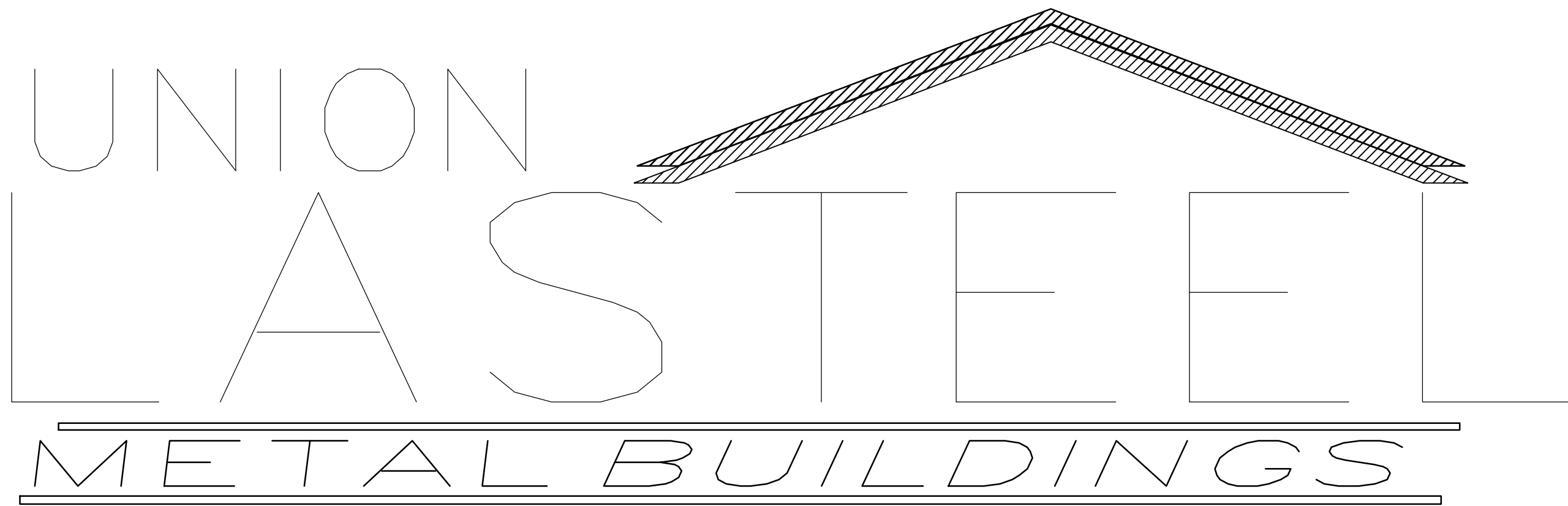




PO BOX 537 Lake Bulter, FL 32054
(386) 496-0681 (386) 496-4705 (fax)

BUILDING LOADS / DESCRIPTION:

WIDTH: 30 LENGTH: 60 HEIGHT: 11 /16
(BUILDING DIMENSIONS ARE NOMINAL. REFER TO PLANS).

THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED
AND APPLIED AS REQUIRED BY : FBC 20 .

THE CONTRACTOR IS TO CONFIRM THAT THESE LOADS COMPLY
WITH THE REQUIREMENTS OF THE LOCAL BUILDING DEPARTMENT.

ROOF DEAD LOAD: 2.000 PSF (ROOF PANELS & PURLINS)

COLLATERAL LOAD: 1 PSF SNOW EXPOSURE: 1.0000

ROOF LIVE LOAD: 20.00 PSF WIND EXPOSURE: B

ROOF SNOW LOAD: 0 PSF INTERNAL PRESSURE COEFF.:

GROUND SNOW LOAD: 0 PSF 0.18 / -0.18

BASIC WIND SPEED: 120 MPH SPECTRAL RESPONSE COEFF. MAPPED SPECTRAL RESPONSE ACC.

SEISMIC ZONE: B Sds 0.10 Ss 0.09

THERMAL FACTOR: 1.00 Sd1 0.08 St 0.05

IMPORTANCE FACTORS: DESIGN BASE SHEAR, V:

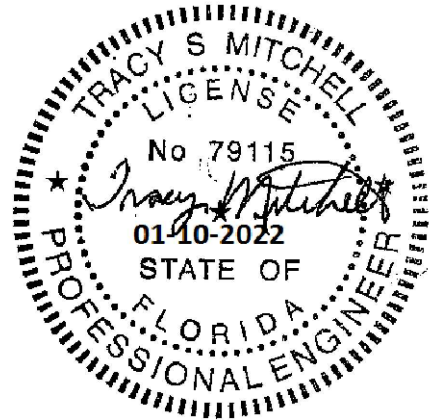
WIND LOAD	1.00	EXPANDED FORMULA	0.667*Ie*Fa*Ss*W/R
SNOW LOAD	1.0000	LONGITUDINAL	0.76
SEISMIC LOAD	1.00	TRANSVERSE	0.44

GENERAL NOTES:

- 1) MATERIALS : MINIMUM YIELD:
- | | |
|------------------------|-----------------------|
| HOT ROLLED BAR | Fy = 50.0000 ksi MIN. |
| STRUCTURAL STEEL SHEET | Fy = 55.0000 ksi MIN. |
| STRUCTURAL STEEL PLATE | Fy = 50.0000 ksi MIN. |
| COLD FORMED SHAPES | Fy = 57.0000 ksi MIN. |
| WALL SHEETING | Fy = 60.0000 ksi MIN. |
| ROOF SHEETING | Fy = 60.0000 ksi MIN. |
| BOLTS | A307 & A325 |
- THE METAL BUILDING MANUFACTURER RESERVES THE RIGHT TO
SUBSTITUTE THE ABOVE MATERIALS WITH EQUAL OR BETTER MATERIAL.

- 2) BOLT TIGHTENING REQUIREMENTS:
- ALL HIGH STRENGTH BOLTS ARE A325 UNLESS NOTED OTHERWISE.
HIGH STRENGTH BOLTS SHALL BE TIGHTENED BY THE TURN OF THE NUT METHOD
IN ACCORDANCE WITH THE LATEST EDITION AISC "SPECIFICATION FOR
STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS". A325 BOLTS SHALL BE
INSTALLED WITH OUT WASHERS WHEN TIGHTENED BY THE "TURN OF THE NUT"
METHOD. ALL BOLTED CONNECTIONS, FOR SHEAR/BEARING CONNECTION TYPE
WITH BOLT THREADS EXCLUDED FROM THE SHEAR PLANE SHALL BE SNUG TIGHT
ONLY.

- 3) ALL STRUCTUAL STEEL TO RECEIVE A RUST INHIBITIVE PRIMER. THIS PAINT
IS NOT INTENDED FOR LONG TERM EXPOSURE TO THE ELEMENTS.



ROOF PANELS:

COLOR: Galvalume Plus

WALL PANELS:

COLOR: Need Sill Poly Color

TRIM COLORS:

CABLE: Need Sill Poly Color

CORNER: Need Sill Poly Color

EAVE: Need Sill Poly Color

FRAMED OPENINGS: Need Sill Poly Color

LINER PANELS:

COLOR: N/A

LINER TRIM:

COLOR: N/A

DEFLECTION LIMTS:

EW COL:	180
EW RAF LIVE:	180
EW RAF WIND:	180
WALL GIRT:	90
PURL LIVE:	180
PURL WIND:	150
WALL PANEL:	60
ROOF PANEL LIVE:	60
ROOF PANEL WIND:	60
RF HORIZONTAL:	60
RF VERTICAL:	180
WIND BENT:	60
RF CRANE:	60
RF SEIS:	50
WIND BENT SEIS:	50

BUILDER / CONTRACTOR RESPONSIBILITIES

IT IS THE RESPONSIBILITY OF THE BUILDER/CONTRACTOR TO INSURE THAT ALL PROJECT
PLANS AND SPECIFICATIONS COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY
GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND
DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN
AGREEMENT THAT THE METAL BUILDING SYSTEM MANUFACTURER OR ITS DESIGN ENGINEER IS
ACTING AS THE ENGINEER OF RECORD OR DESIGN PROFESSIONAL FOR A CONSTRUCTION
PROJECT.

THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE
APPROPRIATE AGENCY AS REQUIRED. APPROVAL OF THE METAL BUILDING SYSTEM
MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE METAL BUILDING
SYSTEM MANUFACTURER CORRECTLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE
CONTRACT DRAWINGS AND SPECIFICATIONS. (SECT. 4.2.1 AISC CODE OF STANDARD
PRACTICES, 9TH ED.) WHERE DISCREPANCIES EXIST BETWEEN THE METAL BUILDING SYSTEM
MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR OTHER TRADES, THE
STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE
9TH ED.)

DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT
FURNISHED BY THE METAL BUILDING SYSTEM MANUFACTURER ARE THE RESPONSIBILITY OF
THE CONTRACTORS AND ENGINEERS OTHER THAN THE METAL BUILDING SYSTEM
MANUFACTURER'S ENGINEER UNLESS SPECIFICALLY INDICATED.

THE CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK
IN COMPLIANCE WITH THE METAL BUILDING SYSTEM MANUFACTURER "FOR CONSTRUCTION"
DRAWINGS.

ALL BRACING AS SHOWN AND PROVIDED BY THE METAL BUILDING SYSTEM MANUFACTURER
FOR THIS BUILDING IS REQUIRED AND SHALL BE INSTALLED BY THE ERECTOR AS A
PERMANENT PART OF THE STRUCTURE.

TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR
OTHER ELEMENTS REQUIRED FOR THE ERECTION OPERATION WILL BE DETERMINED AND
FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE
THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST LOADS
COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING
FROM WIND, SEISMIC FORCES AND ERECTION OPERATIONS, BUT NOT THE LOADS RESULTING
FROM THE PERFORMANCE OF WORK BY OR THE ACTS OF OTHERS, NOR SUCH
UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION, OR COLLISION. (SECT. 7.9.1
AISC CODE OF STANDARD PRACTICE, 9TH ED.)

WARNING: IN NO CASE SHOULD GALVALUME STEEL PANELS BE USED IN CONJUNCTION
WITH LEAD OR COPPER. BOTH LEAD AND COPPER HAVE HARMFUL CORROSION EFFECTS ON
THE ALUMINUM ZINC ALLOY COATING WHEN THEY ARE USED IN CONTACT WITH GALVALUME
STEEL PANELS. EVEN RUN-OFF FROM COPPER FLASHING, WIRING, OR TUBING ONTO
GALVALUME SHOULD BE AVOIDED.

APPROVAL NOTES

THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS
APPROVAL DRAWINGS: IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS BE MADE
IN CONTRASTING INK (PREFERABLY RED INK), HAVE ALL INSTANCES OF CHANGE CLEARLY
INDICATED, AND BE LEGIBLE AND UNAMBIGUOUS. A SIGNATURE AND DATE IS REQUIRED ON
ALL PAGES. MANUFACTURER RESERVES THE RIGHT TO RE-SUBMIT DRAWINGS WITH EXTENSIVE
OR COMPLEX CHANGES REQUIRED TO AVOID MISFABRICATION. THIS MAY IMPACT THE
DELIVERY SCHEDULE. APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE
METAL BUILDING SYSTEM MANUFACTURER HAS CORRECTLY INTERPRETED THE CONTRACT
REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS DRAWN WITH
INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED BY
MANUFACTURER. ANY CHANGES NOTED ON THHE DRAWINGS NOT IN CONFORMANCE WITH THE
TERMS AND REQUIREMENTS OF THE CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER
ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED
AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE DOCUMENTATION.
MANUFACTURER RECONGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING
APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED.
HOWEVER, MANUFACTURER DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL
TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILIAR INDICATION
OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO MANUFACTURER'S DRAWINGS
BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE CONSIDERED AS
UNACCEPTABLE ALTERNATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE
CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS
CUSTOMER.

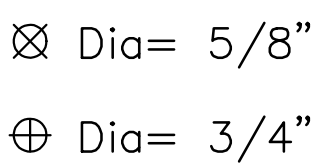
IMPORTANT NOTE: FINAL DETAILING, FABRICATION, AND DELIVERY DATE OF THIS PROJECT
CANNOT BE COMPLETED UNTIL THE SIGNED APPROVALS ARE RETURNED TO THE METAL
BUILDING MANUFACTURER.

SHEET 1 OF 13

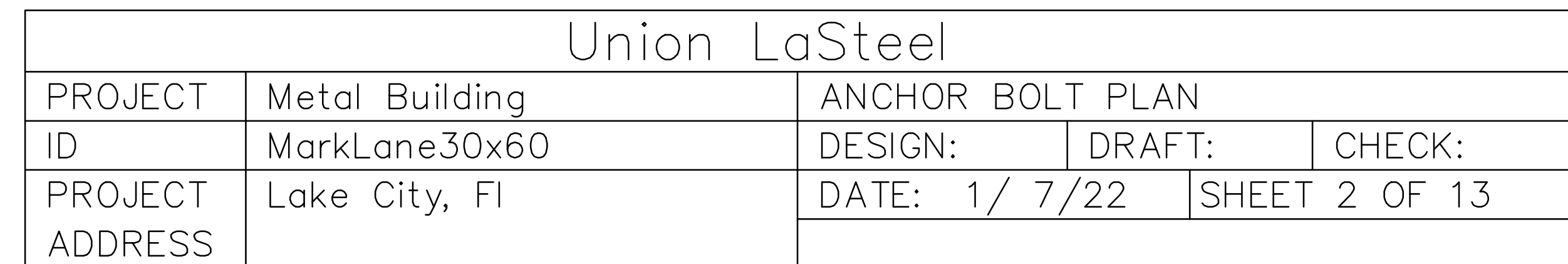
Lake City, Fl. 32025

△		
△		
△		
△		
△	01/10/22	FOR PERMIT
REV.	DATE	REVISION

PURCHASER: Franks & Lane Heating & Air
PROJECT: Metal Building
JOB NUMBER: MarkLane30x60



TRACY S MITCHELL
LICENSE
No 79115
01/10/2022
STATE OF
FLORIDA
PROFESSIONAL ENGINEER



NOTES FOR REACTIONS

Building reactions are based on the following building data:

Width (ft)	=	30.0
Length (ft)	=	60.0
Eave Height (ft)	=	11.0/ 16.0
Roof Slope (rise/12)	=	2.0
Dead Load (psf)	=	2.0
Collateral Load (psf)	=	1.0
Roof Live Load(psf)	=	20.0
Frame Live Load(psf)	=	12.0
Wind Speed (mph)	=	120.0
Wind Code	=	FBC 20 (IBC 18)
Exposure	=	B
Closed/Open	=	C
Importance Wind	=	1.00
Importance Seismic	=	1.00
Seismic Zone	=	B
Seismic Coeff (Fa*Ss)	=	0.15

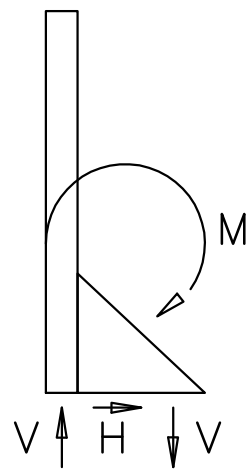
ID Description

- 1 Dead+Collateral+Live
- 2 Dead+0.6Wind_Long2R
- 3 Dead+Collateral+0.75Live+0.45Wind_Left1
- 4 Dead+Collateral+0.75Live+0.45Wind_Right1
- 5 0.6Dead+0.6Wind_Left1
- 6 0.6Dead+0.6Wind_Right1
- 7 0.6Dead+0.6Wind_Left2
- 8 0.6Dead+0.6Wind_Right2
- 9 0.6Dead+0.6Wind_Long1L
- 10 0.6Dead+0.6Wind_Suction+0.6Wind_Long1L
- 11 0.6Dead+0.6Wind_Pressure+0.6Wind_Long1L

