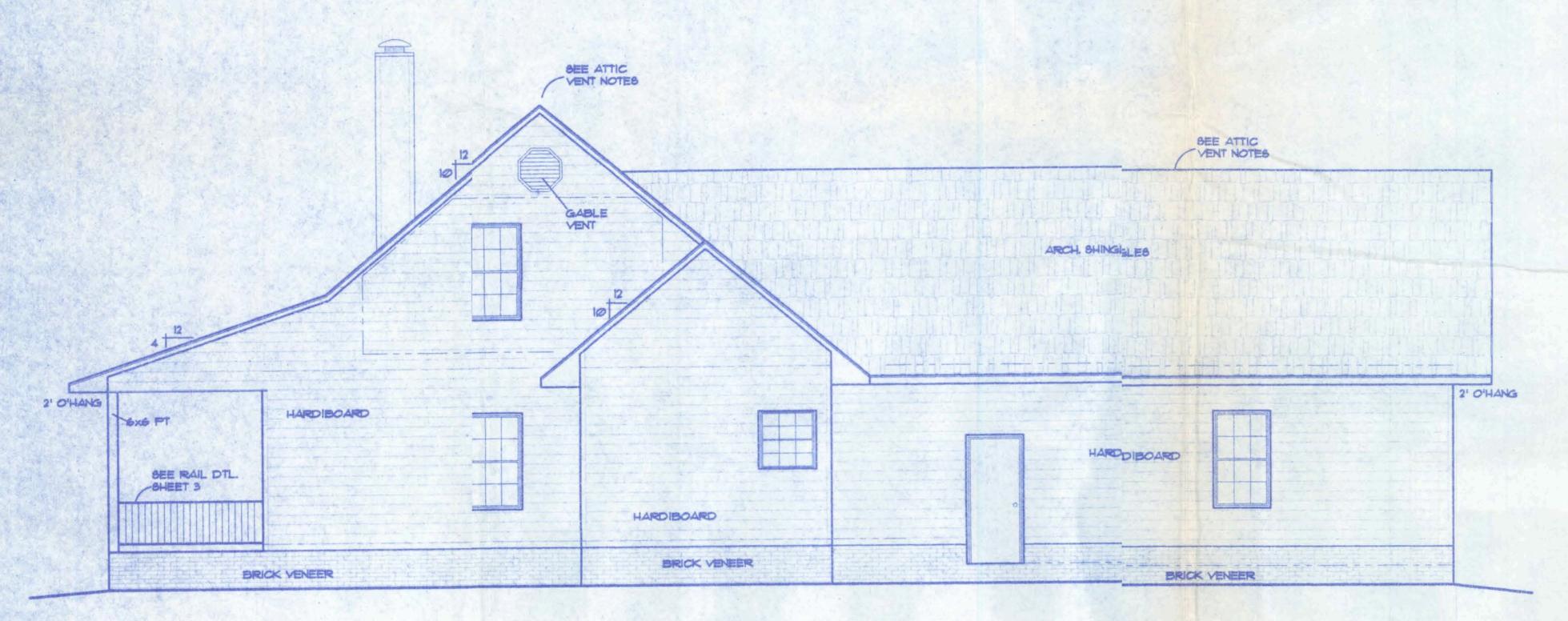
SEE ATTIC PRAME CHIMNEY VENT NOTES APPROX RIDGE. HT.= 25'-0" HARDIBOARD 2nd FL. CEILING -ARCH. SHINGLES ARCH, SHINGLES 2nd FL. FLOOR let FL CEILING HARDIBOARD 2' O'HANG -HARDIBOARD lat FL. FLOOR STEPS BY OWNER BRICK VENEER & BRIC VENEER

FRONT ELEVATION SCALE: 1/4 IN. = 1 FT.



RIGHT ELEVATION SCALE: 1/4 IN. = 1 FT.

ATTIC VENTILATION

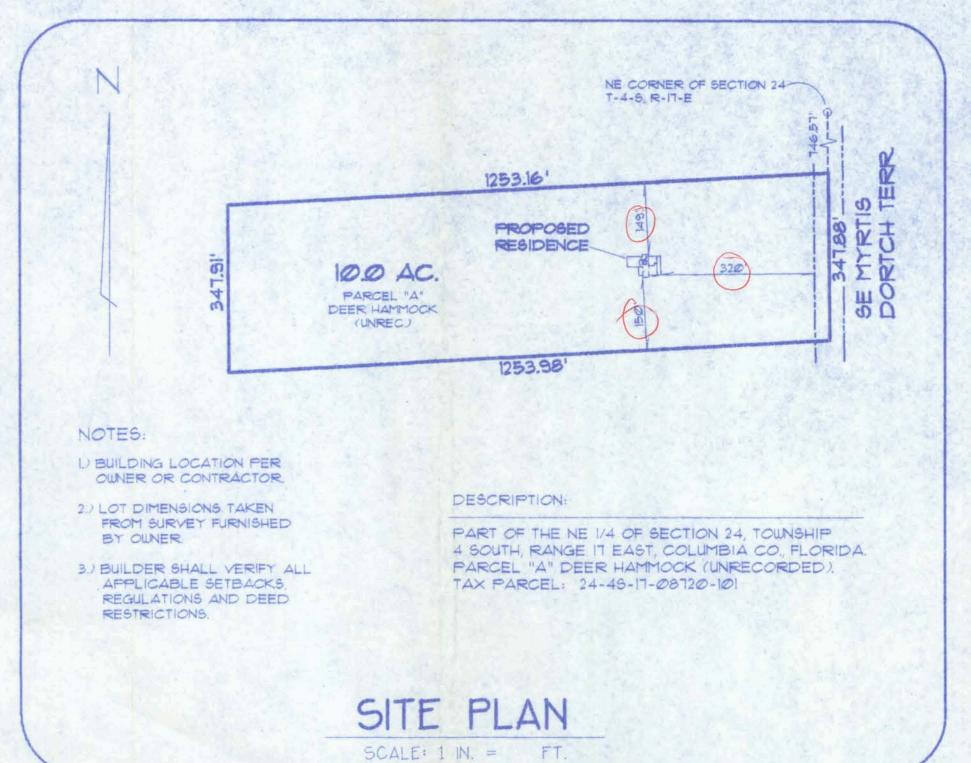
Enclosed attics and enclosed rafter spaces formed wher ceilings are applied directly to the underside of roof rafters shall he cross ventilation for each separate space by ventilating openins protected against the entrance of rain. Ventilating openings shall b provided with corrosion-resistant wire mesh, wit h 1 / 8 inch (3.2 m) minimum to 1/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less tha 1 to 150 of the area of the space ventilated except that the total areas permitted to be reduced to 1 to 300, provided at least 50 percent and ot more than 80 percent of the required ventilating area is provided by intilators located in the upper portion of the space to be ventilated at lest 3 feet (914 mm) above eave or cornice vents with the balance of te required ventilation provided by eave or cornice vents.

