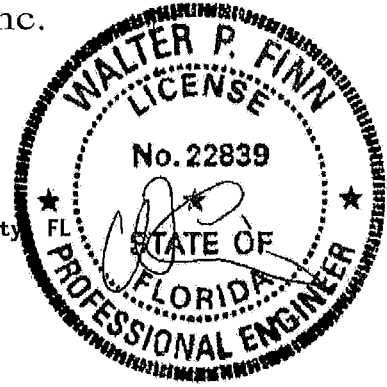


ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID IV1J487-Z0122093524



11/22/2013

Walter P Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

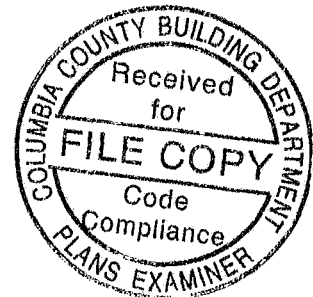
Truss Fabricator **Anderson Truss Company**
Job Identification **13-281--BRYAN ZECHER /Columbia County Builders -- Lake City**
Truss Count **36**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Versions 12.03, 13.01.**
Structural Engineer of Record **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61615-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Details: 14015EC1-GBLLETIN-BRCLBSUB-PB16010-

#	Ref	Description	Drawing#	Date
1	28444-A	38'9"8 Stepdow	13326001	11/22/13
2	28445-A1	38'9"8 Stepdo	13326002	11/22/13
3	28446-B	19'7"8 Common	13326003	11/22/13
4	28447-B1	19'7"8 Common	13326004	11/22/13
5	28448-BG1	11'7" Common	13326035	11/22/13
6	28449-BGE	19'7"8 Gable	13326022	11/22/13
7	28450-CJ1	1' Jack	13326023	11/22/13
8	28451-CJ3	3' End Jack	13326024	11/22/13
9	28452-CJ5	5' Jack	13326025	11/22/13
10	28453-EJ3	3' End Jack	13326026	11/22/13
11	28454-EJ7	7' End Jack	13326027	11/22/13
12	28455-EJ7A	7' End Jack	13326005	11/22/13
13	28456-EJ7B	7' End Jack	13326006	11/22/13
14	28457-EJ7B	7' End Jack	13326007	11/22/13
15	28458-EJ7C	7' End Jack	13326008	11/22/13
16	28459-H11	38'9"8 Stepd	13326009	11/22/13
17	28460-H11A	26'4" Stepd	13326010	11/22/13
18	28461-H13	38'9"8 Stepd	13326011	11/22/13
19	28462-H13A	26'4" Stepd	13326012	11/22/13
20	28463-H15	38'9"8 Stepd	13326013	11/22/13
21	28464-H15A	26'4" Stepd	13326014	11/22/13
22	28465-H17	38'9"8 Stepd	13326015	11/22/13
23	28466-H17A	26'4" Stepd	13326016	11/22/13
24	28467-H19	38'9"8 Stepd	13326017	11/22/13
25	28468-H19A	26'4" Stepd	13326018	11/22/13
26	28469-H3	11'7" Stepdow	13326019	11/22/13
27	28470-H5	11'7" Stepdow	13326020	11/22/13
28	28471-H7	38'9"8 Stepdo	13326021	11/22/13
29	28472-H7A	26'4" Stepdo	13326036	11/22/13
30	28473-H9	38'9"8 Stepdo	13326028	11/22/13
31	28474-H9A	26'4" Stepdo	13326029	11/22/13
32	28475-HJ3	4'2"15 Hip J	13326030	11/22/13
33	28476-HJ7	9'10"13 Hip	13326031	11/22/13
34	28477-HJ7A	9'10"13 Hip	13326032	11/22/13
35	28478-PBA	4'9"6 Common	13326033	11/22/13
36	28479-PBA1	4'9"6 Stepd	13326034	11/22/13



ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837

Page 1 of 1 Document ID 1V1J487-Z0122093524

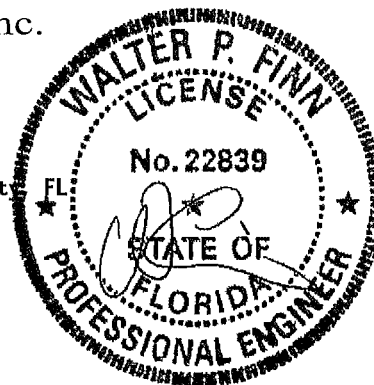
Truss Fabricator **Anderson Truss Company**
Job Identification **13-281--BRYAN ZECHER /Columbia County Builders -- Lake City**
Truss Count **1**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Versions 12.03, 13.01.**
Structural Engineer of Record
Address
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Revised Trusses

#	Ref	Description	Drawing#	Date
1	28472-H7A	26' 4" Stepdo	13326036	11/22/13



11/22/2013

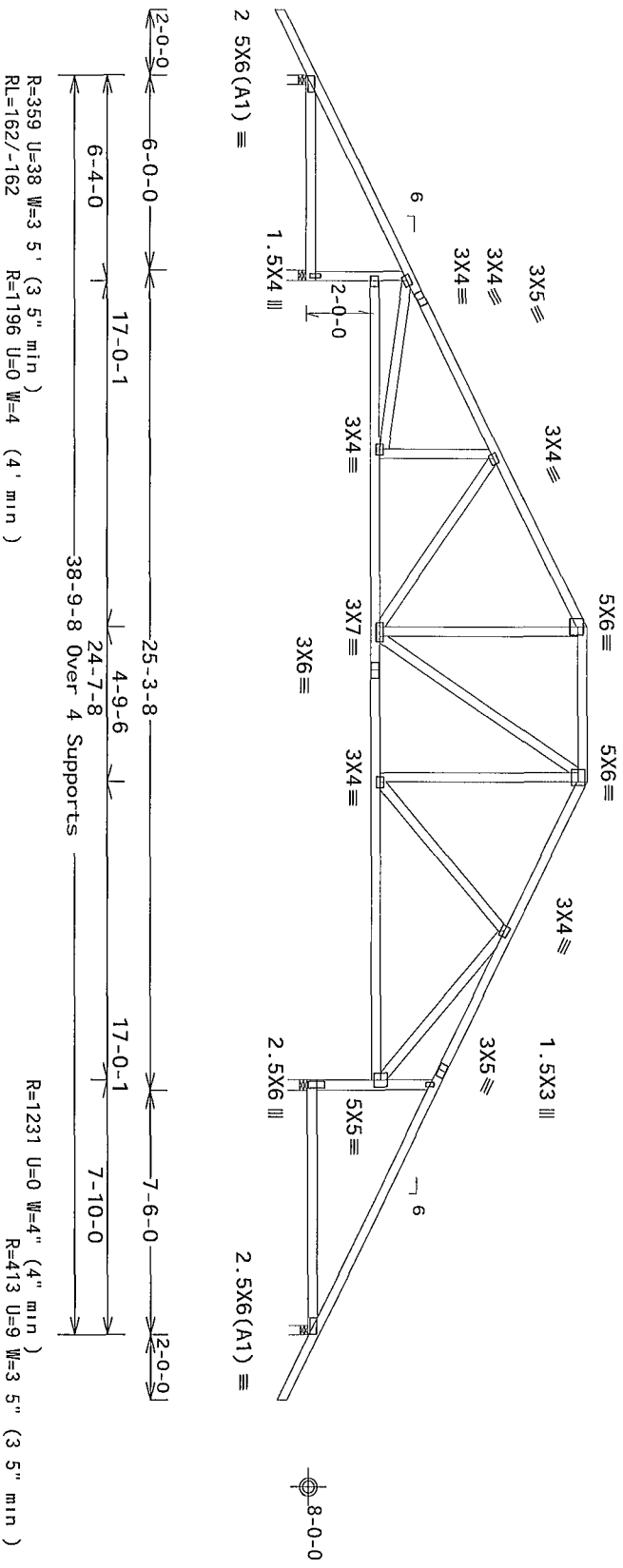
-Truss Design Engineer-
Walter P Finn

1950 Marley Drive
Haines City, FL 33844

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

MIN-RS loads based on trusses located at least 15 00 ft from roof edge

factor for dead load is 1.30

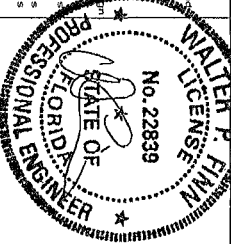


Scale = .1875"/Ft.

****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS. Trussess require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of the Building Component Safety Information by TPI and WTCO for safety practices, proper use and performance of these functions. The manufacturer shall provide temporary bracing per BSCEI and the manufacturer shall provide a permanent lateral restraint of the truss. The manufacturer shall have a properly attached rigid connection. Local area shown for permanent lateral restraint of web. I have braced per BSCEI sects 8B, 8C, 8D as applicable.

Any TPI Bu id ng Components Group Inc. (TMBSC) shall not be responsible for any damage from this design. Failure to build the truss in conformance with ANSI/TPI 1 or for handling any device on the truss installed on bracing of trusses. Apply plates to each place of truss and post as shown above and on the joint. Details unless noted otherwise. Refer to drawngs 1600-2 for standard place posts. A seal on the drawing or cover page relating to a drawing and acceptance of professional engineering need not be the responsibility of the manufacturer. The manufacturer shall provide the structure's design and the responsible party of the Bu id ng Designer per ANSI/TPI 1 Sec 2. For more information see the general notes page 17W-BDC www tdbco com TPI www tpi net org WTCO www industry com

17W-BDC www tdbco com TPI www tpi net org WTCO www industry com



TC LL	20 0 PSF	REF	R9114- 28444
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	HCUSR9114 13326001
BC LL	0 0 PSF	HC-ENG	AP/AP
TOT LD	37 0 PSF	SEQN-	330910
DUR.FAC.	1.25		
SPACING	24 0"	JREF -	1V1J487_Z01

(13-281--BRYAN ZECHER /Columbia County Builders -- Lake City, FL - A1 38'9"8 Steepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B' use design values approved
1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24'
OC

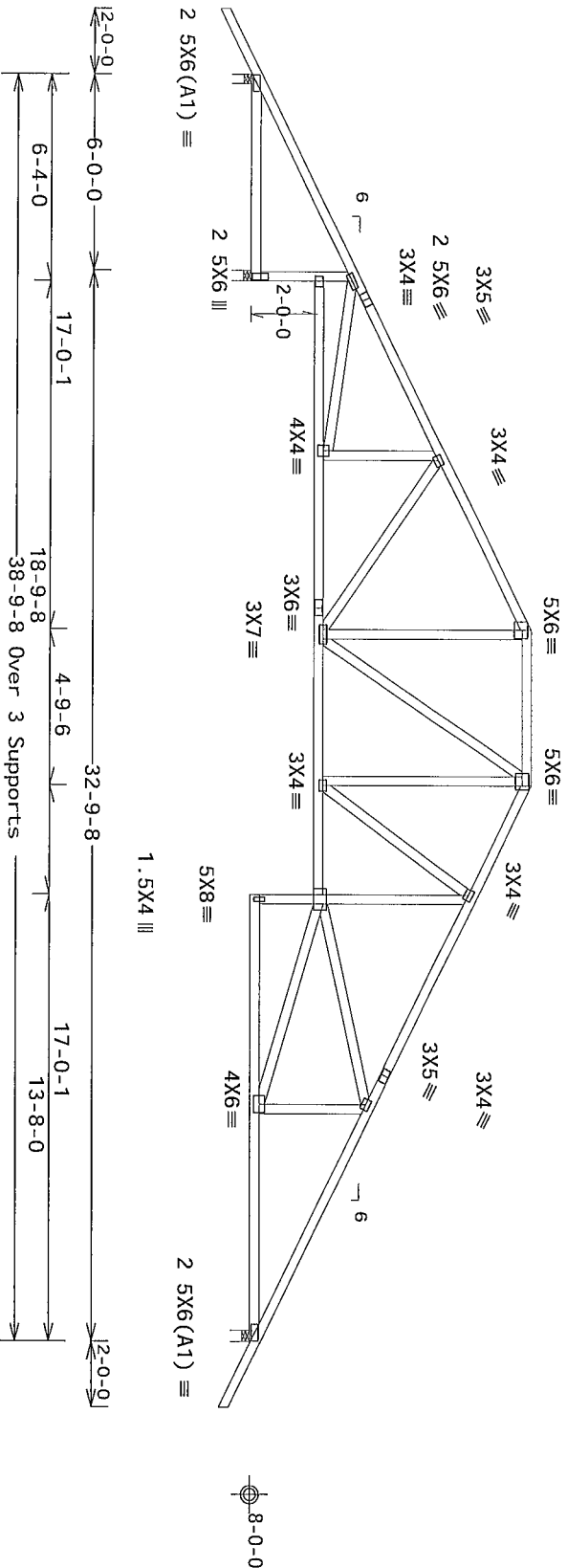
MMFRS loads based on trusses located at least 15 00 ft from roof
edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf,
wind BC DL=5 0 psf GCP(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member
design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50



R=351 U=39 W=3 5" (3 5" min)
R=162/-162 R=1496 U=0 W=4" (4 min)

R=1355 U=0 W=4" (4" min)

PLT TYP Wave

Design Crit FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

12 03 00

QTY: 1 FL/-/5/-/-/R/-

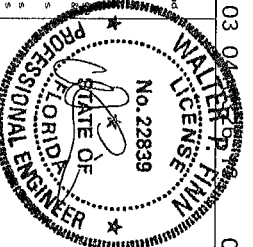
Scale = 1875"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL 32837
FL COA #0278

****IMPORTANT** READ AND FOLLOW ALL NOTES ON THIS SHEET.**
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Components Safety Institute) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached girding. Location shown for permanent lateral restraint of webs. The Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. Any alteration to this design without written approval from ITWBCG is void. Refer to drawings 1804.2 for bracing details and on the job site. Drawing of cover page listing this drawing. The suitability and use of this design for any structure is the responsibility of the building engineer. The suitability and use of this design for any structure is the responsibility of the building engineer. For more information on section 5.1.1, see the general notes page. ITW BCG www.itwbcg.com TPI www.tpi.net.org WIDA www.sbc industry.com IBC www.icbsafe.org



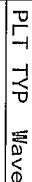
TC LL	20 0 PSF	REF	R9114- 28445
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	HCSR9114 13326002
BC LL	0 0 PSF	HC-ENG	AP/AP
TOT LD	37 0 PSF	SEQN	330912
DUR FAC	1 25		
SPACING	24.0"	JREF	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50


$$\overline{FT/RT} = 10\%(0\%) / 0(0)$$

QTY 2 FL/-/5/-/-/R/-

Scale = 3125"/Ft

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

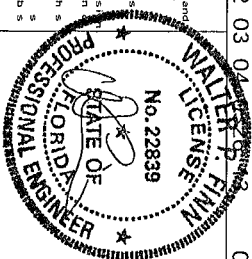
****IMPORTANT--FURNISH THIS DESIGN TO ALL CONTRACTORS, INCLUDING INSTALLERS.**

Tusneen requ re extreme care in fab cat ng hand ng msta lng and brace ng Refer to and follow the latest edition of BCS (Build ng Components Safety Information on by TPI and WTCO) for safety practice and pr or to performing these functions. Installers shall prov e temporary brace ng per BCS1 shall have a proper attachment method. All components shall have properly attached structural cleavah ng and bottom chord lateral restraint of mod shall have brace ng method per BCS1 sect ons B. 87 or B10 as appl cable

1TW Bu Id ng Components Group Inc. (TWBCG) shall not be responsible for any damage from this design any failure to bu Id the truss in conformance with the ANSI/TPI 1 or 2 or for hand ng ship ng material on bracing of trusses. Apply plates to each face of truss and position as shown above and on the Jo nt Details unless noted otherwise. Refer to drawings 180A-2 for standard plate pos tions. A seal on the truss members covers the end of the truss and the design of the seal indicates acceptance of professional engineering responsibility for the design of the truss. For more information on the responsibility of the design of the truss, refer to ANSI/TPI 1 Sec 2 or for more information on the design of the truss, refer to ANSI/TPI 2.

general notes page 1TW BCS www twbcg com TPI www tpi net org WTCO www sbc industry com

CDL www cdcsl org



11/22/2013

TC LL	20 0 PSF	REF R9114- 28446
TC DL	7.0 PSF	DATE 11/22/13
BC DL	10 0 PSF	DRW HOUSE114 13326003
BC LL	0 0 PSF	HC-ENG AP/AP
TOT LD	37 0 PSF	SEQN- 339741
DUR.FAC	1 25	
SPACING	24.0"	JREF- 1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

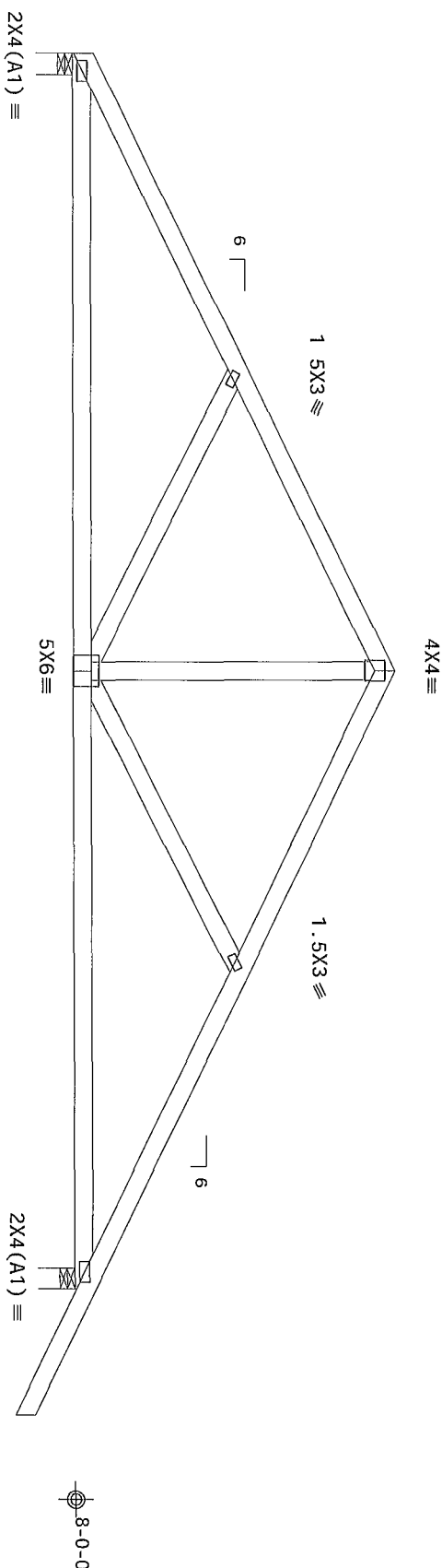
Bottom chord checked for 10 00 psf non-concurrent live load

MMFRS loads based on trusses located at least 7 50 ft from roof edge

130 mph wind 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4.50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf 6Cp1(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

[illegible]

PLT TYP Wave

Design Crit	FBC2010Res/TP1-2007(STD)	FT/RT=10%(0%)/0(0)

12.03 04 55 26 123

QTY 8 FL/-/5/-/-/R/-

Scale = 375"/Ft

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trussos require extreme care in fabricating and bracing. Refer to and follow the latest edition of BCS1 (Building Component Safety Information by TPI and WTA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCS1 and approved drawings. Do not exceed approved structural loads and do not remove bracing until approved.

shall have bracing installed per BCS sections B3, B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from the design.

any failure to build the truss in conformance with ANSI/HPI-1 or for handling shipping installation or bracing of trusses. Apply plates to each face of truss and position as shown above and on the joint

Details unless noted otherwise. Refer to drawing 160A-Z for standard plate positions. A seal on the drawing does not cover material. Do not draw to indicate acceptance of prototype and approval.

The su tab ility and use of th s des gn for any structure s

the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job's general notes page (TW-BCC www.twbcs.com TPI www.tpi-nst.org WTCA www.sbc-industry.com)

CC 0 www.lccsafe.org

ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0 278

11/22/2013

TC LL	20 0 PSF	REF	R9114- 28447
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	H05R9114 13326004
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT.LD	37 0 PSF	SEQN-	339608
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

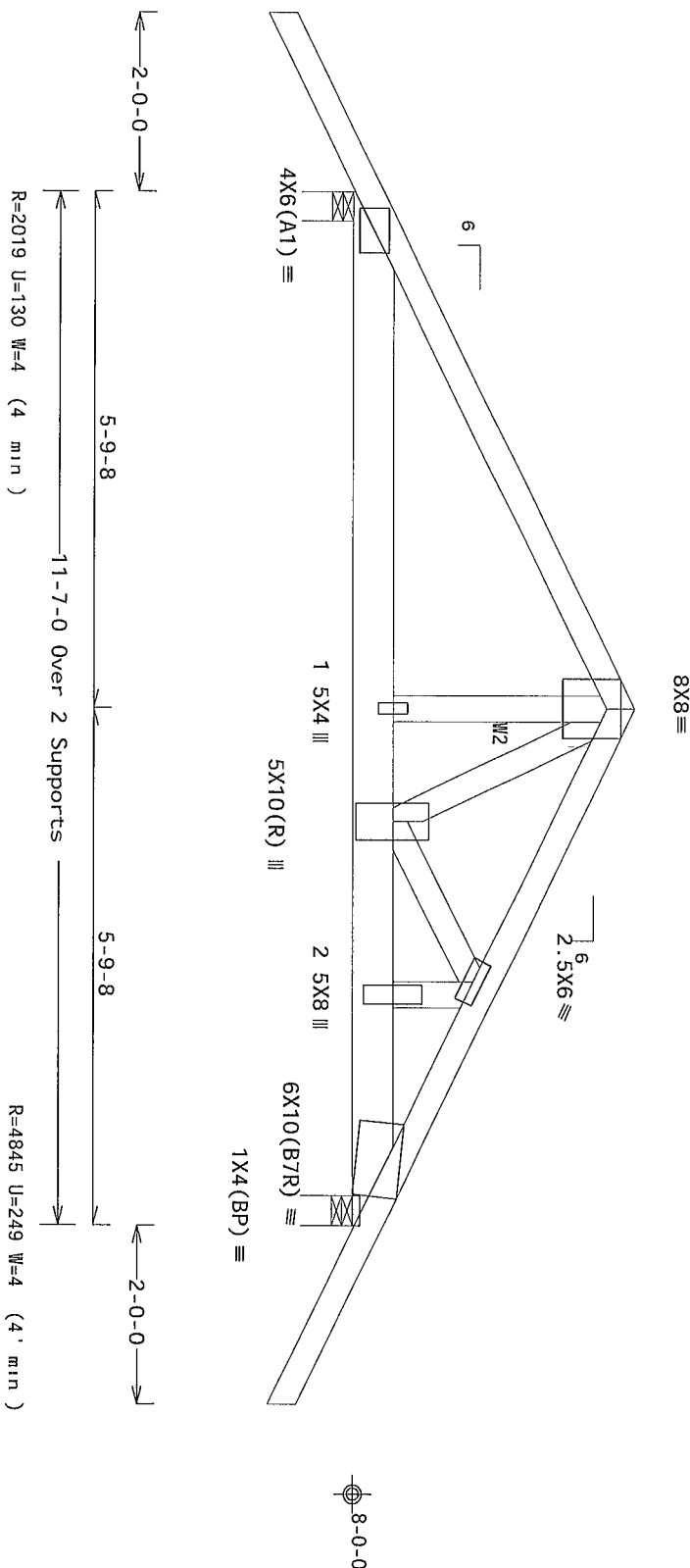
Webs 2x4 SP #3-13B W2 2x4 SP #1-13B

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCpl(+/-)=0 18

Wind loads and reactions based on MWFRS

Deflection meets L/240 live and L/180 total load Creep increases factor for dead load is 1.50

Special loads			
	Dur	Fac =1 25 /	Plate Dur Fac =1 25)
-----Lumber			
TC-From	56 p1f at -2 00 to	56 p1f at 5 79	
TC-From	56 p1f at 5 79 to	56 p1f at 13 58	
BC-From	4 p1f at -2 00 to	4 p1f at 0 00	
BC-From	20 p1f at 0 00 to	20 p1f at 7 06	
BC-From	10 p1f at 7 06 to	10 p1f at 11 58	
BC-From	4 p1f at 11 58 to	4 p1f at 13 58	
BC-2873 16 1b Conc	Load at 7 06		
BC-1462 97 1b Conc	Load at 9 06		
BC-1456 92 1b Conc	Load at 11 06		
Bottom chord checked for 10 00 psf non-concurrent live load			



PLT TYP Wave

Design Crit	FBC2010Res/TP1-2007(STD), FT/RT=10%(0%)/0(0)

12 03 04 05 06 07 08 09 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49 50 51 52 53 54 55 56 57 58 59 60 61 62 63 64 65 66 67 68 69 70 71 72 73 74 75 76 77 78 79 80 81 82 83 84 85 86 87 88 89 90 91 92 93 94 95 96 97 98 99 100 101 102 103 104 105 106 107 108 109 110 111 112 113 114 115 116 117 118 119 120 121 122 123 124 125 126 127 128 129 130 131 132 133 134 135 136 137 138 139 140 141 142 143 144 145 146 147 148 149 150 151 152 153 154 155 156 157 158 159 160 161 162 163 164 165 166 167 168 169 170 171 172 173 174 175 176 177 178 179 180 181 182 183 184 185 186 187 188 189 190 191 192 193 194 195 196 197 198 199 200 201 202 203 204 205 206 207 208 209 210 211 212 213 214 215 216 217 218 219 220 221 222 223 224 225 226 227 228 229 230 231 232 233 234 235 236 237 238 239 240 241 242 243 244 245 246 247 248 249 250 251 252 253 254 255 256 257 258 259 260 261 262 263 264 265 266 267 268 269 270 271 272 273 274 275 276 277 278 279 280 281 282 283 284 285 286 287 288 289 290 291 292 293 294 295 296 297 298 299 300 301 302 303 304 305 306 307 308 309 310 311 312 313 314 315 316 317 318 319 320 321 322 323 324 325 326 327 328 329 330 331 332 333 334 335 336 337 338 339 340 341 342 343 344 345 346 347 348 349 350 351 352 353 354 355 356 357 358 359 360 361 362 363 364 365 366 367 368 369 370 371 372 373 374 375 376 377 378 379 380 381 382 383 384 385 386 387 388 389 390 391 392 393 394 395 396 397 398 399 400 401 402 403 404 405 406 407 408 409 410 411 412 413 414 415 416 417 418 419 420 421 422 423 424 425 426 427 428 429 430 431 432 433 434 435 436 437 438 439 440 441 442 443 444 445 446 447 448 449 450 451 452 453 454 455 456 457 458 459 460 461 462 463 464 465 466 467 468 469 470 471 472 473 474 475 476 477 478 479 480 481 482 483 484 485 486 487 488 489 490 491 492 493 494 495 496 497 498 499 500 501 502 503 504 505 506 507 508 509 510 511 512 513 514 515 516 517 518 519 520 521 522 523 524 525 526 527 528 529 530 531 532 533 534 535 536 537 538 539 540 541 542 543 544 545 546 547 548 549 550 551 552 553 554 555 556 557 558 559 560 561 562 563 564 565 566 567 568 569 570 571 572 573 574 575 576 577 578 579 580 581 582 583 584 585 586 587 588 589 590 591 592 593 594 595 596 597 598 599 600 601 602 603 604 605 606 607 608 609 610 611 612 613 614 615 616 617 618 619 620 621 622 623 624 625 626 627 628 629 630 631 632 633 634 635 636 637 638 639 640 641 642 643 644 645 646 647 648 649 650 651 652 653 654 655 656 657 658 659 660 661 662 663 664 665 666 667 668 669 670 671 672 673 674 675 676 677 678 679 680 681 682 683 684 685 686 687 688 689 690 691 692 693 694 695 696 697 698 699 700 701 702 703 704 705 706 707 708 709 710 711 712 713 714 715 716 717 718 719 720 721 722 723 724 725 726 727 728 729 730 731 732 733 734 735 736 737 738 739 740 741 742 743 744 745 746 747 748 749 750 751 752 753 754 755 756 757 758 759 760 761 762 763 764 765 766 767 768 769 770 771 772 773 774 775 776 777 778 779 780 781 782 783 784 785 786 787 788 789 790 791 792 793 794 795 796 797 798 799 800 801 802 803 804 805 806 807 808 809 810 811 812 813 814 815 816 817 818 819 820 821 822 823 824 825 826 827 828 829 830 831 832 833 834 835 836 837 838 839 840 841 842 843 844 845 846 847 848 849 850 851 852 853 854 855 856 857 858 859 860 861 862 863 864 865 866 867 868 869 870 871 872 873 874 875 876 877 878 879 880 881 882 883 884 885 886 887 888 889 890 891 892 893 894 895 896 897 898 899 900 901 902 903 904 905 906 907 908 909 910 911 912 913 914 915 916 917 918 919 920 921 922 923 924 925 926 927 928 929 930 931 932 933 934 935 936 937 938 939 940 941 942 943 944 945 946 947 948 949 950 951 952 953 954 955 956 957 958 959 960 961 962 963 964 965 966 967 968 969 970 971 972 973 974 975 976 977 978 979 980 981 982 983 984 985 986 987 988 989 990 991 992 993 994 995 996 997 998 999 1000 1001 1002 1003 1004 1005 1006 1007 1008 1009 1010 1011 1012 1013 1014 1015 1016 1017 1018 1019 1020 1021 1022 1023 1024 1025 1026 1027 1028 1029 1030 1031 1032 1033 1034 1035 1036 1037 1038 1039

QTY 1 FL/-/5/-/-/R/-

Scale = .5"/Ft

ALPINE

ITW Building Components Group Inc.