BUILDING BRACING REACTIONS

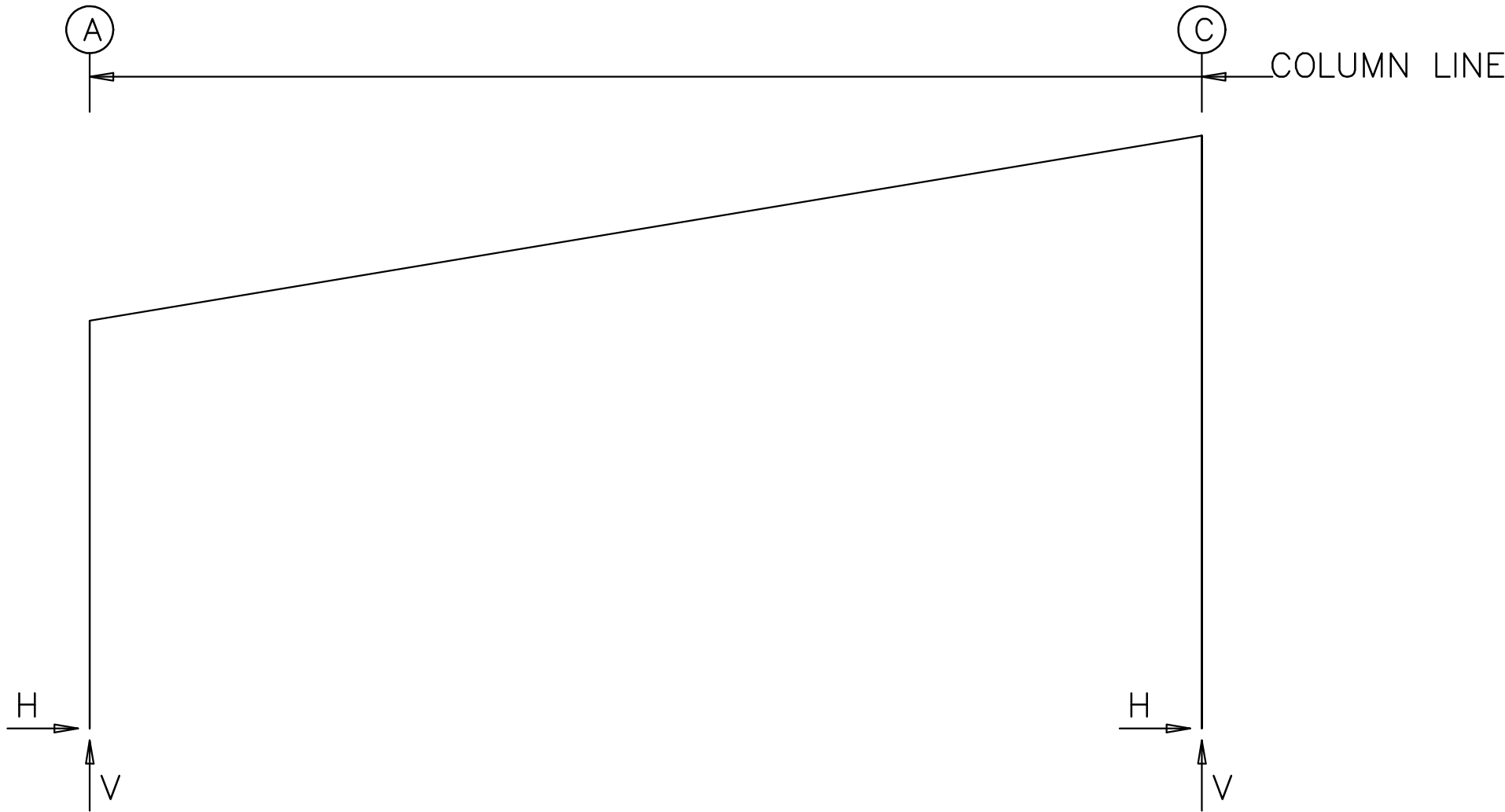
—Wall—		Col		± Reactions(k)				Panel_Shear		Note
Loc	Line	Line	Line	Wind	Seismic	Horz	Vert	Wind	Seis	
L_EW	1							67	9	
F_SW	C	2						70	9	(g)
R_EW	4									
B_SW	A	3,2		1.5	0.7	0.2	0.1			
(g)Wind column at column line										

WIND COLUMN REACTIONS



—Wall—		Col	R/L	Load_ID	± Reactions			Anc_Qty	Bolt_Dia	Base_Plate(in)		
Loc	Line	Line			Horz (k)	Vert (k)	Moment (f-k)			Width	Length	Thick
F_SW	C	2	R	Wind Seismic	2.3 0.6	30.7 7.2	35.8 8.4	2	0.750	8.000	10.500	0.750

FRAME LINES: 2 3



RIGID FRAME: MAXIMUM REACTIONS, ANCHOR BOLTS, & BASE PLATES

Frm Line	Col Line	Column_Reactions(k)						Bolt(in)		Base_Plate(in)			Grout (in)
		Load Id	Hmax H	V Vmax	Load Id	Hmin H	V Vmin	Qty	Dia	Width	Length	Thick	
2*	A	4	2.2	2.8	7	-2.3	-1.8	4	0.750	8.000	9.94	0.375	0.0
		1	1.5	4.7	5	-2.0	-3.3						
2*	C	8	2.0	-1.7	3	-1.5	2.2	4	0.750	8.000	9.94	0.625	0.0
		2	-0.6	17.3	9	0.1	-21.4						
2*	Frame lines: 2 3												

RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	-----Dead-----		-----Collateral-----		-----Live-----		-----Wind_Left1-----		-----Wind_Right1-----		-----Wind_Left2-----	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	A	0.2	0.9	0.1	0.3	1.1	3.5	-3.6	-6.4	2.3	-2.2	-4.1	-3.9
2*	C	-0.2	1.0	-0.1	0.3	-1.1	4.3	-0.7	-5.3	3.4	-6.2	-0.2	-3.0
Frame Line	Column Line	-----Wind_Right2-----		-----Wind_Long1-----		-----Wind_Long2-----		-----Seismic_Left-----		-----Seismic_Right-----		-----Seismic_Long-----	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2*	A	2.1	0.2	0.2	-6.2	0.4	-4.4	-0.1	-0.1	0.1	0.1	0.0	-0.3
2*	C	3.6	-3.8	0.3	-36.7	-0.6	-34.2	0.0	0.1	0.0	-0.1	0.0	-9.0

2* Frame lines: 2 3

ENDWALL COLUMN REACTIONS(k)

MAXIMUM VERTICAL Dead+Collateral+Live = 4.2
MAXIMUM VERTICAL Dead+Wind = -3.0
MAXIMUM HORIZONTAL Dead+Wind = 1.5

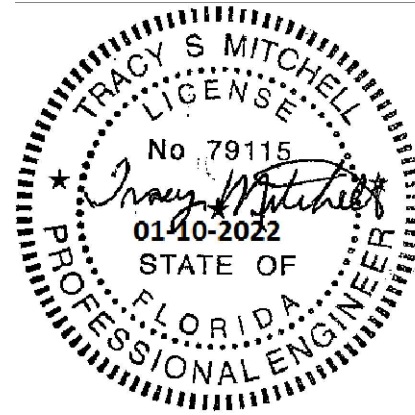
ANCHOR BOLT SUMMARY

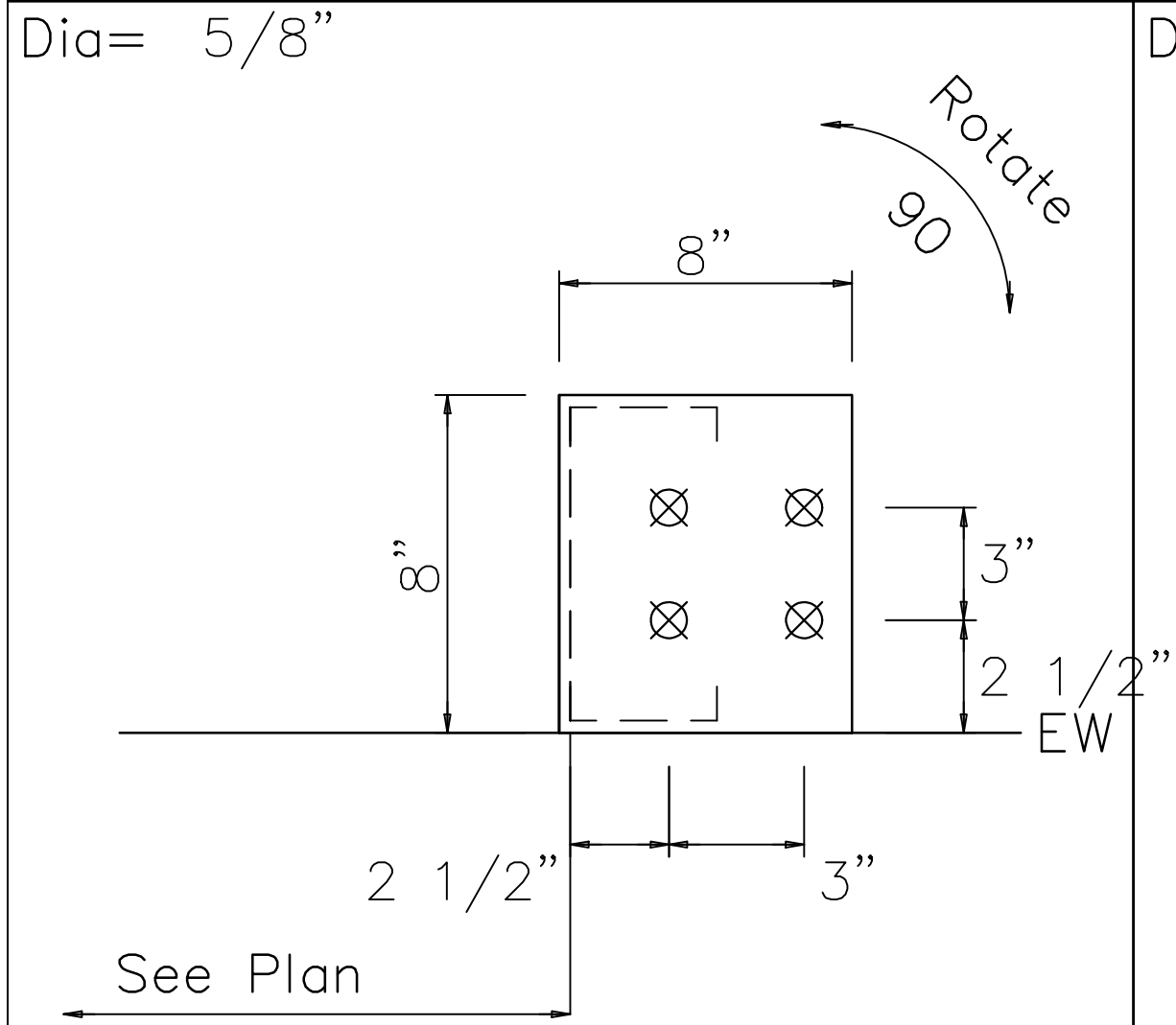
Qty	Locate	Dia (in)	Type	Proj (in)
⊗ 24	Jamb	5/8"	A307	3.00
⊗ 24	Endwall	5/8"	A307	3.00
⊕ 16	Frame	3/4"	A307	3.00
⊕ 2	WindCol	3/4"	A307	3.00



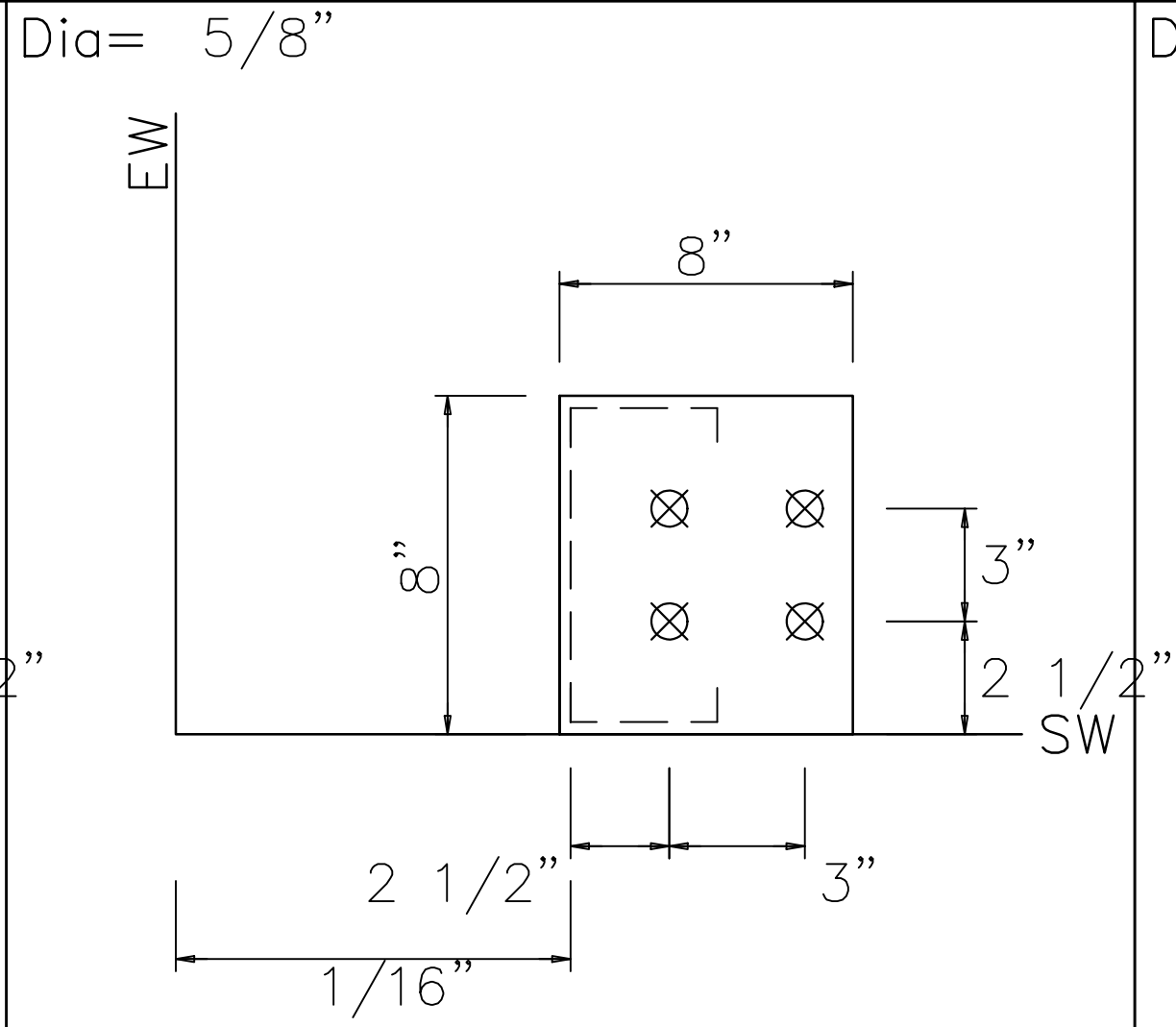
Union LaSteel

PROJECT	Metal Building	ANCHOR BOLT REACTIONS		
ID	MarkLane30x60	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Lake City, Fl	DATE: 1/ 7/22	SHEET 3 OF 13	