Index to Sheets

SHEET A-1	ELEVATIONS + GEN. NOTES
SHEET A-2	ELEVATIONS
SHEET A-3	FLOOR PLAN + SITE PLAN
SHEET A-4	FOUNDATION + SECTIONS
SHEET A-5	ELECTRICAL
SHEET 5-1	WIND ENGINEERING

Raines Residence



GENERAL NOTES

- 1.) See Wind Load Detail Sheet S-1' and Wind Engineer's Notes for data pertaining to Wind Design and compliance w/ Florida Building Code.
- 2.) All concrete used to be 2500 PSI strength or greater.
- 3.) HVAC duct and unit size/design is by engineered shop drawings from the AC contractor.
- 4.) Windows to be alum. framed and double glazed. Sizes shown are nominal and may vary with manufacturer.
- 5.) Roof Truss design is the responsibility of the supplier.
- 6.) The Truss Manufactuer shall prepare Shop Drawings Indicating Truss placement. Girder locations, Truss-to-Truss Connections and any point loads. The Contractor shall notify the Designer of any point loads in excess of 2.0k for Fnd. Modification.
- 7.) Site analysis of preparation information is not a part of this plan and is the responsibility of the owner.
- 8.) Cabinet and millwork detail is not a part of this plan. The plan is a general design and details shall be the responsibility of the owner and/or contractor.

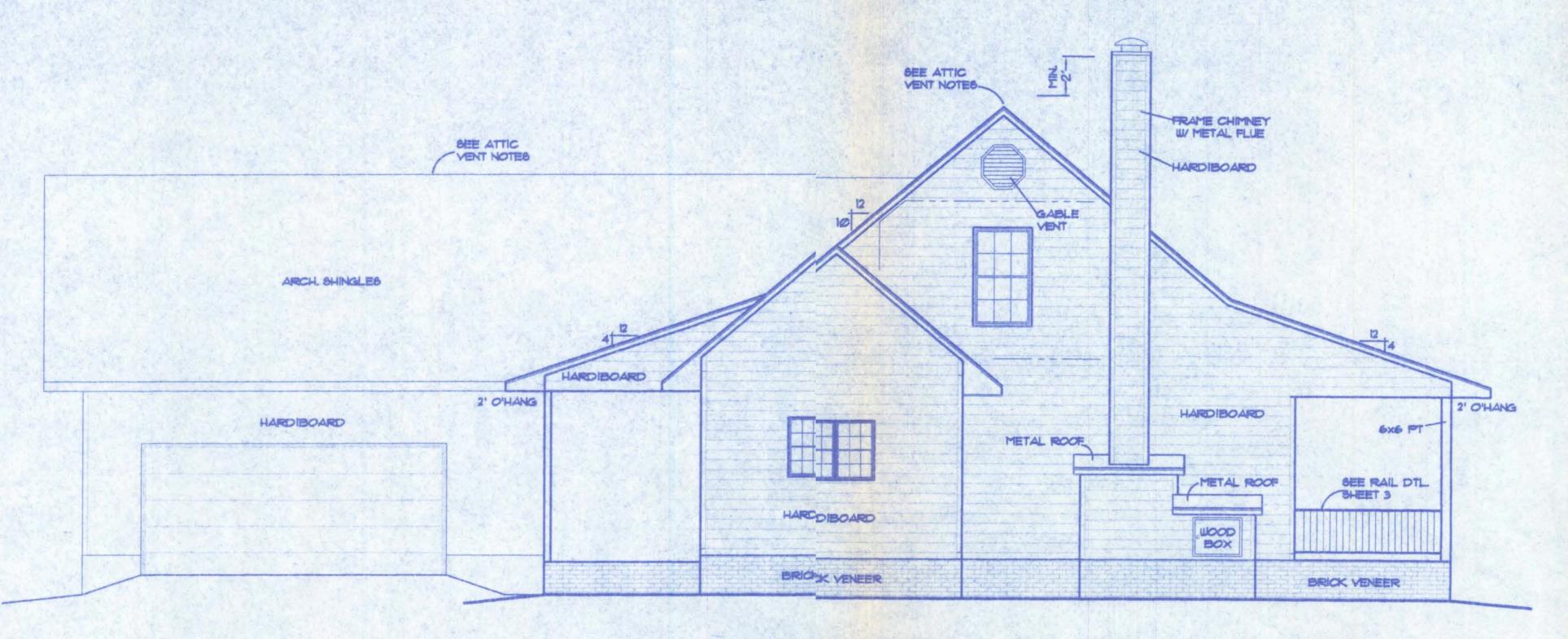
WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