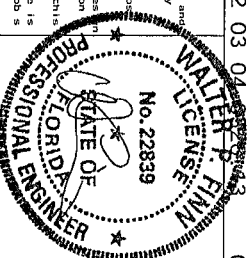
Orlando FL, 32837
FL COA #0278

*****IMPORTANT*** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS**

Trustees require extensive care in their cat ng handling an pp ng install ng and brace ng follow the latest edition of BCSP's (Building Component Safety) Information on by TPI and WTCO for all safety practice case prior to performing these functions. Installers shall provi de temporary brace ng per BCSP's latest noted otherwise. No chord shall have properly attached structural lateral ng and bottom chord shall have bracing installed per BCSP sections B3, B7 or B10 as appli cable.

17W Bldg. Idring Components Ground Inc. (ITMBCB) shall not be responsi ble for any drawi ng being this case any failure to build to build the truss in conformance with ANSI/TPI-1 or for handling an pp ng installa on of trusses. Apply plates to each face of truss and position as shown above and on the Joist. Brace is unless noted otherwise. Refer to drawi ng 100A-Z for standard brace posit ions. A seal on this drawing or cover page, let ng on a drawing and caries consequence of professional engineering or engineering design. The design of the truss is the responsibility of the engineer. For more information on see ANSI/TPI-1, Sec 2. This job s general notes page 17W BCG www.tlwdco.com TPI www.trinort.org WTCO www.sbcindustry.com

www.tlwdco.org



11/22/2013

TC LL	20 0 PSF	REF	R9114- 28448
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCUSR9114 13326035
BC LL	0 0 PSF	HC-ENG	AP/AP
TOT LD	37.0 PSF	SEQN-	324379
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

(13-281--BRYAN ZECHER /Columbia County Builders -- Lake City FL - BGE 19'7 8 Gable)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B
Stack Chord SC1 2x4 SP #1-13B Stack Chord SC2 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

See DWGS A14015ENC100212 & GBLLETIN0212 for more requirements

Stacked top chord must NOT be notched or cut in area (NML) Dropped top chord braced at 24" o c intervals Attach stacked top chord (SC) to dropped top chord in notched area using 3x4 tie-plates 24" o c Center plate on stacked/dropped chord interface, plate length perpendicular to chord length Splice top chord in notched area using 3x6

130 mph wind, 15'00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf GCpl(+/-)=0.18

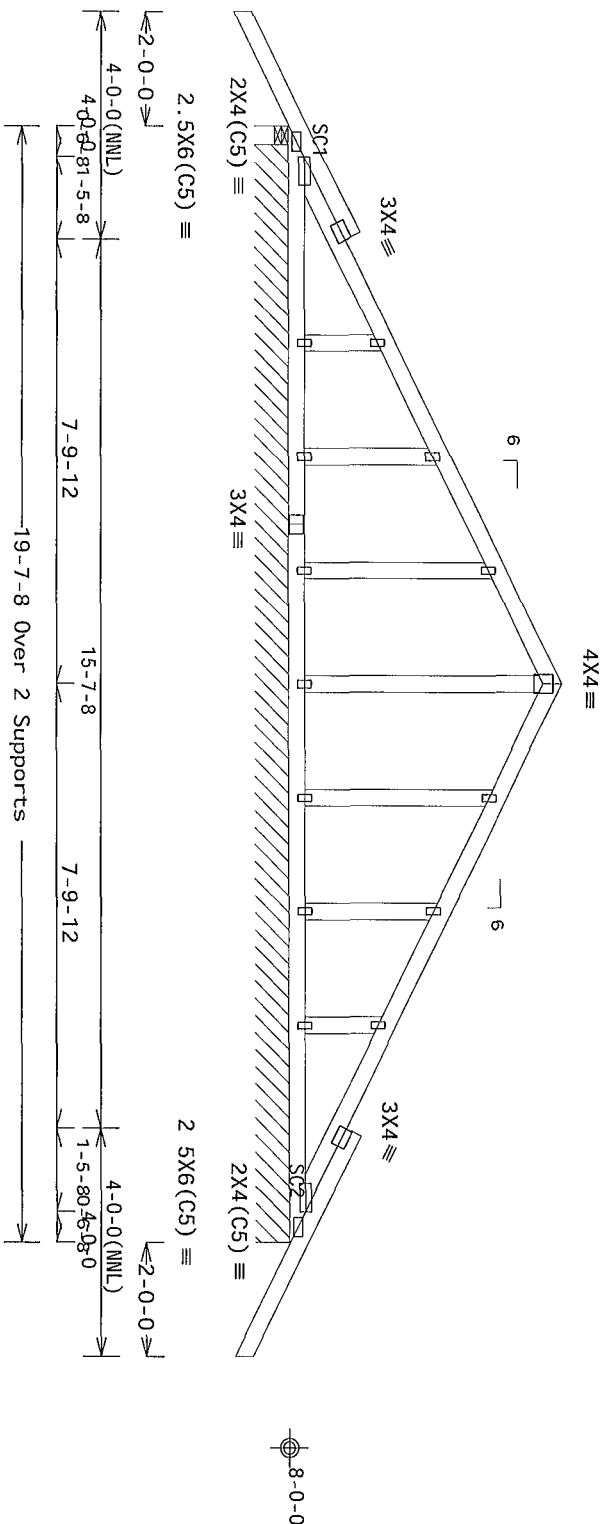
Wind loads and reactions based on MWFRS with additional C&C member design

Truss spaced at 24" o c designed to support 2-0-0 top chord outlookers Cladding load shall not exceed 10'00 PSF Top chord must not be cut or notched

In lieu of structural panels use purlins to brace TC @ 24" o c

Bottom chord checked for 10'00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50



R=578 U=153 W=4' (4" min)
R=146 RLE20621286F W=19-3-8

Note All Plates Are 1 5X3 Except As Shown

Design Crit: FBC2010Res/TP1-2007 (STD)

PLT TYP Wave

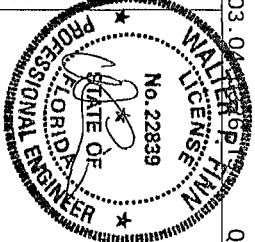
IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS SHEET

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety) Information by TPI and WTC for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r g d c e t i n g. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sheet one B3 B7 or B10 as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. Any failure to build the truss in accordance with ANSI/TP1 or for handling, shipping, installation & bracing shall be the responsibility of the contractor. Refer to drawings 1604-27 for standard plates. Details unless noted otherwise. Refer to drawings 1604-27 for standard plates. Drawing or cover page listing this drawing and notes shall be used for any structure. The responsibility of the building designer per ANSI/TP1 Sec 2. For more information see the responsible page ITW-BCG www.itwbcg.com www.print.org WTC www.structure.com

ALPINE

ITW Building Components Group Inc.
Orlando FL 32837
FL COA #0278



11/22/2013

QTY: 1		FL/-/5/-/-/R/-		Scale = .3125"/Ft.	
TC LL	20 0 PSF	REF	R9114- 28449		
TC DL	7 0 PSF	DATE	11/22/13		
BC DL	10.0 PSF	DRW	HCSR9114 13326022		
BC LL	0.0 PSF	HC-ENG	AP/AP		
TOT.LD.	37.0 PSF	SEQN-	339601		
DUR.FAC.	1.25				
SPACING	24.0"	JREF	1V1J487_Z01		

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

130 mph wind, 15 00 ft mean hgt ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf $G C_p (+/-) = 0.18$

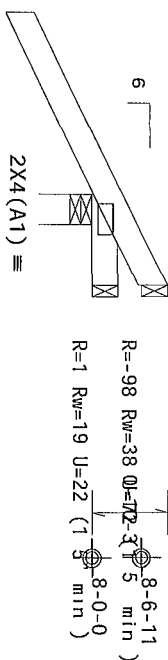


Diagram of a continuous beam with three supports. The beam is divided into two equal spans of 20'-0" each. The total length is 40'-0". The beam is labeled "Over 3 Supports". The beam is supported by three vertical supports. The beam is labeled "R=326 U=75 W=4 (4" min)" and "R1=29/-24".

PLT TYP Wave

Design Crit.	FBC2010Res/TP1-2007(STD)
	FT/RT=10%(0%)/0(0)

12.03.04 15:06 D13 QTY:8

QTY:8 FL/-/5/-/-/R/-

Scale = .5" / Ft.

ALPINE

ITW Building Components Group Inc.

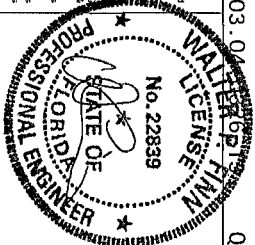
Orlando FL, 32837
FL COA #0278

****IMPORTANT** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS**

Trussing require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BGSI (Building Component Safety Information) from TP1 and WTCA for safety practice and prior to performing these functions. Installers shall provide temporary bracing per BGSI. Trusses should otherwise be top chord shall have properly attached structural sheathing and bottom chord shall have three inch installed per BGSI sections 85, 87 or 810. No other bracing is applicable.

117B Build on Components Ground line (117B/CG) shall not be responsible for any deviation from this design. Failure to build the truss in conformance with ANSI/TP1 1 or for handing or shipping installation & bracing of Trusses. Apply plates to each face of truss and post or as shown above and on the Joint. Details unless noted otherwise. Refer to drawings 180A-Z for standard plate positions. A seal on this drawing or cover page listing the drawing indicates acceptance of process and engineering. The responsible party of the Build ng Designer per ANSI/TP1 1 Sec 2. For more information see the general notes page 117B-CGS. www.ctbdcg.com www.tp1.com www.abcdindustry.com www.wtca.com

ICC www.iccgate.org



11/22/2013

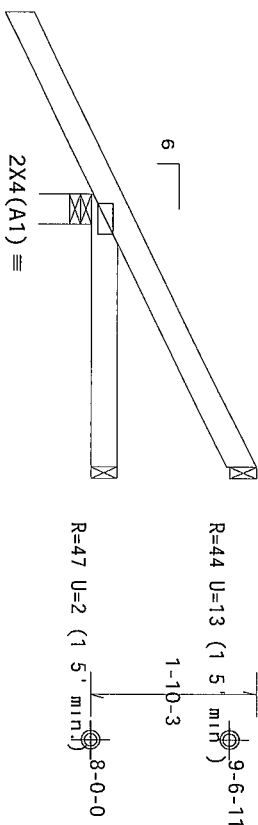
TC LL	20.0 PSF	REF R9114- 28450
TC DL	7.0 PSF	DATE 11/22/13
BC DL	10.0 PSF	DRW HCUR9114 13326023
BC LL	0.0 PSF	HC-ENG AP/AP
TOT.LD	37.0 PSF	SEQN- 339603
DUR.FAC.	1 25	
SPACING	24.0"	JREF- 1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCpi(+/-)=0 18

Wind loads and reactions based on MNFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50



2-0-0-0

3-0-0 Over 3 Supports

R=288 U=35 W=4' (4' min)
RL=48/-28

PLT TYP. Wave

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)
-------------	--

12.03.04.0326.13

QTY:4 FL/-/5/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.

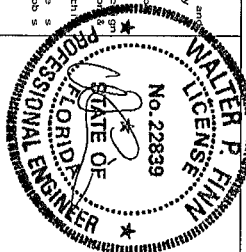
Orlando FL, 32837
FL COA #0278

****IMPORTANT****
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

These require care in fabricating, handling, splicing, installing, and testing. Refer to applicable sections of the following documents for more information:

- *Building Component Safety Information* by TPI and WIDA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI practices noted elsewhere as top chord shall have properly attached structural sheath and bottom chord shall have a properly attached rigid ceiling. Loose ends shown for permanent lateral restraint or wind shall have been installed per BCSI sections 83, 87 or 810 as apply cable

ITW Bu Id ng Components Group Inc (ITWBCG) shall not be responsible for any discrepancy from this contract
any failure to bu Id the truss in conformance with ANSI/TPI-1 or for handling shipping installation or
bracing of trusses. Applied plates to each face of truss and position as shown above and on the Jo int
beats is unless noted otherwise. Refer to drawings 1804-2 for standard girth plate notes. A seal on the
truss is unless noted otherwise. This seal shall be the responsibility of professional engineer and member
of the responsible b l ty of the Bu Id ng Dis gn per ANSI/TPI-1 Section 2. For more information on seal
general practices refer ITW BCG www.itwbcg.com TPI www.tpi.com WTCA www.abctindustry.com
CCC www.cccsf.org



11/22/2013

TC LL	20 0 PSF	REF	R9114- 28451
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCSR9114 13326024
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT. LD.	37 0 PSF	SEQN-	339703
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

(13-281--BRYAN ZECHER /Columbia County Builders -- Lake City, FL - CJS 5 Jack)

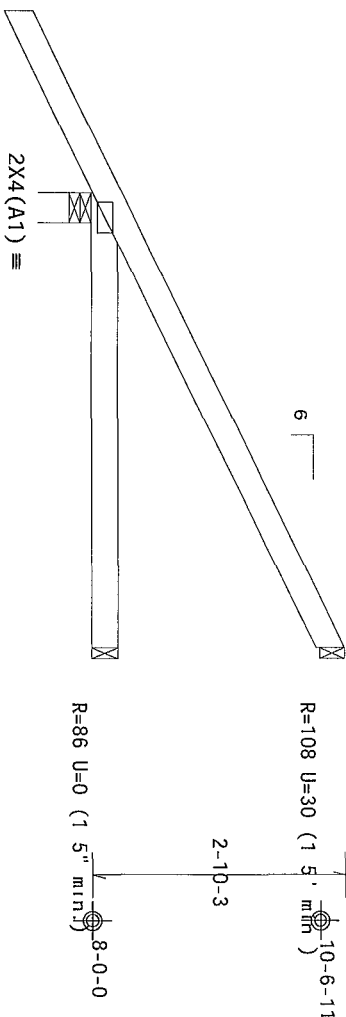
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

Bottom chord checked for 10 00 psf non-concurrent live load

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf,
wind BC DL=5 0 psf GCPI (+/-)=0 18
Wind loads and reactions based on MWFRS with additional C&C member
design
Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50



2-0-0
5-0-0 Over 3 Supports
R=343 U=32 W=4" (4" min)
RL=67/-32

PLT TYP Wave

Design Crit FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

12.03.04 0226 13

QTY: 4 FL/-/5/-/-/R/-

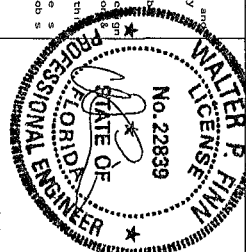
Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL 32837
FL COA #0278

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET**
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTC) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI section 83 B7 or B10 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any device or from this design any failure to build the truss in conformance with ANSI/TPI 1 or for handling, shipping, installing, bracing, or erecting the truss. Refer to drawings 160A-2 for standard practices. A seal on the drawing or cover page listing this drawing and the acceptance of professional engineering and the responsibility of the Building Designer per ANSI/TPI 1 Section 2. For more information see the general notes page ITW-200 www.itwbcg.com www.tpiinc.org WTC www.sbcindustry.com IBC www.international.org



TC LL	20.0 PSF	REF	R9114- 28452
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	H05R9114 13326025
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT. LD.	37.0 PSF	SEQN-	339691
DUR. FAC.	1 25		
SPACING	24.0"	JREF	1V1J487_Z01

11/22/2013

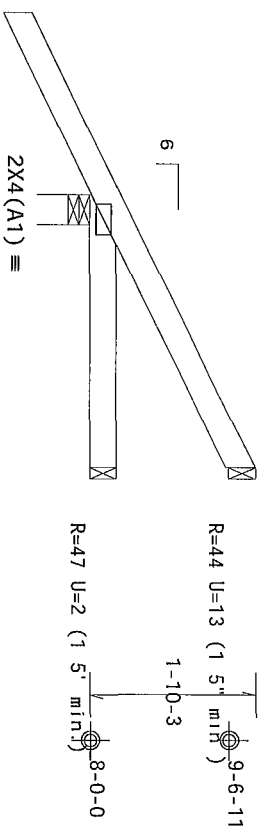
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, EXP B, wind TC DL=3 5 psf, wind BC

$$DL=5.0 \text{ psf} \quad GC_{PI} (+/-)=0.18$$

Wind loads and reactions based on MMFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50



R=288 U=35 W=4" (4' min)
RL=48/-28

PLT TYP	Wave
---------	------

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)
1.0	1.0
2.0	2.0
3.0	3.0
4.0	4.0
5.0	5.0
6.0	6.0
7.0	7.0
8.0	8.0
9.0	9.0
10.0	10.0
11.0	11.0
12.0	12.0
13.0	13.0
14.0	14.0
15.0	15.0
16.0	16.0
17.0	17.0
18.0	18.0
19.0	19.0
20.0	20.0
21.0	21.0
22.0	22.0
23.0	23.0
24.0	24.0
25.0	25.0
26.0	26.0
27.0	27.0
28.0	28.0
29.0	29.0
30.0	30.0
31.0	31.0
32.0	32.0
33.0	33.0
34.0	34.0
35.0	35.0
36.0	36.0
37.0	37.0
38.0	38.0
39.0	39.0
40.0	40.0
41.0	41.0
42.0	42.0
43.0	43.0
44.0	44.0
45.0	45.0
46.0	46.0
47.0	47.0
48.0	48.0
49.0	49.0
50.0	50.0
51.0	51.0
52.0	52.0
53.0	53.0
54.0	54.0
55.0	55.0
56.0	56.0
57.0	57.0
58.0	58.0
59.0	59.0
60.0	60.0
61.0	61.0
62.0	62.0
63.0	63.0
64.0	64.0
65.0	65.0
66.0	66.0
67.0	67.0
68.0	68.0
69.0	69.0
70.0	70.0
71.0	71.0
72.0	72.0
73.0	73.0
74.0	74.0
75.0	75.0
76.0	76.0
77.0	77.0
78.0	78.0
79.0	79.0
80.0	80.0
81.0	81.0
82.0	82.0
83.0	83.0
84.0	84.0
85.0	85.0
86.0	86.0
87.0	87.0
88.0	88.0
89.0	89.0
90.0	90.0
91.0	91.0
92.0	92.0
93.0	93.0
94.0	94.0
95.0	95.0
96.0	96.0
97.0	97.0
98.0	98.0
99.0	99.0
100.0	100.0

12 03.04 0326 13

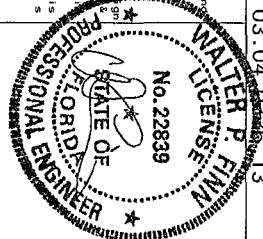
QTY:4 FL/-/5/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

[illegible]

TC LL	20.0 PSF	REF	R9114- 28453
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCUSR9114 13326026
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD	37.0 PSF	SEQN-	339703
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

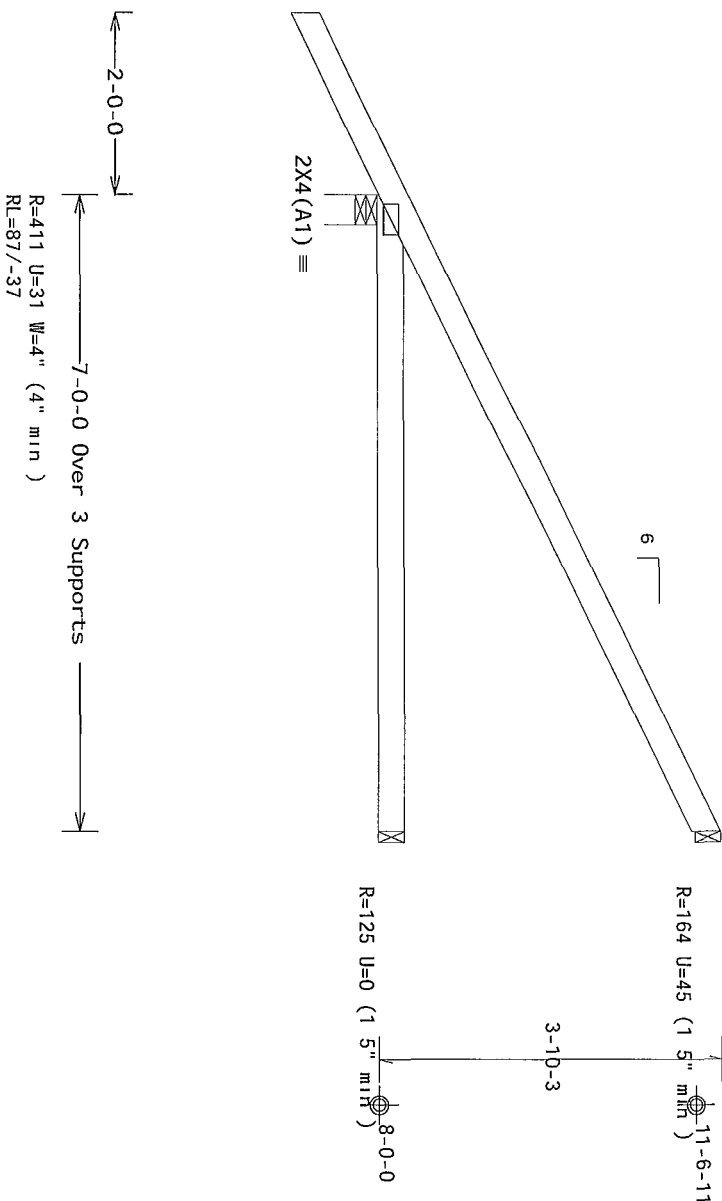
Bottom chord checked for 10 00 psf non-concurrent live load

MINFRS loads based on trusses located at least 7 50 ft from roof edge

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, located anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf Gcpl(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Deflection meets $L/240$ live and $L/180$ total load Creep increase factor for dead load is 1.50



PLT TYP Wave

Design Crit	FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)	

12.03 04 0326 13

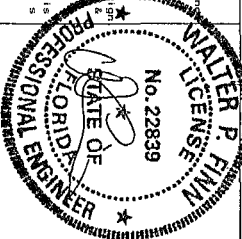
QTY-8 FL/-/5/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc

Orlando FL, 32837
FL COA #0278

[illegible]

11/22/2013

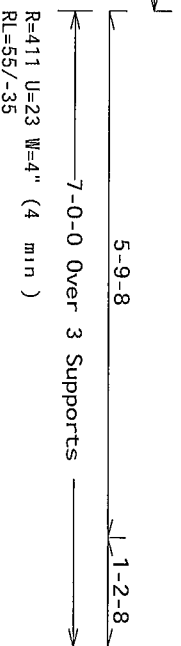
TC LL	20 0 PSF	REF	R9114- 28454
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCUSR9114 13326027
BC LL	0 0 PSF	HC-ENG	AP/AP
TOT LD.	37 0 PSF	SEON-	339602
DUR FAC.	1 25		
SPACING	24.0"	JREF -	1VJ487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Deflection meets factor for dead load is	L/240 live and total load is 1 50	Creep increase

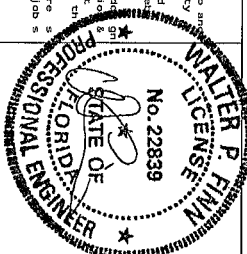


Scale = 5"/Ft.

INTER P. F.

