	STRUCTURAL ABBREVIATION	S				DESIGN CRITERIA				
# NUMBER OR POUND Ø ROUND OR DIAMETER □ SQUARE & AND @ AT ALUM ALUMINUM AR ANCHOR ROD ARCH ARCHITECTURE ASSY ASSEMBLY B/ BOTTOM OF BETWN BETWEEN BLDG BUILDING BM BEAM BOT BOTTOM BPL BASE PLATE BRG BEARING C TO C CENTER TO CENTER CANT CANTILEVER CJ CONSTRUCTION JOINT CL CENTER LINE CLR CLEAR CMU CONCRETE MASONRY UNIT COL COLUMN CONC CONCRETE CONN CONNECTION CONT CONTRACTION CONT CONTRACTION CONT CONTRACTION CONT CONTRACTION CONT CONTRACTION CTR CENTER DBA DEFORMED BAR ANCHOR <td>GA GAGE GALV GALVANIZED GB GRADE BEAM GC GENERAL CONTRACTOR GT GIRDER TRUSS HGT HEIGHT HORIZ HORIZONTAL HSA HEADED STUD ANCHOR HT HIP TRUSS IF INSIDE FACE INFO INFORMATION INT JOINT JST JOIST LB POUND LG LONG LEG HORIZONTAL LLV LONG LEG HORIZONTAL LLV LONG LEG VERTICAL LSV LONG SIDE HORIZONTAL LSV LONG SIDE VERTICAL MANUF MANUFACTURER MAT'L MATERIAL MAX MAXIMUM MECH MECHANICAL MIN MINIMUM MISC MISCELLANEOUS NIC NOT IN CONTRACT NO NUMBER NS NEAR SIDE NTS NOT TO SCALE OC ON CENTER OD OUTSIDE DIAMETER</td> <td>S SCHED SECT SIM SOG SPA SPEC SQ STD STIFF STL STR STRUCT SYM T&B T/ T/U PANEL TE THK THRD TRANSV TS TYP UON VER VERT W/ W/ W/ W/ W/ W/ W/ W/ W/ W/ W/ W/ W/</td> <td>SCHEDULE SECTION SIMILAR SLAB ON GRADE SPACE SPECIFICATION SQUARE STANDARD STIFFENER STEEL STRAIGHT STRUCTURAL SYMMETRICAL TOP & BOTTOM TOP OF TILT-UP PANEL THICKENED EDO THICK THREADED TRANSVERSE THICKENED SLA TYPICAL UNLESS OTHER VERIFY VERTICAL WITH WITHOUT WORK POINT WATERSTOP WELDED WIRE F</td> <td>E B WISE NOTEI</td> <td></td> <td>DESIGN CRITERIA DESIGN PER 2020 FL IVE LOADS: ROOFS AND CANOF DEAD LOADS (SUPER ROOF VIND LOADS (SUPER NOMINAL WIND SP MEAN ROOF HEIGHT RISK CATE WIND EXPOSURE _ ENCLOSURE CLASS INTERNAL PRESSU DIRECTIONALITY F/ SHAPE FACTORS _ RAIN LOADS: RAIN LOADS: RAIN LOADS: RAIN LOADS: RAIN LOADS: RAIN LOADS: RAIN LOADS: RAIN LOADS CONCRETE (DESIGN SLAB ON GRADE _ FOOTINGS ALL OTHER CONCF ALL REINFORCING S DRAWINGS TO BE W ACCORDANCE WITH WELDED WIRE FABR CONCRETE MASONF COMPRESSIVE ST STRUCTURAL STEEL OTHER SHAPES OTHER SHAPES & F HSS SQUARE & REI HSS ROUND SHAPE STEEL PIPES</td> <td>ORIDA BUIL PIES (REDU RIMPOSED PEED: (ASCI EED GORY SIFICATION RE COEFFI ACTOR (Kd) PER CURR ELDED SHA ELDED SHA AWS D1.4. IC RENGTH _ (DESIGN P RIALS SHAL PLATES CTANGULA ES</td> <td>LDING COD</td> <td>DE, UNLESS OTHERWISE</td> <td>NOTED. </td>	GA GAGE GALV GALVANIZED GB GRADE BEAM GC GENERAL CONTRACTOR GT GIRDER TRUSS HGT HEIGHT HORIZ HORIZONTAL HSA HEADED STUD ANCHOR HT HIP TRUSS IF INSIDE FACE INFO INFORMATION INT JOINT JST JOIST LB POUND LG LONG LEG HORIZONTAL LLV LONG LEG HORIZONTAL LLV LONG LEG VERTICAL LSV LONG SIDE HORIZONTAL LSV LONG SIDE VERTICAL MANUF MANUFACTURER MAT'L MATERIAL MAX MAXIMUM MECH MECHANICAL MIN MINIMUM MISC MISCELLANEOUS NIC NOT IN CONTRACT NO NUMBER NS NEAR SIDE NTS NOT TO SCALE OC ON CENTER OD OUTSIDE DIAMETER	S SCHED SECT SIM SOG SPA SPEC SQ STD STIFF STL STR STRUCT SYM T&B T/ T/U PANEL TE THK THRD TRANSV TS TYP UON VER VERT W/ W/ W/ W/ W/ W/ W/ W/ W/ W/ W/ W/ W/	SCHEDULE SECTION SIMILAR SLAB ON GRADE SPACE SPECIFICATION SQUARE STANDARD STIFFENER STEEL STRAIGHT STRUCTURAL SYMMETRICAL TOP & BOTTOM TOP OF TILT-UP PANEL THICKENED EDO THICK THREADED TRANSVERSE THICKENED SLA TYPICAL UNLESS OTHER VERIFY VERTICAL WITH WITHOUT WORK POINT WATERSTOP WELDED WIRE F	E B WISE NOTEI		DESIGN CRITERIA DESIGN PER 2020 FL IVE LOADS: ROOFS AND CANOF DEAD LOADS (SUPER ROOF VIND LOADS (SUPER NOMINAL WIND SP MEAN ROOF HEIGHT RISK CATE WIND EXPOSURE _ ENCLOSURE CLASS INTERNAL PRESSU DIRECTIONALITY F/ SHAPE FACTORS _ RAIN LOADS: RAIN LOADS: RAIN LOADS: RAIN LOADS: RAIN LOADS: RAIN LOADS: RAIN LOADS: RAIN LOADS CONCRETE (DESIGN SLAB ON GRADE _ FOOTINGS ALL OTHER CONCF ALL REINFORCING S DRAWINGS TO BE W ACCORDANCE WITH WELDED WIRE FABR CONCRETE MASONF COMPRESSIVE ST STRUCTURAL STEEL OTHER SHAPES OTHER SHAPES & F HSS SQUARE & REI HSS ROUND SHAPE STEEL PIPES	ORIDA BUIL PIES (REDU RIMPOSED PEED: (ASCI EED GORY SIFICATION RE COEFFI ACTOR (Kd) PER CURR ELDED SHA ELDED SHA AWS D1.4. IC RENGTH _ (DESIGN P RIALS SHAL PLATES CTANGULA ES	LDING COD	DE, UNLESS OTHERWISE	NOTED.
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ROOF PLAN (GENE	ERIC BUILDING SHOWN)	OVERHANG ZO	ONE 2 -63.0 -57	.2 -49.5	-43.7	-37.9 -33.1	-30.2	-30.2		
		OVERHANG ZO	ONE 3 -63.0 -57	.2 -49.5			-30.2	-30.2		
	/ /		RAPET PRESSURF			50 SF 100 SF	200 SF	500 SF		
		CASE A	ZON	IE 2: 90.1	84.3	76.5 70.7	64.9	57.2		
 h h f h f h f	4P 5 4 .6h	CASE B AREA NEGATIVE ZONE 4 NEGATIVE ZONE 5 POSITIVE ZONE 4	ZON INTERIOR ZO CORNER ZO WALL SU 10 SF 4 -32.1 5 -39.5 & 5 29.6	IE 3: 90.1 DNE: -53.2 DNE: -60.8 IRFACE	84.3 -50.5 -56.8 PRESS 100 SF -27.7 -30.8 25.2	76.5 70.7 -47.0 -44.3 -51.4 -47.4 CURES 200 SF -26.4 -28.1 23.9	64.9 -41.6 -43.4 500 -2 -2 -2 22	57.2 -38.0 -38.0 0 SF 4.7 4.7 2.2		
		NOTES: 1) TABLE PRESS FOR OTHER T LINEARLY INTI 2) POSITIVE PRE NEGATIVE PRI 3) SEE DIAGRAM 4) PRESSURES S MULTIPLY VAL	URES ARE FOR THE S RIBUTARY AREAS, ERPOLATE BETWEEN SSURES ACT TOWARI ESSURES ACT AWAY F IS FOR LOCATION OF 2 SHOWN ARE ULTIMATE UES BY 0.6 FOR NOMI	QUARE FOO VALUES SHO D THE BUILD FROM THE B ZONES. E PRESSURE NAL PRESSI	DT (SF) TRIBU DWN ABOVE ING. UILDING. ES, JRES.	JTARY AREA SHOWI	N. 			