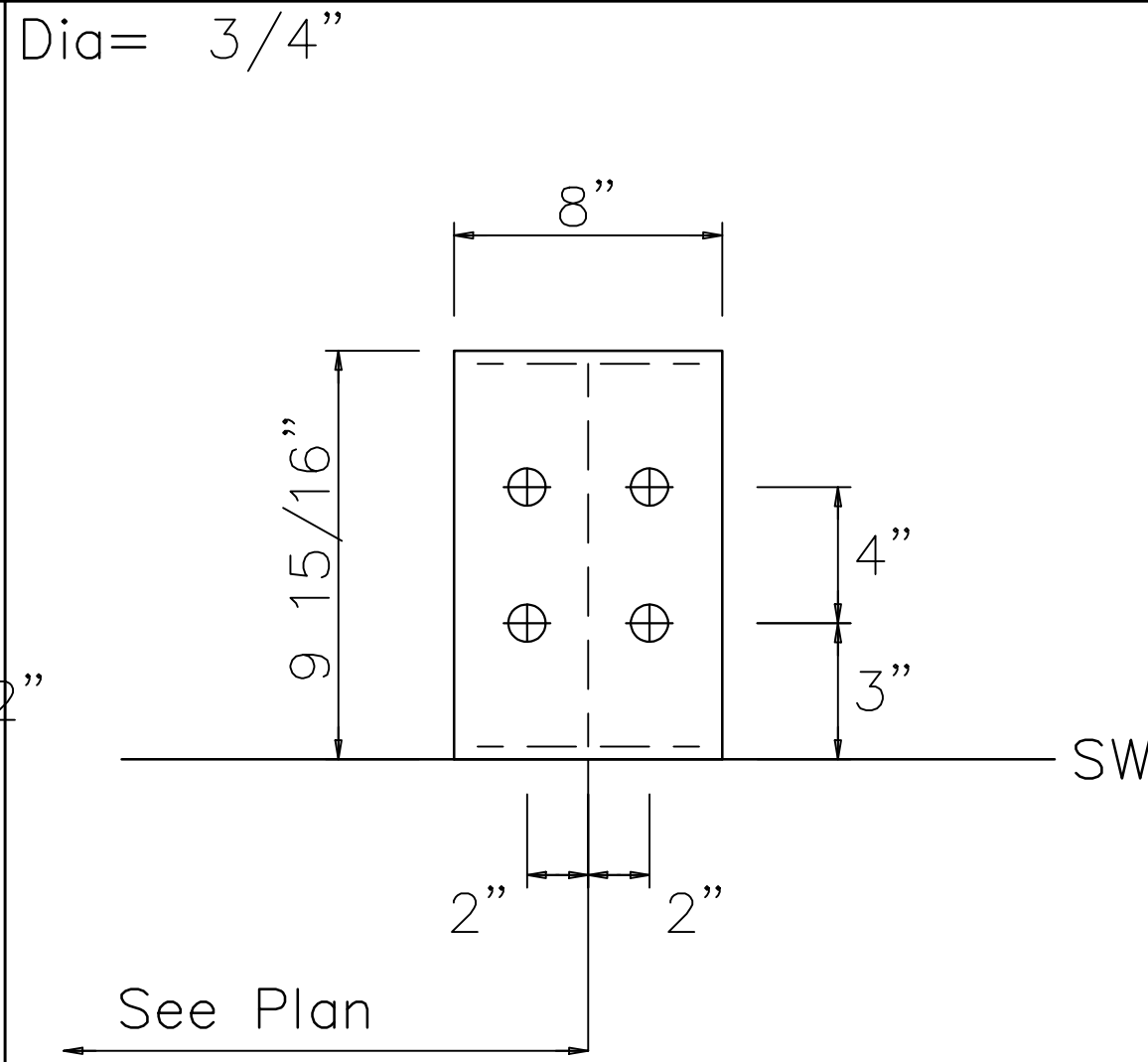




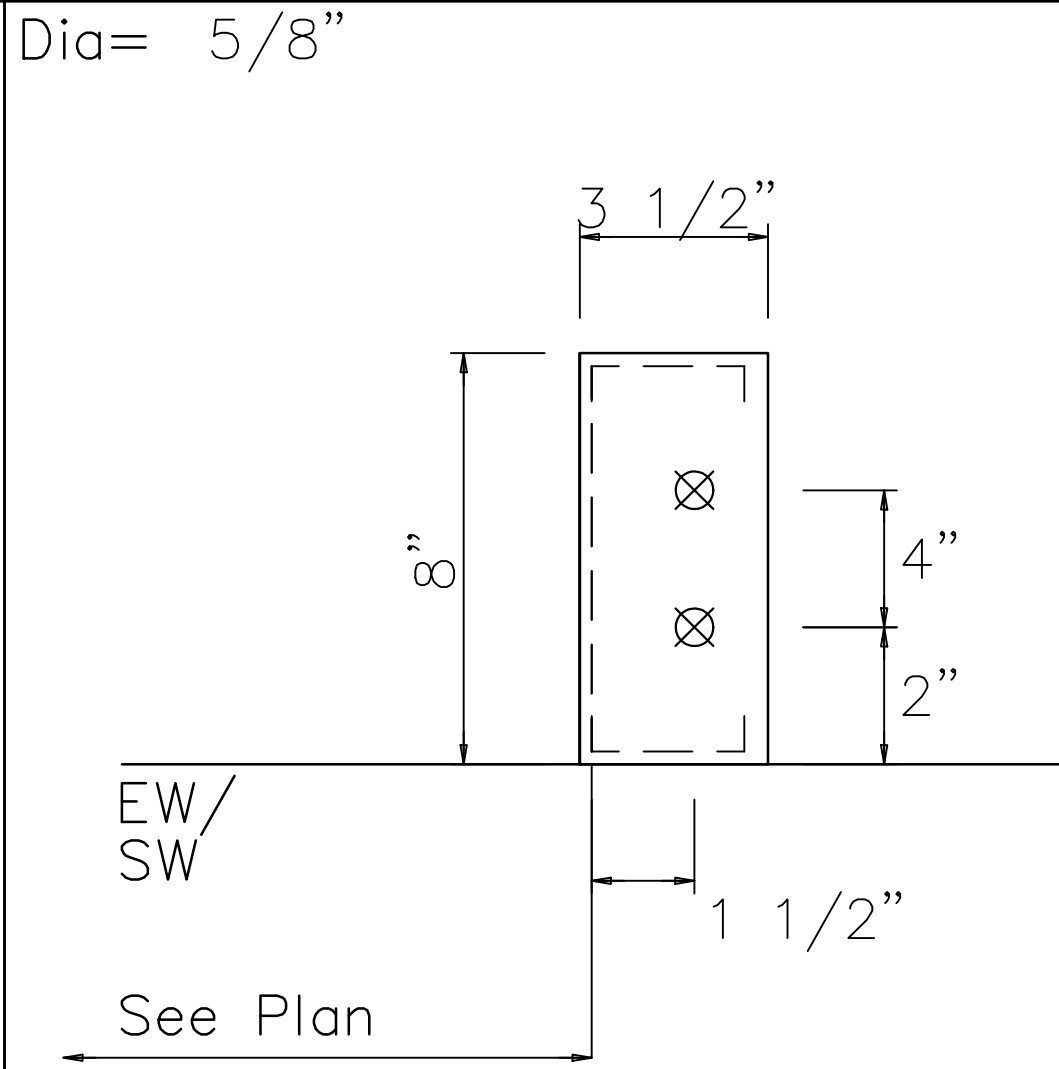
DETAIL A



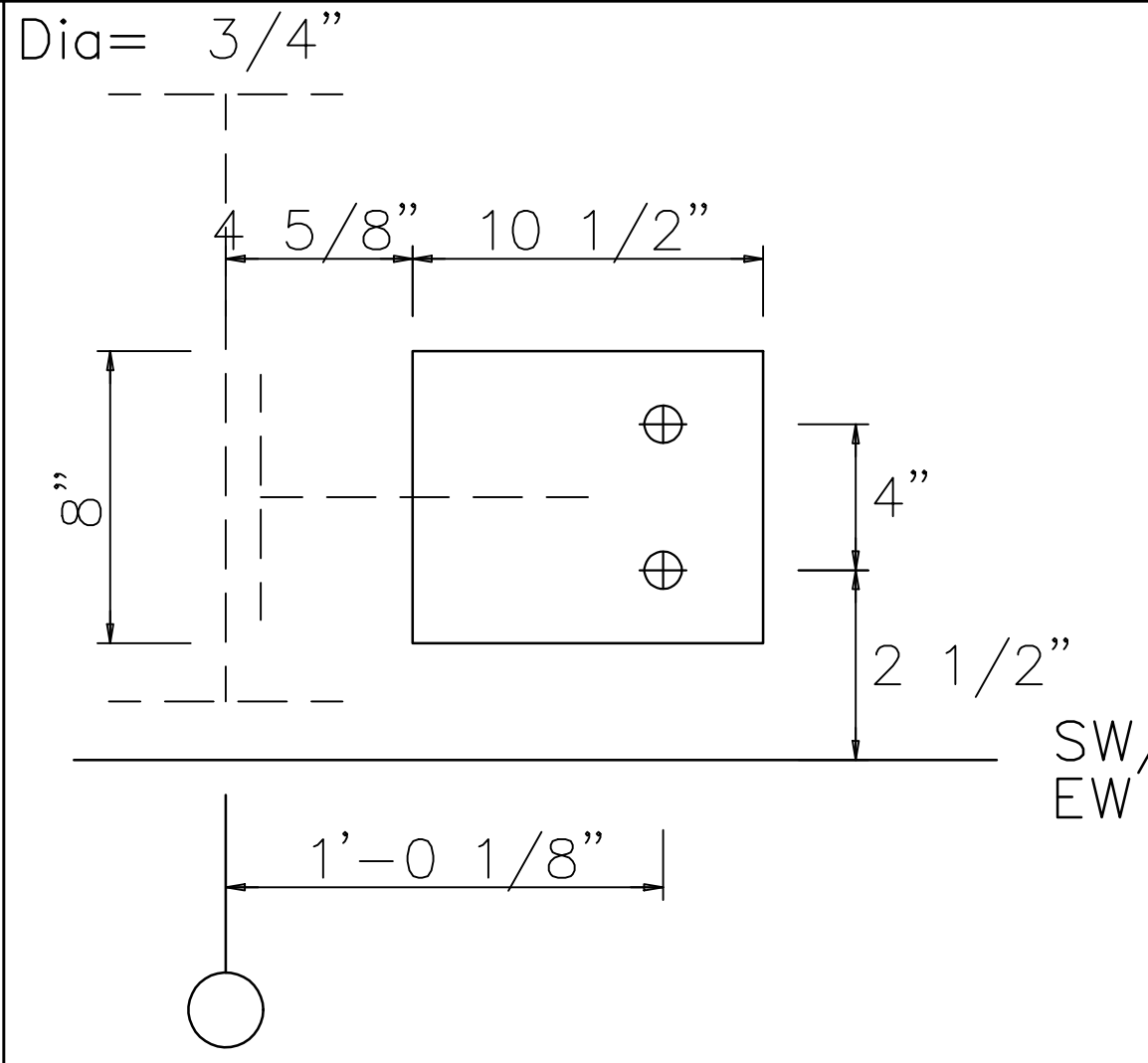
DETAIL B



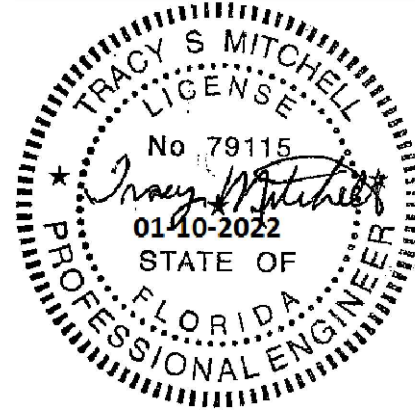
DETAIL C



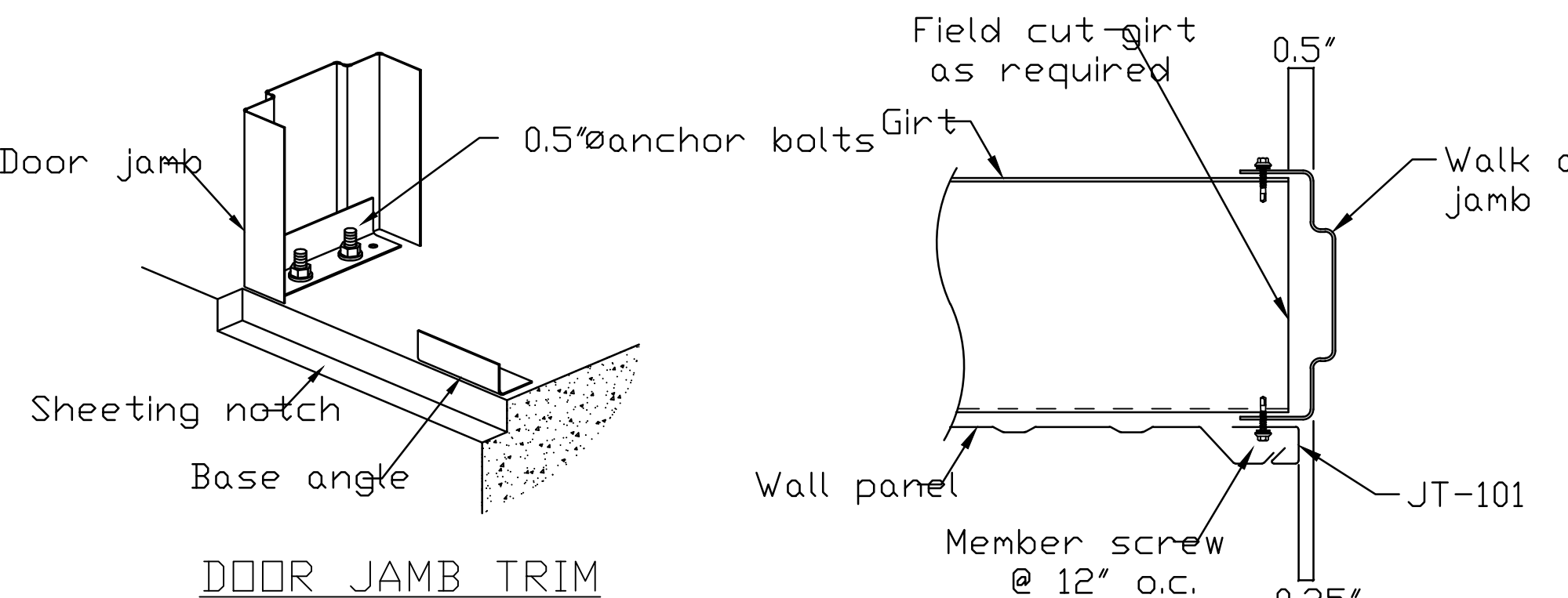
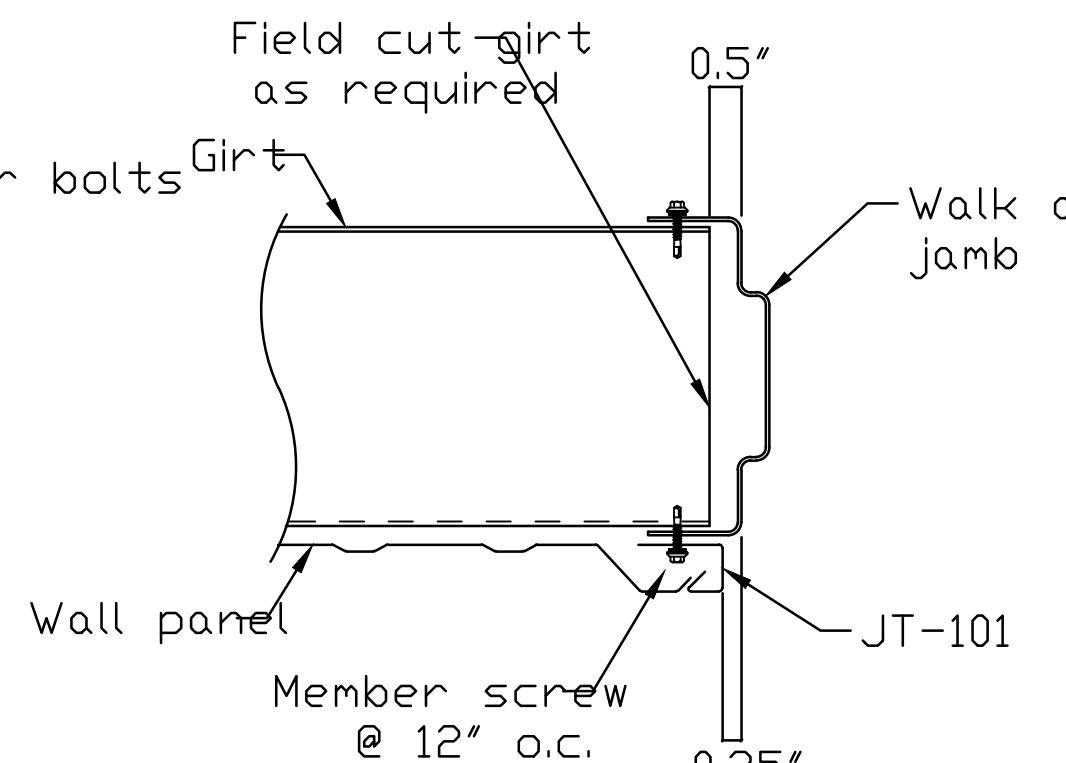
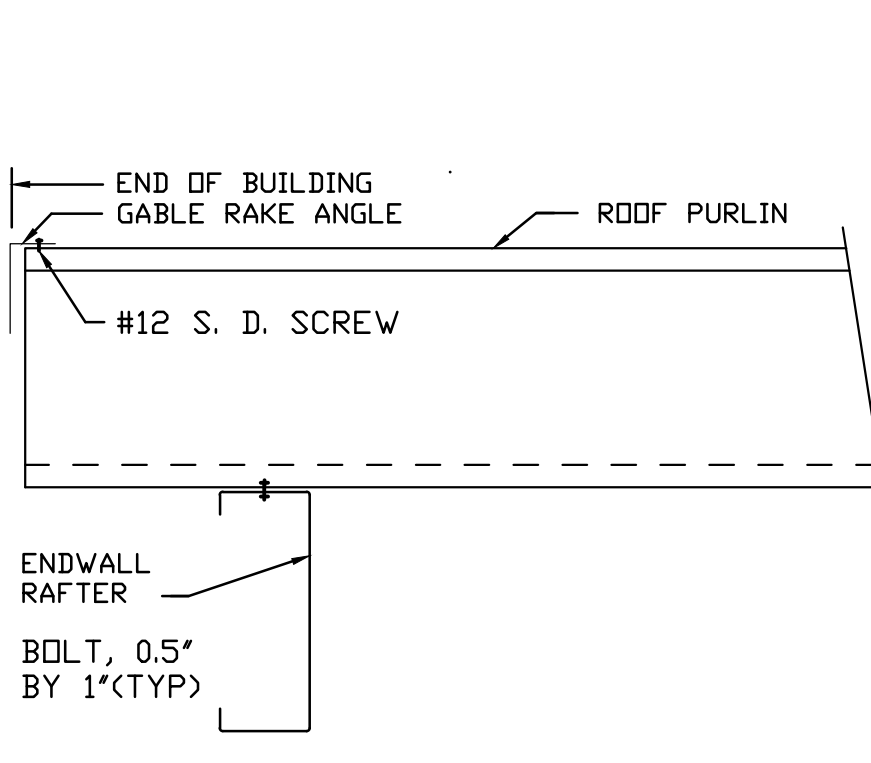
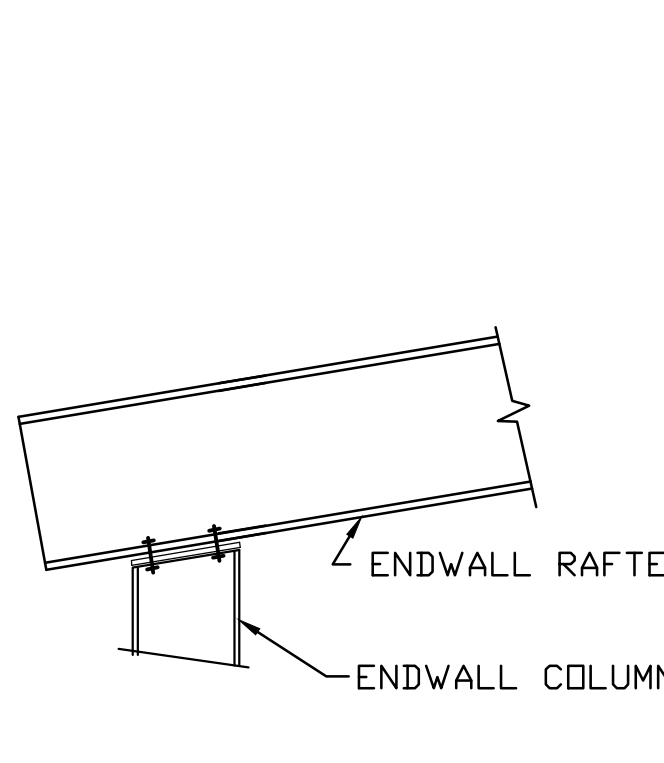
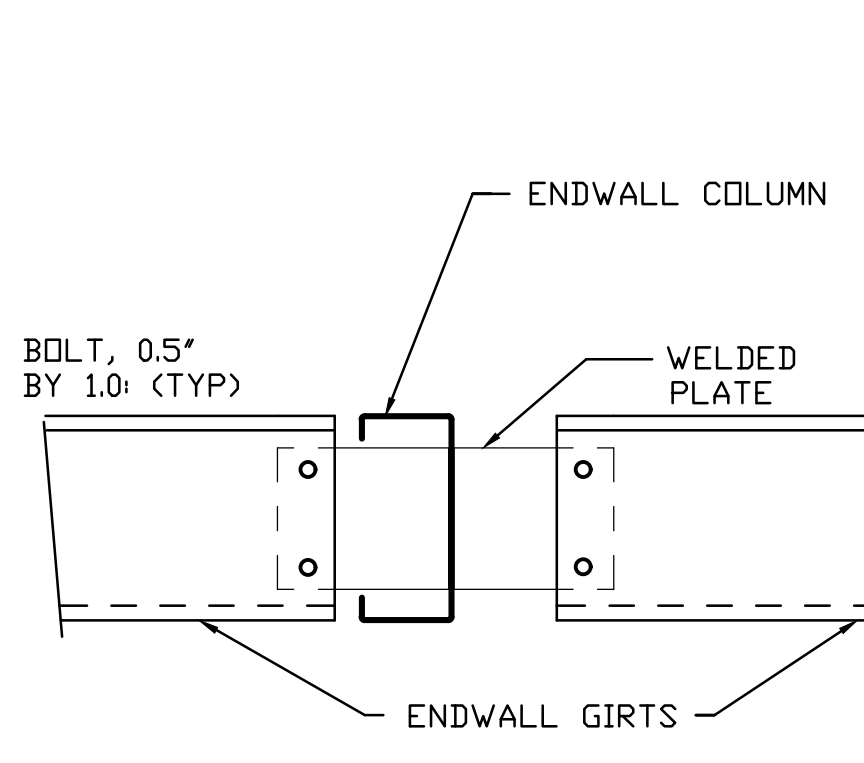
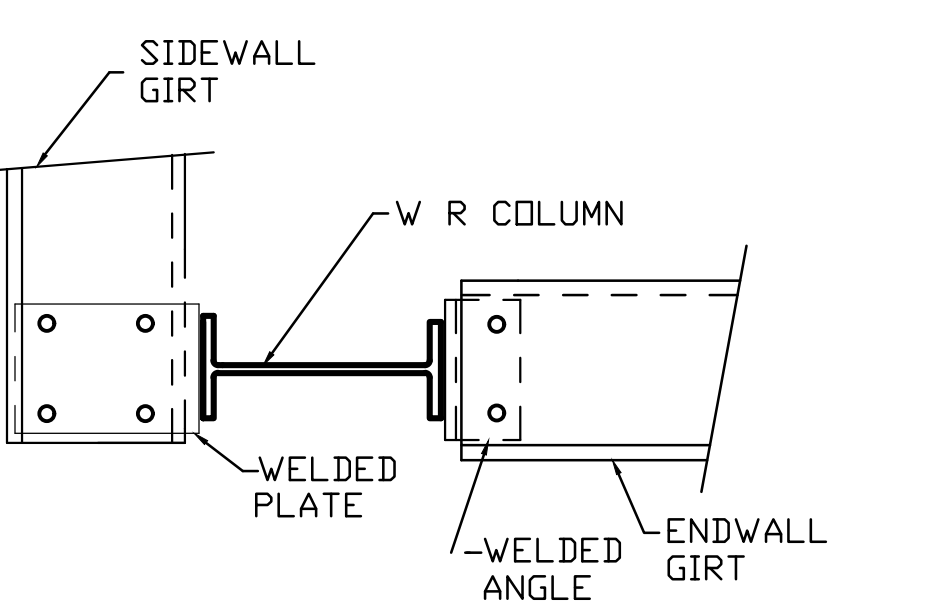