ITV Building Components Group Inc

Orlando FL, 32837
FL COA #0278



11/22/2013

TC LL	20 0 PSF	REF	R9114 - 28455
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	HCUSR9114 13326005
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD	37 0 PSF	SEQN-	339857
DUR FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

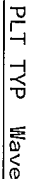
THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCp1(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

factor for dead load is 1.50



Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)
-------------	--

12 03 04 05 06 07 08 09 10 11 12 13

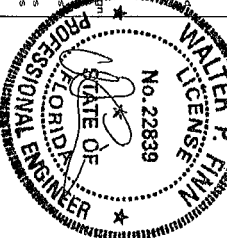
QTY 2 FL/-/5/-/-/R/-

Scale = 5"/Ft

ALPINE

ITW Building Components Group Inc
Orlando FL, 32837
FL COA #0278

*****IMPORTANT***** **WARNING***** **READ AND FOLLOW ALL NOTICES ON THIS SHEET**
 Transusers require extreme care in fabricating and handling metalizing and brazing
 Follow the latest edition of BCSI (Butling Component Safety Information by TPI and WITCA) for safety
 practices per or to perform these functions. Installers shall provide temporary bracing per BCSI
 Unless noted otherwise, all third shall have properly attached structural sheathing and bottom chord
 shall have been properly installed per the following:
 1. All third shall be installed per BCSI Section 83, B7 or B10 as applicable.
 2. Butling and Component Groups Inc. (ITMBG) shall not be responsible for any design or from this sheet
 any failure to butling this trans in conformance with ANSI/TPI 1 or for handling or shipping metalizing
 draws 15 unless noted otherwise. Refer to draws 1604-7 for additional plates above and on the job site.
 drawing or cover plates indicating this drawing and all notices accordance of professional engineering on the
 responsible. It is solely for the design shown. The submittal and use of this design for any structure is
 the responsibility of the butling design firm. per ANSI/TPI 1 Sec 2. For more information on see This job is
 general notes page. ITMBG www.itmbg.com TPI www.tpinet.org WITCA www.sbc-industry.com
 CC www.itscale.org

~~11/22/2013~~

TC LL	20.0 PSF	REF	R9114- 28456
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	HCUSR9114 13326006
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD.	37.0 PSF	SEQN-	339694
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

(13-281--BRYAN ZECHER /Columbia County Builders -- Lake City FL - EJ7B 7 End Jack)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with '13B' use design values approved
1/30/2013 by ALSC

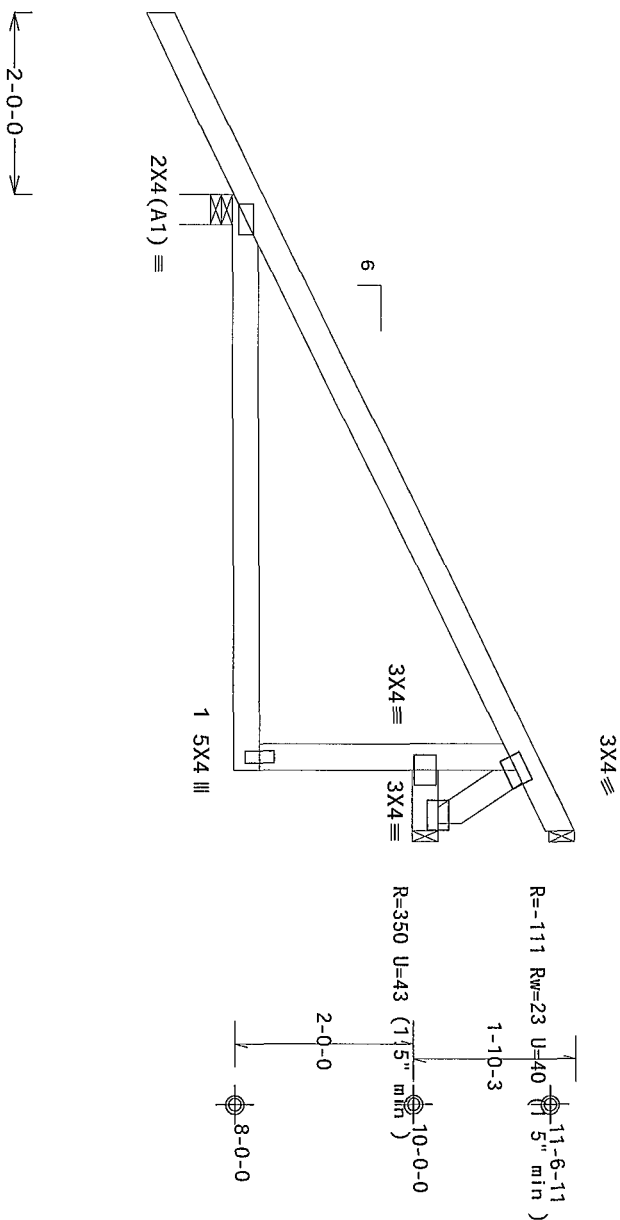
Bottom chord checked for 10 00 psf non-concurrent live load

MMFRS loads based on trusses located at least 7 50 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCpl(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member
design

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50



R=411 U=14 W=4' (4" min)
RL=74/-31

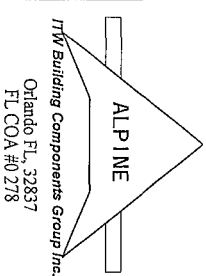
PLT TYP Wave

Design Crit FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

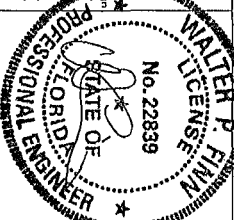
12 03 04 0906-13

QTY 9 FL/-/5/-/-/R/-

Scale = 5"/Ft.



****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information) by TPI and WFOA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of web shall have bracing installed per BCSI sections B3, B7 or B10 as applicable.
TPI Building Components Group Inc. (TPI/BCG) shall not be responsible for any deviation from this design and specifications. The user shall verify the design and specifications are correct for the intended application. The user shall verify the design and specifications are correct for the intended application. The user shall verify the design and specifications are correct for the intended application.
Data is unless noted otherwise. Refer to drawings 1604-Z for standard plate positions. A seal on the drawing or cover page at the end of this drawing. The seal shall verify and use of this design for any structure is the responsibility of the user. The seal shall verify and use of this design for any structure is the responsibility of the user. The seal shall verify and use of this design for any structure is the responsibility of the user.
TPI BCSI www.tpi.com www.bcsi.org WFOA www.wfoa.com



TC LL	20 0 PSF	REF R9114- 28457
TC DL	7 0 PSF	DATE 11/22/13
BC DL	10 0 PSF	DRW HCUR9114 13326007
BC LL	0 0 PSF	HC-ENG AP/AP
TOT LD	37 0 PSF	SEQN- 330594
DUR FAC.	1 25	
SPACING	24 0"	JREF- 1V1J487_Z01

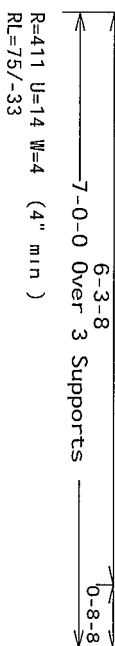
11/22/2013

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf Gcp1(+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase Factor for dead load is 1.50



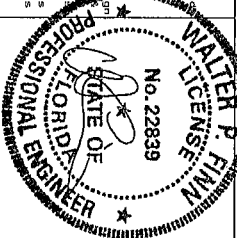
Scale = .5"/Ft

ITW Building Components Group Inc

Orlando FL, 32837
FL COA #0278

****IMPORTANT**** SUBMIT THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Tensun require extra time each in fabric cat rig handling installing and brace ng Refer to and follow the latest edition of BCS(But all Component Safety Information on by TPI and WTCO) for safety practices prior to performing these functions Installers shall provid temporary brace ng per BS6079. If these are noted otherwise, no other shall have properly attended structural sheathing and bracing thereof shall have been meeting installed per BS21 sections B3, B7 or B10 as applicable

All Tensun Components Group Inc. (TMBCOG) shall not be responsible for any damage to any fabric due to build it the truss in conformance w/ ANSI/TPI 1 or for hand ng shi pping installation brace ng of trusses Apply policies to each face of truss and pos t on as shown above and on the Joint brace ns unless noted otherwise Refer to draw ngs TBKA-Z for standard plate pos tions A seal on thins drawing or cover page listing thins in drawing not cases acceptance of profess ionals and peer ng review of drawings by design engineer shall constitute final approval of drawings for construction This Job is the responsibility of the Build ng Designer per ANSI/TPI 1 Section 2 For more information see WTCO general notes page 1TW-BGCG www twbgcg com TPI www tpiinc org WTCO www abc industry com CCC www cccscale org



TC LL	20 0 PSF	REF	R9114 - 28458
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	H05R9114 13326008
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD.	37.0 PSF	SEQN-	330432
DUR FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

~~11/22/2013~~

Top chord 2x4 SP #1-13B T1 2x4 SP 2850F-2 3E
Bot chord 2x4 SP #1-13B B2 2x4 SP M-30
Webs 2x4 SP #3-13B
W2 W3, W4, W11, W12 2x4 SP #2-13B
Lt Slider 2x4 SP #3-13B BLOCK LENGTH = 1 500'

Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

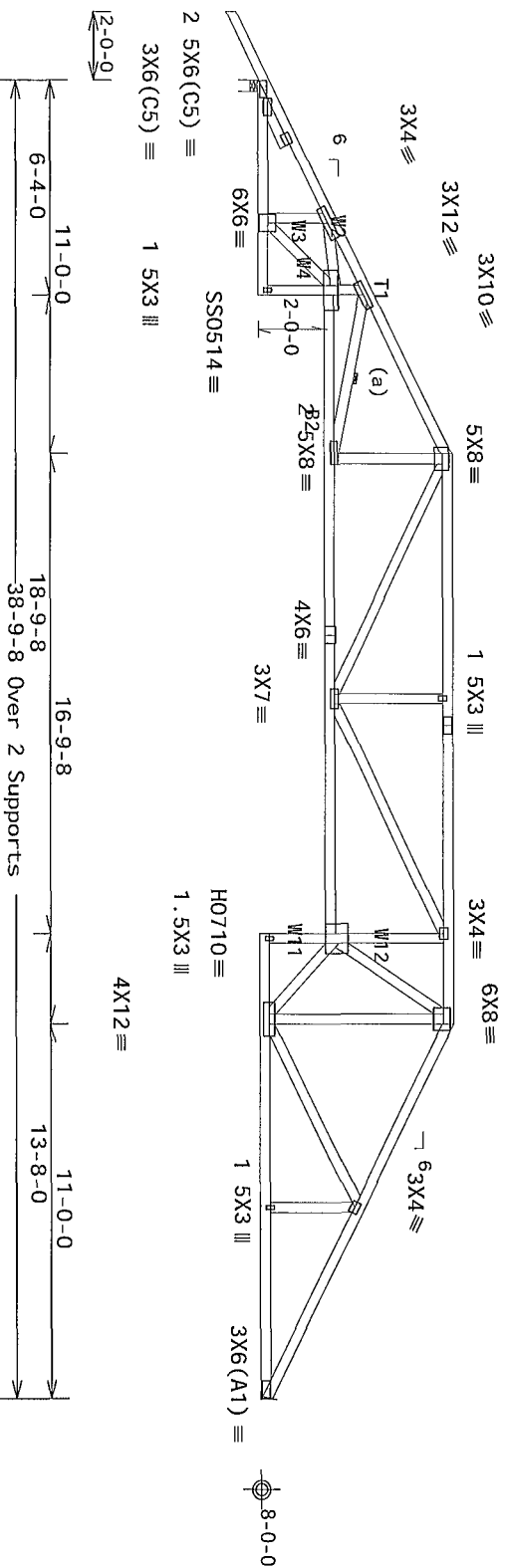
H = recommended connection based on manufacturer tested capacities and
calculations Conditions may exist that require different connections
than indicated Refer to manufacturer publication for additional
information

These support conditions used at bearings indicated
(H1) = HUS26 w/ (1)2x6 SP M-26 supporting member
(14) 0 148 x3" nails into supporting member,
(4) 0 148 x3 nails into supported member

Bottom chord checked for 10 00 psf non-concurrent live load
Calculated vertical deflection is 0 46" due to live load and 0 60' due
to dead load at X = 18-2-8

3X6

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf,
wind BC DL=5 0 psf GCp1 (+/-)=0 18
Wind loads and reactions based on MMFRS with additional C&C member
design
Calculated horizontal deflection is 0 28" due to live load and 0 37"
due to dead load
Hanger specified assumes connection to supporting chord is located a
minimum of five times the depth of the supporting chord from any
unsupported end, unless unsupported chord end has 85% plating
coverage
(a) Continuous lateral restraint equally spaced on member
In lieu of structural panels use purlins to brace all flat TC @ 24"
Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50
MMFRS loads based on trusses located at least 7 50 ft from roof edge



R=1598 U=73 W=4" (4' min)
RL=96/-104

R=1457 U=54
H=H1

PLT TYP 20 Gauge HS, 18 Gauge HS, Design Crit FBC2010Res/TP1-2007(STD)
Wave FT/RT=10%(0%/0/0)

12 03 04 2013

QTY. 1 FL/-/5/-/1/R/-

Scale = .1875"/Ft.

IMPORTANT READ AND FOLLOW ALL NOTES ON THIS SHEET! FURNISH THIS DESIGN TO ALL CONTRACTORS, INCLUDING INSTALLERS			
Trusses require extreme care in fabrication, handling, shipping, and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety) Information by TPI and WTC for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r/g d ceiling. Leave ends shown for permanent lateral restraint of webs. Shall have bracing installed per BCSI (sect 83 B7 or B10 as appl) cable.			
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from the design or any failure to build in accordance with ANSI/TPI 1 or for handling, shipping, installing, or bracing. Does not constitute a warranty. Refer to drawings 1604-Z for standard plate positions. A seal on the drawing or cover page 1 at the time of drawing indicates acceptance of professional engineering responsibility by the designer shown. The suitability and use of this design for any structure is the responsibility of the building designer per ANSI/TPI 1 Sec 2. For more information on the general notes, please refer to the ITWBCG website: www.itwbcg.com. This is Job's general notes page. ITWBCG www.itwbcg.com TPI www.tpi.net or WTC www.wtc-industry.com (C) www.itwbcg.com			
ALPINE ITW Building Components Group Inc. Orlando, FL 32837 FL COA #0278			
		DATE 11/22/13	
TC DL		20.0 PSF	REF R9114-28459
BC DL		7.0 PSF	DATE 11/22/13
BC LL		10.0 PSF	DRW HCUR9114 13326009
TOT LD		0.0 PSF	HC-ENG AP/AP
DUR. FAC		37 0 PSF	SEQN- 330718
SPACING		24.0"	JREF- 1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCP1 (+/-)=0 18

wind BC DL=5 0 psf GCp1 (+/-)=0 18

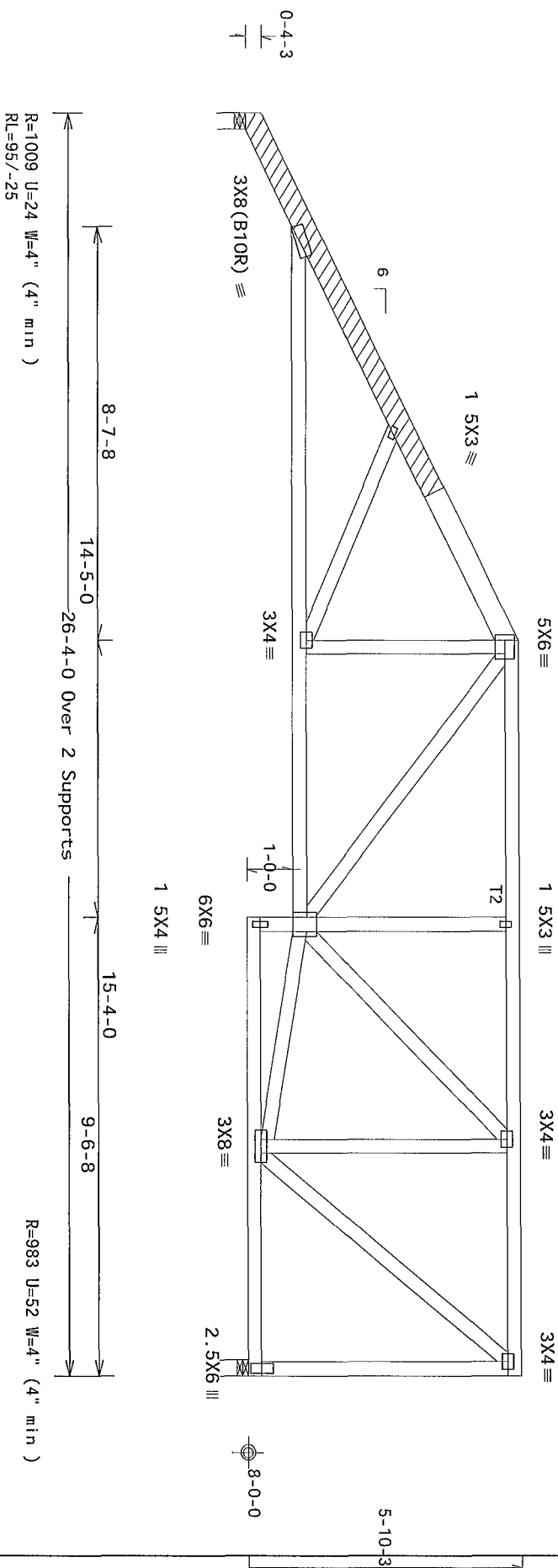
Wind loads and reactions based on MMFRS with additional C&C member design

Right end vertical not exposed to wind pressure

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

MMFRS loads based on trusses located at least 7 50 ft from roof edge

(1) 2x6x8-10-9 SP M-26 scab at left end Attach scab to face of chord with 0 131 x3', min nails @ 8" OC, plus additional nail clusters at BRG (4), heel (6), 1st panel point (2)



Scale = .3125"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

••IMPORTANT•• FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trustees require certificate from a fabricating shop handling all piping, steeling and bracing. Refer to and follow the latest edition of BCSI (Bu id ing Component Safety Information on by TPI and WITPA) for safety practices noted or to perform any these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, no top chord shall have properly attached structural bracing and bottom chord shall have a properly attached r and a long. Least one shown for permanent lateral restraint or webs shall have bracing installed per BCSI sect ions 83 B7 or B10 as appl cable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any delay on from this date.

bracing of trusses. Apply plates to each face of truss and post on as shown above and on the joint any in line to build the truss in conformance with ANSI/APA 1 of for nailing spacing on &

Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on the draw no at cover page list no this draw no nd gates acceptance of protest onal and near no

The suitability and use of this design for any structure is responsibility solely for the design shown.

the responsibility of the building designer per ANSI/AP1-1 sec 2 for more info on see this joo s

general notes page 1TW BCG www twbcg com TP1 www tp nst org WTCA www sbc industry com

ICC www.ccsafe.org

STATE OF

FIELD

CHIP

ADDITIONAL

● 2016年1月1日起实施

11

BC LL 00 H

TOT ID 37 0 F

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52	53	54	55	56	57	58	59	60	61	62	63	64	65	66	67	68	69	70	71	72	73	74	75	76	77	78	79	80	81	82	83	84	85	86	87	88	89	90	91	92	93	94	95	96	97	98	99	100	101	102	103	104	105	106	107	108	109	110	111	112	113	114	115	116	117	118	119	120	121	122	123	124	125	126	127	128	129	130	131	132	133	134	135	136	137	138	139	140	141	142	143	144	145	146	147	148	149	150	151	152	153	154	155	156	157	158	159	160	161	162	163	164	165	166	167	168	169	170	171	172	173	174	175	176	177	178	179	180	181	182	183	184	185	186	187	188	189	190	191	192	193	194	195	196	197	198	199	200	201	202	203	204	205	206	207	208	209	210	211	212	213	214	215	216	217	218	219	220	221	222	223	224	225	226	227	228	229	230	231	232	233	234	235	236	237	238	239	240	241	242	243	244	245	246	247	248	249	250	251	252	253	254	255	256	257	258	259	260	261	262	263	264	265	266	267	268	269	270	271	272	273	274	275	276	277	278	279	280	281	282	283	284	285	286	287	288	289	290	291	292	293	294	295	296	297	298	299	300	301	302	303	304	305	306	307	308	309	310	311	312	313	314	315	316	317	318	319	320	321	322	323	324	325	326	327	328	329	330	331	332	333	334	335	336	337	338	339	340	341	342	343	344	345	346	347	348	349	350	351	352	353	354	355	356	357	358	359	360	361	362	363	364	365	366	367	368	369	370	371	372	373	374	375	376	377	378	379	380	381	382	383	384	385	386	387	388	389	390	391	392	393	394	395	396	397	398	399	400	401	402	403	404	405	406	407	408	409	410	411	412	413	414	415	416	417	418	419	420	421	422	423	424	425	426	427	428	429	430	431	432	433	434	435	436	437	438	439	440	441	442	443	444	445	446	447	448	449	450	451	452	453	454	455	456	457	458	459	460	461	462	463	464	465	466
---	---	---	---	---	---	---	---	---	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----	-----

DUR.FAC. 1.25

CDAC IAWC 24 0"

SPACING 24.0

HC-ENG AP/AP

SFON- 330487

0	0
1	0
2	0
3	0
4	0
5	0
6	0
7	0
8	0
9	0
10	0
11	0
12	0
13	0
14	0
15	0
16	0
17	0
18	0
19	0
20	0
21	0
22	0
23	0
24	0
25	0
26	0
27	0
28	0
29	0
30	0
31	0
32	0
33	0
34	0
35	0
36	0
37	0
38	0
39	0
40	0
41	0
42	0
43	0
44	0
45	0
46	0
47	0
48	0
49	0
50	0
51	0
52	0
53	0
54	0
55	0
56	0
57	0
58	0
59	0
60	0
61	0
62	0
63	0
64	0
65	0
66	0
67	0
68	0
69	0
70	0
71	0
72	0
73	0
74	0
75	0
76	0
77	0
78	0
79	0
80	0
81	0
82	0
83	0
84	0
85	0
86	0
87	0
88	0
89	0
90	0
91	0
92	0
93	0
94	0
95	0
96	0
97	0
98	0
99	0

INDEX 1V/1 1A07 701

JKET- 1V1J481_201

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Bottom chord checked for 10 00 psf non-concurrent live load

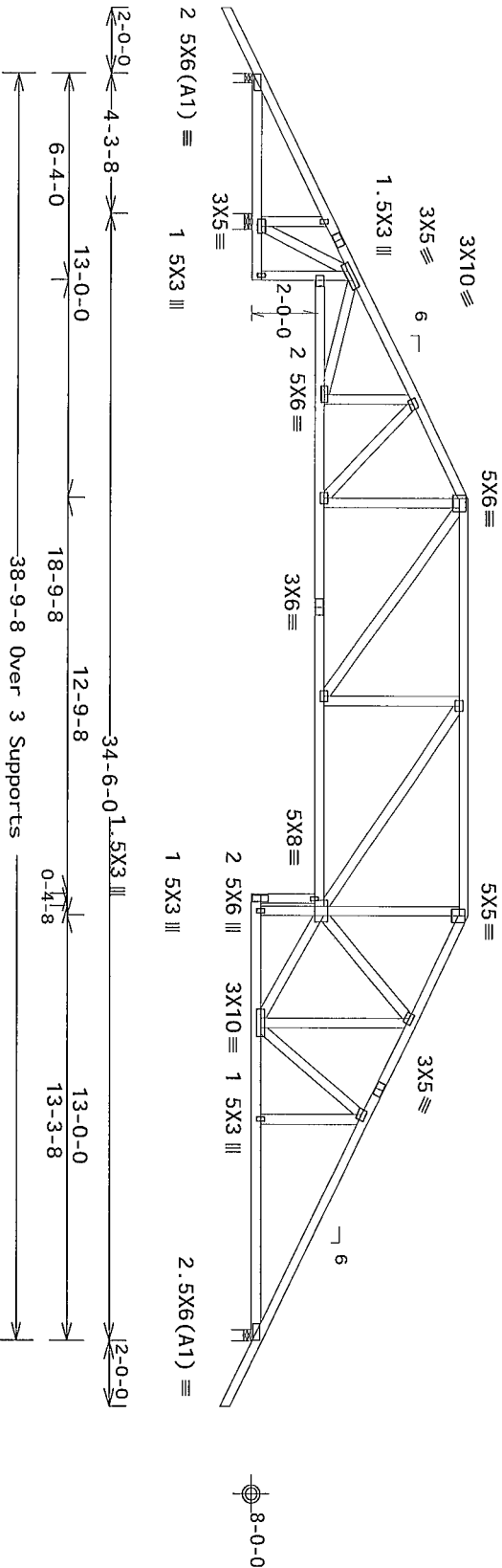
MMFRS loads based on trusses located at least 15 00 ft from roof edge

Negative reaction(s) of -303# MAX (See below) from a non-wind load case requires uplift connection

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI (+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



R=128/-304 U=133 W=3 5" (3 5" min)
RL=131/-131 R=1921 U=0 W=5 657" (5 657" min)

R=1362 U=0 W=4" (4" min)

Note All Plates Are 3x4 Except As Shown

PLT TYP Wave Design Crit FBC2010Res/TPI-2007(STD)
FT/RT=10%(0%)/0(0)

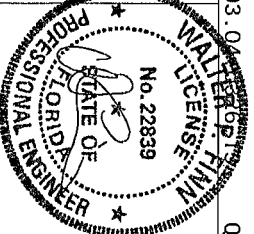
QTY: 1 FL/-/5/-/-/R/- Scale = .1875"/Ft

ALPINE

ITW Building Components Group Inc.

Orlando FL 32837
FL COA #0278

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, shipping, handling, erecting and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information by TPI and WTC) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections 83, 87 or 810 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or for any damage to the truss or its components due to improper handling, shipping, erecting, installation, or use. The user of this design shall be responsible for obtaining all necessary permits and for ensuring that the design is used in accordance with the applicable building code and all other applicable regulations. The user shall also be responsible for obtaining all necessary permits and for ensuring that the design is used in accordance with the applicable building code and all other applicable regulations. The user shall also be responsible for obtaining all necessary permits and for ensuring that the design is used in accordance with the applicable building code and all other applicable regulations.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or for any damage to the truss or its components due to improper handling, shipping, erecting, installation, or use. The user of this design shall be responsible for obtaining all necessary permits and for ensuring that the design is used in accordance with the applicable building code and all other applicable regulations. The user shall also be responsible for obtaining all necessary permits and for ensuring that the design is used in accordance with the applicable building code and all other applicable regulations. The user shall also be responsible for obtaining all necessary permits and for ensuring that the design is used in accordance with the applicable building code and all other applicable regulations.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or for any damage to the truss or its components due to improper handling, shipping, erecting, installation, or use. The user of this design shall be responsible for obtaining all necessary permits and for ensuring that the design is used in accordance with the applicable building code and all other applicable regulations. The user shall also be responsible for obtaining all necessary permits and for ensuring that the design is used in accordance with the applicable building code and all other applicable regulations. The user shall also be responsible for obtaining all necessary permits and for ensuring that the design is used in accordance with the applicable building code and all other applicable regulations.