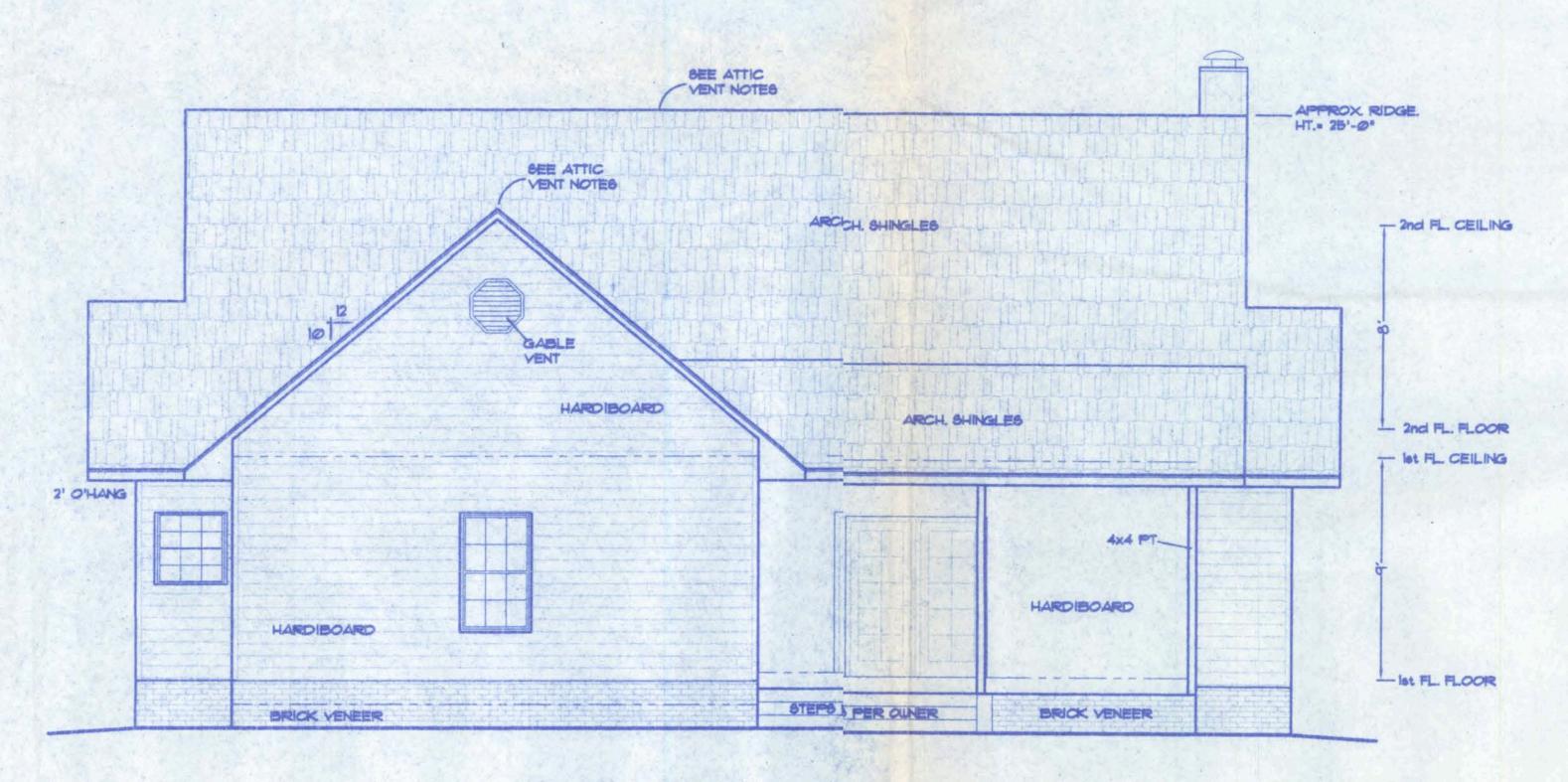
LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

SE MYRTIS DORTCH TERRACE Location: LAKE CITY, FL

O7-006	RAINES	SHEET: 1 OF 5			
6-1-07	RESIDENCE	CAD FILE: 07000			
VN: TAD	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV: 1-5-08			
CK:	192 SW Sagewood Gln. Lake City. FL 32024 Phone (386) 755-5891	REV:			



LEFT ELEWATION SCALE: 1/4 IN. == 1 FT.



ATTIC VENTILATION

Enclosed attics and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain. Ventilating openings shall be provided with corrosion—resistant wire mesh, wit h 1 / 8 inch (3.2 mm) minimum to 1/4 inch (6.4 mm) maximum openings.

The total net free ventilating area shall not be less than 1 to 150 of the area of the space ventilated except that the total area is permitted to be reduced to 1 to 300, provided at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents.

REAR ELEVATION SCALE: 1/4 IN. = 1 FT.