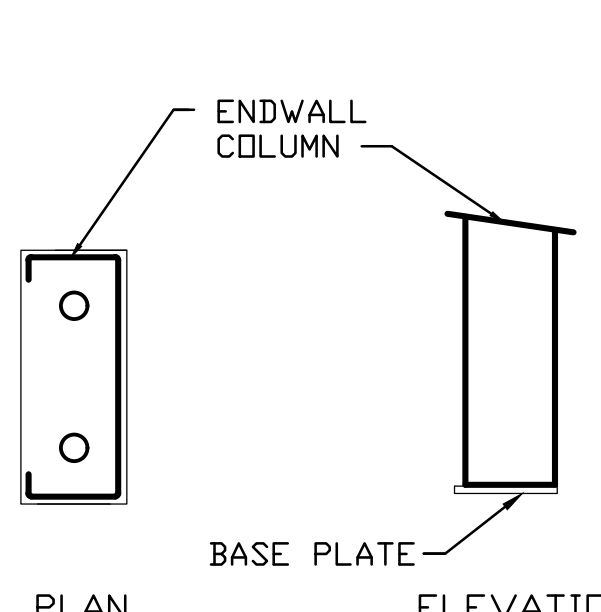
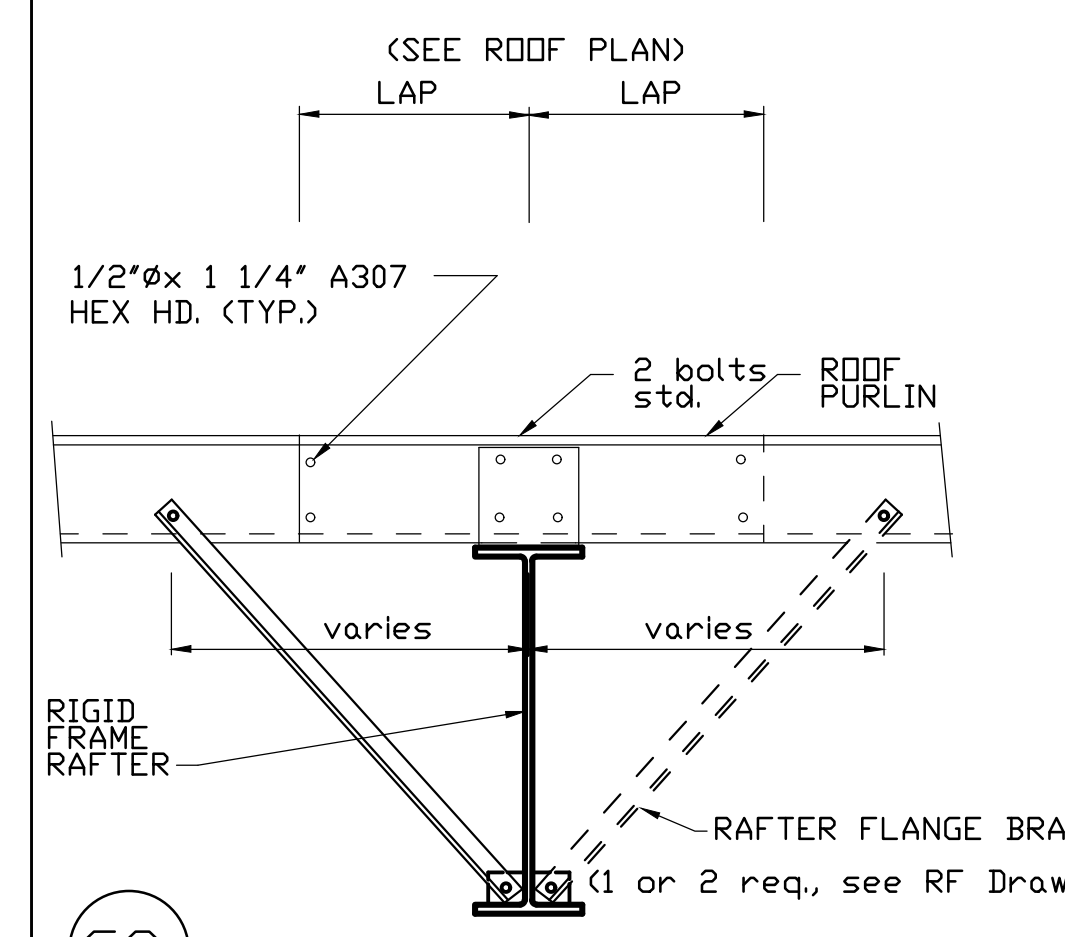
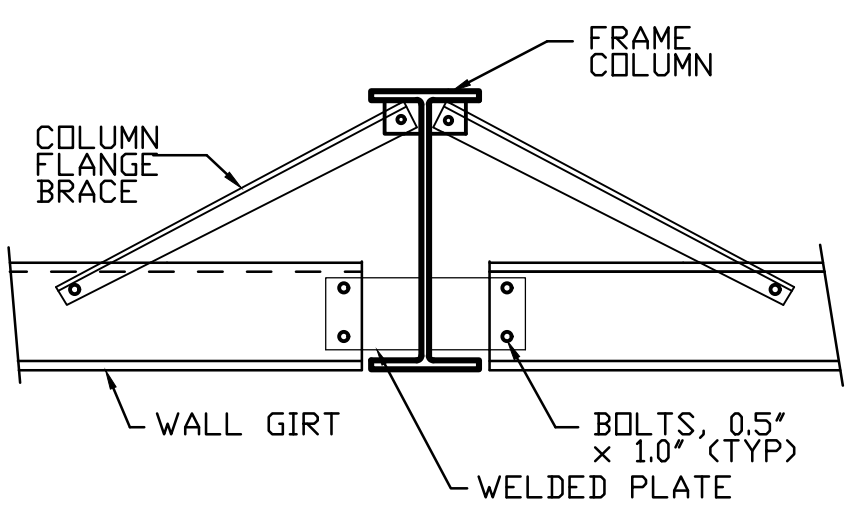
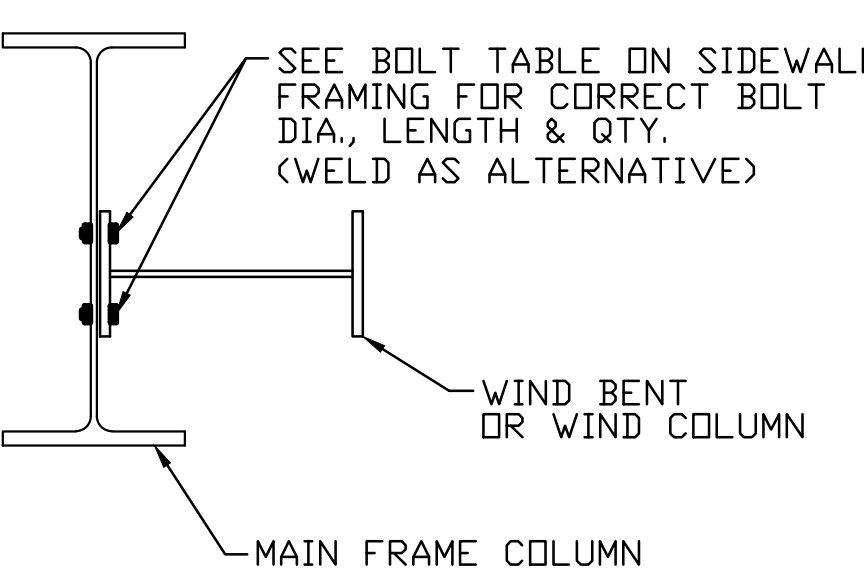
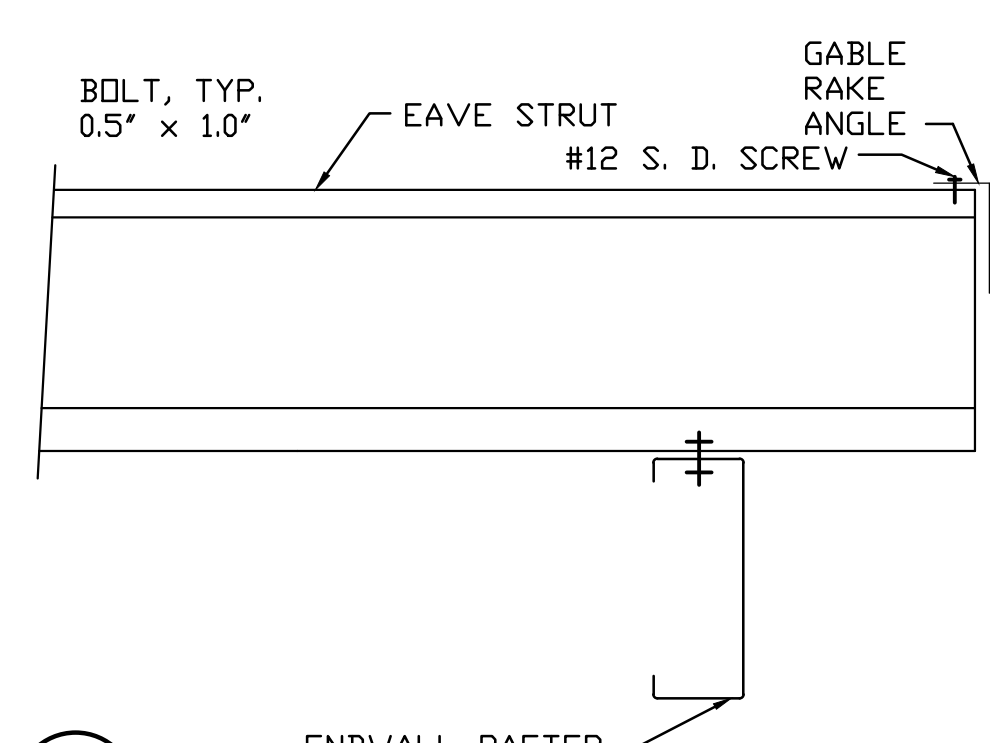
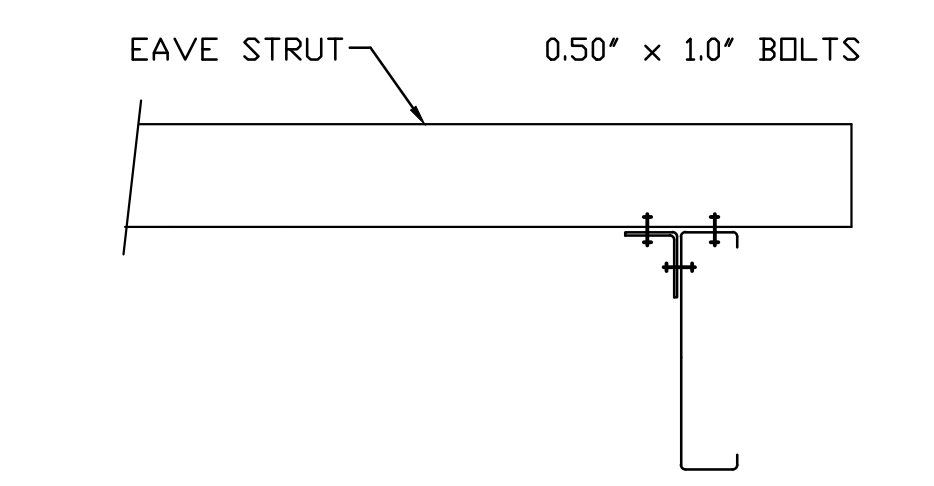
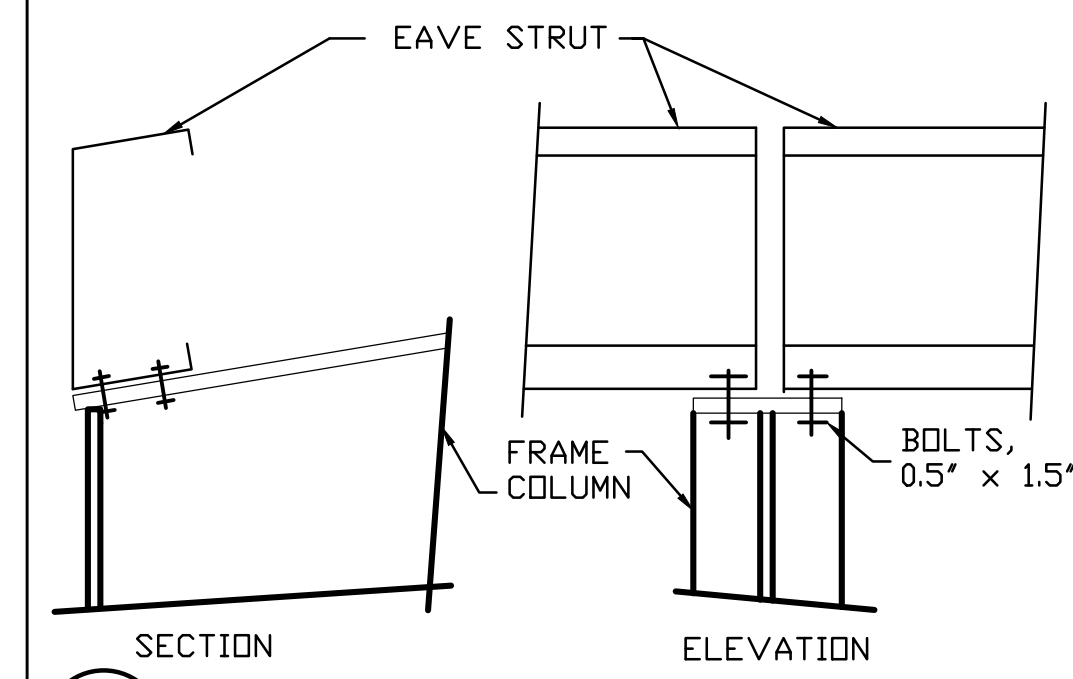
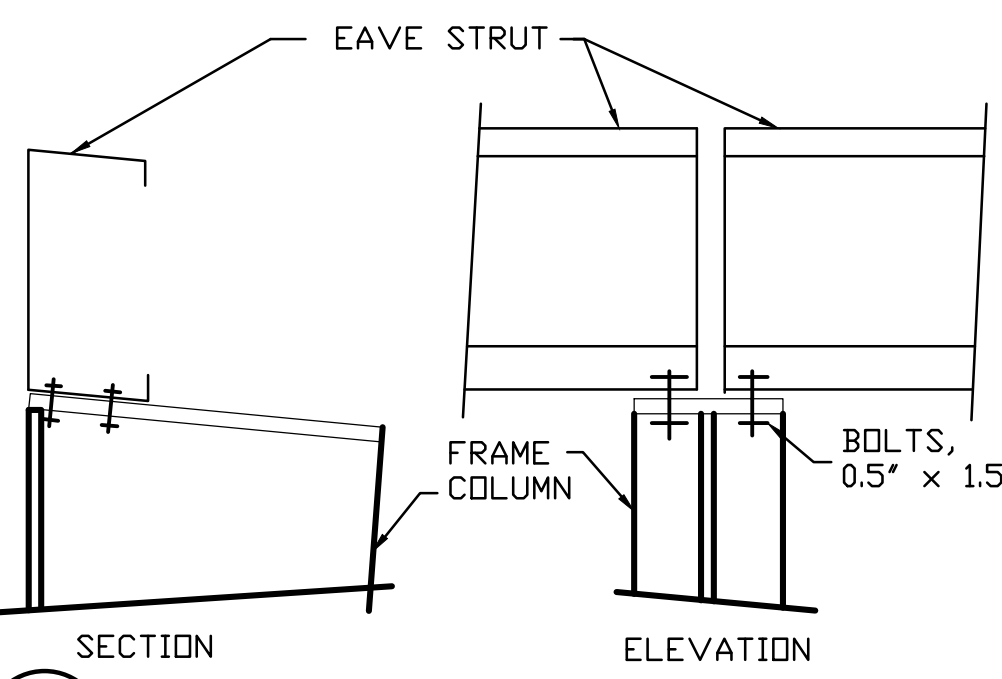
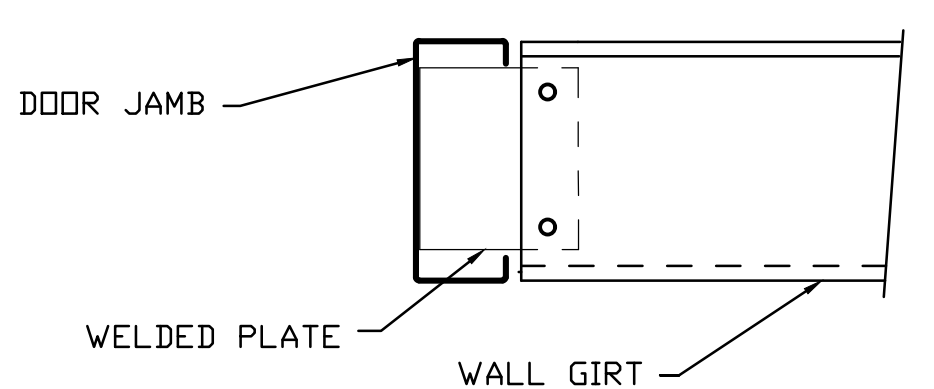
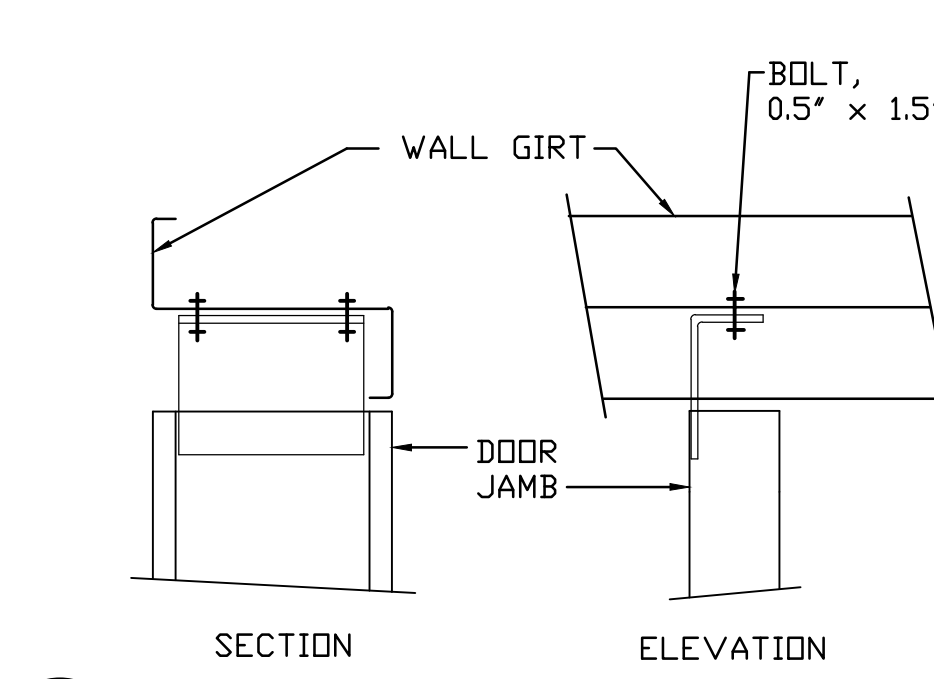
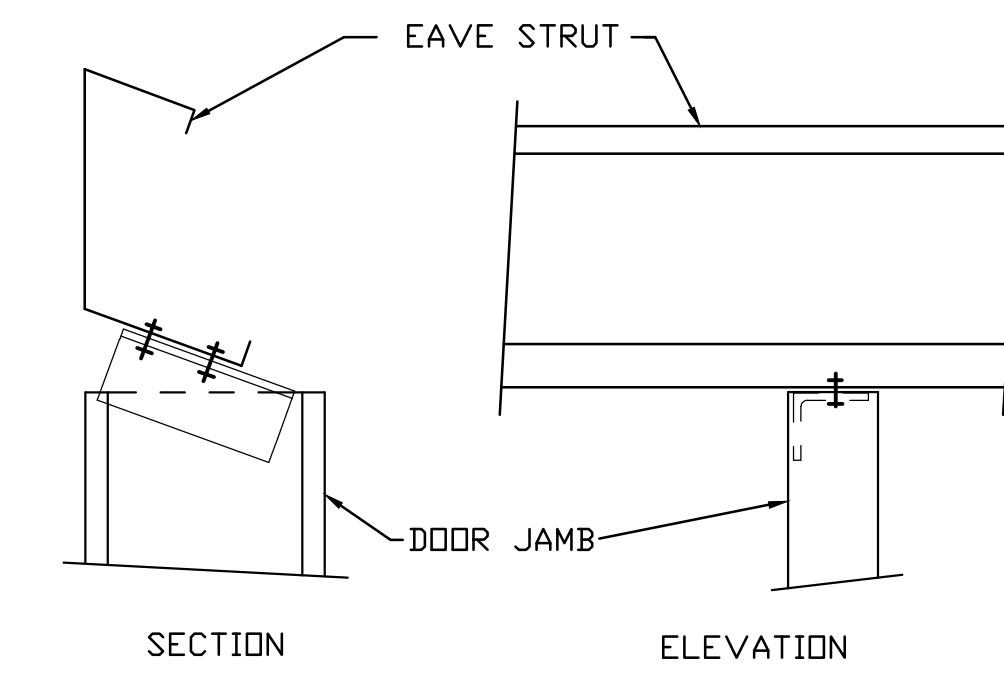
DETAIL D