TC LL	20 0 PSF	REF R9114- 28461
TC DL	7 0 PSF	DATE 11/22/13
BC DL	10 0 PSF	DRW HOURS9114 13326011
BC LL	0 0 PSF	HC-ENG AP/AP
TOT LD	37 0 PSF	SEQN- 330663
DUR FAC	1.25	
SPACING	24.0"	JREF- 1V1J487_Z01

Top chord 2x6 SP M-26 T2 2x6 SP #2-13B
T3 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

Calculated horizontal deflection is 0.11' due to live load and 0.15
due to dead load

Bottom chord checked for 10.00 psf non-concurrent live load

(1) 2x6x10-1-15 SP M-26 scab at left end Attach scab to face of chord
with 0.131"x3, min nails @ 8" OC, plus additional nail clusters at
BRG (4), heel (5), 1st panel point (2)

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 9.00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf,
wind BC DL=5.0 psf GCPI(+/-)=0.18

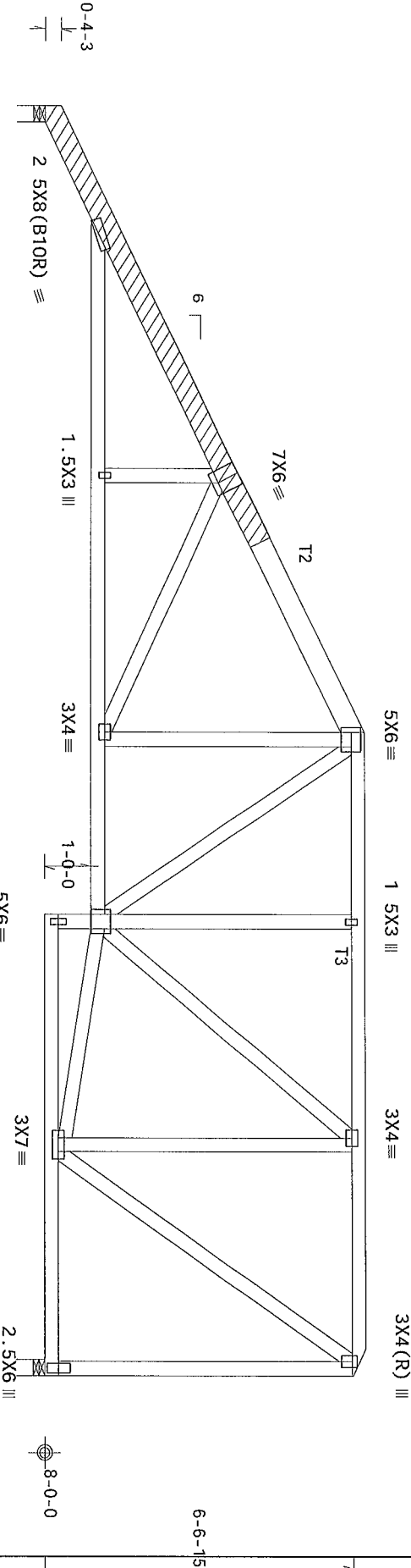
Wind loads and reactions based on MMFRS with additional C&C member
design

Right end vertical not exposed to wind pressure

In lieu of structural panels use purlins to brace all flat TC @ 24"
OC

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1.50

MMFRS loads based on trusses located at least 7.50 ft from roof edge



R=1009 U=19 W=4" (4 min)
RL=111/-28
R=983 U=46 W=4" (4 min)

PLT TYP. Wave

Design Crit FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%/0(0))

12.03.04.08.06.13

QTY 1 FL/-/5/-/-/R/-

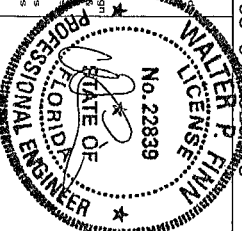
Scale = .3125"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL 32837
FL COA #0278

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS.
Trusses from the extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of the BCSI (Building Components Safety Information) by TPI and WTC for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Loads shown for permanent lateral resistance of web shall have bracing installed per BCSI section 83.87 or 81.0 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design. Any deviation to build the truss in conformance with ANSI/TP1-1 or for handling, shipping, installing, bracing, or any other function shall be the responsibility of the contractor. A seal on the drawings indicates acceptance of professional practice and use of this design for any structure is the responsibility of the building designer per ANSI/TP1-1 Section 2. For more information see this job's general notes page ITW BCG www.bcgeng.com TPI www.tpi.net or WTC www.sbcindustry.com This job's IBC www.icbcare.org



TC LL	20.0 PSF	REF	R9114-28462
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCSR9114 13326012
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD	37.0 PSF	SEQN	330493
DUR FAC.	1.25		
SPACING	24.0"	JREF	1V1J487_Z01

11/22/2013

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with '13B' use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

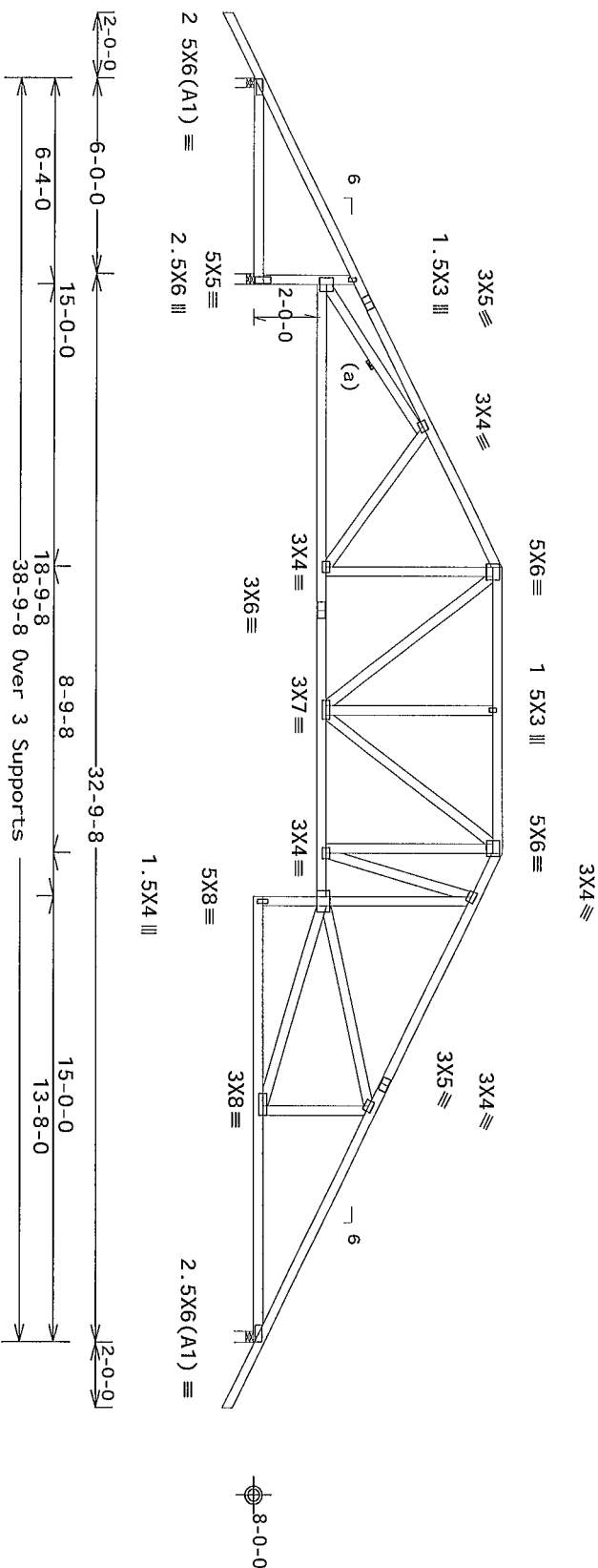
Bottom chord checked for 10 00 psf non-concurrent live load

MFIRS loads based on trusses located at least 15 00 ft from roof edge

Wind loads and reactions based on MMFRS with additional C&C member design

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets $L/240$ live and $L/180$ total load Creep increase factor for dead load is 1.50



R=364 U=32 W=3 5' (3 5' min)
RL=146/-146 R=1454 U=0 W=4' (4' min)

R=1361 U=0 W=4' (4" min)

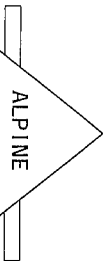
PLT TYP Wave

Design Crit	FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)	

12 03 04 0226 13

QTY 1 FL/-/5/-/-/R/-

Scale = 1875"/Ft.



ITW Building Components Group Inc.
Orlando FL, 32837
FL COA #0278

••IMPORTANT•• FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to any follow the latest edition of BCST (Building Construction Safety Council) and TDI (Tool and Die) for more information.

practicing prior to performing these functions. Installers shall provide temporary bracing per BCSI

shall have a properly attached rigid ceiling locations shown for permanent lateral restraint of web.

TW Bu Idm Component Group Inc (TWBCG) shall not be responsible for any damage or loss sustained by the customer.

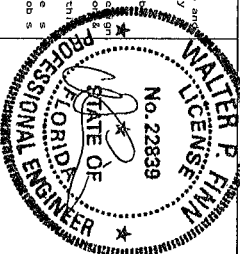
any failure to build the cruss in conformance with ANSI/TPI 7 or for handling shipping and installation.

Details unless noted otherwise Refer to drawing 160A-Z for standard plate positions A seal on this

The suitability and use of the design for any structure is responsibily solely for the design shown

general notes page 17M BCG www twbcg com TPI www tp nst org WTCA www sbc industry com

ICC www.ccsafe.org



TC LL	20 0 PSF	REF	R9114 - 28463
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCUSR9114 13326013
BC LL	0 0 PSF	HC-ENG	AP/AP
TOT LD	37.0 PSF	SEQN-	330427
DUR FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

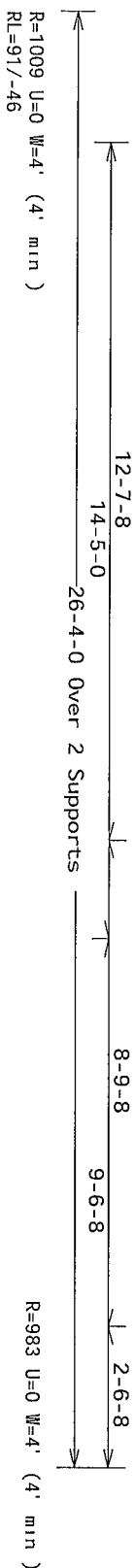
Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC

Calculated horizontal deflection is 0.12 due to live load and 0.16 due to dead load

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

(1) 2x6x11-3-6 SP M-26 scab at left end Attach scab to face of chord with 0 131 x3 min nails @ 8 OC plus additional nail clusters at BRG (4) heel (5) 1st panel point (2)

MMFRS loads based on trusses located at least 15 00 ft from roof edge



PLT TYP Wave

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)
-------------	--

12 03 04 05 06 07 08 09 10 11 12 13 14 15 16 17 18 19 20 21 22 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49 50 51 52 53 54 55 56 57 58 59 60 61 62 63 64 65 66 67 68 69 70 71 72 73 74 75 76 77 78 79 80 81 82 83 84 85 86 87 88 89 90 91 92 93 94 95 96 97 98 99 100 101 102 103 104 105 106 107 108 109 110 111 112 113 114 115 116 117 118 119 120 121 122 123 124 125 126 127 128 129 130 131 132 133 134 135 136 137 138 139 140 141 142 143 144 145 146 147 148 149 150 151 152 153 154 155 156 157 158 159 160 161 162 163 164 165 166 167 168 169 170 171 172 173 174 175 176 177 178 179 180 181 182 183 184 185 186 187 188 189 190 191 192 193 194 195 196 197 198 199 200 201 202 203 204 205 206 207 208 209 210 211 212 213 214 215 216 217 218 219 220 221 222 223 224 225 226 227 228 229 230 231 232 233 234 235 236 237 238 239 240 241 242 243 244 245 246 247 248 249 250 251 252 253 254 255 256 257 258 259 260 261 262 263 264 265 266 267 268 269 270 271 272 273 274 275 276 277 278 279 280 281 282 283 284 285 286 287 288 289 290 291 292 293 294 295 296 297 298 299 300 301 302 303 304 305 306 307 308 309 310 311 312 313 314 315 316 317 318 319 320 321 322 323 324 325 326 327 328 329 330 331 332 333 334 335 336 337 338 339 340 341 342 343 344 345 346 347 348 349 350 351 352 353 354 355 356 357 358 359 360 361 362 363 364 365 366 367 368 369 370 371 372 373 374 375 376 377 378 379 380 381 382 383 384 385 386 387 388 389 390 391 392 393 394 395 396 397 398 399 400 401 402 403 404 405 406 407 408 409 410 411 412 413 414 415 416 417 418 419 420 421 422 423 424 425 426 427 428 429 430 431 432 433 434 435 436 437 438 439 440 441 442 443 444 445 446 447 448 449 450 451 452 453 454 455 456 457 458 459 460 461 462 463 464 465 466 467 468 469 470 471 472 473 474 475 476 477 478 479 480 481 482 483 484 485 486 487 488 489 490 491 492 493 494 495 496 497 498 499 500 501 502 503 504 505 506 507 508 509 510 511 512 513 514 515 516 517 518 519 520 521 522 523 524 525 526 527 528 529 530 531 532 533 534 535 536 537 538 539 540 541 542 543 544 545 546 547 548 549 550 551 552 553 554 555 556 557 558 559 560 561 562 563 564 565 566 567 568 569 570 571 572 573 574 575 576 577 578 579 580 581 582 583 584 585 586 587 588 589 590 591 592 593 594 595 596 597 598 599 600 601 602 603 604 605 606 607 608 609 610 611 612 613 614 615 616 617 618 619 620 621 622 623 624 625 626 627 628 629 630 631 632 633 634 635 636 637 638 639 640 641 642 643 644 645 646 647 648 649 650 651 652 653 654 655 656 657 658 659 660 661 662 663 664 665 666 667 668 669 670 671 672 673 674 675 676 677 678 679 680 681 682 683 684 685 686 687 688 689 690 691 692 693 694 695 696 697 698 699 700 701 702 703 704 705 706 707 708 709 710 711 712 713 714 715 716 717 718 719 720 721 722 723 724 725 726 727 728 729 730 731 732 733 734 735 736 737 738 739 740 741 742 743 744 745 746 747 748 749 750 751 752 753 754 755 756 757 758 759 760 761 762 763 764 765 766 767 768 769 770 771 772 773 774 775 776 777 778 779 780 781 782 783 784 785 786 787 788 789 790 791 792 793 794 795 796 797 798 799 800 801 802 803 804 805 806 807 808 809 810 811 812 813 814 815 816 817 818 819 820 821 822 823 824 825 826 827 828 829 830 831 832 833 834 835 836 837 838 839 840 841 842 843 844 845 846 847 848 849 850 851 852 853 854 855 856 857 858 859 860 861 862 863 864 865 866 867 868 869 870 871 872 873 874 875 876 877 878 879 880 881 882 883 884 885 886 887 888 889 890 891 892 893 894 895 896 897 898 899 900 901 902 903 904 905 906 907 908 909 910 911 912 913 914 915 916 917 918 919 920 921 922 923 924 925 926 927 928 929 930 931 932 933 934 935 936 937 938 939 940 941 942 943 944 945 946 947 948 949 950 951 952 953 954 955 956 957 958 959 960 961 962 963 964 965 966 967 968 969 970 971 972 973 974 975 976 977 978 979 980 981 982 983 984 985 986 987 988 989 990 991 992 993 994 995 996 997 998 999 1000 1001 1002 1003 1004 1005 1006 1007 1008 1009 1010 1011 1012 1013 1014 1015 1016 1017 1018 1019 1020 1021 1022 1023 1024 1025 1026 1027 1028 1029 1030 1031 1032 1033 1034 1035 1036 1037 1038 1039

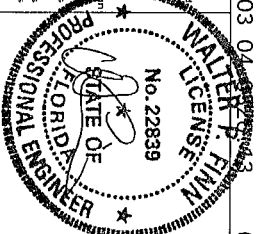
QTY 1 FL/-/5/-/-/R/-

Scale = 3125"/Ft

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

[illegible]~~11/22/2013~~

TC LL	20 0 PSF	REF R9114- 28464
TC DL	7 0 PSF	DATE 11/22/13
BC DL	10 0 PSF	DRW HCURS9114 13326014
BC LL	0.0 PSF	HC-ENG AP/AP
TOT LD	37 0 PSF	SEQN- 330501
DUR FAC.	1 25	
SPACING	24.0"	JREF- 1V1J487_Z01

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

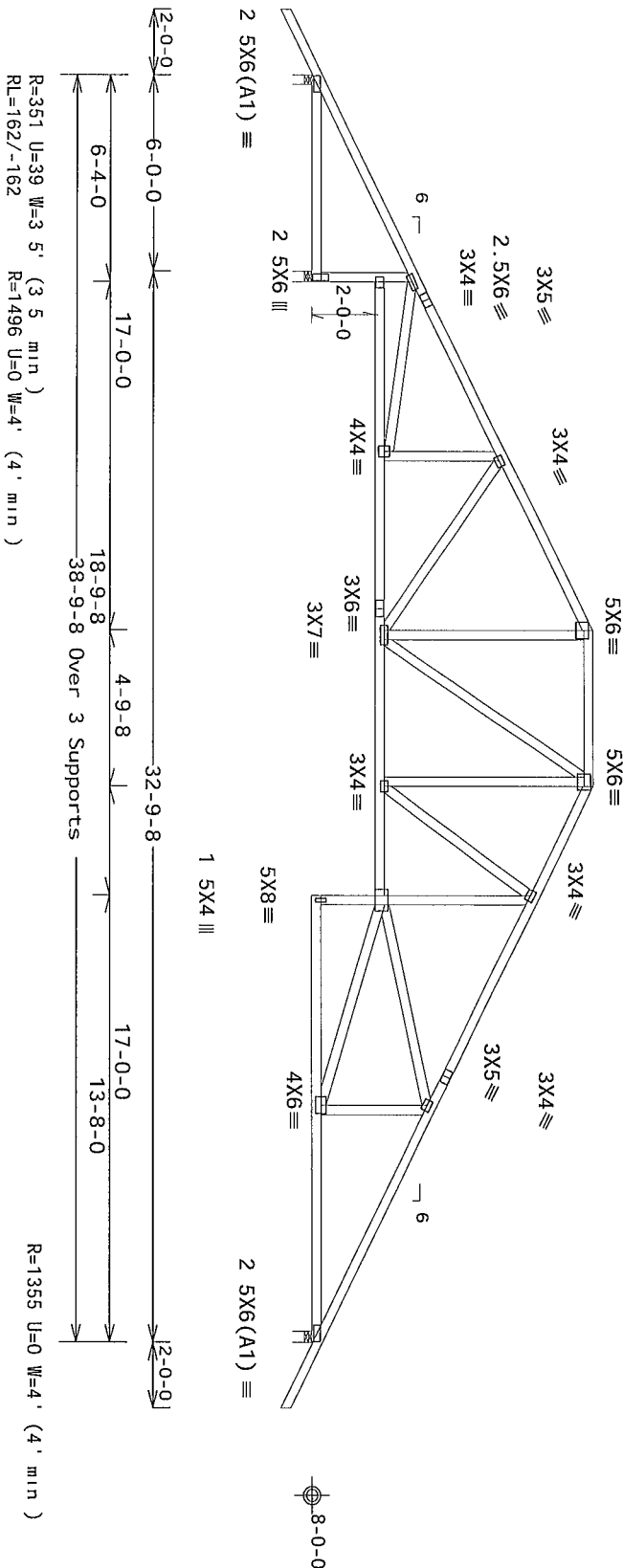
Lumber grades designated with 13B' use design values approved 1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24' OC
MMFRS loads based on trusses located at least 15 00 ft from roof edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50



PLT TYP Wave

Design Crit FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

12 03 04 00 20 13

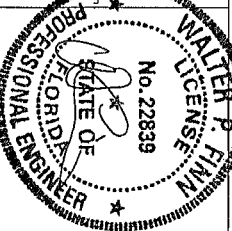
QTY 1 FL/-/5/-/-/R/-

Scale = 1875"/Ft

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety) information by TPI and WTC for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. All connections shall be made in accordance with the applicable code. All building components shall be installed in accordance with the applicable code. All building components shall be installed in accordance with the applicable code. All building components shall be installed in accordance with the applicable code.

TPW Building Components Group Inc.
Orlando FL 32837
FL COA #0278



11/22/2013

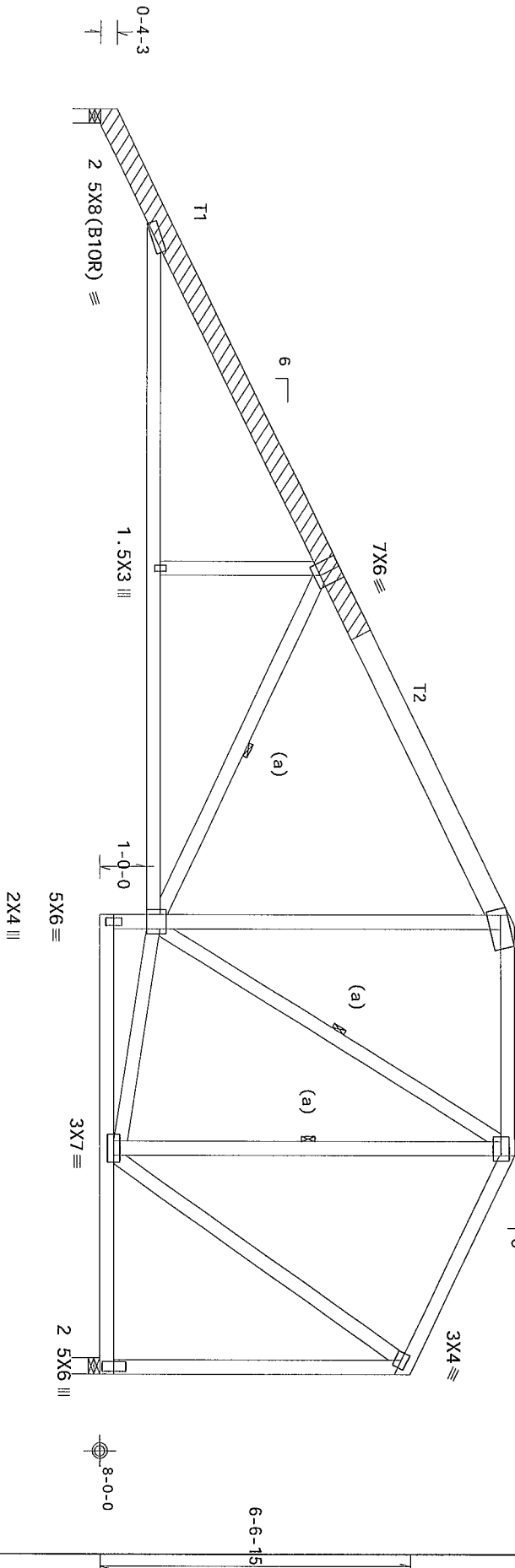
TC LL	20 0 PSF	REF R9114- 28465
TC DL	7 0 PSF	DATE 11/22/13
BC DL	10 0 PSF	DRW HCUR9114 13326015
BC LL	0 0 PSF	HC-ENG AP/AP
TOT LD	37 0 PSF	SEQN- 330415
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1V1J487_Z01

(13-281--BRYAN ZECHER /Columbia County Builders -- Lake City, FL - H17A 26 4 Steepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1-13B T1 2x6 SP M-26
 T2 2x6 SP #2-13B
 Bot chord 2x4 SP #1-13B
 Webs 2x4 SP #3-13B
 Lumber grades designated with 13B use design values approved 1/30/2013 by ALSC
 (a) Continuous lateral restraint equally spaced on member
 Bottom chord checked for 10 00 psf non-concurrent live load
 Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50
 (2) 2x6x12-3-6 SP M-26 scabs at left end Attach one scab to each outer face of chord with 0 131 x3 min nails @ 8 OC Plus additional nail clusters at BRG (3) heel (3) 1st panel point (0)

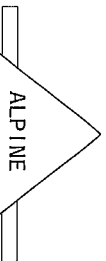
120 mph wind 15 00 ft mean hgt, ASCE 7-10 CLOSED bldg not located within 9 00 ft from roof edge RISK CAT II EXP B wind TC DL=3.5 psf wind BC DL=5.0 psf GCPI (+/-)=0.18
 Wind loads and reactions based on MMFRS with additional C&C member design
 Right end vertical not exposed to wind pressure
 In lieu of structural panels use purlins to brace all flat TC @ 24 OC
 MMFRS loads based on trusses located at least 15 00 ft from roof edge



14-5-0 14-5-0 26-4-0 Over 2 Supports 5-0-0 9-6-8 4-6-8
 R=1007 U=0 W=3.5' (3.5" min)
 RL=105/-60
 R=985 U=0 W=4" (4' min)

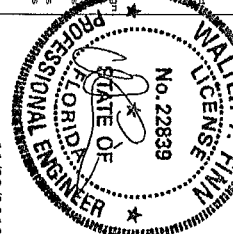
PLT TYP Wave Design Crit FBC2010Res/TP1-2007(STD)
 FT/RT=10%(0%)/0(0)

12.03.04 12.03.04 QTY: 1 FL/-/5/-/-/R/- Scale = .3125"/Ft



ITW Building Components Group Inc.
 Orlando FL, 32837
 FL COA #0278

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET
 WARNING FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
 Truss design is based on the following assumptions: 1. All members are made of steel. 2. All connections are made with steel plates and bolts. 3. The truss is supported by a rigid foundation. 4. The truss is subjected to the following loads: 1. Dead load (DL) = 3.5 psf. 2. Live load (LL) = 0 psf. 3. Wind load (WL) = 120 mph. 4. Seismic load (SL) = 0. The design is based on the following code: 1. ASCE 7-10. 2. FBC 2010. 3. TP1-2007. 4. FBC 2010 Res. The design is based on the following assumptions: 1. All members are made of steel. 2. All connections are made with steel plates and bolts. 3. The truss is supported by a rigid foundation. 4. The truss is subjected to the following loads: 1. Dead load (DL) = 3.5 psf. 2. Live load (LL) = 0 psf. 3. Wind load (WL) = 120 mph. 4. Seismic load (SL) = 0. The design is based on the following code: 1. ASCE 7-10. 2. FBC 2010. 3. TP1-2007. 4. FBC 2010 Res.



11/22/2013

TC LL	20.0 PSF	REF R9114-28466
TC DL	7.0 PSF	DATE 11/22/13
BC DL	10.0 PSF	DRW HCUR9114 13326016
BC LL	0.0 PSF	HC-ENG AP/AP
TOT. LD	37.0 PSF	SEQN- 330510
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1V1J487_Z01

(13-281--BRYAN ZECHER /Columbia County Builders -- Lake City FL - H19 38 9 8 Steepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24"

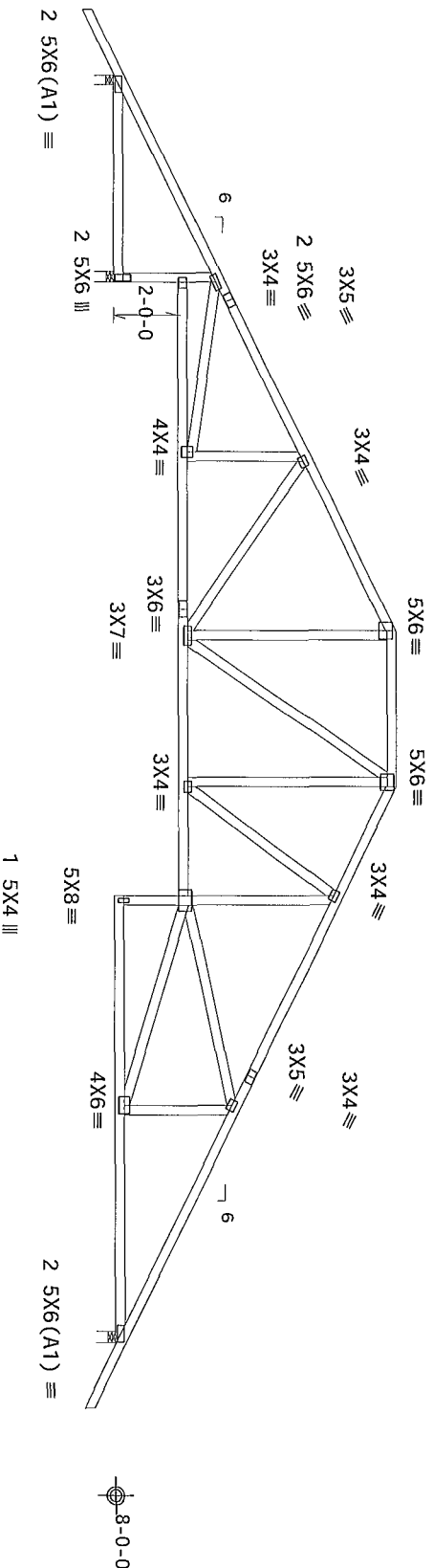
OC
MWFRS loads based on trusses located at least 15 00 ft from roof
edge

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf,
wind BC DL=5 0 psf GCPI (+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member
design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50



12-0-0 6-0-0 17-0-1 4-9-6 32-9-8 17-0-1 13-8-0 12-0-0
6-4-0 18-9-8 38-9-8 Over 3 Supports
R=351 U=39 W=3 5" (3 5 min)
RL=162/-162 R=1496 U=0 W=4 (4 min)
R=1355 U=0 W=4" (4" min)

PLT TYP Wave Design Crit FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

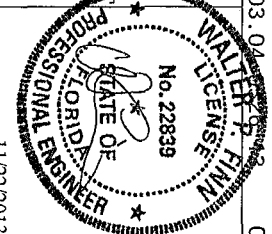
12.03.04 QTY: 1 FL/-/5/-/-/R/- Scale = .1875"/Ft.