WALLS (GENERIC DUILDING SHOWN) WIND PRESSURE DIAGRAMS

2 S0.1 SCALE: NTS

GENERAL NOTES

FOUNDATION

IF FOOTING ELEVATIONS SHOWN OCCUR IN A DISTURBED, UNSTABLE, OR UNSUITABLE SOIL, THE ENGINEER SHALL BE NOTIFIED.

THE BOTTOM OF ALL FOUNDATIONS SHALL EXTEND A MINIMUM OF 18 INCHES BELOW THE TOP OF FINISH GRADE.

STEPS IN WALL FOOTINGS SHALL NOT EXCEED A SLOPE OF ONE (1) VERTICAL TO TWO (2) HORIZONTAL. <u>CONCRETE</u>

UNLESS OTHERWISE NOTED (UON) ON THE DRAWINGS, MINIMUM COVER FOR REINFORCING SHALL BE AS FOLLOWS:

SLABS ON GRADE _ _ _ _ 2" FROM TOP ALL REINFORCING SHALL BE HELD SECURELY IN POSITION WITH STANDARD ACCESSORIES IN CONFORMANCE WITH CRSI MANUAL OF STANDARD PRACTICE AND ACI 315 DURING THE PLACING OF THE CONCRETE.

UNLESS OTHERWISE NOTED, SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE AS FOLLOWS:

ALL HOOKS IN REINFORCING BARS SHALL BE AN ACI STANDARD HOOK, UNLESS OTHERWISE NOTED.

DOWELS FROM FOUNDATIONS OR SLABS TO WALLS SHALL MATCH WALL REINFORCING, UNLESS OTHERWISE NOTED. DOWELS SHALL BE PLACED BEFORE CONCRETE IS POURED. THEY SHALL NOT BE PUSHED INTO THE CONCRETE.

MASONRY

ALL LOAD BEARING WALLS AND EXTERIOR WALLS SHALL BE COMPOSED OF ASTM C90 HOLLOW CONCRETE MASONRY UNITS WITH ASTM C270 TYPE "S" MORTAR. GROUT SHALL CONFORM TO THE REQUIREMENTS OF ASTM C476 AND HAVE A COMPRESSIVE STRENGTH OF 3000 PSI.

ALL EXTERIOR CMU WALLS SHALL BE REINFORCED FULL HEIGHT IN A GROUT FILLED CELL WITH 1-#5 AT:

• EA CORNER, WALL ENDS, WALL INTERSECTIONS EA SIDE OF CONTROL JOINTS AND

AT A MAXIMUM SPACING OF 4'-0" OC, UON

• SEE DETAIL 1/S3.1 FOR TYPICAL REINFORCING AT WALL OPENINGS. AT BEAM & JOIST GIRDER BEARING LOCATIONS ADD REINFORCING AS SHOWN IN PLAN.