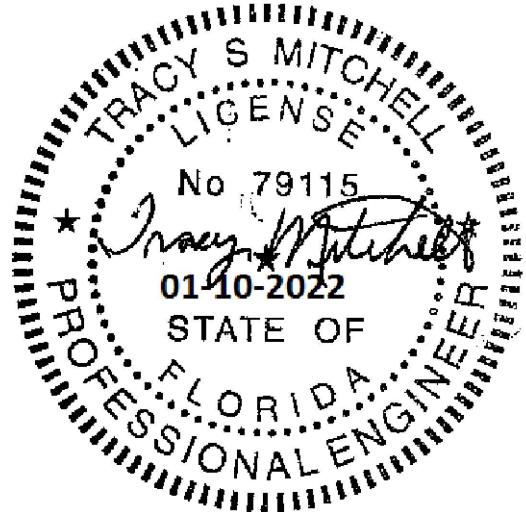


DETAIL E



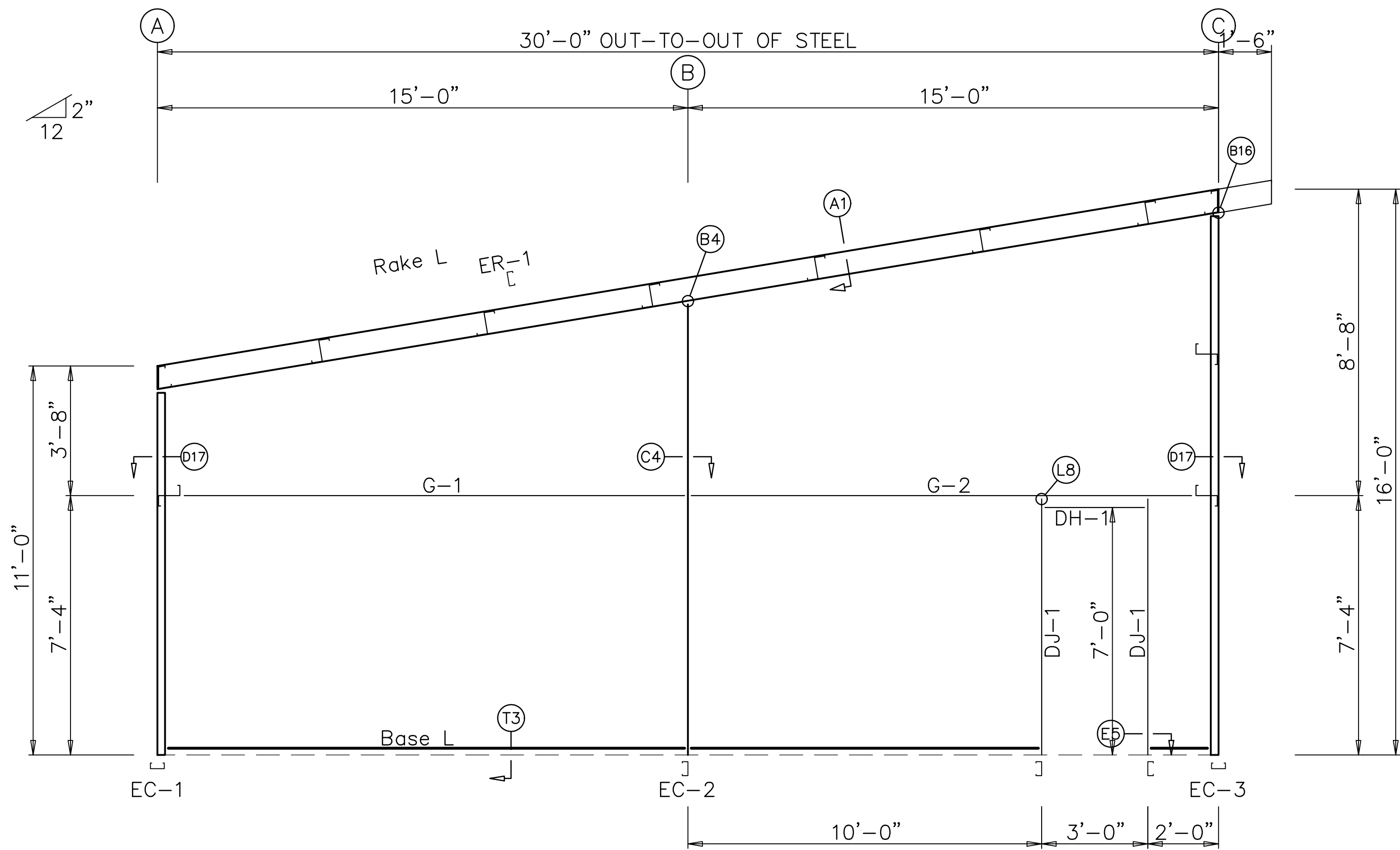
Union LaSteel					
PROJECT	Metal Building		ANCHOR BOLT DETAILS		
ID	MarkLane30x60		DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Lake City, Fl		DATE: 1/ 7/22		SHEET 4 OF 13

 DOOR JAMB TRIM	 A1 SECTION THRU ENDWALL RAFTER	 B4 ENDWALL RAFTER TO COLUMN	 B16 CORNER COLUMN TO ENDWALL RAFTER	 C4 ENDWALL COLUMN TO WALL GIRT	
 D17 CORNER COLUMN TO WALL GIRT	 E5 BASE PLATE TO ENDWALL COLUMN OR DOOR JAMB	 G2 TYPICAL SECTION THROUGH PURLIN LAP	 H6 WALL GIRT TO FRAME COLUMN	 H9 PORTAL COLUMN TO MAIN FRAME COLUMN CONNECTION	 I6 EAVE STRUT TO ENDWALL RAFTER
 I19 EAVE STRUT TO ENDWALL RAFTER	 J1 EAVE STRUT TO RIGID FRAME	 J7 EAVE STRUT TO RIGID FRAME	 K3 WALL GIRT TO DOOR JAMB	 L8 FRAMED OPENING TO WALL GIRT	 L16 DOOR JAMB TO EAVE STRUT