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses are the extreme care in fabricating, shipping, handling, and bracing
for the truss. The truss shall be installed in accordance with the design
project. Prior to performing these functions, the installer shall provide temporary bracing per BCS1
unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord
shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs
shall have bracing installed per BCS1 sections B3 B7 or B10 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design
any failure to build the truss in accordance with ANSI/TP1 1 or for handling, shipping, installing, or
bracing of trusses. Apply plates to each face of truss and position as shown above and on the Joist
beams. Unless noted otherwise, refer to drawings 1804-2 for standard plate positions. Seal on this
drawing is the responsibility of the Building Designer. The suitability and use of this design for any structure is
the responsibility of the Building Designer per ANSI/TP1 1 Sec 2. For more information on see This Job's
general notes page ITW-BCG www.itwbcg.com TP1 www.tp1.net.org WTCA www.sbc industry.com



TC LL	20.0 PSF	REF R9114- 28467
TC DL	7.0 PSF	DATE 11/22/13
BC DL	10.0 PSF	DRW HCUR9114 13326017
BC LL	0.0 PSF	HC-ENG AP/AP
TOT. LD.	37.0 PSF	SEQN- 330908
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

T3 2x4 SP #1-13B

Bot	chord	2x4	SP	#1-13B
	Webs	2x4	SP	#3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

(a) Continuous lateral restraint equally spaced on member

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

(2) 2x6x12-3-6 SP M-26 scabs at left end Attach one scab to each outer face of chord with 0 131"x3", min nails @ 8" OC, Plus additional nail clusters at BRG (3), heel (3), 1st panel points (0)

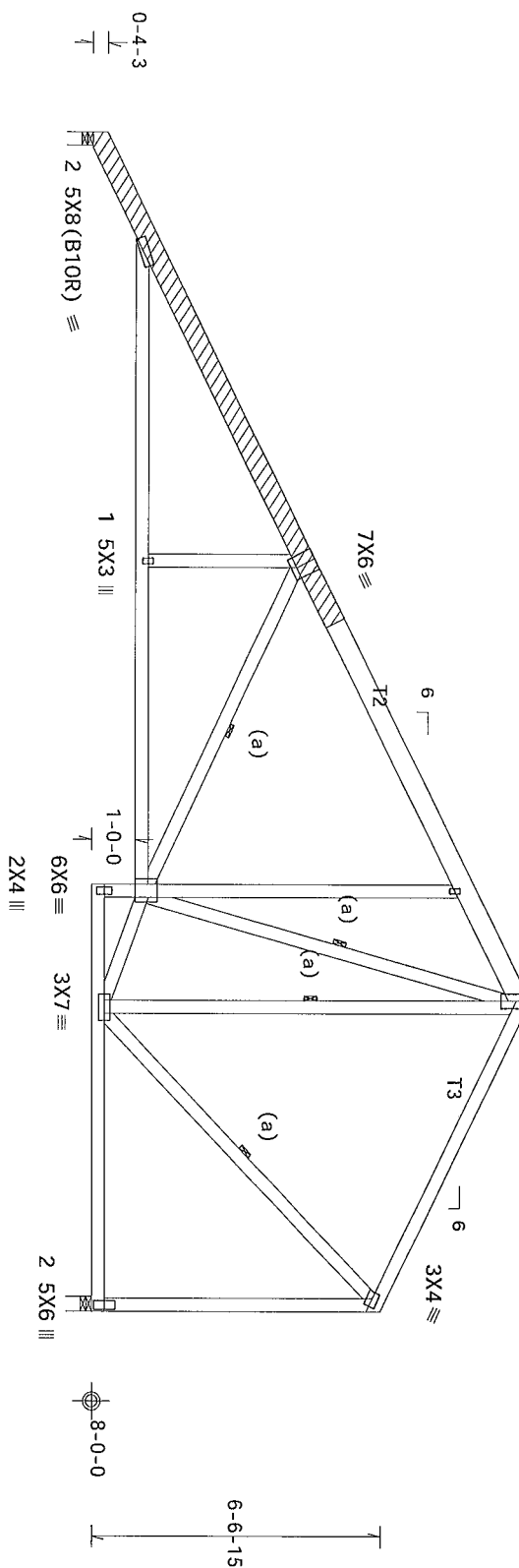
120 mph wind 15 00 ft mean hgt., ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf wind BC DL=5 0 psf 6Cp1(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Right end vertical not exposed to wind pressure

Bottom chord checked for 10 00 psf non-concurrent live load

MMFRS loads based on trusses located at least 15 00 ft from roof edge



17-0-4

14-5-0

26-4-0 Over 2 Supports

9-6-8

6-11-4

R=1007 U=0 W=3 5" (3 5" min)

R=985 U=0 W=4" (4" min)

PLT TYP Wave

Design Crit	FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)	

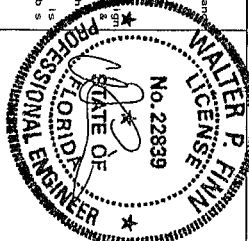
12.03.04 0326 13 QTY:1 FL/-/5/-/-/R/-

Scale = .25"/Ft.

ALPINE

ITW Building Components Group Inc

Orlando FL, 32837
FL COA #0278

[illegible]

11/22/2013

TC LL	20.0 PSF	REF	R9114 - 28468
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCUSR9114 13326018
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD.	37.0 PSF	SEQN-	330516
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Special loads

Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)
TC 15 4 3 00 4 3 00

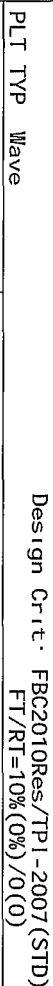
TC-From	TC-To	TC-From	TC-To
28 p/f at 56	3 00 to 58	28 p/f at 56	3 00 to 58
TC-From	TC-To	TC-From	TC-To
28 p/f at 56	3 00 to 58	28 p/f at 56	3 00 to 58
TC-From	TC-To	TC-From	TC-To
28 p/f at 56	3 00 to 58	28 p/f at 56	3 00 to 58

BC-From	20 pif at	0 00 to	20 pif at	3 03
BC-From	10 pif at	3 03 to	20 pif at	8 55
BC-From	20 pif at	8 55 to	20 pif at	11 58
BC-From	4 pif at	11 58 to	4 pif at	13 58

Tc-70	Conc	Load at load at	8 06, 4 53
16			55
58			55
16			55

BC- 111	48 lb Conc	Load at	3 03, 8 55
BC- 46	51 lb Conc	Load at	5 06, 6 52

Deflection meets L/240 live and L/180 total load Creep increases factor for dead load is 1.50



QTY:1 FL/-/5/-/-/R/-

Scale = .5"/Ft.

ITW Building Components Group Inc

Orlando FL, 32837
FL COA #0278

••IMPORTANT•• FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to any

practices prior to performing these functions. Installers shall provide temporary bracing per BCSI

Unless noted otherwise top chord shall have properly attached structural sheathing and bottom chord shall have a separately attached rigid insulation. Location of sheathing and bottom chord at all walls shall be as shown on drawings. Location of bottom chord at all walls shall be as shown on drawings.

shall have bracing installed per BCS1 sections B3 B7 or B10 as applicable

ITW Building Components Group Inc (ITWBCG) shall not be responsible for any deviation from this design.

Apply plates to each face of truss and fasten as shown above and on the joint bracing of trusses. Apply plates to each face of truss and fasten as shown above and on the joint bracing of trusses.

Details unless noted otherwise Refer to drawings 160A-Z for standard plate positions A seal on the

responsibility solely for the design shown. The suitability and use of this design for any structure is the user's responsibility and use of this design for any structure is the user's responsibility.

the responsibility of the Building Designer per ANSI/TPI 1 Sec 2 For more information see This job :

ICC www.iccsafe.org

[illegible]

11/22/2013

TC LL	20.0 PSF	REF R9114 - 28469
TC DL	7.0 PSF	DATE 11/22/13
BC DL	10.0 PSF	DRW HCU9R114 1332601
BC LL	0.0 PSF	HC-ENG AP/AP
TOT LD.	37.0 PSF	SEQN- 339854
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1V1J487_Z01

JREF- 1W1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

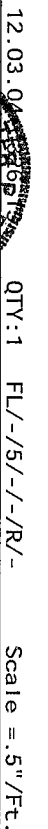
130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCp1(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Bottom chord checked for 10 00 psf non-concurrent live load

Deflection meets $L/240$ live and $L/180$ total load Creep increases

factor for dead load is 1 50



TC LL	20.0 PSF	REF R9114- 28
-------	----------	---------------

STATE OF
No. 22839
WALTER P. FINN
LICENSE



11/22/2013

TC LL	20.0 PSF	REF	R9114 - 28470
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DR#	HCUSR9114 1332602
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT. LD.	37 0 PSF	SE0N-	339734
DUR. FAC.	1.25		
SPACING	24.0"	JREF - 1V1J487	Z01

Top chord 2x4 SP 2850F-2 3E T2, T3 2x6 SP M-26
Bot chord 2x4 SP 2850F-2 3E B2, B3 2x6 SP M-26
Webs 2x4 SP #3-13B W3, W4, W14 2x4 SP 2850F-2 3E
W5, W12, W13 2x4 SP #1-13B

Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

120 mph wind 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf 60psi(+/-)=0 18

(J) Hanger Support Required, by others

In lieu of structural panels use purlins to brace all flat TC @ 24
OC

Calculated vertical deflection is 1 08" due to live load and 1 43" due
to dead load at X = 17-7-11

2 COMPLETE TRUSSES REQUIRED

Nail Schedule 0 131"x3", min nails
Top Chord 1 Row @12 00" o c
Bot Chord 1 Row @12 00" o c
Webs 1 Row @ 4' o c
Use equal spacing between rows and stagger nails
in each row to avoid splitting

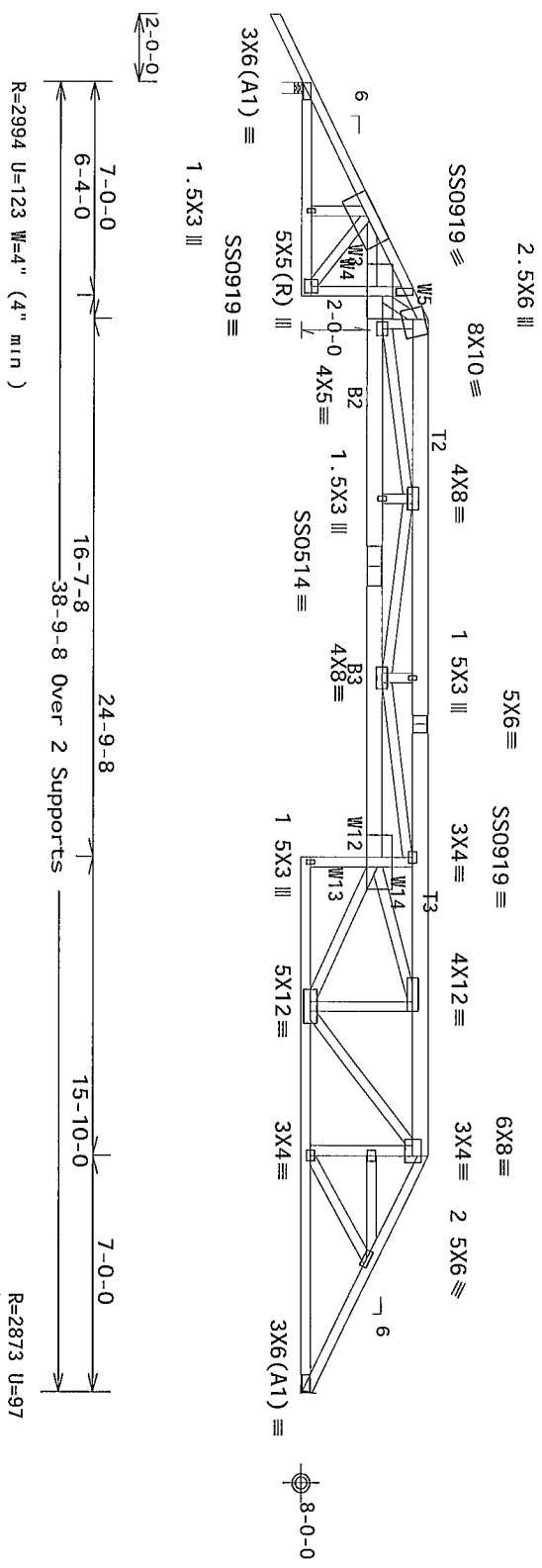
Wind loads and reactions based on MMFRS

Calculated horizontal deflection is 0 46" due to live load and 0 61"
due to dead load

#1 hip supports 7-0-0 jacks with no webs

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1 50

WARNING Furnish a copy of this DWG to the installation contractor
Special care must be taken during handling, shipping and installation
of trusses See "WARNING" note below



PLT TYP. 18 Gauge HS, Wave
Design Crit. FBC2010Res/TP1-2007 (STD)
FT/RT=10%(0%)/0(0)

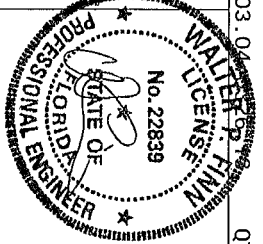
WARNING READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

ALPINE

ITW Building Components Group Inc.
Orlando FL 32837
FL COA #0278

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Component Safety Information) by TPI and WFOA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10 as applicable.

ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design or for any damage to the truss or its components. The user of this design shall be responsible for the design of the building and the use of this design for any structure is the responsibility of the building designer per ASCE/TP1 Sec 2. For more information see the ITWBCG website at www.itwbcg.com or call 1-800-368-7273.



11/22/2013

QTY: 1		FL/-/5/-/-/R/-		Scale = .1875"/ft	
TC LL	20.0 PSF	REF	R9114- 28471		
TC DL	7.0 PSF	DATE	11/22/13		
BC DL	10.0 PSF	DRW	HCSR9114 13326021		
BC LL	0.0 PSF	HC-ENG	AP/AP		
TOT LD.	37.0 PSF	SEQN-	324376		
DUR.FAC.	1.25	FROM	JMW		
SPACING	24.0"	JREF-	1V1J487_Z01		

THIS DING PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI (+/-)=0 18

Wind loads and reactions based on MWFRS with additional C&C member design

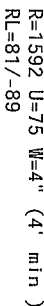
Calculated horizontal deflection is 0.34' due to live load and 0.45' due to dead load

Hanger specified assumes connection to supporting chord is located a minimum of five times the depth of the supporting chord from any unsupported end, unless unsupported chord end has 85% plating

In lieu of structural panels use purlins to brace all flat TC @ 24 OC

Deflection meets $L/240$ live and $L/180$ total load Creep increase factor for dead load is 1.50

MMFRS loads based on trusses located at least 7 50 ft from roof edge



R=1463 U=56
H=H1

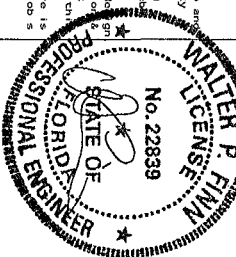
12.03.04 0226 13

QTY:1 FL/-/5/-/-/R/-

Scale = .1875"/Ft

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

[illegible]

TC LL	20 0 PSF	REF	R9114- 28473
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCSR9114 13326028
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD.	37 0 PSF	SEQN-	330752
DUR FAC	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

Top chord 2x4 SP #1-13B T1 2x6 SP M-26
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with 13B use design values approved
1/30/2013 by ALSC

In lieu of structural panels use purlins to brace all flat TC @ 24"
OC

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1.50

(1) 2x6x7-8-14 SP M-26 scab at left end Attach scab to face of chord
with 0 131'x3, min nails @ 8" OC, plus additional nail clusters at
BRG (4), heel (6), 1st panel point (3)

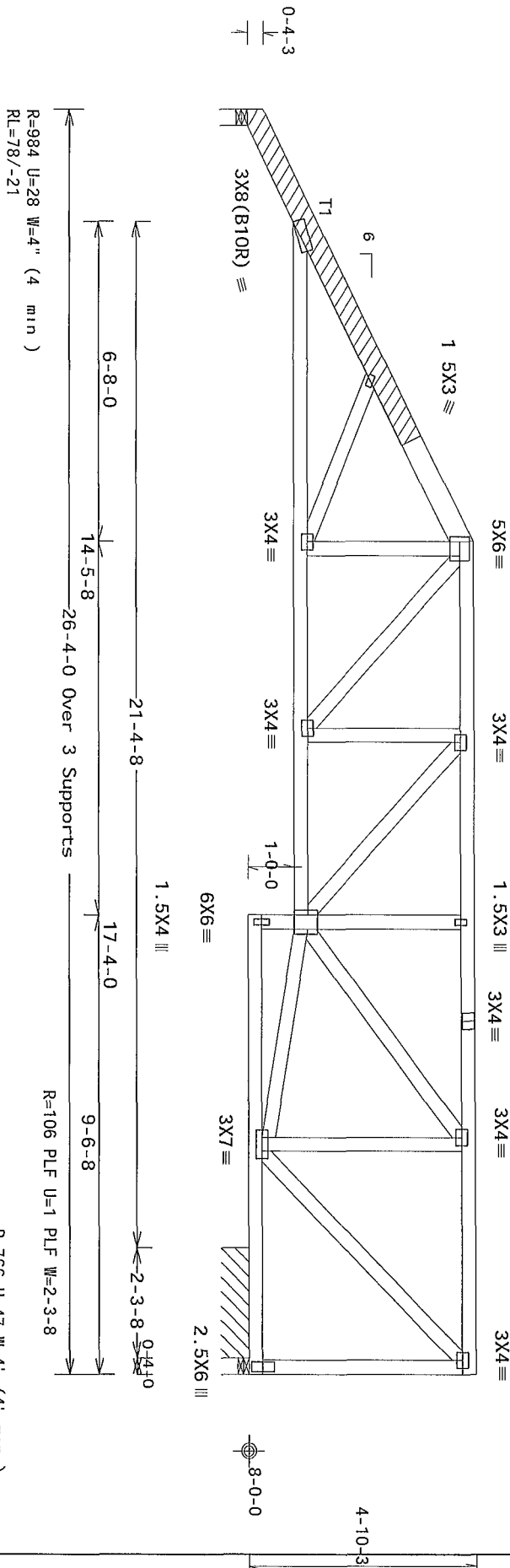
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located
within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3.5 psf,
wind BC DL=5.0 psf GCPI (+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member
design

Right end vertical not exposed to wind pressure

Bottom chord checked for 10 00 psf non-concurrent live load

MMFRS loads based on trusses located at least 7.50 ft from roof edge

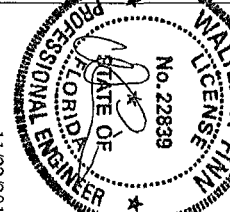


PLT TYP Wave Design Crit FBC2010Res/TP1-2007(STD) 12 03 04 2013 QTY: 1 FL/-/5/-/1/R/- Scale = 3125"/Ft.

ALPINE

ITW Building Components Group Inc.
Orlando FL 32837
FL COA #0778

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Before to and
follow the latest edition of BCSI (Building Component Safety Information by TPI and WTC) for safety
product details or to perform these functions. Installers shall provide temporary bracing per BCSI
unless noted otherwise. Top chord shall have properly attached structural sheathing and bottom chord
shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs
shall have bracing installed per BCSI section 83 B7 or B10 as applicable.
ITW Building Components Group Inc. (ITWBCG) shall not be responsible for any deviation from this design
any failure to build the truss in accordance with ANSI/TP1-1 or for handling, shipping, installation,
bracing or use of trusses. Apply plates between face of chord and peak to one side above and on the joint.
Do not cut, drill, nail, or otherwise alter the truss. Do not use the truss for any other purpose than that
shown in the drawing. The user shall be responsible for the design and use of the truss. This job is
the responsibility of the Building Designer per ANSI/TP1-1 Sec 2. For more information see
general notes page ITW-BDG www.trussorg WTC www.structure.com
ICC www.iccsafe.org



TC LL	20 0 PSF	REF	R9114- 28474
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	HOUSR9114 13326029
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT. LD	37 0 PSF	SEQN-	330480
DUR. FAC.	1.25		
SPACING	24.0"	JREF	1V1J487_Z01

11/22/2013

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

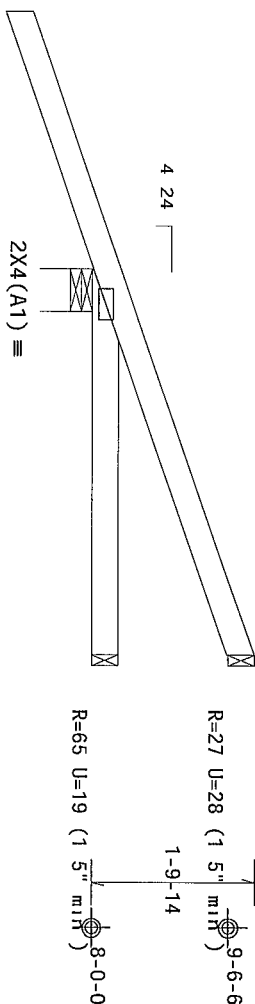
Special loads
----- (Lumber Dur Fac = 1.25 / Plate Dur Fac = 1.25)

TC- BC- From	2 pif at 0 pif at 2 pif at	0 00 to -2 83 to 0 00 to	2 pif at 4 pif at 2 pif at	4 24 0 00 4 24
TC- BC- From	2 pif at 0 pif at 2 pif at	0 00 to -2 83 to 0 00 to	2 pif at 4 pif at 2 pif at	4 24 0 00 4 24

IC-63	45 lb Conc	Load at 1 48
BC-	1 08 lb Conc	Load at 1 48

Defect rate = $1/340$ from July 1/1980 to

Deflection meets L/240 live and L/180 total load Creep increases factor for dead load is 1.50



2-9-15

←4-2-15 Over 3 Supports →

R=195 U=149 W=5 657" (5 657" min)

PLT TYP. Wave

Design Crit	FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)	

12 03 04 03-26-13

QTY:2 FL/-/5/-/-/R/-

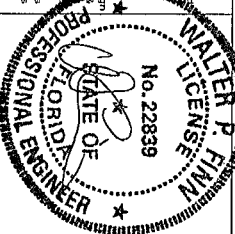
Scale = .5"/Ft

****IMPORTANT****
****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

ADVICE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

[illegible]

11/22/2013

TC LL	20 0 PSF	REF R9114- 28475
TC DL	7 0 PSF	DATE 11/22/13
BC DL	10.0 PSF	DRW HCUSR9114 13326030
BC LL	0 0 PSF	HC-ENG AP/AP
TOT.LD.	37.0 PSF	SEQN- 339851
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

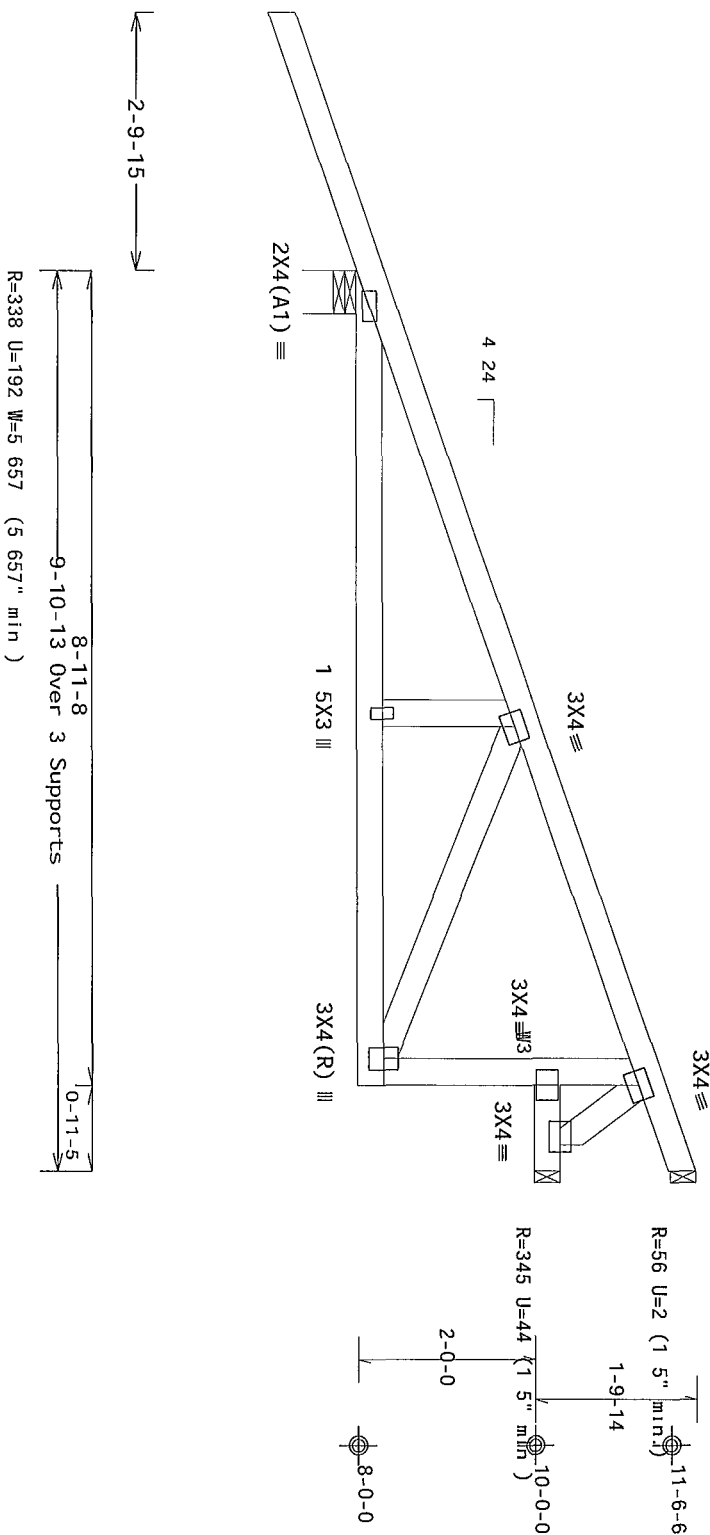
Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bidg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI (+/-)=0 18

Wind loads and reactions based on MWFRS

Bottom chord checked for 10 00 psf non-concurrent live load

Special loads					
-----Lumber Dur Fac =1 25 / Plate Dur Fac =1 25)					
TC- From	0 pif at -2 83	to	55 pif at 0 00		
TC- From	2 pif at 0 00	to	2 pif at 9 90		
BC- From	0 pif at -2 83	to	4 pif at 0 00		
BC- From	2 pif at 0 00	to	2 pif at 8 96		
BC- From	2 pif at 8 96	to	2 pif at 9 90		
TC- 63 45 lb Conc	Load at		1 48		
TC- 87 80 lb Conc	Load at		4 31		
TC- 215 63 lb Conc	Load at		7 13		
BC- 1 08 lb Conc	Load at		1 48		
BC- 93 02 lb Conc	Load at		4 31		
BC- 172 85 lb Conc	Load at		7 13		
Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1 50					



PLT TYP Wave

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)
-------------	--

12.03 04 0326 13

QTY: 1

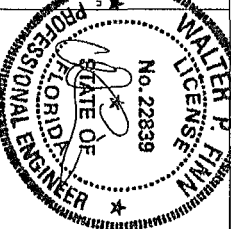
FL/-/5/-/-/R/-

Scale = .5"/Ft.