LAPPED BARS SHALL BE SECURED WITH WIRE TIES OR OTHER MEANS TO ENSURE THAT THE BAR IS NOT DISPLACED DURING GROUT PLACEMENT OUTSIDE THE TOLERANCES ESTABLISHED BY ACI 530. LAP BARS WITH THE FOLLOWING MINIMUM LENGTH.

BAR	BARS CTR'D	BARS CTR'D
EA FACE	<u>8" CMU</u>	<u>12" CMU</u>
16"	16"	16"
26"	21"	21"
40"	26"	26"
54"	43"	40"
63"	60"	46"
	BAR <u>EA FACE</u> 16" 26" 40" 54" 63"	BAR BARS CTR'D <u>EA FACE</u> 8" CMU 16" 16" 26" 21" 40" 26" 54" 43" 63" 60"

GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT IN 5'-0" MAXIMUM LIFTS. DO NOT BEGIN PLACEMENT OF GROUT UNTIL ALIGNMENT OF CELLS ARE INSPECTED AND APPROVED.

FILL ALL CELLS BELOW FINISHED GRADE.

PROVIDE HORIZONTAL JOINT REINFORCEMENT IN WALLS AT 16" OC VERTICALLY, UON. IN ADDITION. INSTALL JOINT REINFORCING IN THE FIRST TWO MORTAR JOINTS ABOVE & BELOW OPENINGS, EXTENDING AT LEAST 24 INCHES BEYOND THE OPENING. PROVIDE HORIZONTAL JOINT REINFORCEMENT IN PARAPETS AND FREE STANDING WALLS & 8" OC VERTICALLY. LAP JOINT REINFORCEMENT 6" MINIMUM. HORIZONTAL REINFORCING SHALL CONSIST OF AT LEAST TWO W1.7 WIRES OR GREATER.

SEE ARCHITECTURAL DRAWING FOR EXPANSION OR CONTROL JOINTS. IF NOT SHOWN, LOCATE VERTICAL CONTROL JOINTS AT 25'-0" OC MAXIMUM, BUT NOT LESS THAN 2'-0" FROM A JOIST OR BEAM BEARING PLATE. AT BUILDING CORNERS, PROVIDE ONE JOINT IN ONE OF THE TWO WALL SIDES NO MORE THAN 5'-0" FROM THE BUILDING CORNER. HORIZONTAL REINFORCING SHALL CONSIST OF W1.7 JOINT REINFORCEMENT OR GREATER.

ALL PRECAST OR POURED LINTELS SHALL BE REINFORCED WITH TWO #4 TOP & BOTTOM WITH #3 TIES @ 12" AS A MINIMUM AND HAVE A MINIMUM MASONRY END BEARING OF 8".

BOND/TIE BEAM REINFORCEMENT SHALL BE CONTINUOUS ACROSS CONTROL JOINTS.

16" U-BLOCK OR BOND BEAM SHALL CONSIST OF TWO 8" KNOCK-OUT BLOCKS.

BARS SPECIFIED TO BE EA FACE SHALL BE HELD IN PLACE WITH SPACERS AND SHALL BE LOCATED 2 3/8" FROM EA FACE TO THE CENTER OF THE BAR.

MASONRY WORK SHALL BE INSPECTED IN ACCORDANCE WITH TMS 402 QUALITY ASSURANCE LEVEL 2.

a=8.80 FT



FASTENERS

EXPANSION BOLTS SHALL BE HILTI KWIK BOLT 3, SIMPSON STRONG-TIE STRONG-BOLT2, DEWALT POWER-STUD+ SD1 OR APPROVED EQUAL, UON. EMBEDMENT DEPTH INTO CONCRETE OR SOLID GROUTED MASONRY SHALL BE AT LEAST 7 TIMES THE BOLT DIAMETER UON. CLEAN HOLE AND INSTALL IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

SCREW ANCHORS SHALL BE HILTI KWIK HUS-EZ, SIMPSON STRONG-TIE TITEN HD, DEWALT SCREW-BOLT+ OR APPROVED EQUAL, UON. EMBEDMENT IN CONCRETE OR SOLID GROUTED MASONRY SHALL BE AT LEAST 9 TIMES THE BOLT DIAMETER, UON. CLEAN HOLE AND INSTALL IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS.

POWER ACTUATED FASTENERS (PAF) SHALL BE 0.157" DIAMETER HILTI X-U, SIMPSON STRONG-TIE PDPA, DEWALT CSI PIN OR EQUAL, UON. EMBED MIN 1-1/4" INTO CONCRETE AND CMU, UON. DO NOT PLACE WITHIN 1" OF CMU MORTAR JOINT. PAF SHALL COMPLETELY PENETRATE STRUCTURAL STEEL.