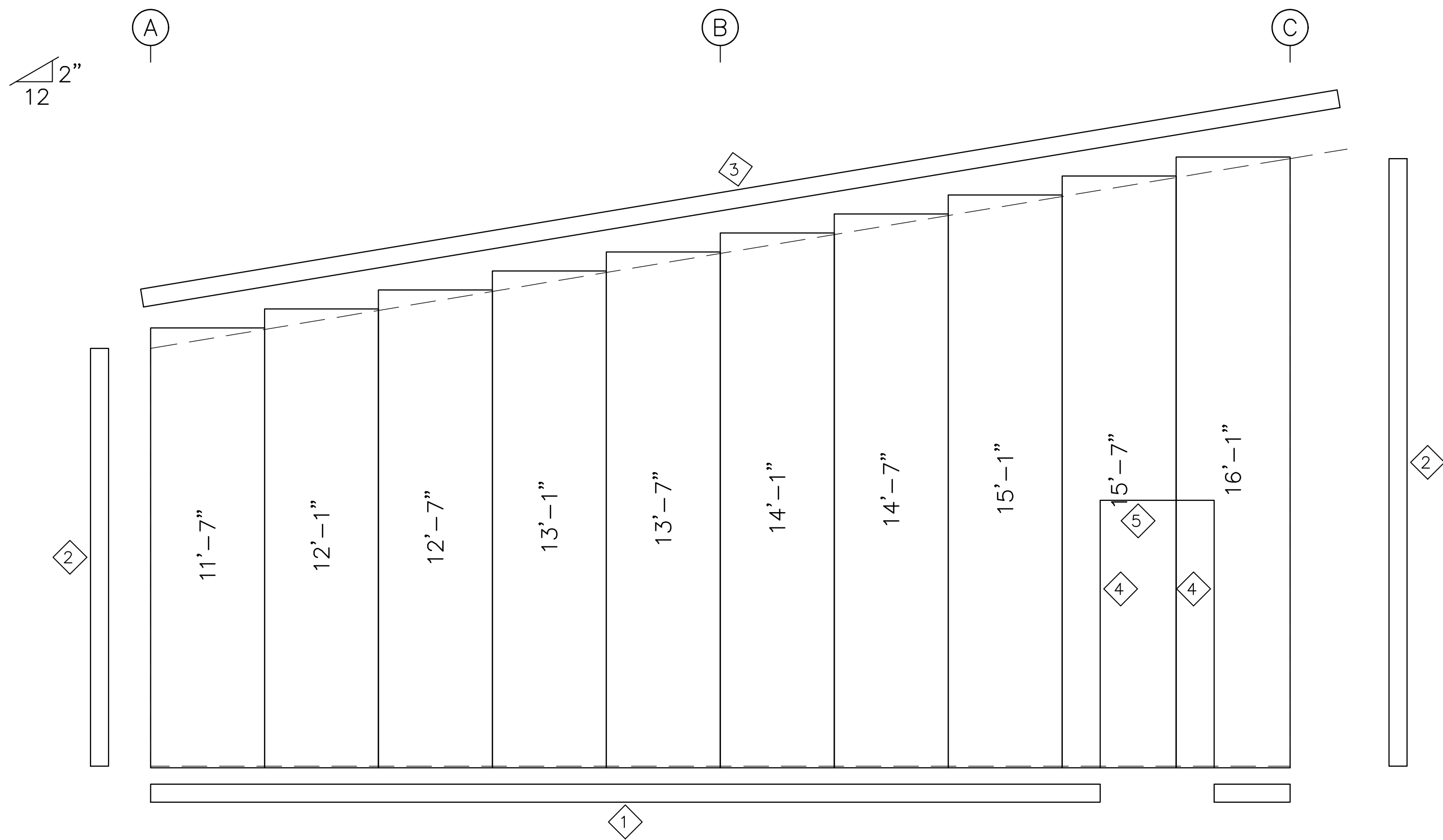


ASG LLC
AMERICAN STRUCTURAL GROUP
 CONSULTING STRUCTURAL ENGINEERS
 225 SOUTH MAIN STREET
 MONROE, OHIO 45050
 PH 513-402-7600
 WWW.ASG-ENG.COM
 "Value Engineering the First Time"

Union LaSteel				
PROJECT	Metal Building	STRUCTURAL DETAILS		
ID	MarkLane30x60	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Lake City, Fl	DATE: 1/ 7/22	SHEET 5 OF 13	



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

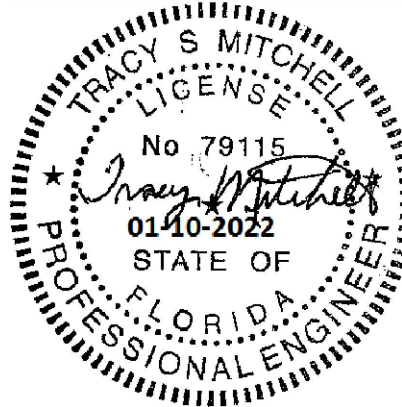
PANELS: 26 Ga. SX – Need Sill Poly Color

ASG LLC
AMERICAN STRUCTURAL GROUP
CONSULTING STRUCTURAL ENGINEERS
225 SOUTH MAIN STREET
MONROE, OHIO 45050
PH 513-402-7600
WWW.ASG-ENG.COM
"Value Engineering the First Time"

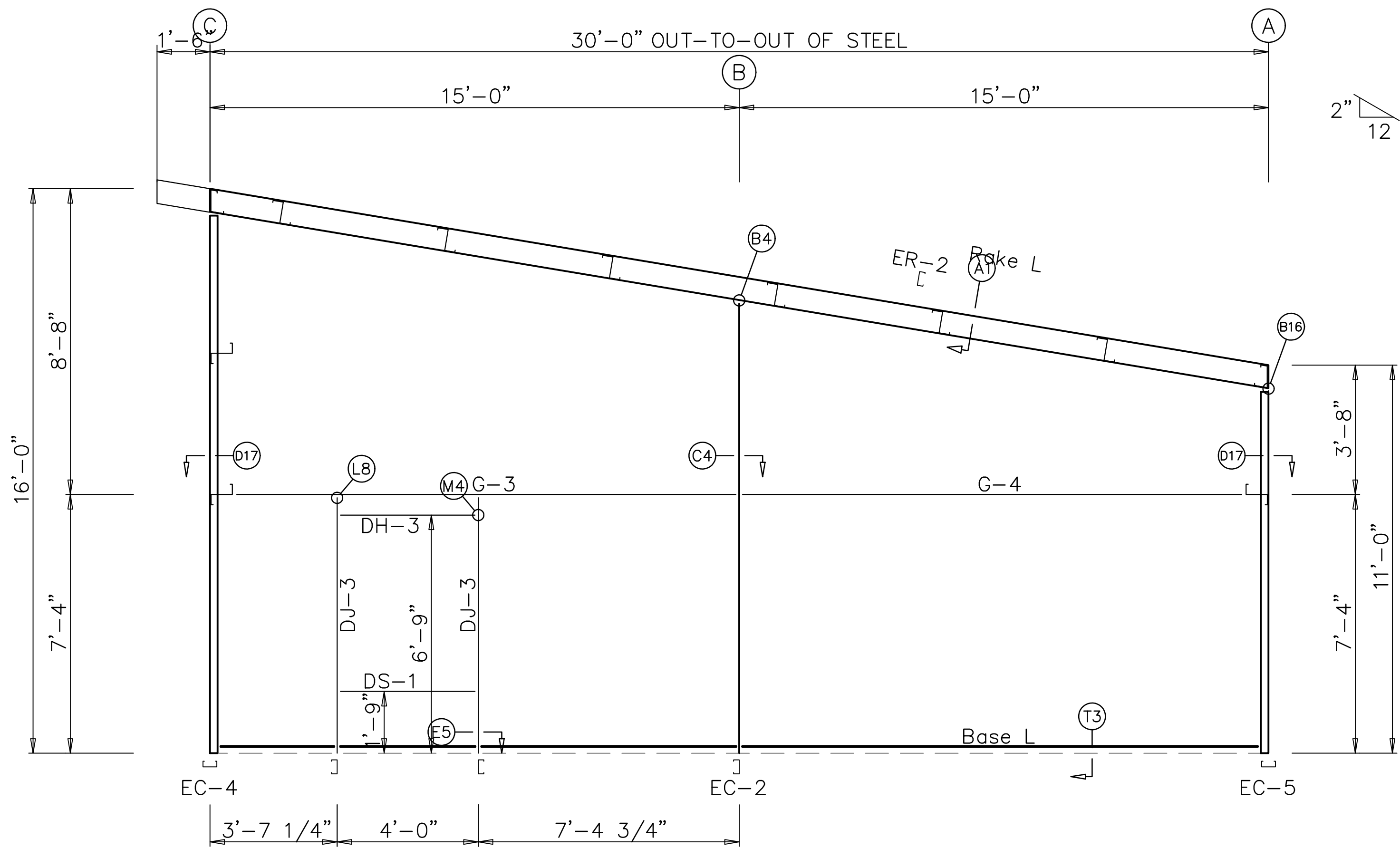
BOLT TABLE				
FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
Cor_Column/Raf	2	A325	5/8"	2"
EC-2/ER-1	2	A325	5/8"	1 1/2"

TRIM TABLE			
FRAME LINE 1			
ID	PART	LENGTH	DETAIL
1	BT-101	10'-3"	TRIM_68
2	CT-102	10'-3"	TRIM_323
3	RT-101	10'-3"	TRIM_229
4	JT-101	10'-3"	TRIM_401
5	HT-101	10'-3"	TRIM_232

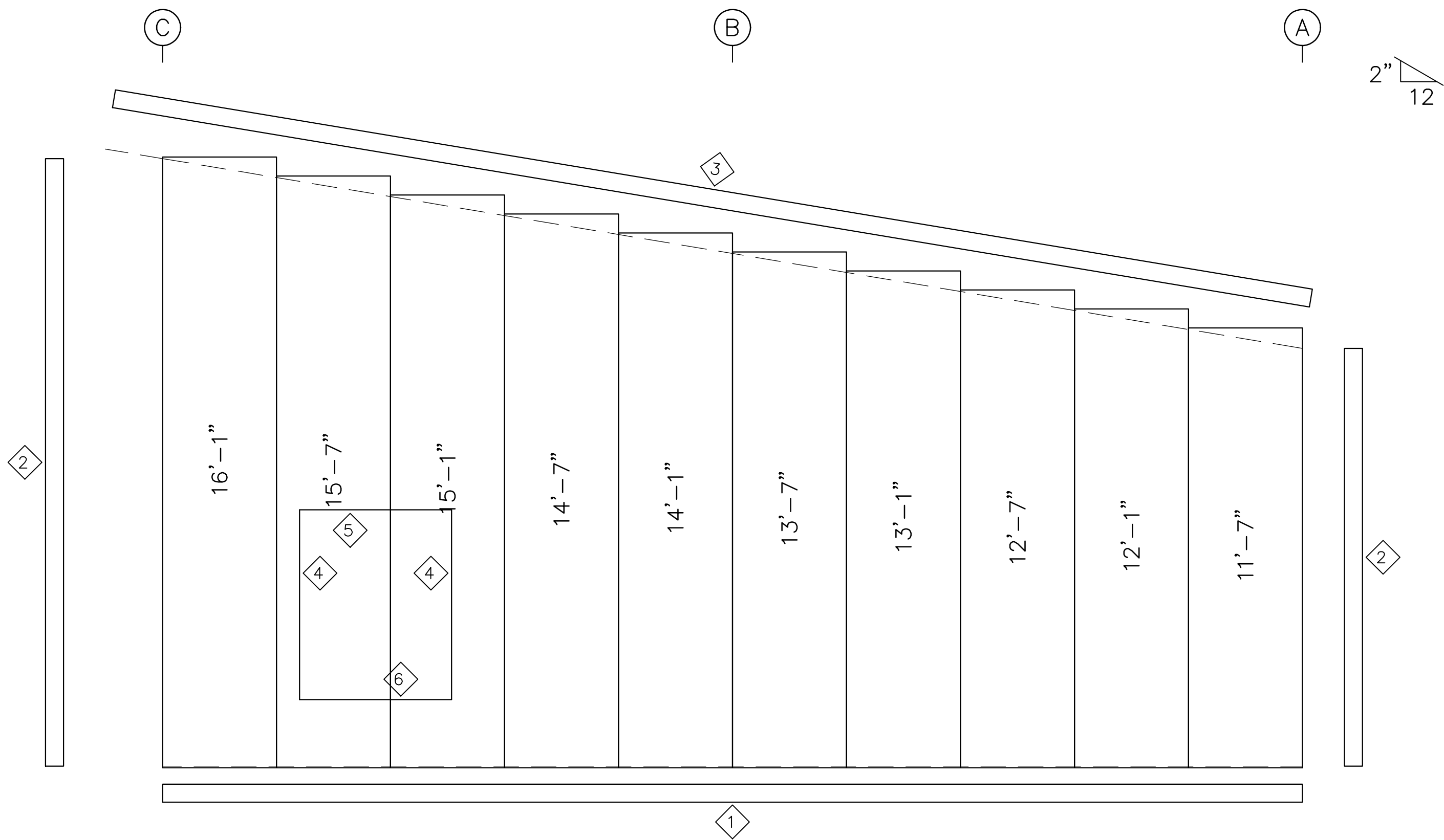
MEMBER TABLE		
FRAME LINE 1		
MARK	PART	LENGTH
EC-1	8X35C16	10'-3 3/16"
EC-2	8X35C16	12'-7 5/8"
EC-3	8X35C16	15'-1 7/8"
ER-1	10X35C14	31'-7 7/8"
DJ-1	8X35C16	7'-4"
DH-1	8X35C16	3'-0"
G-1	8X25Z16	13'-11 3/4"
G-2	8X25Z16	14'-3 1/2"



Union LaSteel				
PROJECT	Metal Building	ENDWALL FRAMING		
ID	MarkLane30x60	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Lake City, FL	DATE: 1/ 7/22	SHEET 6 OF 13	



ENDWALL FRAMING: FRAME LINE 4



ENDWALL SHEETING & TRIM: FRAME LINE 4

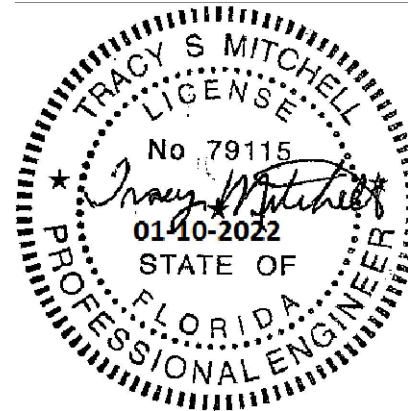
PANELS: 26 Ga. SX – Need Sill Poly Color



BOLT TABLE				
FRAME LINE 4				
LOCATION		QUAN	TYPE	DIA
Cor_Column/Raf		2	A325	5/8"
EC-2/ER-2		2	A325	5/8"

TRIM TABLE			
FRAME LINE 4			
ID	PART	LENGTH	DETAIL
1	BT-101	10'-3"	TRIM_68
2	CT-102	10'-3"	TRIM_323
3	RT-101	10'-3"	TRIM_229
4	JT-101	10'-3"	TRIM_239
5	HT-101	10'-3"	TRIM_232
6	MT-113	10'-3"	TRIM_261

MEMBER TABLE		
FRAME LINE 4		
MARK	PART	LENGTH
EC-2	8X35C16	12'-7 5/8"
EC-4	8X35C16	15'-1 7/8"
EC-5	8X35C16	10'-3 3/16"
ER-2	10X35C14	32'-0 15/16"
DJ-3	8X35C16	7'-4"
DH-3	8X35C16	4'-0"
DS-1	8X35C16	4'-0"
G-3	8X25Z16	13'-11 3/4"
G-4	8X25Z16	14'-3 1/2"

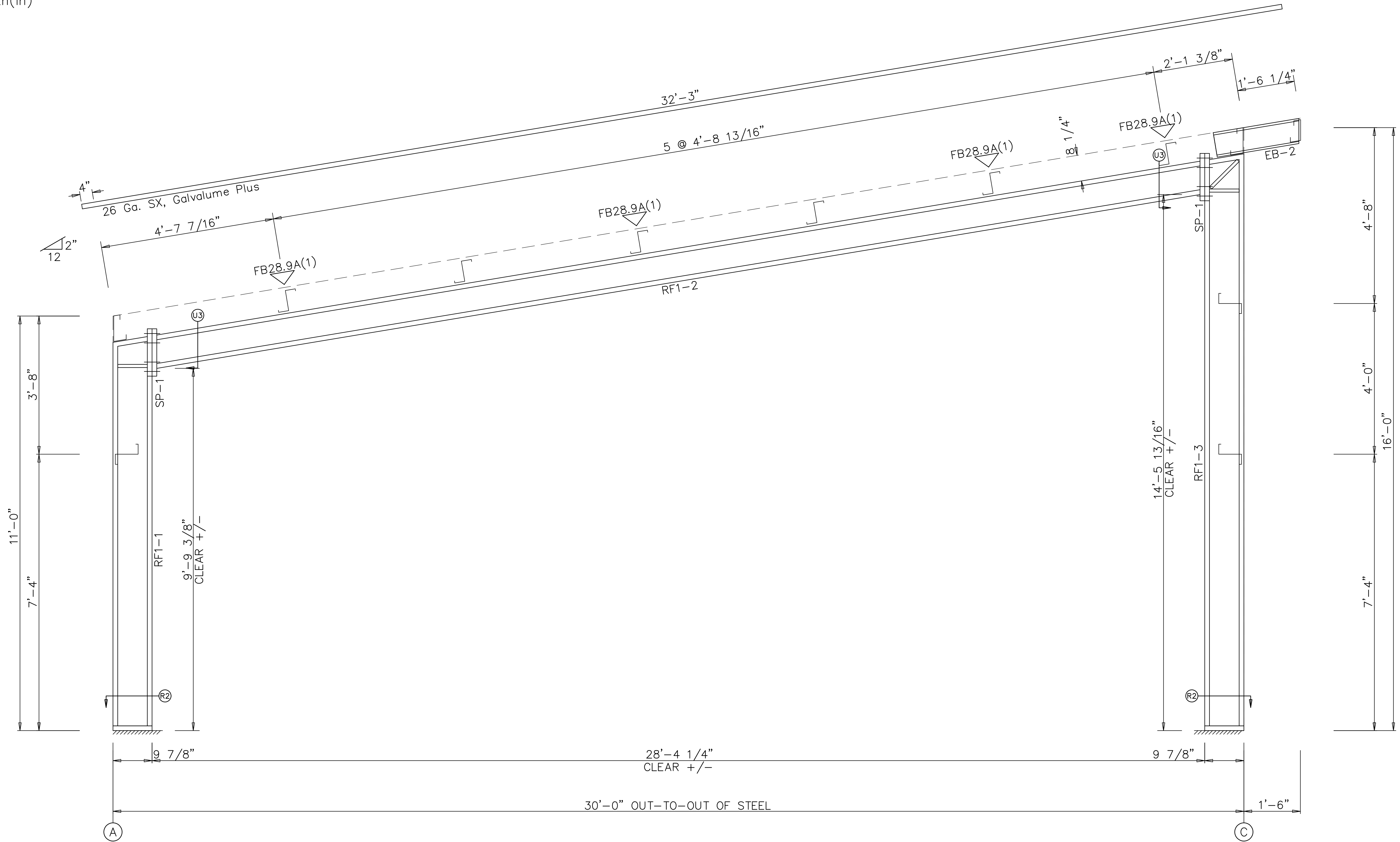


Union LaSteel				
PROJECT	Metal Building	ENDWALL FRAMING		
ID	MarkLane30x60	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Lake City, Fl	DATE: 1/ 7/22	SHEET 7 OF 13	

