2-STORY	32"

NOTE:  
ALL STEM WALL FOUNDATIONS  
OVER 3'-0" IN HEIGHT TO BE POURED SOLID

## OPTIONAL STEM WALL FOUNDATION

Blake Construction

Ronnie Shuman Res

PROJECT ADDRESS:  
205 SW Madison Court  
Lake City, FL 32024

DIMENSIONS:  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915

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STATE OF FLORIDA  
Professional Engineer  
No. 53915  
Mark Disoway

Thursday, August 26, 2021

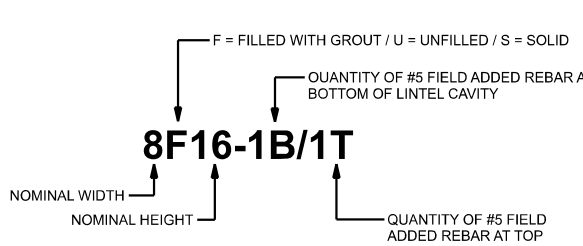
Mark Disoway P.E.  
163 SW Midtown Place  
Suite 103  
Lake City, Florida 32025  
386.754.5419  
disowaydesign@gmail.com

JOB NUMBER:  
211179

S-2.1  
OF 4 SHEETS



# TYPE DESIGNATION



## MATERIALS

1. F-8' precast intel = 3500 psi
2. Fc prestressed intel = 6000 psi
3. Grout per ASTM C476 Fc = 3000 psi w/ maximum 3/8 inch aggregate & 8 to 11 inch slump
4. Concrete Masonry Units (CMU) per ASTM C90 minimum net area compressive strength = 1900 psi
5. Rebar per ASTM A615 grade 60
6. Prestressing strand per ASTM A416 grade 270 low relaxation
7. Mortar per ASTM C270 type M or S

## GENERAL NOTES

1. Provide full mortar bed and head joints.
2. Show field intel as required.
3. Installation of intel must comply with the architectural and/or structural documents.
4. U-Intels are manufactured with 5 1/2" long notches at the ends to accommodate vertical oil reinforcing and grouting.
5. All intel meet or exceed L/500 deflection, except intel 17'-4" and longer with a nominal height of 8' meet or exceed L/180 deflection.
6. Bottom field added rebar to be located at the bottom of the intel cavity.
7. 7/32" diameter wire stirrups are welded to the bottom steel for mechanical anchorage.
8. Cast-in-place concrete may be provided in composite intel in lieu of concrete masonry units.
9. Safe load rating based on rational design analysis per ACI 318 and ACI 530.
10. Product Approvals: Miami-Dade County, Florida No. C-2008-05
11. The exterior surface of intel installed in exterior concrete masonry walls shall have a coating of stucco applied in accordance with ASTM C-208 or other approved coating.
12. Intel loaded simultaneously with vertical (gravity or uplift) and horizontal (lateral) loads should be checked for the combined loading with the following equation:  
Applied vertical load + Applied horizontal load ≤ 1.0 Safe vertical load + Safe horizontal load
13. Additional lateral load capacity can be obtained by the designer by providing additional reinforced concrete masonry above the intel. See detail at right.

## SAFE LOAD TABLE NOTES

1. All values based on minimum 4 inch nominal bearing.
2. Exception: Safe loads for unfilled intel must be reduced by 20% if bearing length is less than 6 1/2 inches.
3. N.R. = Not Rated
4. Safe loads are superimposed allowable loads.
5. Safe loads based on grade 40 or grade 60 field rebar.
6. One #7 rebar may be substituted for two #5 rebars in 9" intel only.
7. The designer may evaluate concentrated loads from the safe load tables by calculating the maximum resisting moment and shear at 0'-away from face of support.
8. For composite intel heights not shown, use safe load from next lower height shown.
9. For intel lengths not shown, use safe load from next longest length shown.
10. All safe loads in units of pounds per linear foot.
11. All safe loads based on simply supported span.
12. The number in the parenthesis indicates the percent reduction for grade 40 field added rebar.
13. Example 7'-6" intel type 8F32-1B safe gravity load = 6472(0.0465) (15)(40.071); w/ 15% reduction 6472 x (.85) = 5501 plf

## SAFE GRAVITY LOADS FOR 8" PRECAST & PRESTRESSED U-INTELS

LENGTH	TYPE	RUB	SAFE LOAD - POUNDS PER LINEAR FOOT									
			8F-16	8F-12-4B	8F-16-4B	8F-20-4B	8F-24-4B	8F-28-4B	8F-32-4B	8F-16-1B	8F-12-4B-1B	8F-16-4B-1B
2'-10" (34")	PRECAST	2231	3069	4605	6113	7547	8974	10394	11809	3069	4605	6113
3'-4" (42")	PRECAST	2231	3069	4605	6113	7547	8974	10394	11809	3069	4605	6113
4'-0" (48")	PRECAST	1966	2893	4379	5865	7351	8837	10323	11809	2893	4379	5865
4'-4" (54")	PRECAST	1599	2189	3275	4361	5447	6533	7619	8705	2189	3275	4361
5'-4" (64")	PRECAST	1217	1663	2509	3355	4201	5047	5893	6739	1663	2509	3355
6'-10" (70")	PRECAST	1062	1451	2189	2927	3665	4403	5141	5879	1451	2189	2927
6'-4" (76")	PRECAST	908	1238	1717	2340	2963	3586	4209	4832	1238	1717	2340
7'-4" (80")	PRECAST	743	1011	1429	1847	2265	2683	3101	3519	1011	1429	1847
8'-4" (112")	PRECAST	584	792	1085	1378	1671	1964	2257	2550	792	1085	1378
10'-4" (126")	PRECAST	475	643	886	1129	1372	1615	1858	2101	643	886	1129
11'-4" (136")	PRECAST	362	502	696	890	1084	1278	1472	1666	502	696	890
12'-0" (144")	PRECAST	337	460	633	806	979	1152	1325	1498	460	633	806
13'-4" (160")	PRECAST	296	411	574	737	900	1063	1226	1389	411	574	737
14'-0" (168")	PRECAST	279	402	565	728	891	1054	1217	1380	402	565	728
14'-8" (176")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
15'-4" (184")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
17'-4" (208")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
19'-4" (232")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
21'-4" (256")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
22'-0" (264")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.
24'-0" (288")	PRESTRESSED	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.	N.R.

## SAFE GRAVITY LOADS FOR 8" PRECAST w/ 2" RECESS DOOR U-INTELS

LENGTH	TYPE	RUB	SAFE LOAD - POUNDS PER LINEAR FOOT									
			8F-16-1B	8F-12-4B-1B	8F-16-4B-1B	8F-20-4B-1B	8F-24-4B-1B	8F-28-4B-1B	8F-32-4B-1B	8F-16-1B	8F-12-4B-1B	8F-16-4B-1B
4'-4" (52")	PRECAST	1635	1541	2281	3021	3761	4501	5241	5981	1541	2281	3021
4'-0" (48")	PRECAST	1494	1399	2039	2679	3319	3959	4599	5239	1399	2039	2679
5'-4" (68")	PRECAST	866	803	1171	1539	1907	2275	2643	2991	803	1171	1539
5'-10" (70")	PRECAST	810	757	1085	1453	1821	2189	2557	2925	757	1085	1453
7'-4" (80")	PRECAST	797	735	1043	1351	1659	1967	2275	2583	735	1043	1351
9'-4" (116")	PRECAST	411	426	599	772	945	1118	1291	1464	426	599	772