- ADHESIVE ANCHORING (EPOXY):
- ADHESIVE ANCHORING FOR CONCRETE SHALL BE HILTI RE-500 V3 CARTRIDGE SYSTEM, SIMPSON STRONG-TIE SET-3G, DEWALT PURE 110+ (OR EQUIVLANT ACRYLIC AC200+, HY 200, OR ATXP) OR APPROVED EQUAL, UON. EMBEDMENT DEPTH SHALL BE AT LEAST 12 TIMES THE INSERT DIAMETER, UON. HOLE DIAMETER SHALL BE NO GREATER THAN RECOMMENDED BY MANUFACTURER. THE HOLE SHALL BE CLEANED PER MANUFACTURER'S RECOMMENDATIONS BY BRUSHING OUT WITH WIRE BOTTLE BRUSH AND BLOWN OUT WITH AIR USING A COMPRESSOR WITH A FUNCTIONAL OIL TRAP (EXCEPT WHERE PERMITTED WHEN USING A DUST EXTRACTION SYSTEM IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS).
- ADHESIVE ANCHORING FOR MASONRY SHALL BE HILTI HIT-HY 70 OR HY 270 CARTRIDGE SYSTEM, SIMPSON STRONG-TIE SET-XP, DEWALT AC100+ OR APPROVED EQUAL, UON. EMBEDMENT DEPTH INTO SOLID GROUTED MASONRY SHALL BE AT LEAST 9 TIMES THE INSERT DIAMETER, UON. HOLE DIAMETER SHALL BE NO GREATER THAN RECOMMENDED BY MANUFACTURER. HOLES SHALL NOT BE PLACED WITHIN 1" OF A VERTICAL MORTAR JOINT. CLEAN HOLE IN ACCORDANCE WITH MANUFACTURER'S PRINTED INSTRUCTIONS.
- GENERAL ANCHORS SHALL MEET THE REQUIREMENTS OF ACI 355.4. INSTALLATION SHALL BE IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTRUCTIONS AND PERFORMED BY AN INSTALLER TRAINED BY THE MANUFACTURER. INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY, WHICH SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED BY A CERTIFIED ADHESIVE ANCHOR INSTALLER IN ACCORDANCE WITH ACI318 AND CONTINUOUSLY INSPECTED PER ACI318. PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL.
- CAPACITIES UON, DESIGN BOND STRENGTH OF ANCHORS HAVE BEEN BASED ON CRACKED CONCRETE, ACI 355.4 TEMPERATURE CATEGORY B, AND INSTALLATIONS INTO DRY HOLES DRILLED WITH A ROTARY IMPACT DRILL OR ROCK DRILL INTO CONCRETE THAT HAS CURED AT LEAST 21 DAYS AND HAS A CONCRETE TEMPERATURE OF AT LEAST 50 DEGREES F AT TIME OF ANCHOR INSTALLATION.

WOOD

CLIPS, CONNECTIONS, HANGERS, HOLD-DOWNS, ETC. SHOWN ON THESE DRAWINGS ARE SIMPSON STRONG-TIE CONNECTORS, UON. FASTENERS OF OTHER MANUFACTURERS MAY BE SUBSTITUTED PROVIDED THE LOAD VALUES OF THE SUBSTITUTED FASTENER FOR GROUP II WOOD SPECIES EQUALS OR EXCEEDS THE SPECIFIED FASTENER.

ALL NAILS SHALL BE COMMON WIRE NAILS UNLESS SHOWN OTHERWISE OR MANUFACTURE'S CONNECTOR LITERATURE SPECIFIES OTHERWISE

NAILING OF ALL MEMBERS SHALL BE IN ACCORDANCE WITH THE BUILDING CODE. SEE CODE FOR TABLE.

ALL LUMBER USED IN EXTERIOR APPLICATIONS, INCLUDING: BALCONY DECK BOARDS, LEDGER, JOISTS, BEAMS, WOOD IN CONTACT WITH EXTERIOR MASONRY OR CONCRETE SLABS OR WALLS, AND SILL PLATES EXPOSED TO CONCRETE SHALL BE TREATED IN ACCORDANCE WITH AWPA U1. USE CATEGORY 2 FOR SILL PLATES, CATEGORY 3B FOR EXTERIOR MEMBERS, AND CATEGORY 4A FOR WOOD IN GROUND CONTACT. SEE AWPA U1 FOR ALL OTHER CASES.

SILL PLATE BOLT AND ANCHOR BOLT WASHERS SHALL BE 1/8"x2"x2" AT BEARING LOCATIONS WITH UPLIFT.

ROOF SHEATHING SHALL BE 19/32" MINIMUM APA RATED SHEATHING, EXPOSURE 1 WITH 32/16 SPAN RATING. HOWEVER, 7/16" MINIMUM APA RATED SHEATHING, EXPOSURE 1 WITH 24/16 SPAN RATING MAY BE USED FOR ASPHALT SHINGLED OR STANDING SEAM METAL ROOFS.

ROOF DECKING SHALL BE NAILED WITH 8D NAILS AT 7/16"& 1/2" DECK AND 10d NAILS AT 5/8" & 3/4" DECK. SPACE NAILS AT 6" AT SUPPORTED EDGES OF DECK (4" AT EXTERIOR WALLS) AND 12" SPACING AT INTERMEDIATE SUPPORTS. AT GABLE ENDS, NAIL ROOF DECK AT 4" AT PANEL EDGES AND AT 6" AT INTERMEDIATE SUPPORTS FOR A DISTANCE OF ?'-?" FROM THE END WALL

PROVIDE 2x4 BLOCKING FOR SUPPORT OF ROOF SHEATHING AT HIPS AND VALLEYS.

WALL SHEATHING SHALL BE 7/16" MINIMUM APA RATED SHEATHING, EXPOSURE 1 WITH 24/16 SPAN RATING. SHEATHING MAY BE ORIENTED VERTICALLY OR HORIZONTALLY FOR FLEXIBLE WALL FINISHES. SHEATHING MUST BE ORIENTED HORIZONTALLY FOR BRITTLE WALL FINISHES (STUCCO) UNLESS STRUCTURAL 1 RATED SHEATHING OR 15/32" 5-PLY/5-LAYER PLYWOOD OR 15/32" OSB IS USED.

HOLES AND NOTCHES MUST BE APPROVED BY THE ENGINEER. IF APPROVED THE NOTCHES ON THE ENDS OF JOISTS SHALL NOT EXCEED ONE-FOURTH THE DEPTH. HOLES BORED FOR PIPE OR CABLE SHALL NOT BE WITHIN THE TOP OR BOTTOM THIRD OF THE JOIST DEPTH AND THE DIAMETER OF SUCH HOLE SHALL NOT EXCEED ONE-THIRD THE JOIST DEPTH NOTCHES FOR PIPES IN THE TOP OR BOTTOM OF JOISTS SHALL NOT EXCEED ONE-SIXTH THE JOIST DEPTH AND SHALL NOT BE LOCATED IN THE MIDDLE ONE-THIRD OF THE SPAN.