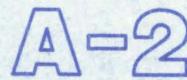
WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

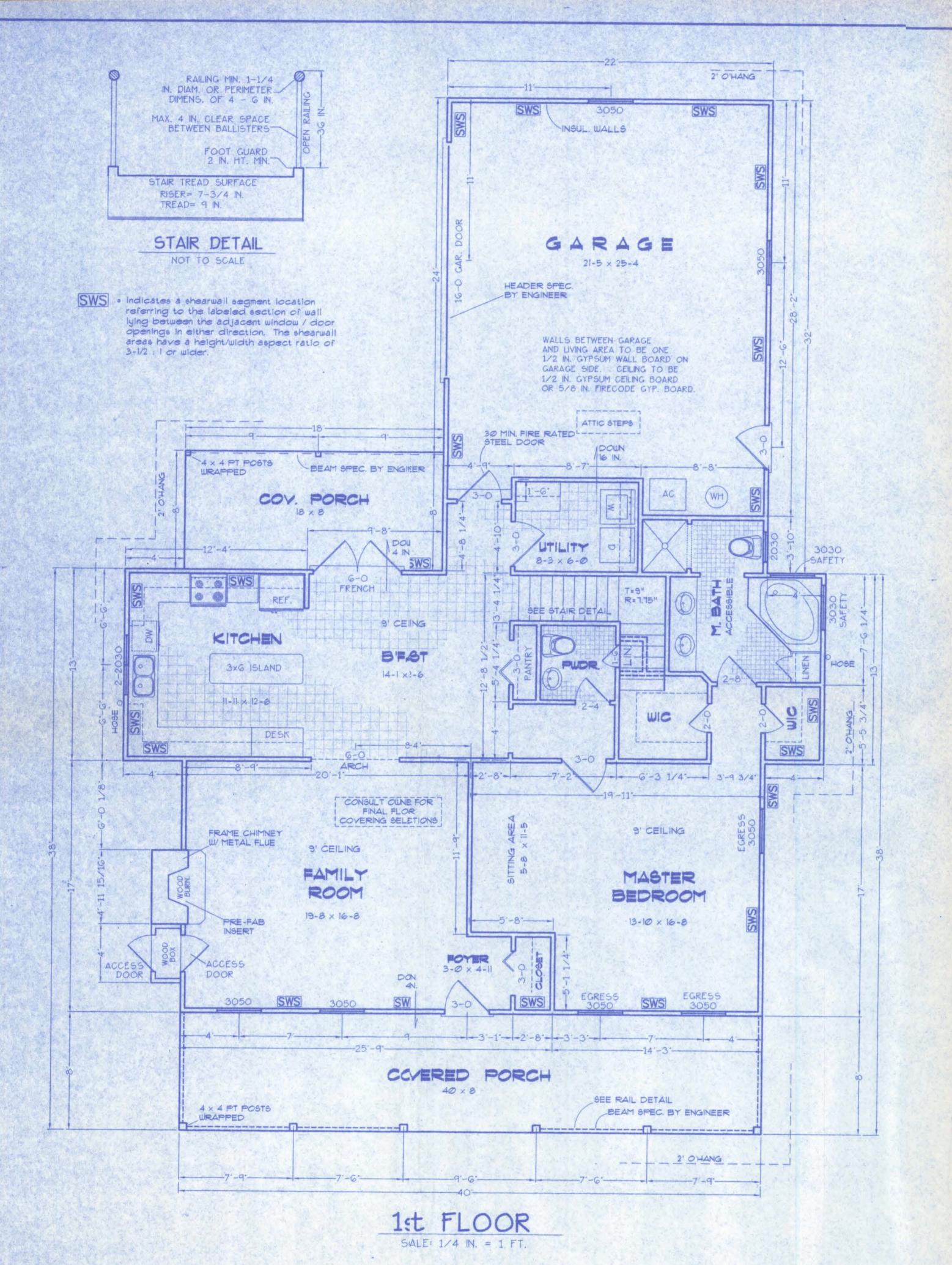
SE MYRTIS DORTCH TERRACE

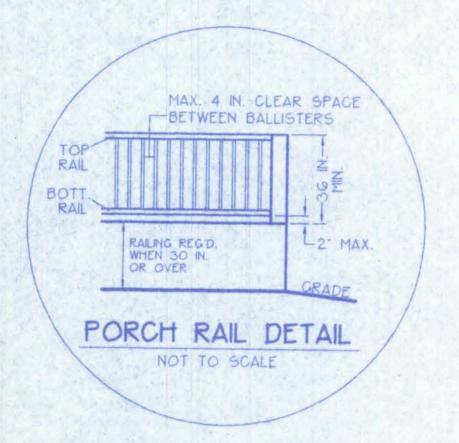
Location: LAKE CITY FL Job No



FILE: 07-006	RAINES	SHEET: 2 OF 5			
DATE: 6-1-07	RESIDENCE	CAD FILE: 07006			
DRAWN: T A D	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV: 1-5-08			
CHECK:	192 SW Sagewood Gh., Lake City, FL 32024 Phone (386) 755-5891	REV:			