ALPINE

ITW Building Components Group Inc

Orlando FL, 32837
FL COA #0278

[illegible]

~~11/22/2013~~

TC LL	20.0 PSF	REF	R9114- 28476
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DR#	HCUSR9114 13326031
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD	37.0 PSF	SEQ#	330597
DUR, FAC	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

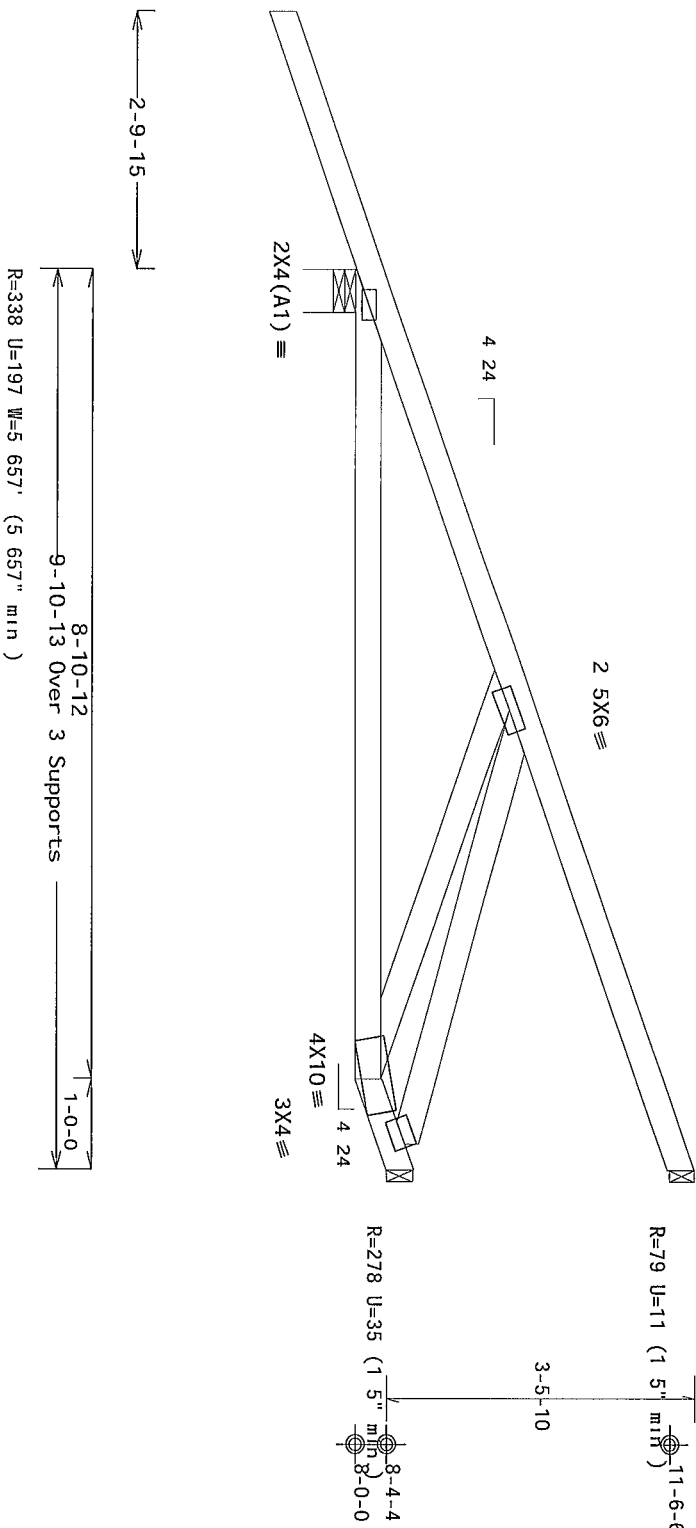
130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 9 00 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI (+/-)=0 18

Wind loads and reactions based on MWFRS

Bottom chord checked for 10 00 psf non-concurrent live load

Shim all supports to solid bearing

Special loads	Deflection for dead load is 1 50	Creep increase
-----Lumber	Dur Fac = 1 25 /	Plate Dur Fac = 1 25)
TC- From	0 pif at -2 83 to	55 pif at 0 00
TC- From	2 pif at 0 00 to	2 pif at 0 00
BC- From	0 pif at -2 83 to	4 pif at 0 00
BC- From	2 pif at 0 00 to	2 pif at 8 90
BC- From	2 pif at 8 90 to	2 pif at 9 90
TC- 63 45 1b Conc	Load at 1 48	
TC- 87 80 1b Conc	Load at 4 31	
TC- 215 63 1b Conc	Load at 7 13	
BC- 1 08 1b Conc	Load at 1 48	
BC- 93 02 1b Conc	Load at 4 31	
BC- 172 85 1b Conc	Load at 7 13	



PLT TYP Wave

Design Crit	FBC2010Res/TP1-2007(STD) FT/RT=10%(0%)/0(0)
-------------	--

12 03.04 2025

QTY.1 FL/-/5/-/-/R/-

Scale = .5"/Ft.

ALPINE

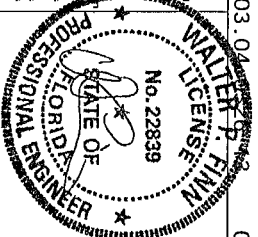
ITW Building Components Group Inc
Orlando FL, 32837
FL COA #0 278

****WARNING** READ AND FOLLOW ALL NOTES ON THIS SHEET!**

FURNISH THIS DESIGN TO ALL CONTRACTORS, INCLUDING INSTALLERS

Tussocks, you're extremely care in fabricating, handling, installing and bracing
Follow the latest edition of BCSI (Building Component Safety) Information on TP1 and WTCA) for safety
practices prior to performing these functions. Installers shall provide temporary bracing per BCSI
Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord
shall have a properly installed rigid ceiling.
Bridges shall have bracing installed per BCSI sections B3, B7 or B10 as applicable.

1TW Build up Components Group Inc. (TWBCS) shall not be responsible for any delay due from this design
any failure to build the truss in conformance with ANSI/TPI-1 or for handling, shipping or installation
bracing of trusses. Apply plates to each face of truss and post on as shown above and on the Joist
details of trusses noted otherwise. Refer to drawing 180A-2 for standard details above and on the Joist
by solely for the design shown. The use tabling and use of this information for any structure is
the response bility of the Build up Design. For more information on this job see
general notes page 1TW-BG0 www.twbcg.com ANSI/TPI-1 Sec 2 This job s
www twbcg org TPI www tpinc.org WTCA www sbcindustry com
CCC localite org



11/22/2013

TC LL	20.0 PSF	REF	R9114 - 28477
TC DL	7.0 PSF	DATE	11/22/13
BC DL	10.0 PSF	DRW	HCUSR9114 13326032
BC LL	0.0 PSF	HC-ENG	AP/AP
TOT LD.	37.0 PSF	SEQN-	339827
DUR FAC	1.25		
SPACING	24.0"	JREF -	1V1J487_Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Webs 2x4 Sp #3-13B

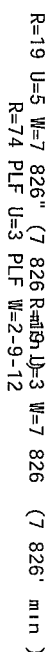
	Dur	Fac = 1	25 / Plate Dur	Fac = 1	(25)
Special loads					
-----(Lumber					
TC-From	56 pif at	0 00 to	56 pif at	2 39	
TC-From	56 pif at	2 39 to	56 pif at	4 78	
BC-From	4 pif at	0 00 to	4 pif at	4 78	

120 mph wind, 17 45 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind Bo
DL=2 0 psf GC_{PI}(+/-)=0 18

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50

Deflection meets L/240 live and L/180 total load Creep increase factor for dead load is 1.50



Scale = .5"/Ft

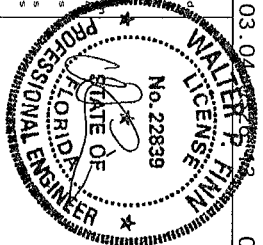
ITW Building Components Group Inc

Orlando FL, 32837
FL COA #0278

****IMPORTANT**** **WARNING** **READ AND FOLLOW ALL NOTES ON THIS SHEET!**
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require the use of a fabric restraining and bracing system to follow the lateral action of BCS. (But if Compartment Safety Information by TPI and WDA) for safety practices not or to performing these functions. Installers shall provide temporary bracing per BCS. Unless noted otherwise, no top chord shall have properly attached structural sheath and bottom chord shall have a properly attached rigid ceiling. Load case shown for permanent lateral restraint of webs shall have brace installed per BCS section 83, 87 or 810 as applicable.

ITW Bu Id ng Components Group Inc (LTHB00) shall not be responsible for any delay or from th s date
any failure to bu id th truss in conformance w th ANSI/TPI 1 or for handling shippng metal on
loading of trucks. Apply plates to each face of truss and position as shown above and on the Jo nt
brace unless noted otherwise. Refer to drawing gns 180A-2 for standard plate positions. A seal on th s
responsible ly solely for the des gn shown. The su itability and use of th s information for any
the responsibility b y of the bu id ng department per ANSI/TPI 1 Sec 2 For more information on any
general noise panel 17H-BG03 www.lawdog.com TPI www.tpi.net.org WJCA www.abcdindustry.com
CDC www.cdc.gov



11/22/2013

TC LL	20.0 PSF	REF R9114- 28478
TC DL	7.0 PSF	DATE 11/22/13
BC DL	10.0 PSF	DRW HCURS9114 13326033
BC LL	0.0 PSF	HC-ENG AP/AP
TOT LD	37 0 PSF	SEQN- 330906
DUR FAC.	1 25	
SPACING	24.0"	JREF- 1V1J487_Z01

(13-281--BRYAN ZECHER /Columbia County Builders -- Lake City, FL - PBAT 4 9'6 Steepdown Hip)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved
1/30/2013 by ALSC

Wind loads and reactions based on MMFRS with additional C&C member
design

In lieu of rigid ceiling use purlins to brace BC @ 24" OC

MMFRS loads based on trusses located at least 17 35 ft from roof
edge

Refer to drawing PB160100212 for piggyback detail Top chord
of supporting truss under piggyback to be braced @ 24" OC,
unless otherwise specified

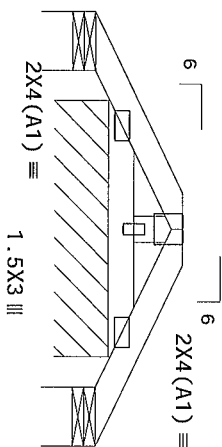
Special Loads

-----Lumber Dur Fac =1.25 / Plate Dur Fac =1.25
TC-From 56 pif at 0.00 to 56 pif at 2.39
TC-From 56 pif at 2.39 to 56 pif at 4.78
BC-From 4 pif at 0.00 to 4 pif at 4.78

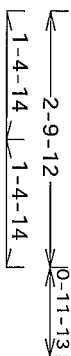
120 mph wind, 17 35 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC
DL=2.0 psf GCpl(+/-)=0.18

Deflection meets L/240 live and L/180 total load Creep increase
factor for dead load is 1.50

4X4 ≡



16-10-35



4-9-6 Over 3 Supports

R=19 U=4 W=7 826" (7 826 R=19 U=3 W=7 826" min)
R=74 PLF U=2 PLF W=2-9-12

PLT TYP Wave

Design Crit FBC2010Res/TP1-2007(STD)

RT/RT=10%(0%)/0(0)

12 03.04 0326 13

QTY. 1

FL/-/5/-/-/R/-

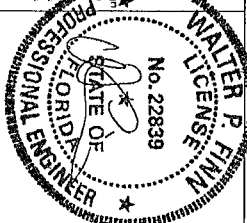
Scale = 5"/Ft.

ALPINE

NTW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

IMPORTANT READ AND FOLLOW ALL NOTES ON THIS SHEET
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
Trusses require extreme care in handling, shipping, installing and bracing. Refer to and
follow the latest edition of BCSI (Building Components Safety Information) by TPI and WTC for safety
practices prior to performing these functions. Installers shall provide temporary bracing per BCSI.
Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord
shall have a properly attached r gird ceiling. Locations shown for permanent lateral restraint of web
shall have bracing installed per BCSI section 83, 87 or 810 as applicable.
NTW Building Components Group Inc. (NTWBCG) shall not be responsible for any deviation from this design
any failure to build the truss in conformance with ANSI/TP1-1 or for handling, shipping, installing or
bracing of trusses. Apply plates to each face of truss and post on as shown above and on the joint on
the end of the truss. The design is based on the assumption that the truss is supported on a level
drawing or cover page listing the design shown. The suitability and use of this design for any structure is
the responsibility of the Building Designer per ANSI/TP1-1 Section 2. For more information see this job's
general notes page TPI-BDCG www.tbdcg.com TPI www.tpinc.net org WTC www.stc industry.com



TC LL	20 0 PSF	REF	R9114- 28479
TC DL	7 0 PSF	DATE	11/22/13
BC DL	10 0 PSF	DRW	HCSR9114 13326034
BC LL	0 0 PSF	HC-ENG	AP/AP
TOT LD	37 0 PSF	SEQN-	330914
DUR FAC	1.25		
SPACING	24.0"	JREF-	1V1J487_Z01

11/22/2013

Dr 120 mph Wind Speed	15	Mean Height	Partially Enclosed	Exposure C	K _{zt} = 1.00
Dr 120 mph Wind Speed	15	Mean Height	Enclosed	Exposure D	K _{zt} = 1.00
Dr 100 mph Wind Speed	15	Mean Height	Partially Enclosed	Exposure D	K _{zt} = 1.00

Bracing Group Species and Grades

Group A

Service-Pine-Fir	Hem-Fir
#1 / #2 Standard	#2 Stud
#3 Stud	#3 Standard

Douglas Fir-Larch

#3
Stud
Standard

Southern Pine*Oak

#3
Stud
Standard

Douglas Fir-Larch

#1
#2

Southern Pine

#1
#2

4 Braces shall be SRB (Stress-Rated Board) *For 1x4 So. Pine use only industrial 55 or industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes

Wind Load deflection criterion is $L/240$

Provide uplift connections for 55 plf over

continuous bearing (5 psf TC Dead Load).
Gable end supports load from 4 0' outlookers

So Pine lumber design values based on the ALSC January 2012 ruling

Attach 'L' braces with 10d (0.128"x3.0" min) nails

* For (1) "L" brace space nails at 2' o.c. in 18" end zones and 4' o.c. between zones.

***For (2) 'L' braces space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.

"L" bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes

Vertical Length	No Splice
Less than 4 0'	1X4 or 2X3

Greater than 4 0" but less than 11 6"	2.5X4
---------------------------------------	-------

Greater than 1!	6'	3x4
-----------------	----	-----

+ Refer to common truss design for

peak splice and heel plates.

Refer to the Building Designer for conditions not addressed by this detail

REF	ASCE7-10-GABI4015
-----	-------------------

DATE 2/14/12

DRWG A14015ENC100213

LD 60 PSF

--	--

ING 240"	
----------	--

11/22/2013

11/22/2013

CT0717711Y

Diagram illustrating a roof truss system with various components and connections. The diagram shows a main truss structure with a gable end, supported by a wall. Various components are labeled with symbols:

- (+)** Refer to a splice web
- (*)** If gable single plate over the end

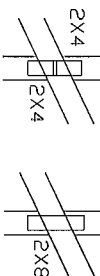
Example

Diagram labels include: Sym About, Gable Vertical Typ, and various connection points marked with (+) and (*).

Refer to appropriate ITW gable detail for minimum plate sizes for vertical studs.

- ① Refer to Engineered truss design for peak splice web and heel plates
- ② If gable vertical plates overlap use a single plate that covers the total area of the overlapped plates to span the web

Example



To convert from 'L' to 'T' reinforcing members multiply 'T' increase by length (based on appropriate ITW gable detail)

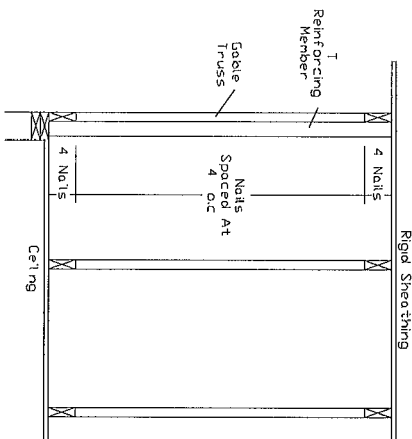
Maximum allowable "T" reinforced gable vertical length is 14 from top to bottom chord.

"T" reinforcing member material must match size specie and grade of the "L" reinforcing member

Web Length Increase w/ "T" Brace

T ⁺ Reinf Mbr Size	T ⁺ Increase
2x4	30 %
2x6	20 %

Example
ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft K_{zt} = 1.00
Gable Vertical = 24'oc SP #3
T Reinforcing Member Size = 2x4
T Reinforce Increase (from Above) = 30% = 1.30
(1) 2x4 TL Brace Length = 8' 7"
Maximum *T* Reinforced Gable Vertical Length
130 x 8' 7" = 11' 2"



This detail to be used with the appropriate ITW gable detail for ASCE wind load

wind load

A13015980109, A12015980109 A11

A13030980109 A12030980109 A11030980109 A10030980109

ASCE 7-02 Gable Detail Drawings

A13015020109 A12015020109 A110

A13030020109 A12030020109 A11

ASCE 7-05 Gable Detail Drawings

A130:5050:09 A120:5050:09 A110

A13030050109 A12030050109 A11

ASCE 7-10 Gable Detail Drawings

AI1015ENL100212	AI2015ENL100212
AI10015ENL100212	AI20015ENL100212

AI0013ENCL00212	AE0013ENCL002
AI1E00ENCL00013	AI0000ENCL000

A1B030ENCL100212	A30030ENCL1002
A1B030ENCL100212	A1E030ENCL1002

See appropriate ITW gable detail for maximum unreinforced gable vertical length.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!
 IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

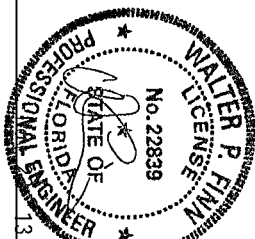


Building Components Group Inc.

Building Components Group Inc.

Earth City MO 63045

ITV Building Components Group Inc shall not be responsible for any deviation from this drawing, any failure to build the Truss in conformance with ANSI/TPI-1 or for handling, shipping, installation or use of the Truss. The responsibility for covering the Truss, including the responsibility for the design of professional engineering or other design, shall be the responsibility of the building designer. For more information see this job's general notes page and these web sites: www.bwbuild.com/tipi and www.ansi.org/tipi



MAX	TOT LD	60 PSF
DUR	FAC	ANY
MAX	SPACING	240"

REF	LET-IN VERT
DATE	2/16/12
DRWG	GBLLETIN0212

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired

Notes

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement
Alternative reinforcement specified in chart below may be conservative for minimum alternative reinforcement, re-run design with appropriate reinforcement type

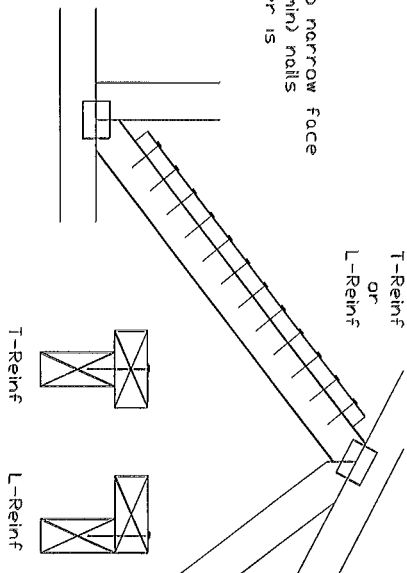
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf	Scab Reinf
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web Apply (1) scab to each face of web

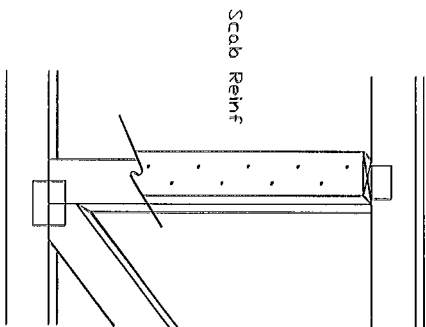
T-Reinforcement or L-Reinforcement

Apply to either side of web narrow face Attach with 10d (0128"x30",min) nails at 6" o.c Reinforcing member is a minimum 80% of web member length



Scab Reinforcement

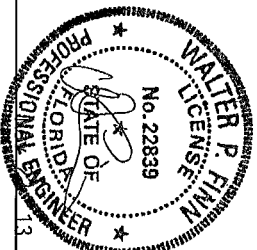
Apply scab(s) to wide face of web. No more than (1) scab per face Attach with 10d (0128"x30",min) nails at 6" o.c Reinforcing member is a minimum 80% of web member length.



Building Components Group Inc.

Earth City, MO 63045

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING. TRUSSES REQUIRE EXTREME CARE IN REPLICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO AND FOLLOW THE LATEST EDITION OF BCSI BUILDING COMPONENT SAFETY INFORMATION, BY TPI AND SBCA FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. INSTALLERS SHALL PROVIDE TEMPORARY BRACING PER BCSI REQUIREMENTS. TRUSSES SHALL BE PROTECTED FROM DAMAGE DURING TRANSPORT AND STORAGE. TRUSSES SHALL HAVE A PROPERLY ATTACHED ROAD CEILING LOCATION SHOWN FOR PERMANENT LATERAL RESTRAINT OF WEBS OF TRUSS AND POSITION AS SHOWN ABOVE AND ON THE JUNT DETAILS, UNLESS NOTED OTHERWISE. REFER TO DRAWINGS FOR STANDARD PLATE POSITIONS. ALL BRACING SHALL BE INSTALLED IN ACCORDANCE WITH THE BCSI REQUIREMENTS. BRACING OF TRUSSES. A SEAL ON THIS DRAWING OR COVER PAGE LISTING THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING DESIGN. THE SEALING OF THIS DRAWING SHALL BE THE RESPONSIBILITY OF THE SEALING ENGINEER. FOR MORE INFORMATION SEE THE JOB'S GENERAL NOTES, PAGE AND THESE WEB SITES: www.bcsinfo.com TPI: www.tpi.org SBCA: www.sbcindustry.org ITC: www.itcinfo.org



TC LL	PSF	REF	CLR Subst
TC DL	PSF	DATE	8/15/13
BC DL	PSF	DRWG	BRCLBSUB0813
BC LL	PSF		
TOT LB	PSF		
DUR FAC			
SPACING			

11/22/2013

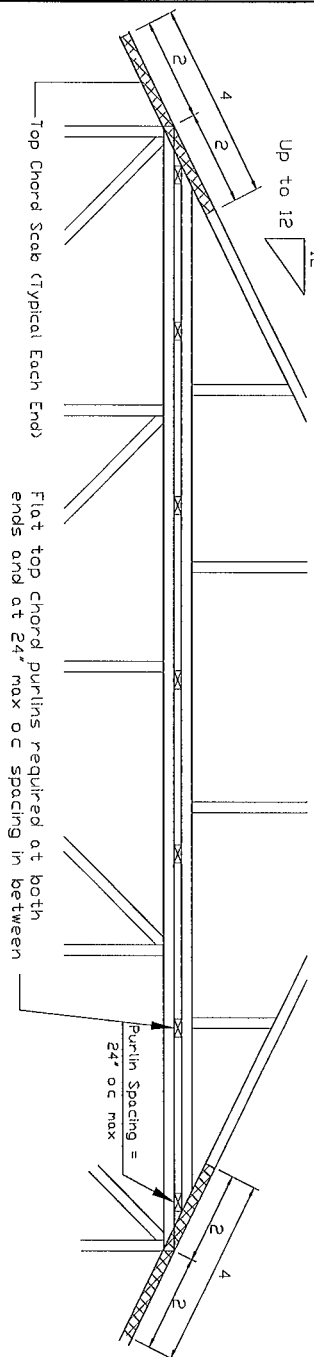
Piggyback Detail - ASCE 7-10 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

160 mph Wind 30.00 ft Mean Hgt ASCE 7-10 Enclosed Bldg located anywhere in roof Exp C Wind DL = 50 psf (min) Kzt=1.0
Or 140 mph wind 30.00 ft Mean Hgt ASCE 7-10 Enclosed Bldg located anywhere in roof Exp D Wind DL = 50 psf (min) Kzt=1.0

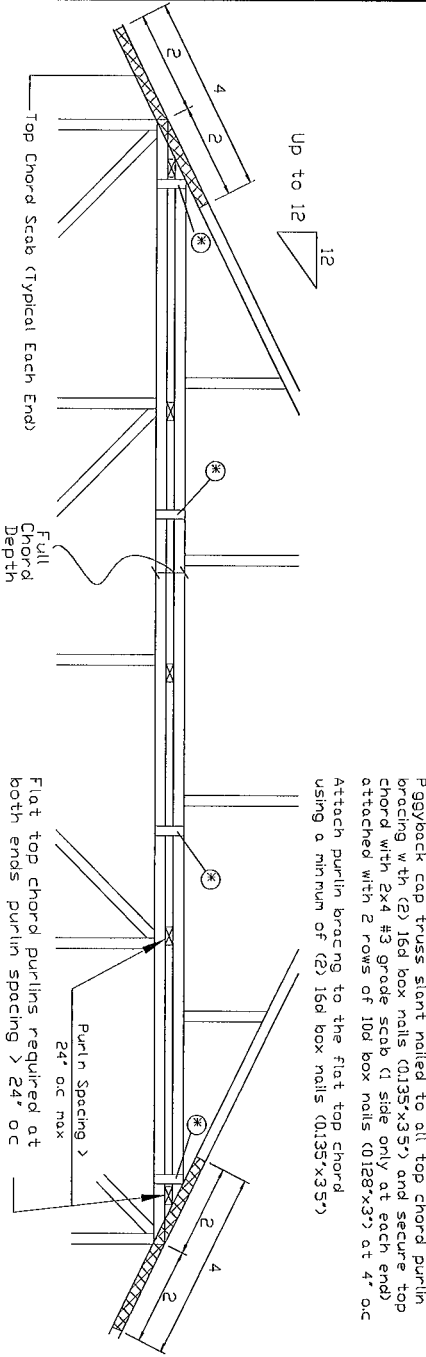
Note Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins and lateral bracing for out of plane loads over gable ends. Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola steeple chimney or drag strut loads

** Refer to Engineer's steel truss design drawing for piggyback and base truss specifications

Detail A Purlin Spacing = 24" o.c. or less



Detail B Purlin Spacing > 24" o.c



Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c. Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5")

Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c. Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5")

The top chord #3 grade 2x4 scab may be replaced with either of the following (1) 3x8 Trufox plate attached with (2) 0.120"x1.375" nails (4) into cap TC & (4) into base truss TC or (1) 2x8PB wave piggyback plate nailed to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note Nailing thru holes of wave plate is acceptable

* In addition provide connection with one of the following methods

Trufox
Use 3x8 Trufox plates for 2x4 chord member and 3x10 Trufox plates for 2x6 and larger chord members. Attach to each face @ 8 o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trufox plates may be staggered 4 o.c. front to back faces

APA Rated Gussset
8"x8"x7/16" (min) APA rated sheathing gusssets (each face) Attach @ 8 o.c. with (8) 5d common (0.113"x2") nails per gussset (4) in cap bottom chord and (4) in base truss top chord. Gusssets may be staggered 4 o.c. front to back faces

2x4 Vertical Scabs
2x4 SPF #2 full chord depth scabs (each face) Attach @ 8 o.c. with (6) 10d box nails (0.128"x3") per scab (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4 o.c. front to back faces

2x8PB Wave Piggyback Plate
Use 2x8PB wave piggyback plate to each face @ 8 o.c. Attach to each face of piggyback at time of fasten to base truss top chord. Ply Piggyback plates may be staggered 4' o.c. front to back faces



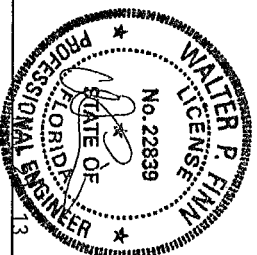
Building Components Group Inc.