STRUCTURAL GLUED LAMINATED TIMBER DESIGN PER CURRENT NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION "NDS" BY THE NATIONAL FOREST PRODUCTS ASSOCIATION, STANDARD SPECIFICATION FOR STRUCTURAL GLUED LAMINATED TIMBER OF SOFTWOOD, AND AITC 119 BY THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC). ALL MEMBERS MUST BE MANUFACTURED IN ACCORDANCE WITH THE CURRENT EDITION OF THE U.S. DEPARTMENT OF COMMERCE VOLUNTARY PRODUCT STANDARD PS.56 "STRUCTURAL GLUED LAMINATED TIMBER".

LVL SHALL BE MICROLLAM 2.0E BY "TRUS JOIST" OR EQUAL, PSL SHALL BE PARALLAM 2.0E BY "TRUS JOIST" OR EQUAL; FOR COLUMNS – PARALLAM 1.8E BY "TRUS JOIST" OR EQUAL, AND LSL SHALL BE TIMBERSTRAND 1.5E BY "TRUS JOIST" OR EQUAL. PSL BEAMS GREATER THAN 18" DEEP SHALL BE PARALLAM 2.2E BY "TRUS JOIST" OR EQUAL.

STRESS GRADE: SOUTHERN PINE NO. 2 OR ENGINEER APPROVED EQUAL. ALL DESIGN VALUES ARE UNDER NORMAL LOADING AND IN DRY CONDITIONS OF SERVICE.

PRESSURE-TREAT LUMBER IN ACCORDANCE WITH THE MANUAL OF RECOMMENDED PRACTICE OF THE AMERICAN WOOD PRESERVERS ASSOCIATION (AWPA).

ALL FASTENERS AND NAILS IN CONTACT WITH PRESSURE TREATED LUMBER SHALL BE MADE OF TYPE 304 OR TYPE 316 STAINLESS STEEL OR ASTM A653 TYPE G185 ZINC COATED STEEL UNLESS THE LUMBER IS TREATED WITH CCA, MCA, MCQ, uCA OR SBX (DOT), [BUT NOT SBX (DOT) WITH SODIUM SILICATE (NaSiO2)]. EXCEPT AT SWIMMING POOLS AND WITHIN 5 MILES OF SALT WATER STAINLESS STEEL MUST BE USED IF IN CONTACT WITH COPPER BASED PRESERVATIVES.

UNLESS OTHERWISE NOTED, USE THE FOLLOWING MINIMUM GRADE OF LUMBER FOR FRAMING.

<u>FRAMING</u> SILL ON FOUNDATION WALLS OR SLAB ON GRADE	MINIMUM GRADE NO. 3 SYP
JOISTS, RAFTERS & HEADERS	NO. 2 SYP
PLATES, CAPS & BUCKS	NO. 2 SYP
STUDS	SEE SCHEDUI
POSTS & COLUMNS (INCLUDE ALL VERTICAL MEMBERS SPECIFICALLY	NO. 2 SYP

CALLED OUT, I.E. 3 - 2x4)

WOOD CONT

CONNECT OVER FRAMING (SUCH AS VALLEY TRUSSES) TO MAIN ROOF FRAMING BELOW WITH SIMPSON VTCR WITH 4-10d NAILS INTO TRUSS AND 5-10d x 1 1/2" NAILS INTO OVERFRAMING OR 1 1/4"x16 ga TWIST STRAP @ 48" MAX w/ 4-10d NAILS EA END OF STRAP.

AND CC OR CCQ SERIES AT CAP.

JOIST HANGERS SHALL BE SIMPSON SERIES LUS, UON.

CONVENTIONAL FASTENING AND STRAPPING HAVE BEEN SHOWN ON THESE DRAWINGS TO RESIST WIND LOADING. AN ALTERNATE SYSTEM USING FULL HEIGHT BOLTED RODS, CABLES, ETC. (SUCH AS QUICK-TIE). MAY BE SUBMITTED AS AN ALTERNATE. SUBMIT DETAILED SHOP DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY A REGISTERED ENGINEER FOR APPROVAL PRIOR TO FABRICATION. THE SUBMITTAL SHALL ADDRESS THE FOLLOWING: THE TOP PLATE MUST BE CONSIDERED AS TWO SEPARATE MEMBERS FOR TRANSFERRING UPLIFT FORCES TO THE ALTERNATE SYSTEM, UNLESS IT IS NAILED TOGETHER TO ACT AS ONE MEMBER (I.E. vQ/It CALCULATION REQUIRED).

- WOOD WALL SHEATHING MAY BE USED TO TRANSFER THE UPLIFT FORCES PROVIDED ADDITIONAL NAILING IS PROVIDED AT THE TOP PLATE (SEE "SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC" BY AWC FOR ACCEPTABLE TABLES AND DETAILING). NAIL SPACING AT SHEARWALLS WILL NEED TO BE INCREASED.
- GYPSUM BOARD WALL SHEATHING (EXCEPT AT GYPBOARD SHEARWALLS) MAY BE USED TO TRANSFER UPLIFT FORCES PROVIDED SPECIAL NAILING IS PROVIDED. IF WALL SHEATHING IS USED TO TRANSFER UPLIFT THE HURRICANE CLIPS SHALL BE INSTALLED UNDER THE SHEATHING AND ON THE SAME SIDE OF THE WALL.

SHEARWALL SILL ANCHOR BOLT SPACING MUST BE PER THE SHEARWALL SCHEDULE. HOWEVER, EACH BOLT FOR THE ALTERNATE SYSTEM MAY REPLACE ONE SCHEDULED ANCHOR BOLT.