1474508 V

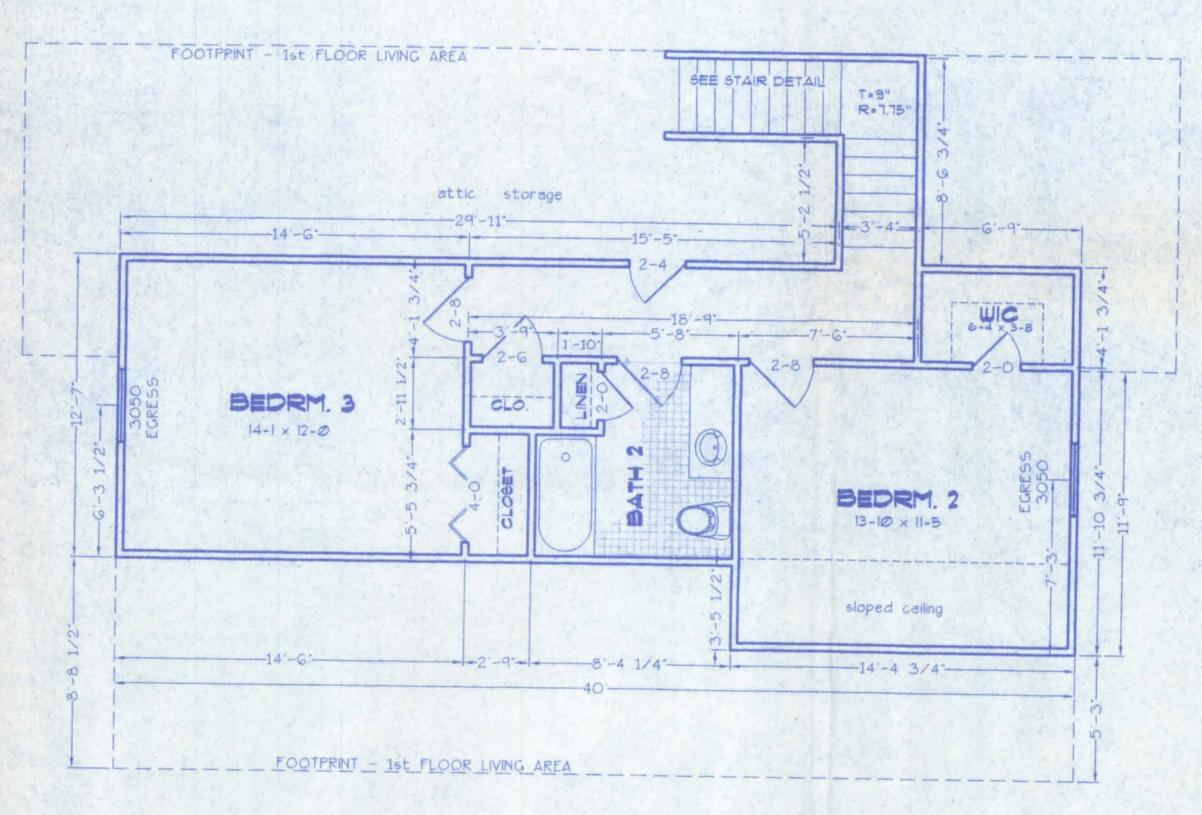




AREA SUMMARY

let FLOOR - - - - - 1416 SF 2nd FLOOR - - - - 553 SF TOTAL CONDITIONED - 1969 SF GARAGE - - - - - 593 SF FRONT PORCH - - - - 320 SF REAR PORCH - - - - 144 SF

TOTAL UNDER ROOF - - 3026 SF



2nd FLOOR SCALE: 1/4 IN. = 1 FT.

WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

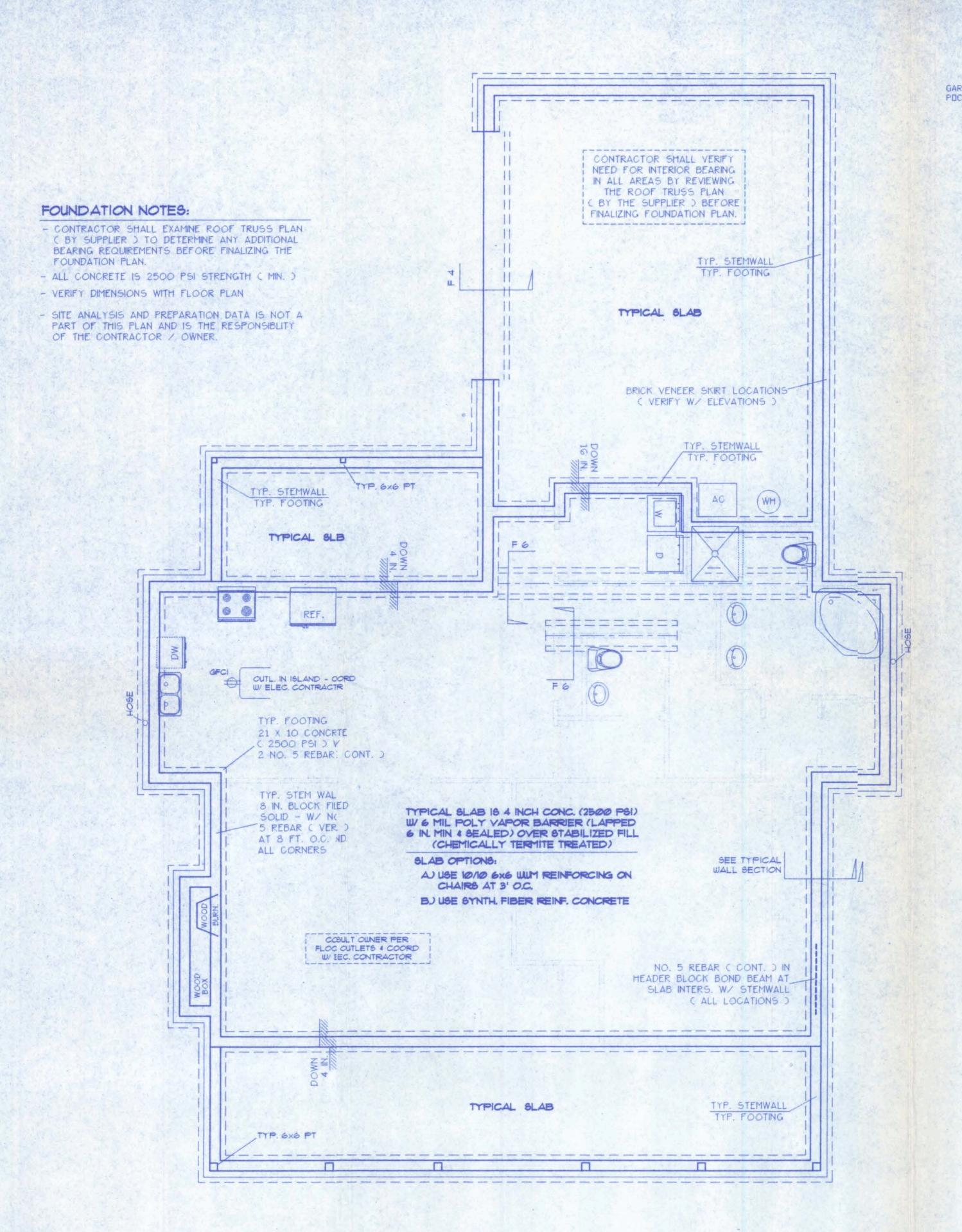
SE MYRTIS DORTCH TERRACE Location: LAKE CITY, FL