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	0	A325	0.750	2.00

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF1-1	W10X12	10'-3 5/8"
RF1-2	W8X10	28'-8 3/8"
RF1-3	W10X12	15'-3 5/8"
EB-2	W8X10	2'-4 1/8"

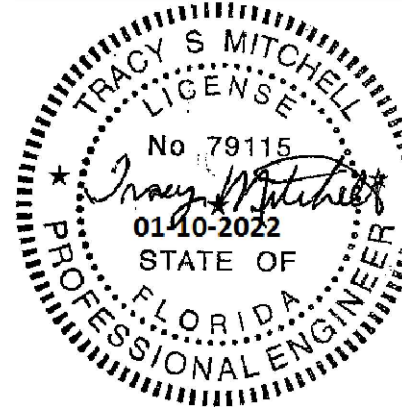
▽FLANGE BRACES: Both Sides(U.N.)
FBxxA(1): xx=length(in)
A - L2x2x14G



RIGID FRAME ELEVATION: FRAME LINE 2 3

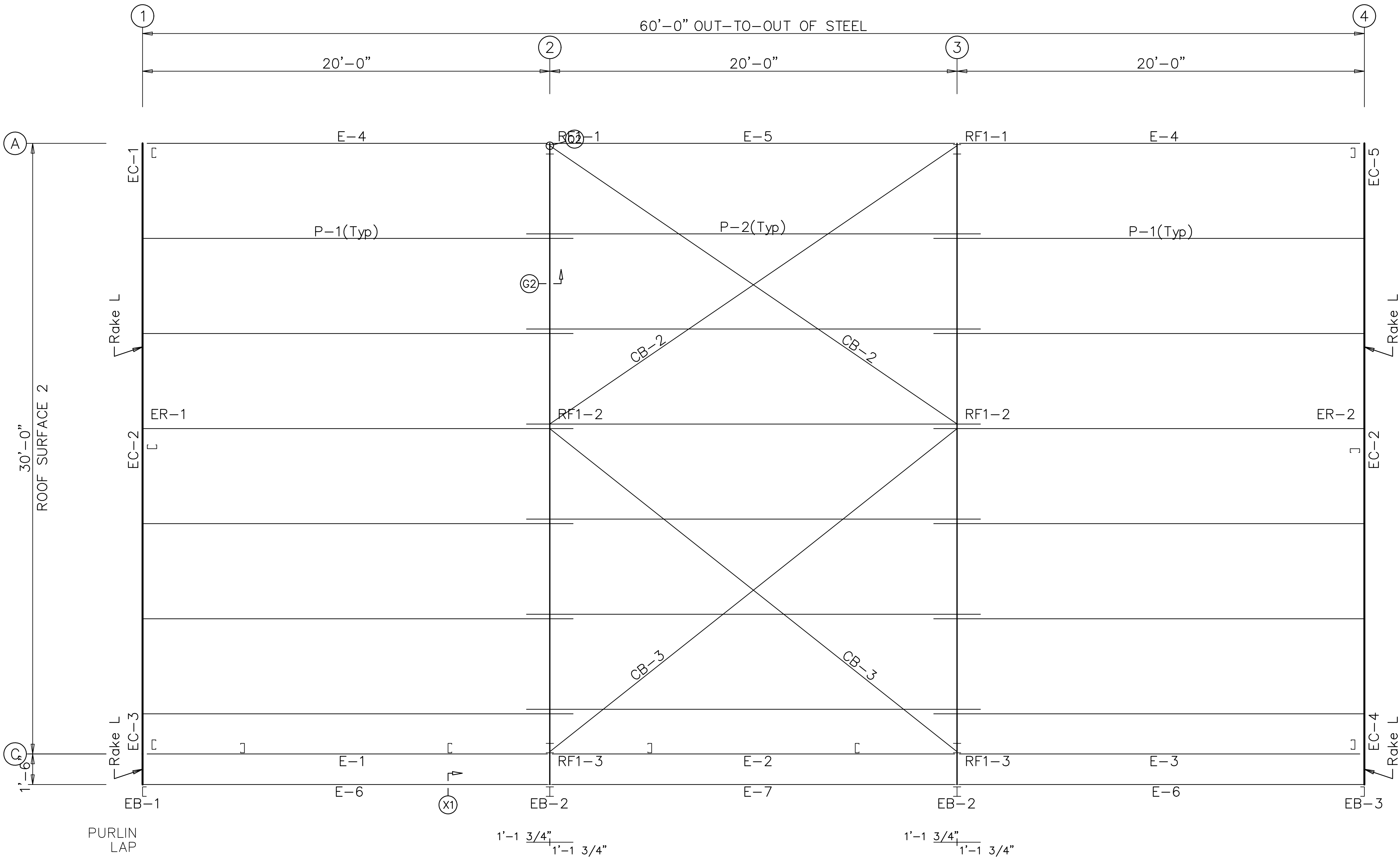
ASG LLC
AMERICAN STRUCTURAL GROUP
CONSULTING STRUCTURAL ENGINEERS
225 SOUTH MAIN STREET
MONROE, OHIO 45050
PH 513-402-7600
WWW.ASG-ENG.COM
"Value Engineering the First Time"

Union LaSteel				
PROJECT	Metal Building	RIGID FRAME ELEVATION		
ID	MarkLane30x60	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Lake City, FL	DATE: 1/ 7/22	SHEET 8 OF 13	



EXTENSION/CANOPY BOLTS				
ROOF PLAN				
MARK	QUAN	TYPE	DIA	LENGTH
EB-2	4	A325	1/2"	1 1/4"

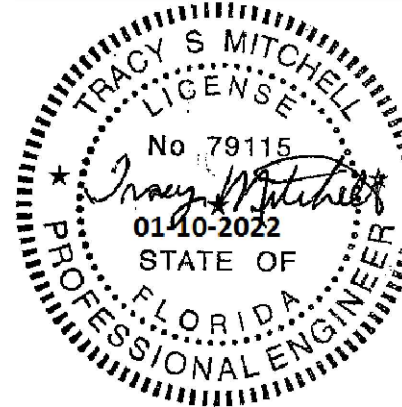
MEMBER TABLE		
ROOF PLAN		
MARK	PART	LENGTH
EB-1	8X2CH16	3'-7 5/8"
EB-2	W8X10	2'-4 1/8"
EB-3	8X2CH16	3'-7 5/8"
P-1	8X25Z16	20'-9 7/16"
P-2	8X25Z16	22'-3 1/2"
E-1	E085342H	19'-3 7/16"
E-2	E085342H	19'-3 1/2"
E-3	E085342H	19'-3 7/16"
E-4	E085342L	19'-7 7/16"
E-5	E085342L	19'-11 1/2"
E-6	E085342H	19'-7 1/2"
E-7	E085342H	19'-3 1/2"
CB-2	0.25_CBL	22'-7 1/2"
CB-3	0.25_CBL	23'-9 1/4"



ROOF FRAMING PLAN

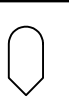
FRONT SW
SOFFIT

ROOF
SHEETING
PANELS: 26 Ga. SX
Galvalume Plus

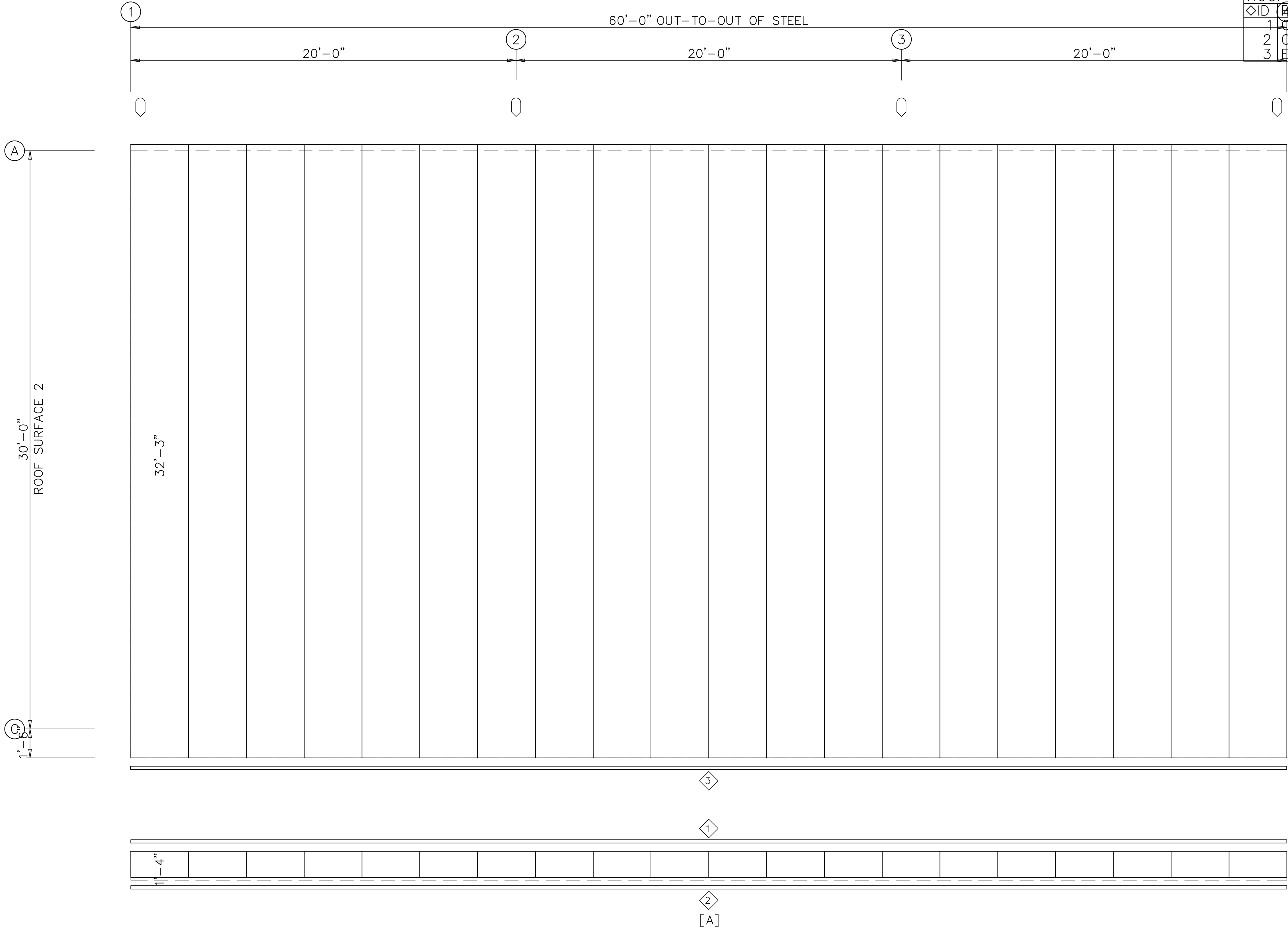


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Union LaSteel				
PROJECT	Metal Building	ROOF FRAMING		
ID	MarkLane30x60	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Lake City, Fl	DATE: 1/ 7/22	SHEET 9 OF 13	

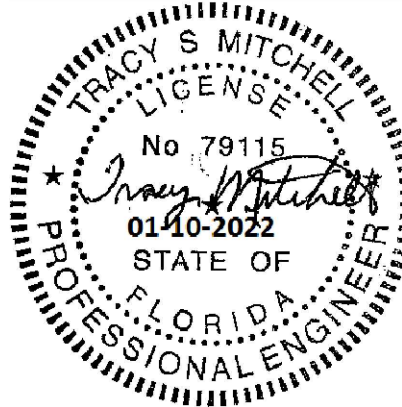
 DOWNSPOUT LOCATIONS

TRIM TABLE			
ROOF PLAN			
◇ ID	PART	LENGTH	DETAIL
1	CF-116	10'-3"	TRIM_267
2	CF-104	10'-3"	
3	ET-702	10'-3"	TRIM_316



ROOF SHEETING PLAN

PANELS: 26 Ga. SX – Galvalume Plus
 [A] SOFFIT PANELS: 26 Ga. SX – Need Sill Poly Color



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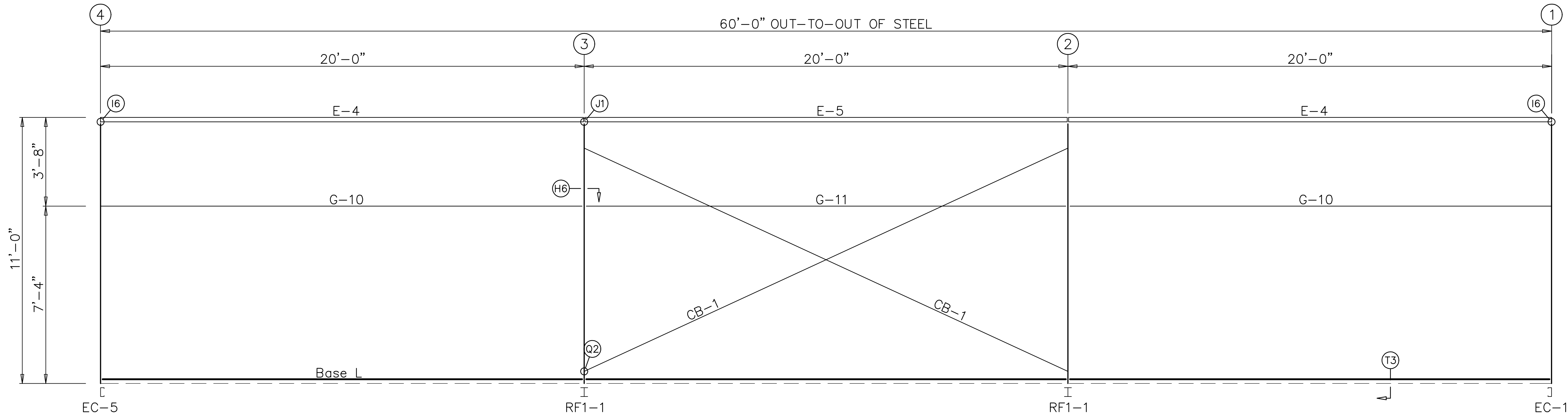
PH 513-402-7600

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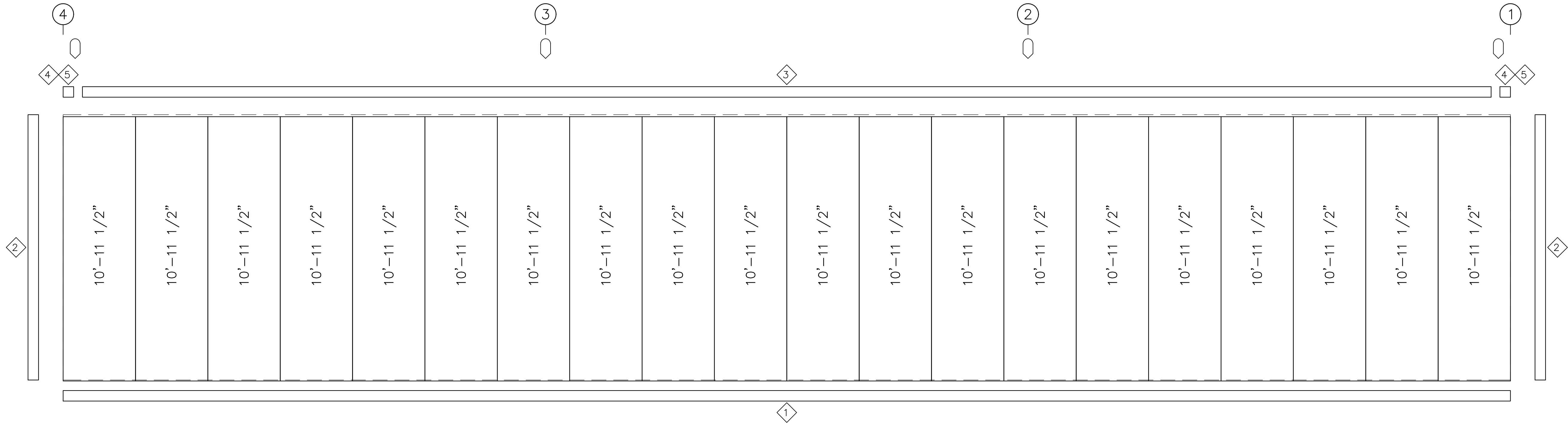
"Value Engineering the First Time"