- SHEARWALL HOLDOWNS AND FLOOR TO FLOOR STRAPPING AT END POSTS MUST BE PROVIDED AS SCHEDULED UNLESS AN ICC REPORT OR OTHER TESTING IS PROVIDED SHOWING THAT LATERAL SHEARWALL DEFLECTION IS WITHIN ACCEPTABLE LIMITS.
- CABLE SYSTEMS MUST BE PRE-TENSIONED TO RESIST UPLIFT LOADING. SUBMIT TENSIONING PROCEDURE FOR APPROVAL.

TRUSSES AND CONVENTIONAL FRAMING WILL BE FASTENED WITH CONVENTIONAL FASTENERS AS SHOWN ON THESE DRAWINGS, UNLESS AN ALTERNATE PROCEDURE IS SUBMITTED FOR APPROVAL.

- RODS OR CABLES SHALL BE TIED OFF @ EA FLOOR. IF NOT, END POSTS SPECIFIED AT THE GROUND FLOOR SHALL BE USED FULL HEIGHT.
- FOR BUILDINGS OVER A SINGLE STORY, TAKE UP DEVICES SHALL BE USED WITH ROD SYSTEMS TO ACCOMMODATE WOOD SHRINKAGE AND CABLE SYSTEMS SHALL BE DESIGNED AND DETAILED TO ACCOMMODATE WOOD SHRINKAGE.

WOOD TRUSSES

TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS INDICATING ACTUAL TRUSS LAYOUT, DESIGN, WIND UPLIFT AT BEARING LOCATIONS, NUMBER AND TYPES OF TRUSSES, ETC. SHOP DRAWINGS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY A REGISTERED PROFESSIONAL ENGINEER. TRUSS MANUFACTURER SHALL COORDINATE AND VERIFY ALL TRUSS DIMENSIONS AND DESIGNS WITH ARCHITECT'S DRAWINGS.

ROOF FRAMING PLAN AND TRUSS TYPES ARE DIAGRAMMATIC AND ARE INTENDED TO INDICATE DESIGN CONCEPT ONLY FOR ROOF CONFIGURATION.

TRUSSES SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH ANSI/TPI1 "NATIONAL DESIGN STANDARDS FOR METAL-PLATE-CONNECTED WOOD TRUSS CONSTRUCTION".

00110	
ROOF	TRUSS DESIGN CRITERIA:
LI	VE LOAD

LIVE LOAD
TILE ROOF
MIN DEAD LOAD (FOR UPLIFT) - ASPHALT SHINGLES
WIND UPLIFT

BRACE BOTTOM CHORD AS REQUIRED FOR WIND UPLIFT.

COORDINATE TRUSS LOCATIONS/CONFIGURATION WITH PLUMBING WALLS AND HVAC EQUIPMENT SO AS TO AVOID CONFLICTS. SEE MECHANICAL DRAWINGS FOR EXACT LOCATIONS OF EQUIPMENT, DUCTS, STACKS, PIPES, ETC. GENERAL CONTRACTOR SHALL ENSURE TRUSS CONFIGURATION ACCOMMODATES ALL EQUIPMENT, DUCTS, ETC.

TEMPORARY TRUSS BRACING SHALL BE INSTALLED IN ACCORDANCE WITH "RECOMMENDED DESIGN SPECIFICATIONS FOR TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES" (TPI-DSB) AND "COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" (TPI-HIB). INSTALL ALL WEB BRACING REQUIRED BY THE TRUSS DESIGNER. TEMPORARY BOTTOM CHORD AND WEB BRACING SHALL REMAIN PERMANENTLY IN PLACE. THE BOTTOM CHORD BRACING SHALL NOT EXCEED 10' SPACING FOR TRUSSES WHERE NO SHEATHING IS ATTACHED TO THE TRUSS BOTTOM CHORD OR WITH TRUSS BOTTOM FILLER. PROVIDE 2x4 LATERAL BRACING @ 36" UNDER PIGGYBACK TRUSSES. ALL BRACING SHALL BE NAILED WITH 2-16d NAILS TO TRUSSES.

AT TRUSSES REQUIRING HORIZONTAL WEB BRACING, PROVIDE 2x4 DIAGONAL BRACE (APPROX 45 DEGREES) @ 20' MAXIMUM SPACING. NAIL THE TOP END OF DIAGONAL TO WEB OF TRUSS AT ROOF, NAIL MIDDLE OF DIAGONAL TO TRUSS WEB AT HORIZONTAL LATERAL BRACING LOCATION AND THE BOTTOM END OF DIAGONAL TO BOTTOM OF WEB OF TRUSS AT CEILING.

SUPPLEMENTARY NOTES

PROVIDE ALL TEMPORARY BRACING, SHORING, GUYING OR OTHER MEANS TO AVOID EXCESSIVE STRESSES AND TO HOLD STRUCTURAL ELEMENTS IN PLACE DURING CONSTRUCTION. THE STRUCTURE SHOULD NOT BE CONSIDERED STABLE UNTIL ALL STRUCTURAL ELEMENTS HAVE BEEN CONSTRUCTED IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

MCVEIGH & MANGUM ENGINEERING, INC OR ANY OF ITS EMPLOYEES SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES OR SEQUENCES FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR OR ANY OTHER PERSONS PERFORMING THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

VISUAL OBSERVATIONS OF THE STRUCTURAL SYSTEM BY MCVEIGH & MANGUM ENGINEERING FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATIONS DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED BY THE INTERNATIONAL BUILDING CODE.

VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

SEE ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS FOR EMBEDS, OPENINGS, SLEEVES, ETC. NOT SHOWN ON THE STRUCTURAL DRAWINGS.

ALL STRUCTURAL OPENINGS AROUND OR AFFECTED BY MECHANICAL, ELECTRICAL AND PLUMBING EQUIPMENT SHALL BE VERIFIED WITH EQUIPMENT PURCHASED BEFORE PROCEEDING WITH STRUCTURAL WORK AFFECTED.

CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS OF EXISTING STRUCTURE AND SITES THAT ARE AFFECTED BY NEW WORK BEFORE PROCEEDING WITH FABRICATION AND CONSTRUCTION.

ANY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF THE PROJECT.

POST BASE AND CAPS FOR 4x4 AND 6x6 POST SHALL BE SIMPSON CB OR CBQ SERIES AT BASE

ESIGN CF	RITERIA	THIS	SHEE	ΞТ
	10 PSF	TOP (CHOR	D
	10 PSF	BOT (CHOR	D
	20 PSF	TOP (CHOR	D
	10 PSF	BOT	CHOR	D
			· 8 PS	ŝF
			15 PS	ŝF

PER CODE

SHOP DRAWINGS AND SUBMITTALS

SHOP DRAWING SUBMITTALS ARE ONLY REVIEWED FOR GENERAL CONFORMANCE WITH THE INFORMATION SHOWN ON THE CONSTRUCTION DOCUMENTS. THE GENERAL CONTRACTOR MUST REVIEW AND APPROVE THE SHOP DRAWINGS PRIOR TO THEIR SUBMITTAL TO THE ARCHITECT. SUBMITTALS WHICH DO NOT CONTAIN THE CONTRACTOR'S SHOP DRAWING STAMP SHALL BE RETURNED WITHOUT REVIEW. ANY REQUESTED CHANGES TO THE CONTRACT DOCUMENTS SHALL BE COMMUNICATED IN WRITING PRIOR TO SUBMITTING THE SHOP DRAWINGS AND CLOUDED ON THE SHOP DRAWINGS.

SHOP DRAWINGS MUST BE SUBMITTED FOR ENGINEER'S REVIEW OF THE FOLLOWING ITEMS: (S/S = SIGNED & SEALED SHOP DRAWING WITH CALCS, SD = SHOP DRAWING FOR REVIEW ONLY)

- CONCRETE REINFORCING LAYOUT CONCRETE CONSTRUCTION JOINT LAYOUT
- MASONRY REINFORCEMENT LAYOUT CONCRETE MIX DESIGNS
- WOOD TRUSS SYSTEMS MISC STEEL FABRICATIONS EXTERIOR CLADDING (CURTAINWALLS)



COMPLETE SHOP DRAWINGS FOR CONSTRUCTION OF EACH BUILDING COMPONENT NOT DESIGNED BY THE DESIGN TEAM OF RECORD AND NOT SPECIFIED ON THE PROJECT CONSTRUCTION DOCUMENTS SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT AND SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.

SOME STRUCTURAL SYSTEMS ARE DEFINED AS VENDOR-DESIGNED COMPONENTS PER THE STRUCTURAL DOCUMENTS. THESE ELEMENTS OF THE DESIGN ARE DEFERRED SUBMITTAL COMPONENTS AND HAVE NOT BEEN PERMITTED UNDER THE BASE BUILDING APPLICATION. VENDOR-DESIGNED COMPONENT SHOP DRAWINGS SHALL BE APPROVED BY THE COMPONENT DESIGNER ENGINEER PRIOR TO CURSORY REVIEW BY THE ENGINEER OF RECORD FOR LOADS IMPOSED ON THE BASE STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. SHOP DRAWINGS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON BASIC STRUCTURE. THE CONTRACTOR SHALL SUBMIT THE STAMPED COMPONENT SYSTEM DOCUMENTS TO THE BUILDING OFFICIAL FOR

SPECIFICATIONS

APPROVAL.

CONCRETE WORK SHALL CONFORM TO THE REQUIREMENTS OF ACI 301. "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS" (LATEST EDITION). EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL EMPLOY AND PAY AN INDEPENDENT TESTING LABORATORY TO PERFORM CONCRETE TESTING.

MASONRY CONSTRUCTION AND MATERIALS SHALL CONFORM TO THE REQUIREMENTS OF "SPECIFICATIONS FOR MASONRY STRUCTURES - ACI 530.1/ASCE 6" (LATEST EDITION), EXCEPT AS MODIFIED BY REQUIREMENTS OF THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL EMPLOY AND PAY AN INDEPENDENT TESTING LABORATORY TO PERFORM MASONRY TESTING.

ALL STRUCTURAL STEEL WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITIONS OF AISC "SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS". AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES", AND AWS D1.1" "STRUCTURAL WELDING CODE", EXCEPT AS MODIFIED BY THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. PROOF OF WELDER CERTIFICATION SHALL BE AVAILABLE AT THE JOB SITE DURING TIMES OF INSPECTION.

BOLTED CONNECTIONS SHALL BE ASSEMBLED AND INSPECTED IN ACCORDANCE WITH RESEARCH COUNCIL ON STRUCTURAL CONNECTIONS "SPECIFICATIONS FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".

GALVANIZING: CONFORM TO ASTM STANDARDS A 123, A 386, AND A 153 AS APPLICABLE WHEREVER SURFACES ARE INDICATED OR SPECIFIED TO BE GALVANIZED. GALVANIZE AFTER FABRICATION UNLESS OTHERWISE INDICATED OR SPECIFIED. REPAIR ALL GALVANIZED COATINGS THAT BECOME DAMAGED IN HANDLING, TRANSPORTING, WELDING, OR BOLTING. MAKE THE REPAIRS BY APPLICATION OF A GALVANIZING REPAIR PAINT CONFORMING TO ASTM A 780. CLEAN ALL AREAS THAT ARE TO BE REPAIRED; REMOVE SLAG FROM WELDS. APPLY REPAIR PAINT TO COLD SURFACES.

A GEOTECHNICAL ENGINEER SHALL BE EMPLOYED TO CONFIRM BEARING PRESSURE STATED PRIOR TO CONSTRUCTION. THE ENGINEER SHALL DEVELOP & ENSURE IMPLEMENTATION OF A SITE PREPARATION PROGRAM AS HE DEEMS NECESSARY TO ACHIEVE THE STATED BEARING PRESSURE.