4-3 RAINES

SHEET: 07-006 3 OF 5 DATE: RESIDENCE CAD FILE: 6-1-07 07006 DRAWN: PREPARED BY: TIM DELBENE
Drafting + Technical Services TAD 1-5-08 CHECK: 192 SW Sagewood Gln. Lake City. FL 32024 Phone (386) 755-5891

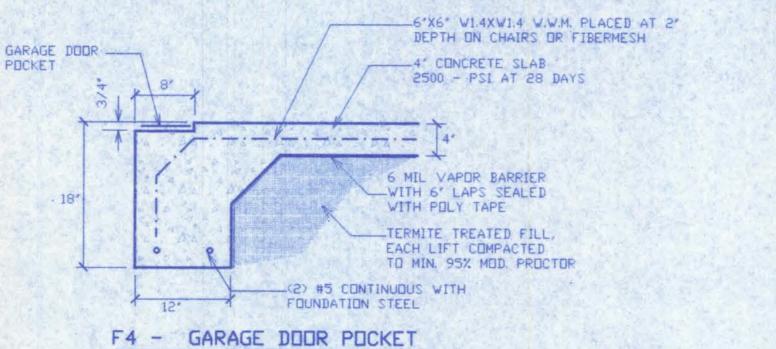
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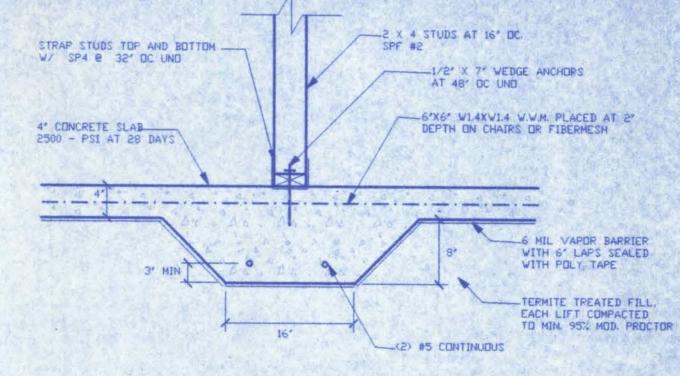
1478408