Earth City MO 63045

IMPORTANT READ AND FOLLOW ALL NOTES ON THIS DRAWING. FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and allow the latest edition of BCIS (Building Component Safety Information, by TPI and VITA) for safety and bracing information. Truss manufacturers shall provide temporary bracing per BCIS. Trusses shall be braced in accordance with the BCIS. Trusses shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCIS sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details unless noted otherwise. Refer to drawings 1604-2 for standard plate positions.

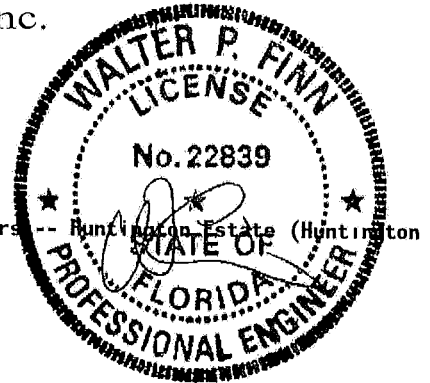
Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of the truss. The responsibility of the building designer is the responsibility of the building designer per ANSI/TPI 1. See drawing for any structure. The responsibility of the building designer per ANSI/TPI 1. See drawing for more information see this job's general notes page and these web sites: TIVBCG: www.tbvbcg.com, TPI: www.tpiinc.org, VITA: www.vita-industry.org, ICC: www.iccsd.org



REF	PIGGYBACK
DATE	2/14/12
DRWG	PB160100212
SPACING	24'0"

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V31487-Z0115131106



01/15/2014

Walter P. Finn
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Truss Fabricator **Anderson Truss Company**
Job Identification **13-281--Columbia County Bldrs Ass /Columbia County Builders**
Truss Count **1**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Version 12.03.**
Structural Engineer of Record **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61015-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 130 MPH ASCE 7-10 -Closed

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Details: 160TL-

#	Ref	Description	Drawing#	Date
1	76811-H3	repair 11'7"	14015003	01/15/14

ALPINE

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR
1'7" Steepdown Hip Girder)

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

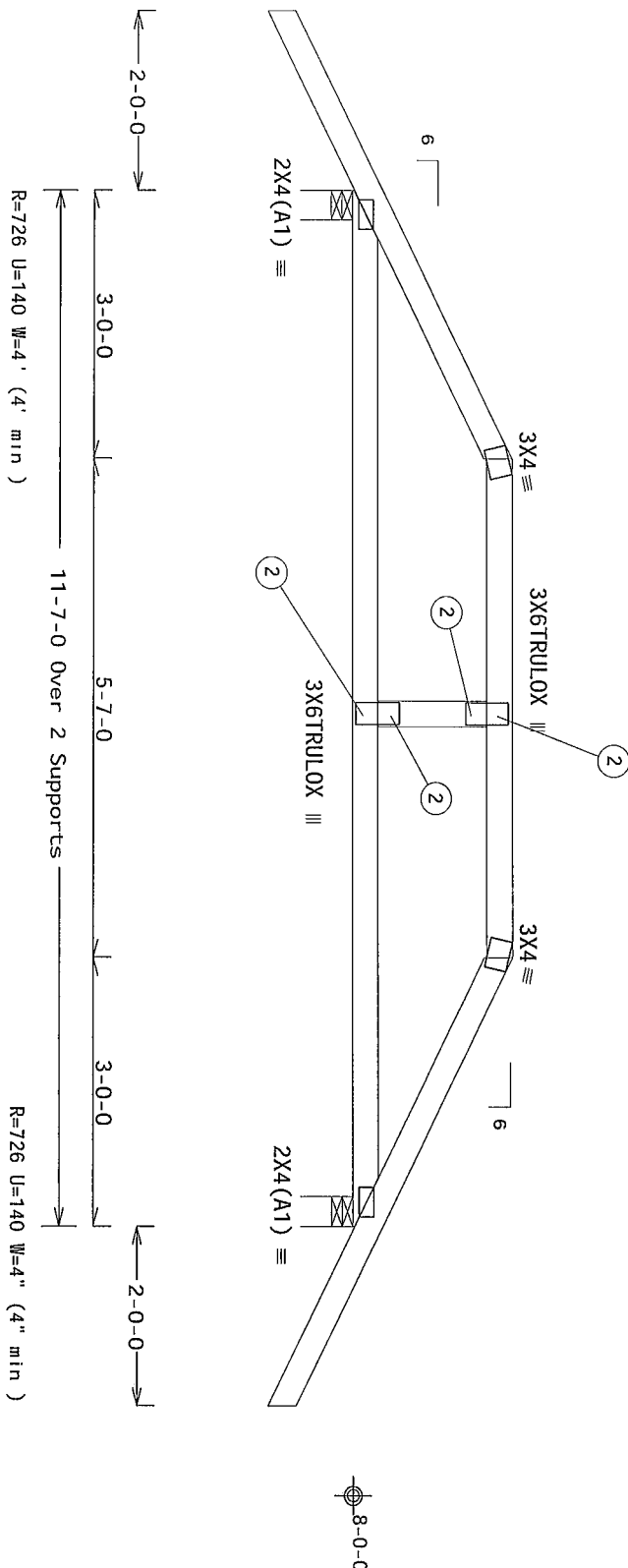
130 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, not located within 4 50 ft from roof edge, RISK CAT II, EXP B, wind TC DL=3 5 psf, wind BC DL=5 0 psf GCPI (+/-)=0 18

Wind loads and reactions based on MWFRS

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets L/240 live and L/180 total load Creep increases factor for dead load is 1.50

Bottom chord checked for 10 00 psf non-concurrent live load



PLT TYP Wave, Trulox

Design Crit	FBC2010Res/TP1 -2007(STD)
FT/RT=10%(0%)/0(0)	

12.03.04 ~~0326~~ 13

QTY:1 FL/-/5/-/-/R/-

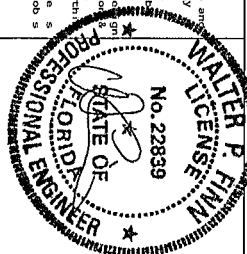
Scale = .5"/Ft.

ALPINE

ITV Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

****IMPORTANT**** ****WARNING**** **READ AND FOLLOW ALL NOTES ON THIS SHEET!**
FINISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS
 Trusses require extreme care in fabricating, handling, and bracing. Refer to the following notes and drawings for details. Follow the design and drawings for safety and follow the latest edition of BCSI (Building Component Safety) Information by TPI and WTCO for safety practices per or to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached r/d of 1/4" or 1/2" as shown for permanent lateral restraint of web. Shall have one (1) installed per BCSI sect. 83. B7 or B10 as applicable.
 TPI Building Components Group Inc. (TIBCO) shall not be responsible for any deviation from the design or any failure to build the truss in conformance with ANSI/TPI 1 or for handling shipping or installation of any truss. Apply plates to each corner of truss and post it on as shown above and on the joint. Drawing of corner plate list on this sheet. Drawings 1000-2 for standard plate and 1000-3 for a seal on the joint.
 The responsibility for safety for the design, design, and use of this design for any structure is the responsibility of the Building Design Group. The availability and use of this design for any structure is the responsibility of the Building Design Group. For more information, on see this job's general notes page. TIBCO www.tibco.com TPI www.tpinet.org WTCO www.stc-industry.com
 IBC www.icbsite.org

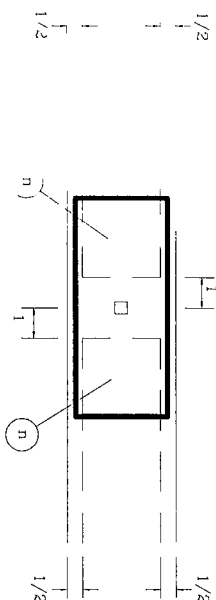


01/15/2014

TC LL	20.0 PSF	REF	R9114- 76811
TC DL	7.0 PSF	DATE	01/15/14
BC DL	10.0 PSF	DRW	HCU8R9114 14015003
BC LL	0.0 PSF	HC-ENG	JB/WMPF
TOT.LD.	37.0 PSF	SEQN-	24462
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V31487_Z01

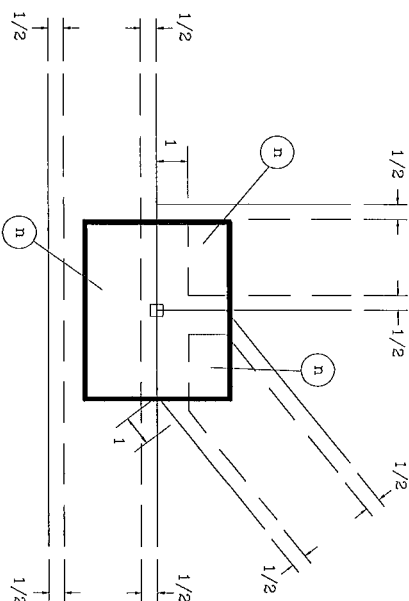
TRULOX INFORMATION DETAIL

TYPICAL OFF PANEL SPLICE

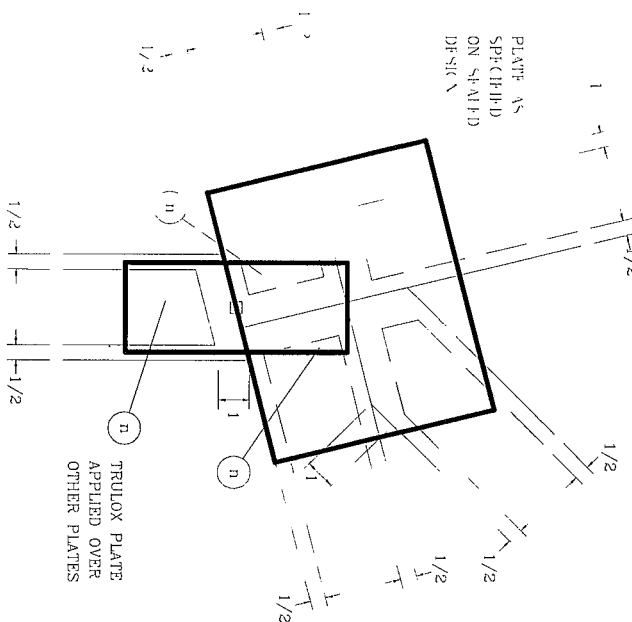


DO NOT APPLY NAILS WITHIN 1/2" OF LUMBER EDGES OR 1" OF LUMBER ENDS ON EACH FACE AS SHOWN BY DASHED LINES
NAILS MUST NOT SPLIT LUMBER

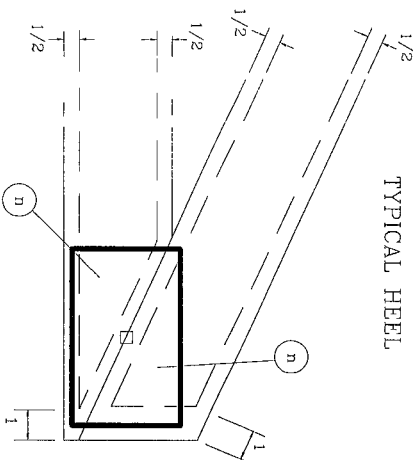
TYPICAL PANEL POINT WITHOUT SPLICE



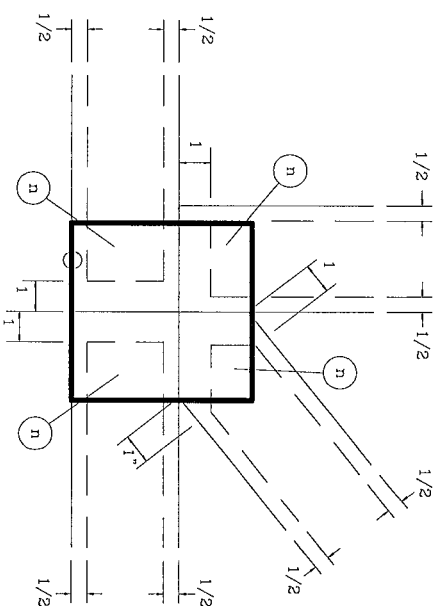
TYPICAL FILLER



TYPICAL HEEL



TYPICAL PANEL POINT SPLICE

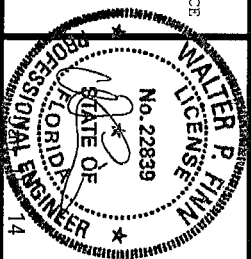


Building Components Group Inc.

Earth City, MO 63045

NOTES

- (n) IS THE REQUIRED NUMBER OF 0.120 X 1.375 NAILS OR EQUAL, PER FACE PER PLATE AS SPECIFIED ON THE SEALED DESIGN REFERENCE THIS DETAIL.
- LOCATES PLATE CORNER OR FLUSH EDGE
- LOCATES PLATE CENTER



TRULOX PLATING

160

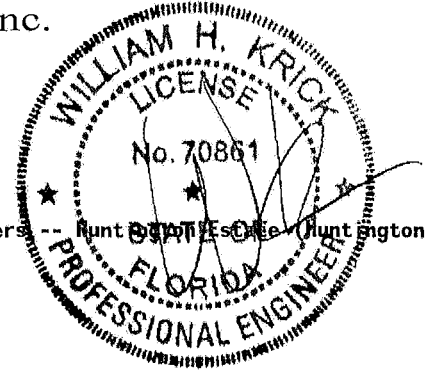
TT

PAGE 1 OF 1
DATE 4/1/09

01/15/2014

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID IV31487-Z0415133529



Truss Fabricator	Anderson Truss Company
Job Identification	13-281--Columbia County Bldrs Ass /Columbia County Builders -- Hunt Bldg Est Co (Huntington
Truss Count	1
Model Code	Florida Building Code 2010
Truss Criteria	FBC2010Res/TPI-2007(STD)
Engineering Software	Alpine Software, Version 12.03.
Structural Engineer of Record	The identity of the structural EOR did not exist as of
Address	the seal date per section 61015-31.003(5a) of the FAC
Minimum Design Loads	Roof - 37.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 120 MPH ASCE 7-10 -Closed

01/15/2014

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

William H Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: -

#	Ref	Description	Drawing#	Date
1	81579-EJ7Cr 7 ¹	End Jac	14015027	01/15/14



THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MEMBER Jack BC scab repair)

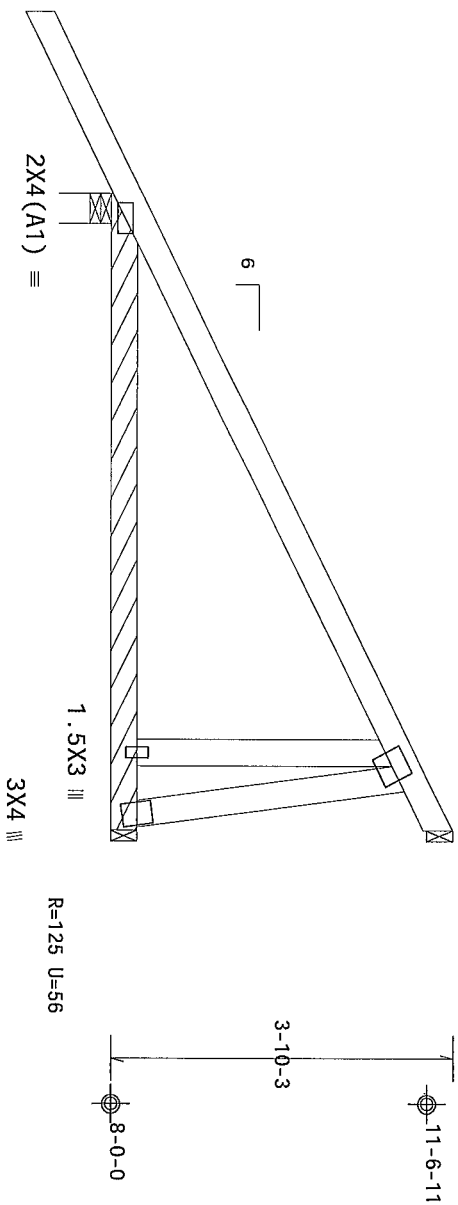
120 mph wind, 15 00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located
anywhere in roof, RISK CAT 11, EXP B, wind TC DL=3 5 psf, wind BC
DL=5 0 psf GCPI(+/-)=0 18

Wind loads and reactions based on MMFRS with additional C&C member design

Deflection meets $\sqrt{240}$ live and $\sqrt{180}$ total load Creep increases factor for dead load is 1.50

MMFRS loads based on trusses located at least 7 50 ft from roof edge

$4 \times 4 \equiv R=128 \quad R_w=35 \quad U=57$



7-0-0 Over 3 Supports (4' min)

12 03 04 0326.13

QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

ALPINE

ITV Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

*** IMPORTANT *** UNIFORM THIS DECISION TO ALL CONTRACTORS INCLUDING INSTALLERS

Trustees require extreme care in their cat rig handling shipping installing and bracing
to follow the latest edition of BCS's Building Component Safety Information on TP1 and WTCA's
practices prior to performing these functions. Installers shall provide temporary bracing
Uniform noted otherwise so no chord shall have permanent structural sheath ng and bolts
shall have been ng installed per BCS sheet as BS 80 or 81 as shown for permanent lateral restraint
ITW Build ng Components General Inc. (IMBOS) shall also supply cable
Any failure due to the trusses in conformance with ANSI/TPI
Deduct s unless noted otherwise As Refer to draw ngs 16A-2-Z for standard plate positions
drawing ng or cover plates listing this drawing The suitability and use of th s design for any
the responsibility of the designer and the user of the design for the intended application
www .itwbuildng.com ITW-BDC www .itwbuildng.com TPI www .nrc.org WTCA www .theindustry.com
CSC general notes page ITW-BDC

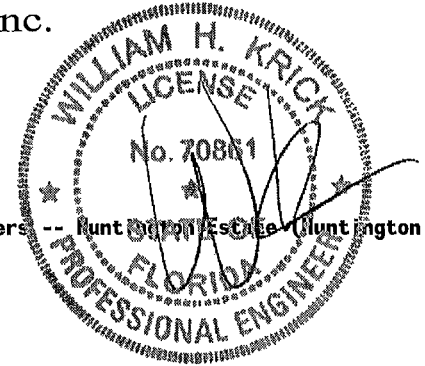
01/15/2014

TC LL	20.0 PSF	REF	R9114- 81579
TC DL	7.0 PSF	DATE	01/15/14
BC DL	10.0 PSF	DRW	HGUSR9114 14015027
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT.LD.	37.0 PSF	SEQN-	24814
DUR.FAC.	1.25		
SPACING	24.0"	JREF-	1V31487_Z04

Permit # 31631

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V31487-Z0415133529



Truss Fabricator **Anderson Truss Company**
Job Identification. **13-281--Columbia County Bldrs Ass /Columbia County Builders -- Hunt Station Escalade Huntington**
Truss Count **1**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Version 12.03.**
Structural Engineer of Record. **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61G15-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

01/15/2014

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

William H Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: -

#	Ref	Description	Drawing#	Date
1	81579-EJ7Cr	7' End Jac	14015027	01/15/14

Top chord 2x4 SP #1-13B
Bot chord 2x4 SP #1-13B
Webs 2x4 SP #3-13B

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

Bottom chord checked for 10.00 psf non-concurrent live load.

(1) 2x4X7-0-0 SP #1-13B Bottom chord scab centered 3-6-0" from left end. Attach to one face of chord with (2) rows of 0.131"x3", min. nails @ 6" O.C., staggered 3".

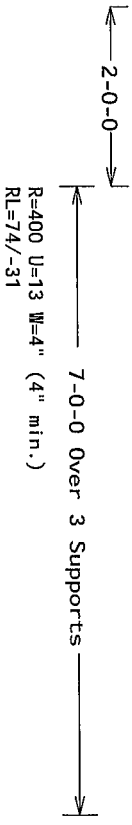
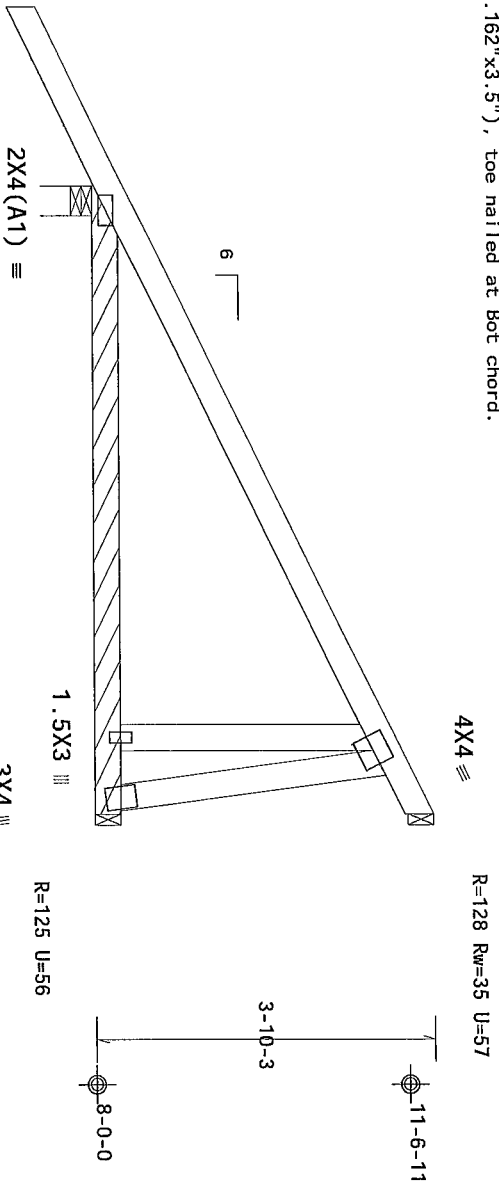
Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Top chord. Provide (2) 16d common nails(0.162"x3.5"), toe nailed at Bot chord.

120 mph wind, 15.00 ft mean hgt, ASCE 7-10, CLOSED bldg, Located anywhere in roof, RISK CAT II, EXP B, wind TC DL=3.5 psf, wind BC DL=5.0 psf, GCPI(+/-)=0.18

Wind loads and reactions based on MMFRS with additional C&C member design.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

MMFRS loads based on trusses located at least 7.50 ft. from roof edge.



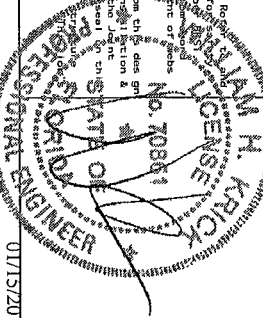
PLT TYP. Wave Design Crit FBC2010Res/TPI-2007(STD) FT/RT=10%(0%)/0(0) 12 03 04 0326.13 QTY:1 FL/-/5/-/-/R/- Scale =.5"/Ft.

****IMPORTANT**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Follow the latest edition of BCSI (Building Safety Information) by TPI and WTCA for all practices prior to performing these functions. Installers shall provide temporary bracing unless noted otherwise. Top chord shall have properly attached structural sheathing and bracing. All bracing shall be properly installed per BCSI Section 83, 87 or 810 as applicable. Truss Building Components Group Inc. (TBCGI) shall not be responsible for any damage to property or injury to persons resulting from the use of this design. Details of trusses shall be applied to each face of truss and position as shown above and indicated on drawing or cover page listing this drawing. Indicates acceptance of professional engineering responsibility by the engineer. For more information see the corresponding TPI/ASCE/IBC/IRC/MSR/11-11-12 For more information see the corresponding TPI/ASCE/IBC/IRC/MSR/11-11-12 For more information see the corresponding TPI/ASCE/IBC/IRC/MSR/11-11-12

ALPINE

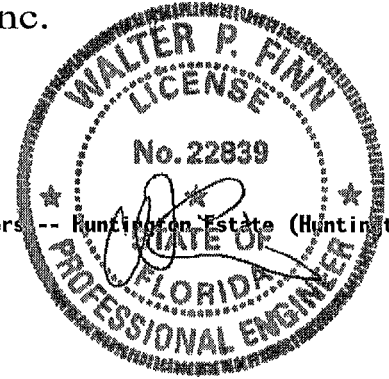
Truss Building Components Group Inc.
Orlando FL 32837
FL COA #0278



TC LL	20.0 PSF	REF	R9114- 81579
TC DL	7.0 PSF	DATE	01/15/14
BC DL	10.0 PSF	DRW	HCUSR9114 14015027
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD.	37.0 PSF	SEQN-	24814
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1V31487_Z04

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
 Florida Engineering Certificate of Authorization Number 0 278
 Florida Certificate of Product Approval # FL1999
 Page 1 of 1 Document ID 1V31487-Z0115131106



01/15/2014

Walter P Finn
 -Truss Design Engineer-

1950 Marley Drive
 Haines City, FL 33844

Truss Fabricator **Anderson Truss Company**
 Job Identification. **13-281--Columbia County Bldrs Ass /Columbia County Builders -- Huntington Estate (Huntington**
 Truss Count. **1**
 Model Code **Florida Building Code 2010**
 Truss Criteria. **FBC2010Res/TPI-2007(STD)**
 Engineering Software **Alpine Software, Version 12.03.**
 Structural Engineer of Record **The identity of the structural EOR did not exist as of**
 Address **the seal date per section 61G15-31.003(5a) of the FAC**
 Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 130 MPH ASCE 7-10 -Closed

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

Details: 160TL-

#	Ref	Description	Drawing#	Date
1	76811-H3	repair 11'7"	14015003	01/15/14

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MEFR
1'7" Stepdoun Hip Girder)

Lumber grades designated with "13B" use design values approved 1/30/2013 by ALSC

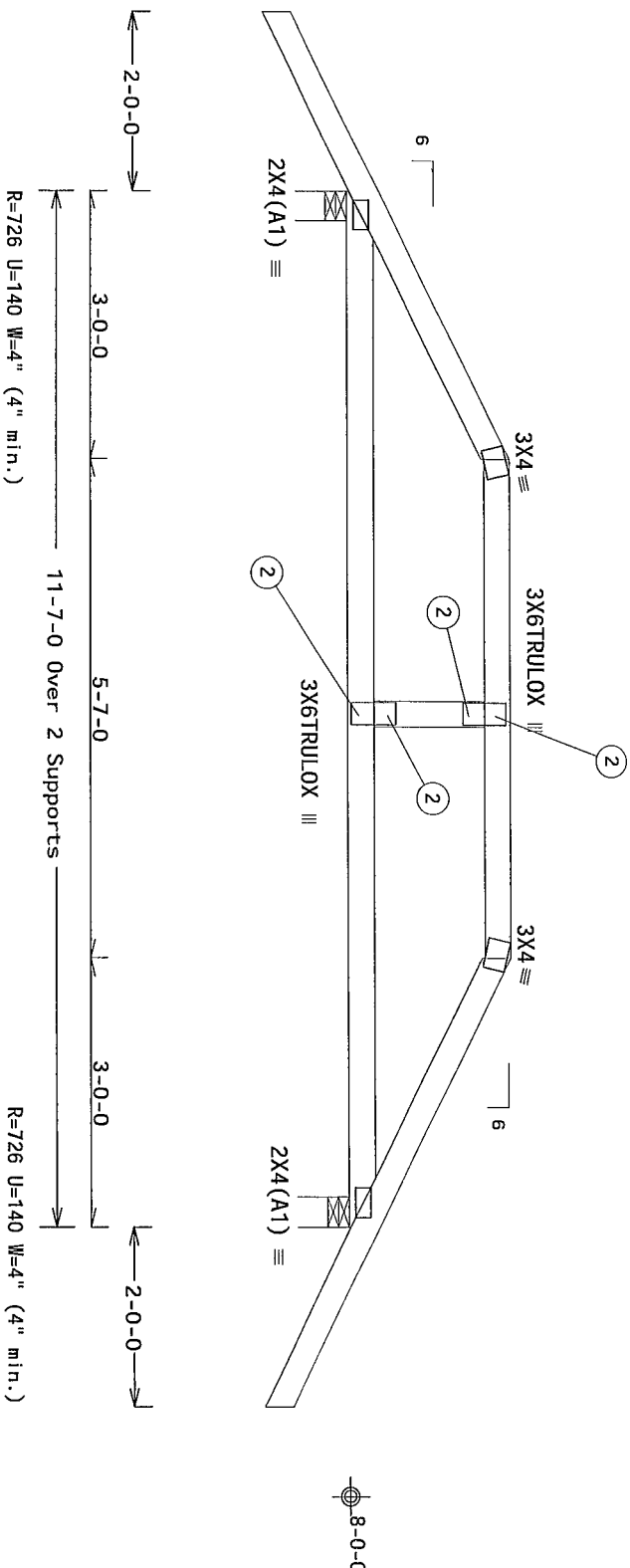
Wind loads and reactions based on MMFRS.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC

Deflection meets $L/240$ live and $L/180$ total load. Creep increase factor for dead load is 1.50

Bottom chord checked for 10.00 psf non-concurrent live load.