FOOTING AND SLAB SUBGRADE PREPARATION SHALL BE IN COMPLIANCE WITH APPLICABLE REQUIREMENTS OF AUTHORITIES HAVING JURISDICTION.









_____ ____



SLAB ON GRADE CONN (EXIST TO NEW)

SCALE: NTS



TYP PIPE PENETRATION THRU WALL



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	FAS	TENER SCHED	DULE			
			<u>C</u>	ONNECTION		
LOCATION ROOF TRUSS (2)	<u>UPLIFT</u> <455# <600# <1200# >1200#	<u>FASTENER (1)</u> 1-H5 1-H2.5A 2-H2.5A SEE DETAIL 4/S3.1	<u>TRUSS</u> 4-8d 5-8d 5-8d	<u>PLATE</u> 4-8d 5-8d 5-8d		
SILL PLATE TO CMU V	VALLS	1/2"Ø ANCHOR BOLT PL WASHER OR "MAS	w/ 2x2x1/8" A" @ 32"			16" O
NOTES: 1) ALL CONNECTOR BE SUBSTITUTED CATALOG. ROOF SHOWN ON THE F 2) IN ADDITION TO S 3) EMBEDMENT OF A <u>ANCHOR BOLT</u> EMBEDDED AN EMBEDDED AN	S LISTED ARE SIMP . NAIL SIZE AND NUI TRUSS CLIPS SHALI ROOF TRUSS SHOP HEDULED HOLD DC ANCHOR BOLTS SH, <u>TYPE</u> CHOR @ INTERIOR CHOR @ EDGE	SON STRONG-TIE, UON MBER SHALL BE IN ACC L BE SELECTED TO PRO DRAWINGS. WN, PROVIDE 3-10d TO ALL BE AS FOLLOWS: <u>1/2'</u> 	. OTHER MANUFACTURERS ORDANCE WITH MANUFAC ⁻ OVIDE THE UPLIFT RESISTAN E NAILS. <u>'Ø <u>5/8"Ø</u> <u>3/4"Ø 7/8</u> 7" 7" 7</u>	MAY FURER'S NCE <u>8"Ø</u> "	TRUSSES	NAIL
EMBEDDED AN EPOXIED THRE EXPANSION AN EDGE DISTANCE DISTANCE FOR H BOLTS SHALL BE	CHOR IN TOP OF CM ADED ROD CHORS FOR SILL PLATE BO OLDDOWNS AND AL HEADED OR BE THF	AU WALL	9" 13" 1 EE GENERAL NOTES EE GENERAL NOTES ⁵ 1/2 OF SILL WIDTH. EDGE 1/2" MIN EMBEDDED ANCHO NUT ATTACHED TO THE EM	8" PR BEDED		NO
			:			
S4.1 SCALE: NTS		SHEDULL			2 	SCA
					2x4 BLOCKING @ EA X-BRACE	
2x4 BLOCK @ X-BRACE -	2 SF		4-12d TOE NAIL	S		
4-16d NAILS	RC La Constantina de la Consta	DOF SHEATHING				
4-12d (2 ES) TOE NAILS – EA END, FROM BLOCK TO TRUSS TOP CHORD			2x4 X-BRACE @ 48" OC*	- 4-10 SEE NOTE #	TOP CHORD OF GABLE TRUSS EXTENDS UP TO CREATE PARAPET, SEE ARCH WALL SHEATHING	3
	3-12d NAILS, TYP @ VERTICALS				BRACE VERTICAL MEMBERS O	M CH I TRU
SIMPSON A33 CLIP	SEE		2x4 SCAB NAILE	EP	GABLE END GIRDER TRUSS PER TRUSS MFR DETAILS 2-12d NAILS	
A33 CLIP (TYP EA			WEB IS NOT PRESENT		BOTTOM CHORD OF GABLE END TRUSS	
CONT 2x4x8' #2 SYP LAT BRACE @ 48" OC w/ 3-120		\boxtimes	12d @ 4" OC FROM			
EA TRUSS BOTTOM C	HORD		BLOCK BELOW 2x4 BLOCKING @ 48"		ANCHOR BOLT	
			& FIRST TRUSS CS20 STRAP w/ 7-10d & (7) 3/16"x2 1/4" LG TAPCC	NAILS		
	1) WHERE "H SHALL BE 2) WHERE "H	H" EXCEEDS 8'-0" X-BRA 2x6. H" EXCEEDS 10'-6" X-BRA				
	WEB (OR 3) CMU REIN	S SHALL BE NAILED TO SCAB) @ EA TRUSS. IF NOT SHOWN FOR CL	ARITY			
4 GABLI	E END T	RUSS ON				



NO PARAPET

TRUSS BEARING





S4.1

SCALE: NTS

- PLYWOOD SHEATHING, SEE PLAN

(N) WOOD TRUSS, SEE PLAN

0 0 0 0 PANEL EDGE BLOCKING PROVIDED $\langle 0 \rangle$ - NAILING TO INTERMEDIATE SUPPORT IS 12" OC MAX, UON 0 - STUDS, ROOF TRUSSES, OR FLOOR JOISTS

NOTES: 1) ROOF SHEATHING SHALL BE STAGGERED AS SHOWN WITH LONG DIRECTION OF PLYWOOD TRANSVERSE TO TRUSSES OR JOISTS.
UNSUPPORTED (UNBLOCKED) PANEL EDGE. PROVIDE BLOCKING IF REQ'D BY NOTES OR SCHEDULE.

NAILING REQUIREMENTS FOR ROOF SHEATHING

TO THE BEST OF THE ENGINEERS KNOWLEDGE, THE PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE MINIMUM BUILDING CODES FOR THIS PART OF THE WORK IN ACCORDANCE WITH THE APPLICABLE FLORIDA STATUTES.



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