Union LaSteel				
PROJECT	Metal Building	ROOF SHEETING		
ID	MarkLane30x60	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Lake City, Fl	DATE: 1/ 7/22	SHEET 10 OF 13	

DOWNSPOUT LOCATIONS



SIDEWALL FRAMING: FRAME LINE A

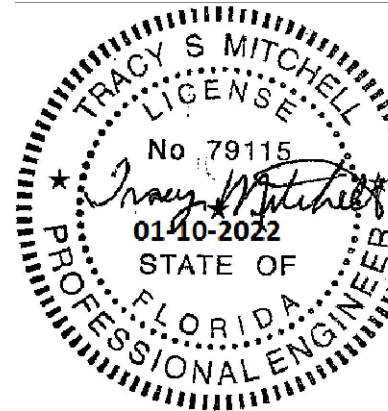


SIDEWALL SHEETING & TRIM: FRAME LINE A

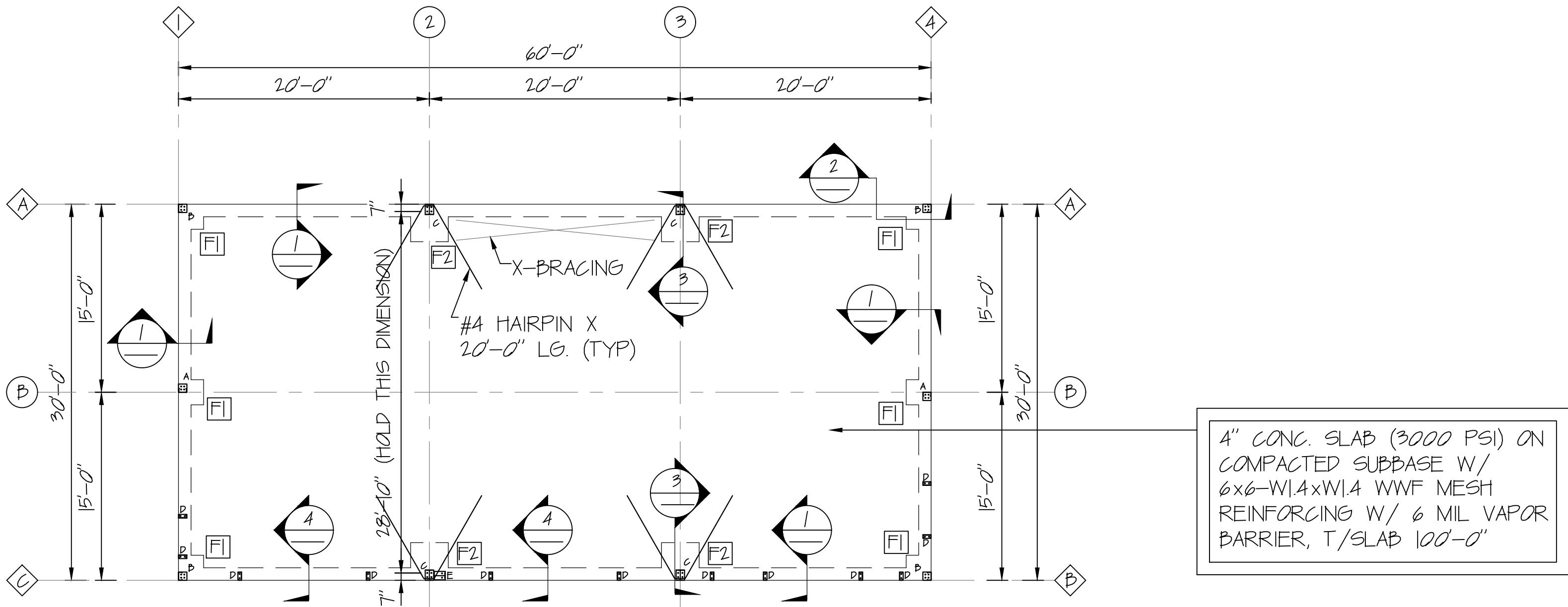
PANELS: 26 Ga. SX – Need Sill Poly Color

TRIM TABLE			
FRAME LINE A			
ID	PART	LENGTH	DETAIL
1	BT-101	10'-3"	TRIM_68
2	CT-102	10'-3"	TRIM_323
3	GU-121	20'-3"	
4	EC-122	6"	
5	SCB	6"	

MEMBER TABLE		
FRAME LINE A		
MARK	PART	LENGTH
E-4	E085342L	19'-7 7/16"
E-5	E085342L	19'-11 1/2"
G-10	8X25Z16	19'-4 5/16"
G-11	8X25Z16	19'-5 1/4"
CB-1	0.25_CBL	20'-8 1/2"



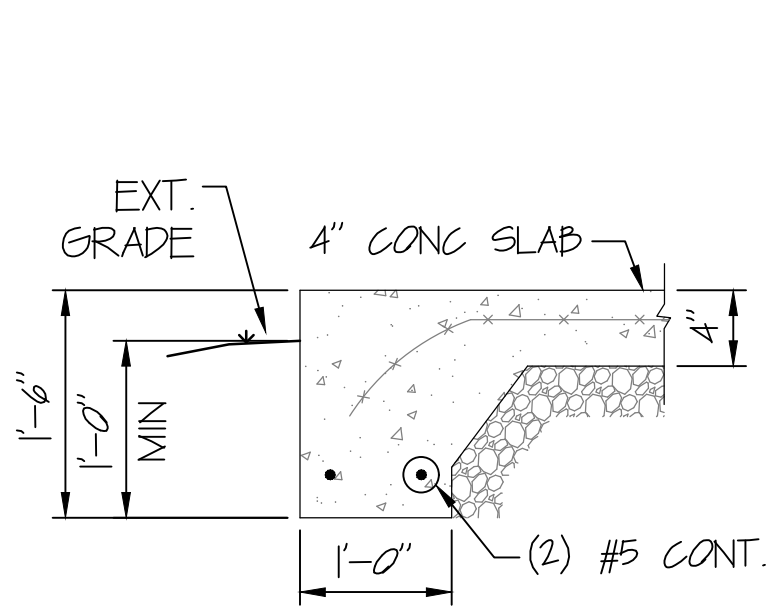
Union LaSteel				
PROJECT	Metal Building	SIDEWALL FRAMING		
ID	MarkLane30x60	DESIGN:	DRAFT:	CHECK:
PROJECT ADDRESS	Lake City, Fl	DATE: 1/ 7/22	SHEET 11 OF 13	



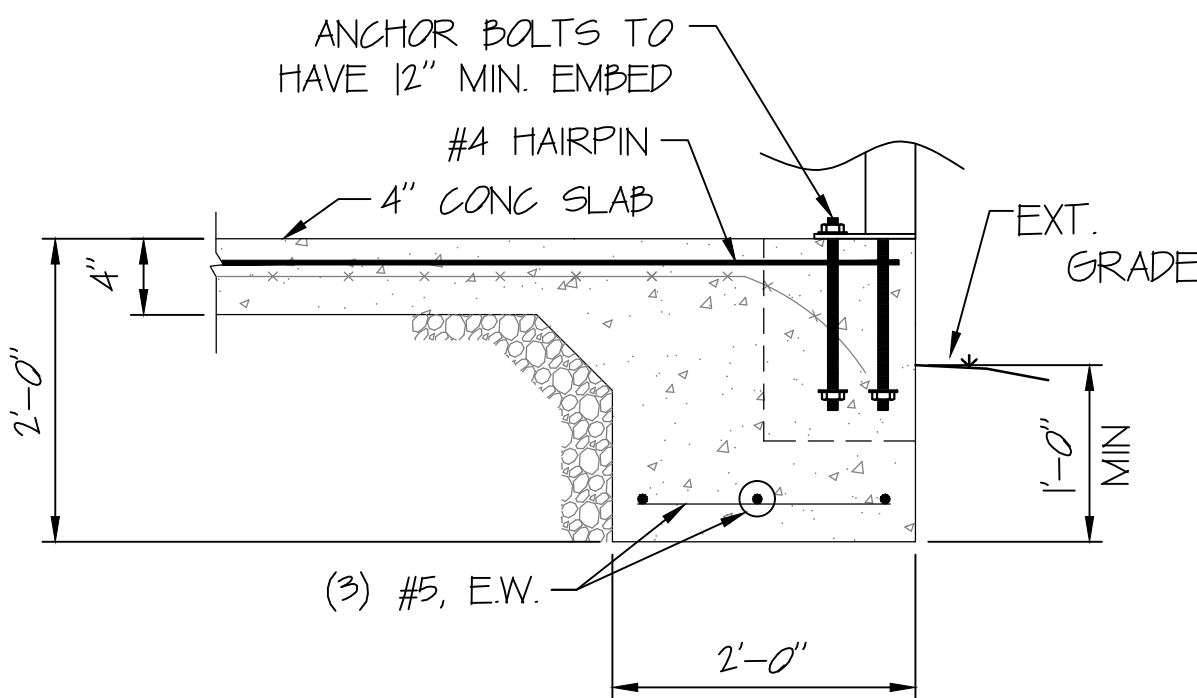
THE SOIL BEARING PRESSURE IS ASSUMED TO BE 1500 PSF MAXIMUM. UPON ENCOUNTERING SOILS LESS THAN THIS CONTACT THE ENGINEER.

FOUNDATION PLAN
NOTE: All Base Plates @ 100'-0" (U.N.)

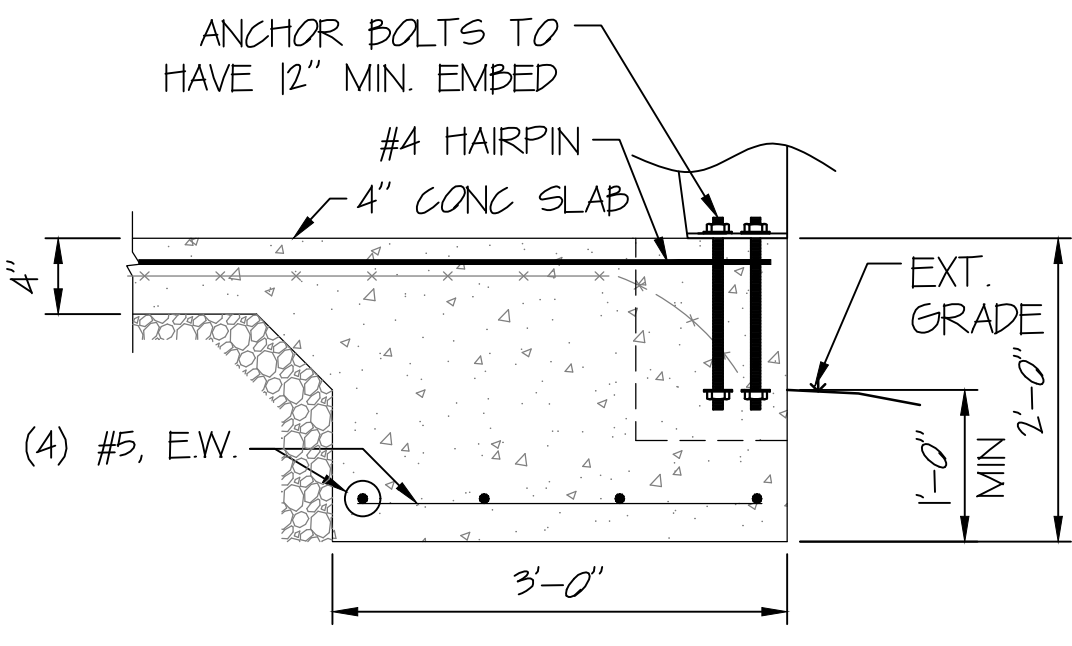
FOOTING SCHEDULE				
FND MARK	FND SIZE (L x B x D)	REINFORCING		TOP OF FOOTING
		TOP, E.W.	BOTT, E.W.	
F1	2'-0" x 2'-0" x 2'-0"	NA	(3)~#5's	100'-0"
F2	3'-0" x 3'-0" x 2'-0"	NA	(4)~#5's	100'-0"



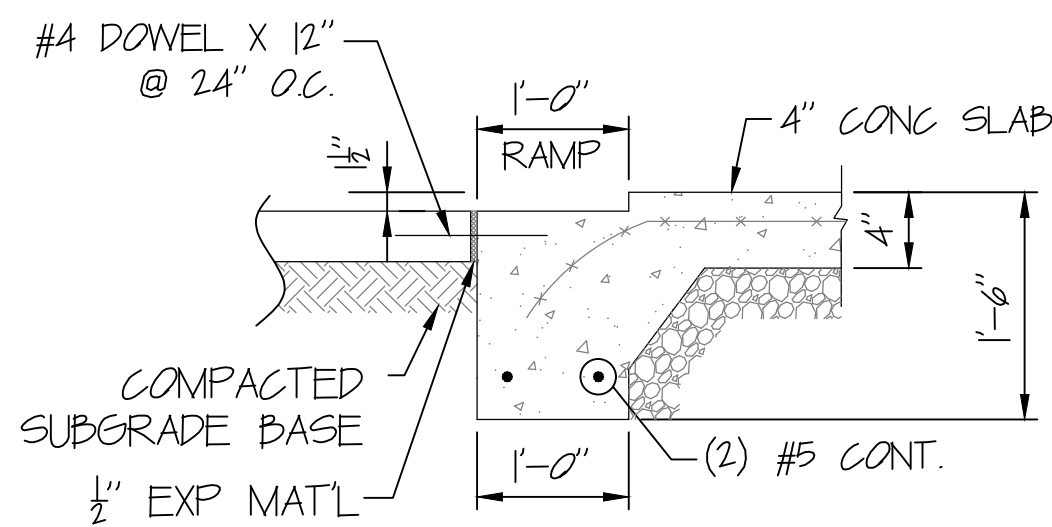
SECTION 1
N.T.S.



SECTION 2
N.T.S.



SECTION 3
N.T.S.



SECTION 4
N.T.S.

CONCRETE

- ALL CONCRETE WORK SHALL CONFORM TO ACI 301-18, "SPECIFICATIONS FOR BUILDINGS", EXCEPT AS NOTED BELOW. REFER TO CHAPTER 16 OF ACI 301 FOR REPORTS FROM TESTS THAT SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER, ARCHITECT, OWNER, ANY APPLICABLE CONTRACTORS, CONCRETE SUPPLIER, AND BUILDING OFFICIAL.
- CONCRETE WORK IN COLD WEATHER SHALL CONFORM TO ALL REQUIREMENTS OF ACI 308R-16 "STANDARD SPECIFICATION FOR COLD WEATHER CONCRETING" AND ACI 308R-16 "COLD WEATHER CONCRETING".
- CONCRETE WORK IN HOT WEATHER SHALL CONFORM TO ALL REQUIREMENTS OF ACI 308R-10 "HOT WEATHER CONCRETING". THE AIR TEMPERATURE, RELATIVE HUMIDITY, CONCRETE TEMPERATURE, AND WIND VELOCITY SHALL BE ENTERED INTO NOMOGRAPH FIG. 2.15 TO DETERMINE IF PRECAUTIONS AGAINST PLASTIC SHRINKAGE ARE REQUIRED.
- REBAR FABRICATION, PLACEMENT, SPLICING, ETC. SHALL BE IN ACCORDANCE WITH THE LATEST EDITIONS OF ACI 318, THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI) HANDBOOK AND ANY OTHER APPLICABLE PUBLICATIONS.
- A SLUMP TEST SHALL BE PERFORMED BEFORE ANY HRWR IS ADDED.
- CONCRETE MATERIAL SPECIFICATION: 3000 PSF f'c BASED ON 28 DAYS STRENGTH.
- UNLESS OTHERWISE NOTED, REINFORCEMENT BARS TO HAVE MINIMUM CONCRETE COVER AS SHOWN BELOW:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3 INCHES
CONCRETE EXPOSED TO EARTH OR WEATHER:	
#5 BARS AND SMALLER	1 1/2 INCHES
OTHER	2 INCHES
CONCRETE NOT EXPOSED TO EARTH OR WEATHER:	
BEAM AND COLUMN BARS INCLUDING TIES, STIRRUPS AND SPIRALS	1 1/2 INCHES
SLABS, WALLS AND JOISTS:	
#11 BARS AND SMALLER	3/4 INCHES
OTHERS	1 1/2 INCHES
- ALL EXPOSED EDGES ARE TO BE CHAMFERED 3/4 INCH, UNO.
- LAP SPLICE LENGTHS TO BE OF CLASS "B" PER ACI.
- CONCRETE REINFORCEMENT SHALL CONFORM TO ASTM A615 OR A616, GRADE 60.
- ISOLATED COLUMN FOOTINGS HAVE BEEN DESIGNED WITH AN ASSUMED ALLOWABLE SOIL BEARING PRESSURE OF 1500 PSF, UPON ENCOUNTERING ANY ADVERSE SOIL CONDITIONS CONTACT THE STRUCTURAL ENGINEER.
- DO NOT CAST CONCRETE AGAINST WATER OR FROZEN WATER.