11 GAUGE (0.120")X1.375" nails required for trulox plate attachment. Nails specified in circles must be applied to each face of each truss ply. See DWG 160TL for nailing and trulox plate requirements.



PLT TYP. Wave, Truiox

Design Crit: FBC2010Res/TP1-2007(STD)
FT/RT=10%(0%)/0(0)

12.03.04 0235 13

QTY: 1

FL/-/5/-/-/R/-

Scale = .5"/Ft.

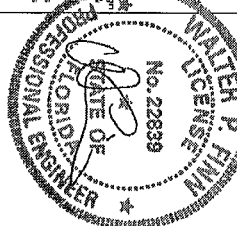
ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

****WARNING**** READ AND FOLLOW ALL NOTES ON THIS SHEET!
****IMPORTANT**** FURNISH THIS DESIGN TO ALL CONTRACTORS INCLUDING INSTALLERS

Unless required, crane cranes are fabricating, handling, installing and bracing. Refer to the latest edition of BCSI (Build n Component Safety Information by TPI and BCSA) for safety practices and to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural slush and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint or web shall have bracing installed per BCSI sections 83, 87 or 810 as applicable.

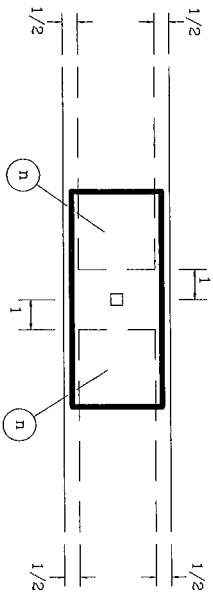
[illegible]

~~01/15/2014~~

TC LL	20.0 PSF	REF R9114- 76811
TC DL	7.0 PSF	DATE 01/15/14
BC DL	10.0 PSF	DRW HCSR9114 14015003
BC LL	0.0 PSF	HC-ENG JB/M/PF
TOT. LD.	37.0 PSF	SEQN- 24462
DUR. FAC.	1.25	
SPACING	24.0"	JREF- 1V31487_Z01

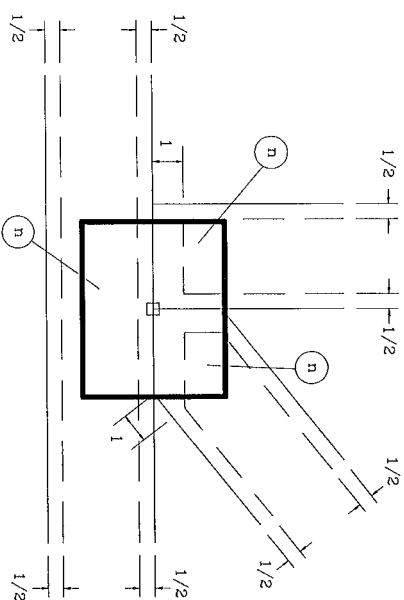
TRULOX INFORMATION DETAIL

TYPICAL OFF PANEL SPLICE

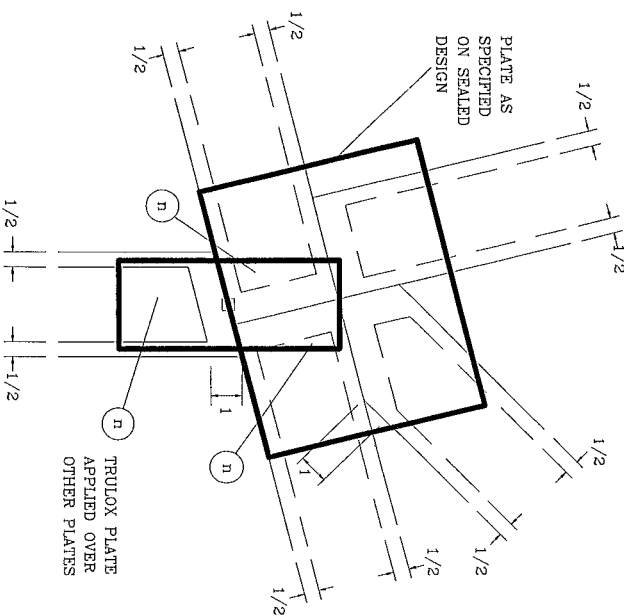


DO NOT APPLY NAILS WITHIN 1/2 OF LUMBER EDGES OR 1 OF LUMBER ENDS ON EACH FACE AS SHOWN BY DASHED LINES
NAILS MUST NOT SPLIT LUMBER

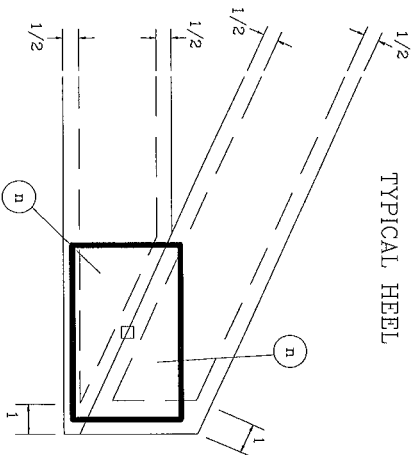
TYPICAL PANEL POINT WITHOUT SPLICE



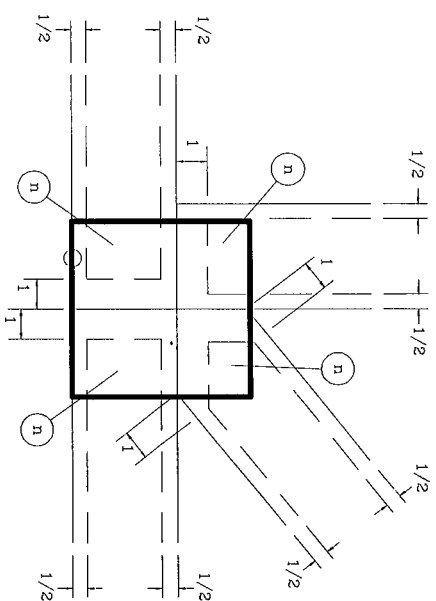
TYPICAL FILLER



TYPICAL HEEL



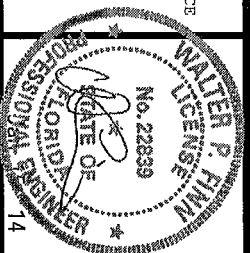
TYPICAL PANEL POINT SPLICE



- NOTES
- (n) IS THE REQUIRED NUMBER OF 0.120 X 1.375 NAILS OR EQUAL, PER FACE PER PLY AS SPECIFIED ON THE SEALED DESIGN REFERENCING THIS DETAIL
 - LOCATES PLATE CORNER OR FLUSH EDGE
 - LOCATES PLATE CENTER



Earth City MO 63045



01/15/2014

TRULOX PLATING

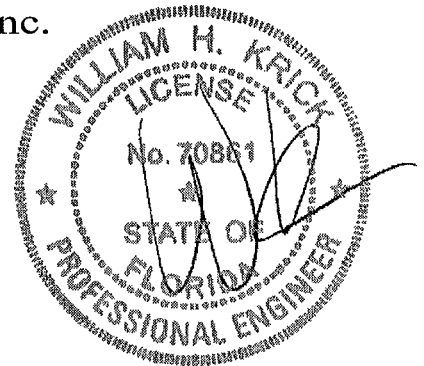
160
TL

PAGE 1 OF 1
DATE 4/1/09

Permit # 31631

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V329114Z0215110638



Truss Fabricator: Anderson Truss Company
Job Identification: REPAIR / 13-281 -H9 -BRYAN ZE (REPAIR / 13 281-)
Truss Count: 1
Model Code: Florida Building Code 2010
Truss Criteria: FBC2010Res/TPI 2007(STD)
Engineering Software: Alpine Software, Version 13.02.
Structural Engineer of Record: The identity of the structural EOR did not exist as of
Address: the seal date per section 61615-31.003(5a) of the FAC
Minimum Design Loads: Roof - 37.0 PSF @ 1.25 Duration
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

01/15/2014

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

William H Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: -

#	Ref	Description	Drawing#	Date
1	82924--H9	Jack Scab	14015002	01/15/14

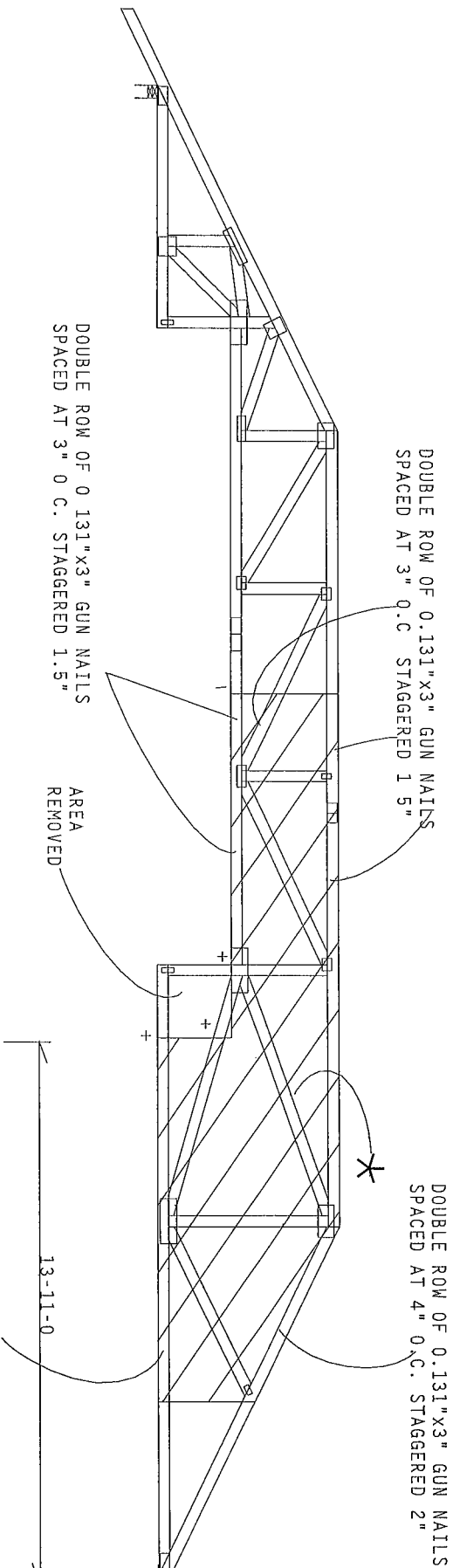
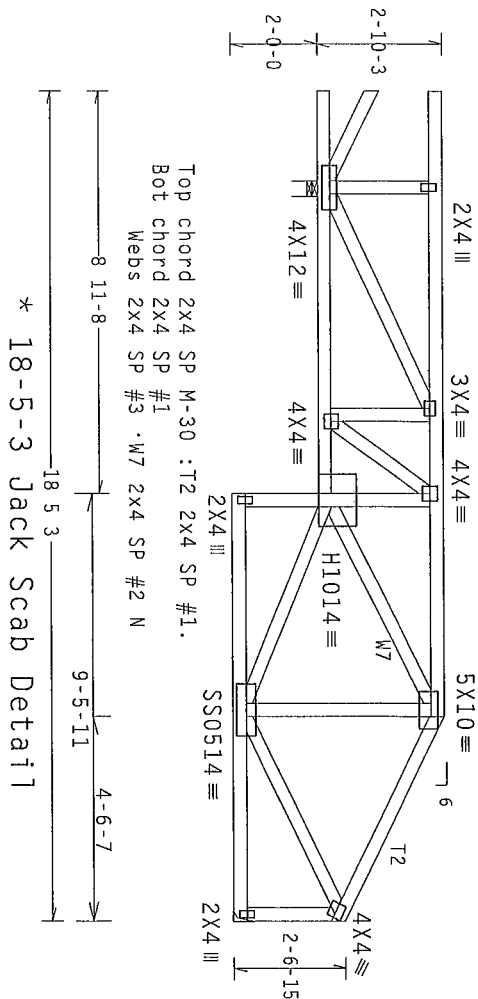
THIS DNG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR

FIELD REPAIR MUST COMPLY WITH ALPINE DESIGNS AND SPECIFICATIONS
REFER TO DRAWING HCUSR9114 13326028 FOR LUMBER, PLATES AND OTHER
DATA NOT GIVEN HERE

NOTE PRIOR TO AND DURING THE REPAIR OPERATION, THIS TRUSS AND ANY SUPPORTED SPANS MUST BE TEMPORARILY BRACED AND SHORED THE DESIGN AND POSITIONING OF THIS BRACING AND SHORING TO BE DESIGNED BY OTHERS.

+ USE A SHARP METAL CUTTING SAW BLADE TO CAREFULLY REMOVE UNWANTED MATERIAL FROM THE TRUSS AS SHOWN DASHED PORTIONS OF TRUSS MUST BE FREE FROM DAMAGE.

* ONE (1) JACK SCAR, CHORDS AND WEBS AS SHOWN IN DETAIL ONE (1) JACK SCAR REQUIRED ATTACH JACK SCAR TO ONE FACE OF TRUSS WITH A DOUBLE ROW OF 0.131"x3" GIN NAILS SPACED AT 24" O C STAGGERED 12" THROUGHOUT MATCHING MEMBERS, UNLESS OTHERWISE NOTED. REFER TO DRAWING CMMALSP0109 FOR ADDITIONAL NAIL SPACING INFORMATION.



PLT TYP.	20 Gauge HS, 18 Gauge HS, Wave	Design Crit	FBC2010Res/TPI-2007(STD)
			FT/RT=10%(0%)/0(0)

13 02 00 09 26 18

QTY:0 FL/-/3/-/-/R/-

Scale = .25" / Ft.

TRUSS REPAIR

ALPINE

ITW Building Components Group Inc.

Orlando FL, 32837
FL COA #0278

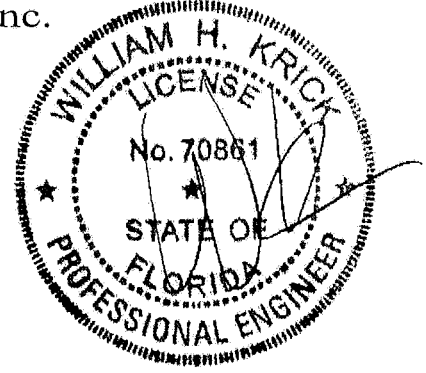
DAMAGED FIBERS MUST BE CAREFULLY EVALUATED TO DETERMINE THE EXTENT OF DAMAGE AND THE FEASIBILITY OF REPAIR. IN SOME CASES THE PRUDENT SOLUTION IS TO SCRAP THE DAMAGED TROUSSER AND REBUILD INTERNAL MOOD FIBER DAMAGE AND EXCESSIVE CONNECTION STRESS FROM REMOVAL OR SHOCK CANON BE READILY DETECTED THEREFORE IT IS VITAL THAT THE TROUSER FABRICATOR AND BUILDING CONTRIBUTOR, CONSIDER THE CAUSE OF THE DAMAGE IN THEIR DECISION WHETHER TO REPAIR OR REBUILD

REPAIR WORK SHOWN ON THIS DRAWING APPLIES ONLY TO THOSE SECTIONS OF THE TROUSER REPORTED BY THE TROUSER MANUFACTURER TO HAVE BEEN DAMAGED. A QUALIFIED THIRD PARTY INSPECTOR SHALL CHECK TROUSERS TO DETERMINE THE EXTENT OF ANY FURTHER DAMAGE. IF ANY DAMAGE IS DETECTED, THE TROUSER MANUFACTURER SHALL BE NOTIFIED IMMEDIATELY AND REPAIR WORK AND VERIFICATION THAT REPAIRS HAVE BEEN PERFORMED AS INDICATED ON THIS DRAWING

TC LL	20.0 PSF	REF	R9114- 82924
TC DL	7.0 PSF	DATE	01/15/14
BC DL	10.0 PSF	DRW	HCSR9114.14015002
BC LL	0 0 PSF	HC-ENG	WHK/WHK
TOT LD	37 0 PSF	SEQN-	5139
DUR.FAC	1.25		
SPACING	24.0"	JREF-	1V329114202

ITW Building Components Group, Inc.

2400 Lake Orange Drive suite 150 Orlando FL 32837
Florida Engineering Certificate of Authorization Number 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID 1V329114Z0215110638



Truss Fabricator **Anderson Truss Company**
Job Identification **REPAIR / 13-281 -H9 -BRYAN ZE (REPAIR / 13-281-)**
Truss Count **1**
Model Code **Florida Building Code 2010**
Truss Criteria **FBC2010Res/TPI-2007(STD)**
Engineering Software **Alpine Software, Version 13.02.**
Structural Engineer of Record **The identity of the structural EOR did not exist as of**
Address **the seal date per section 61015-31.003(5a) of the FAC**
Minimum Design Loads **Roof - 37.0 PSF @ 1.25 Duration**
Floor - N/A
Wind - 120 MPH ASCE 7-10 -Closed

01/15/2014

Notes

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR9114

William H Krick
-Truss Design Engineer-

1950 Marley Drive
Haines City, FL 33844

Details: -

#	Ref	Description	Drawing#	Date
1	82924--H9	Jack Scab	14015002	01/15/14

ALPINE

(REPAIR / 13-281 -H9 -BRYAN ZE - H9 Jack Scab)

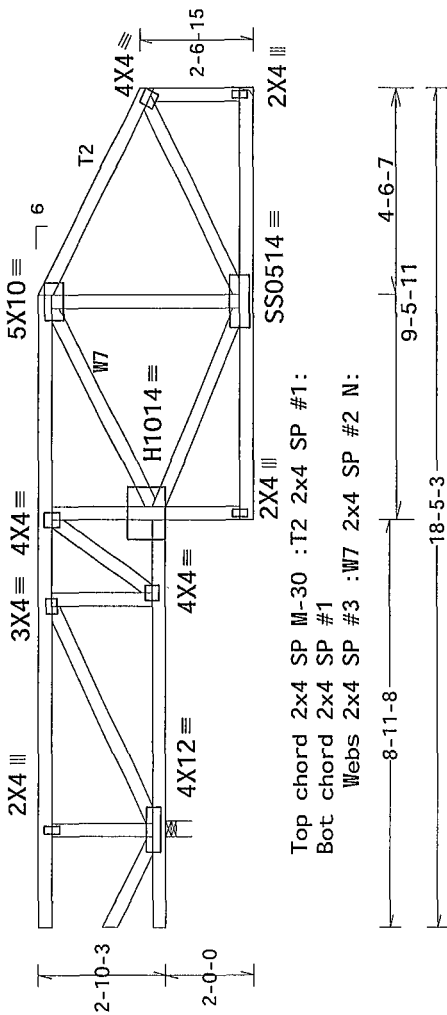
REPAIR TO CHANGE EXTERIOR GEOMETRY OF TRUSS SHOWN BELOW

FIELD REPAIR MUST COMPLY WITH ALPINE DESIGNS AND SPECIFICATIONS
REFER TO DRAWING HCUSR9114 13326028 FOR LUMBER, PLATES AND OTHER
DATA NOT GIVEN HERE

NOTE PRIOR TO AND DURING THE REPAIR OPERATION, THIS TRUSS AND
ANY SUPPORTED SPANS MUST BE TEMPORARILY BRACED AND SHORED
THE DESIGN AND POSITIONING OF THIS BRACING AND SHORING TO
BE DESIGNED BY OTHERS

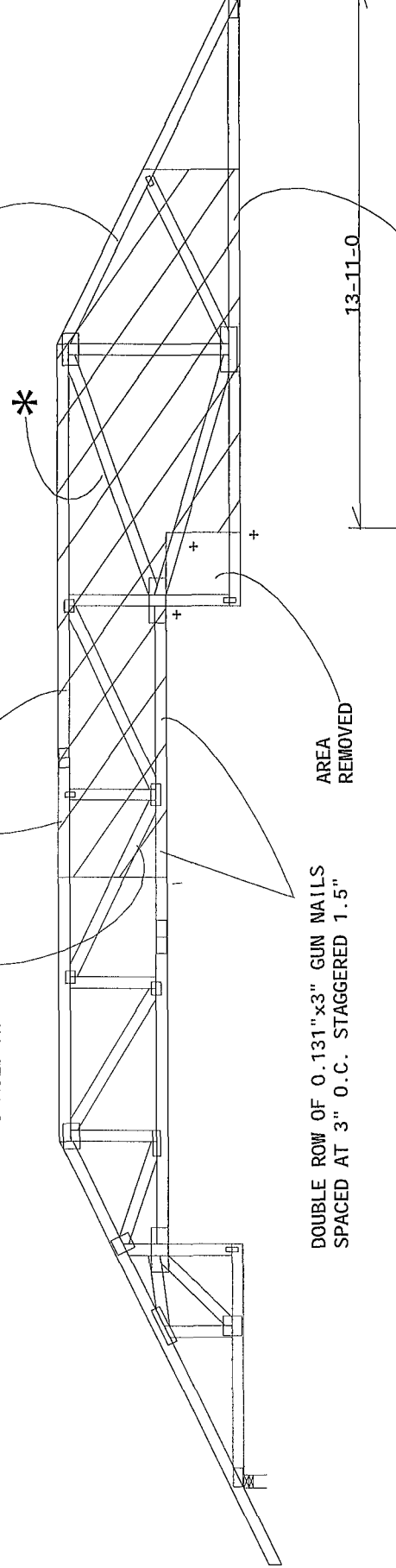
+ USE A SHARP METAL CUTTING SAW BLADE TO CAREFULLY REMOVE
UNWANTED MATERIAL FROM THE TRUSS AS SHOWN DASHED REMAINING
PORTIONS OF TRUSS MUST BE FREE FROM DAMAGE

* ONE (1) JACK SCAB CHORDS AND WEBS AS SHOWN IN DETAIL ONE
(1) JACK SCAB REQUIRED ATTACH JACK SCAB TO ONE FACE OF
TRUSS WITH A DOUBLE ROW OF 0.131"x3" GUN NAILS SPACED AT
24" O.C. STAGGERED 12" THROUGHOUT MATCHING MEMBERS, UNLESS
OTHERWISE NOTED REFER TO DRAWING CNAILSP0109 FOR ADDITIONAL
NAIL SPACING INFORMATION



* 18-5-3 Jack Scab Detail

DOUBLE ROW OF 0.131"x3" GUN NAILS
SPACED AT 3" O.C. STAGGERED 15"



DOUBLE ROW OF 0.131"x3" GUN NAILS
SPACED AT 3" O.C. STAGGERED 1.5"

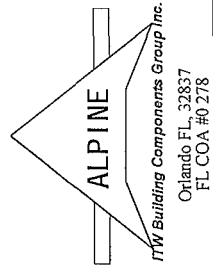
DOUBLE ROW OF 0.131"x3" GUN NAILS
SPACED AT 4" O.C. STAGGERED 2"

PLT TYP 20 Gauge HS, 18 Gauge HS, Design Crit: FBC2010Res/TPI-2007(STD)
Wave FT/RT=10%(0%)/0(0)

TRUSS REPAIR

DAMAGED TRUSSES MUST BE CAREFULLY EVALUATED TO DETERMINE THE EXTENT OF DAMAGE
AND THE FEASIBILITY OF REPAIR. IN SOME CASES THE PRUDENT SOLUTION IS TO SCRAP
THE DAMAGED TRUSSES AND REBUILD. INTERNAL WOOD FIBER DAMAGE AND EXCESSIVE
CONNECTOR STRESS FROM BENDING OR SHOCK CANNOT BE READILY DETECTED THEREFORE
IT IS VITAL THAT THE TRUSS FABRICATOR AND BUILDING CONTRACTOR CONSIDER THE
CAUSE OF THE DAMAGE IN THEIR DECISION WHETHER TO REPAIR OR REBUILD

REPAIR WORK SHOWN ON THIS DRAWING APPLIES ONLY TO THOSE SECTIONS OF THE TRUSS
REPORTED BY THE TRUSS MANUFACTURER TO HAVE BEEN DAMAGED. A QUALIFIED THIRD PARTY
INSPECTOR SHALL CHECK TRUSSES TO DETERMINE THE EXTENT OF ANY FURTHER DAMAGE. IF ANY
AND VERIFY THAT REPAIRS HAVE BEEN PERFORMED AS INDICATED ON THIS DRAWING



13.02.000-0926.18

QTY: 0 FL/-/3/-/-/R/- Scale = .25"/Ft.

TC LL	20.0 PSF	REF	R9114- 82924
TC DL	7.0 PSF	DATE	01/15/14
BC DL	10.0 PSF	DRW	HCUSR9114 14015002
BC LL	0.0 PSF	HC-ENG	WHK/WHK
TOT. LD.	37.0 PSF	SEQN-	5139
DUR. FAC.	1.25		
SPACING	24.0"	JREF-	1V329114Z02