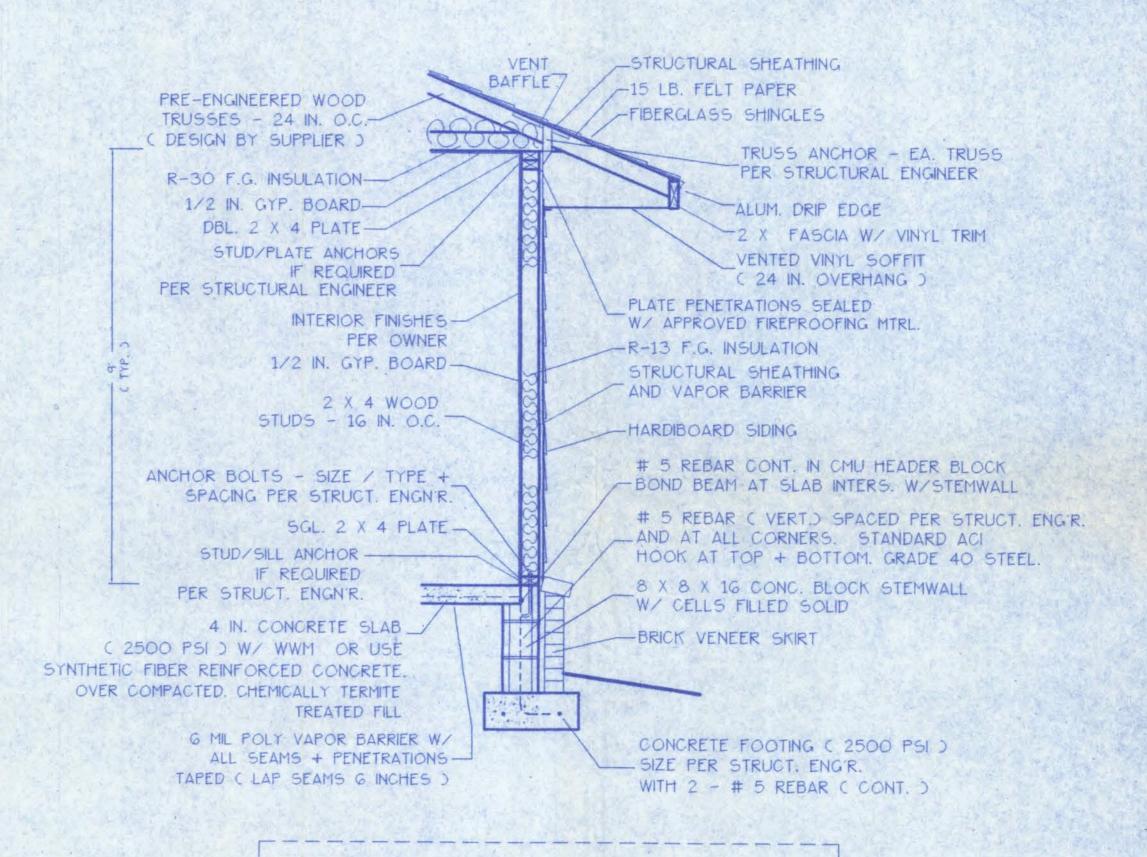
FOUNDATION PLAN

SCALE: 1/4 IN. = 1 FT.





F6 - INTERIOR BEARING FOOTING



WALL SECTION NOTES:

- This Typical Wall Section is for Estimating purposes only.

- All data shown in this Wall Section shall be subject to review . and final input by the Structural Engineer.

DESIGN WALL SECTION NON-STRUCTURAL DATA

SCALE: 3/4 IN. = 1 FT.

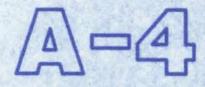
WINDLOAD ENGINEER: Mark Disosway, PE No.53915, POB 868, Lake City, FL 32056, 386-754-5419

CERTIFICATION: These plans and "Windload Engineering", Sheet S-1, attached, comply with Florida Building Code Residential 2004, Section R301.2.1 to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location, permitted within 90 days of signature date. In case of conflict, structural requirements, scope of work, and builder responsibilities on sheet S-1 control.

SE MYRTIS DORTCH TERRACE

Location: LAKE CITY, FL Jo



FILE:

07-006

RESIDENCE

1-5-08

PREPARED BY:

T A D

CHECK:

T A D

CHECK:

T A D

CHECK:

T A D

Phone (386) 755-5891

SHEET:

4 OF 5

CAD FILE:

07006

REV:

REV:

REV:

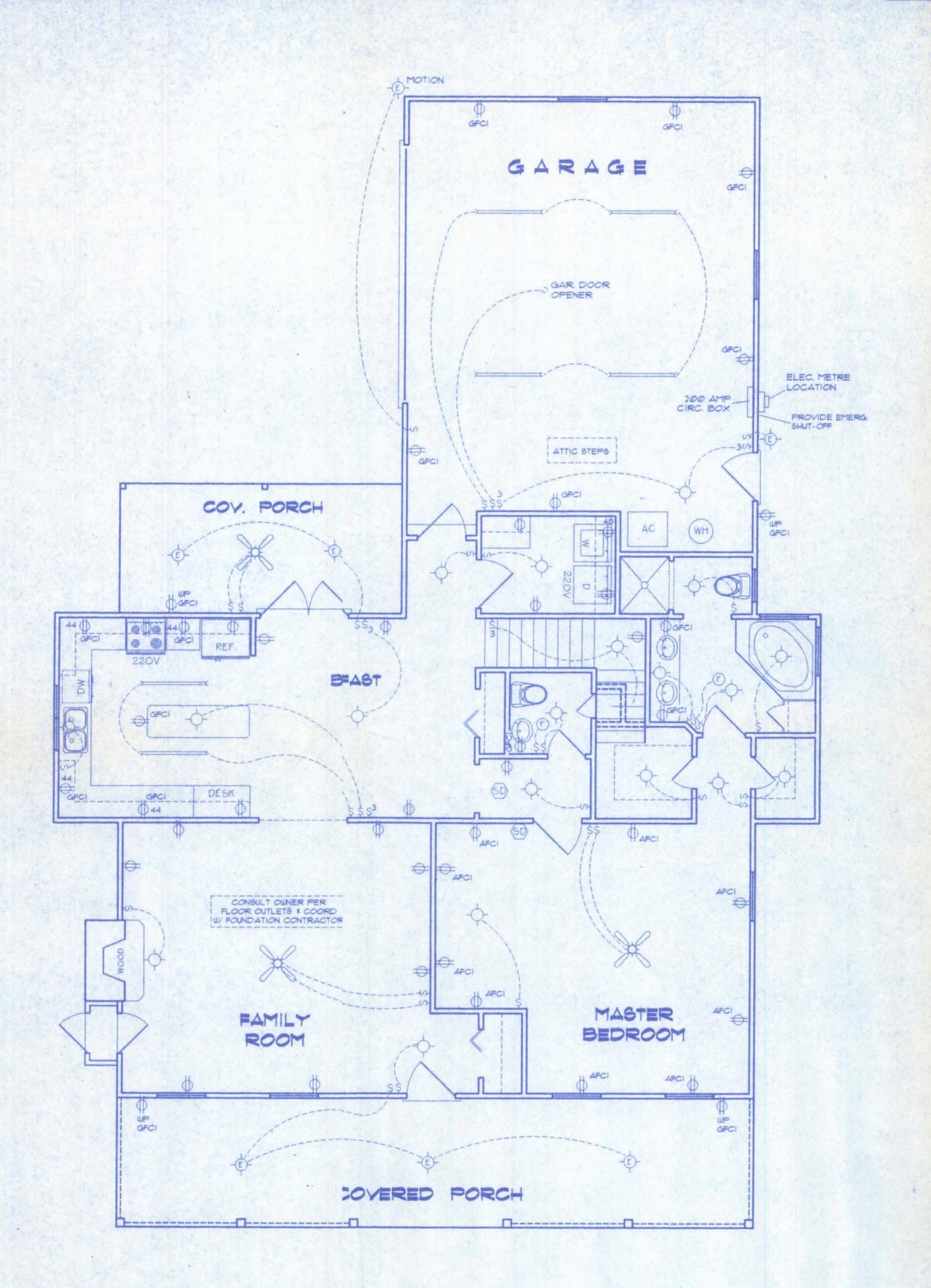
REV:

REV:

REV:

BOCALPI

Moon



ELECTRICAL	SYMBOL LEGEND
	= FLOURESCENT LIGHTING FIXTURE.
\$	= CEILING LIGHT FIXTURE
-¢-	= EXTERIOR LIGHTING FIXTURE
Ş	= LIGHT SWITCH.
\$ ₃	= THREE-WAY SWITCH.
ф	= 110 V. DUPLEX OUTLET.
ф42	= SPECIAL HEIGHT 110 V. DUPLEX OUTLET
φ ^{GFCI}	= GROUND FAULT CIRC. OUTLET
	= ARC FAULT CIRC. OUTLET
ф	= 110 V. SINGLE RECEPTACLE OUTLET.
€220V	= 220 VOLT OUTLET (4 WIRE)
X	= FAN LOCATION
•	= FAN LOCATION (EXHAUST)
6 D	= SMOKE DETECTOR

ELECTRICAL PLAN NOTES

-WIRE ALL APPLIANCES. HVAC UNITS AND OTHER EQUIPMENT PER MANUF. SPECIFICATIONS.

-CONSULT THE OWNER FOR THE NUMBER OF SEPERATE TELEPHONE LINES TO BE INSTALLED.

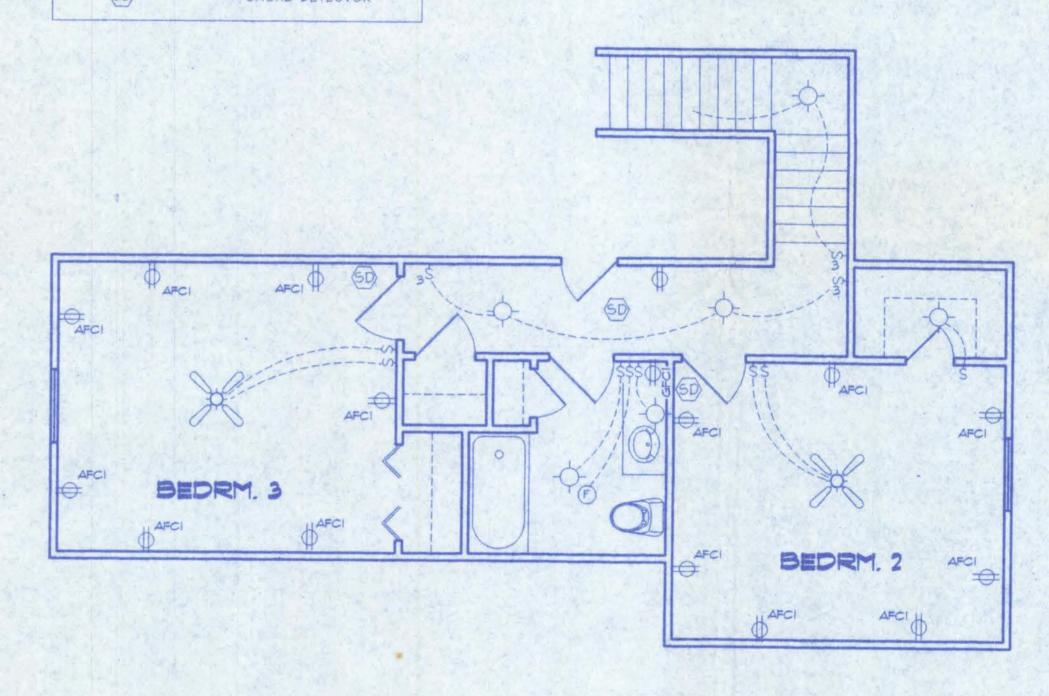
-ALL INSTALLATIONS SHALL BE PER NAT'L ELECTRIC CODE.

-ALL SMOKE DETECTORS SHALL BE 120V W/ BATTERY BACKUP OF THE PHOTOELECTRIC TYPE. AND SHALL BE INTERLOCKED TOGETHER. INSTALL INSIDE AND NEAR ALL BEDROOMS.

-TELEPHONE. TELEVISION AND OTHER LOW VOLTAGE
DEVICES OR OUTLETS SHALL BE AS PER THE OWNER'S
DIRECTIONS. + IN ACCORDANCE W/ APPLICABLE
SECTIONS OF NEC-LATEST EDITION.

-ELECTRICAL CONT'R SHALL BE RESPONSIBLE FOR THE DESIGN + SIZING OF ELECTRICAL SERVICE AND CIRCUITS.

TO BE DETERMINED BY POWER COMPANY.

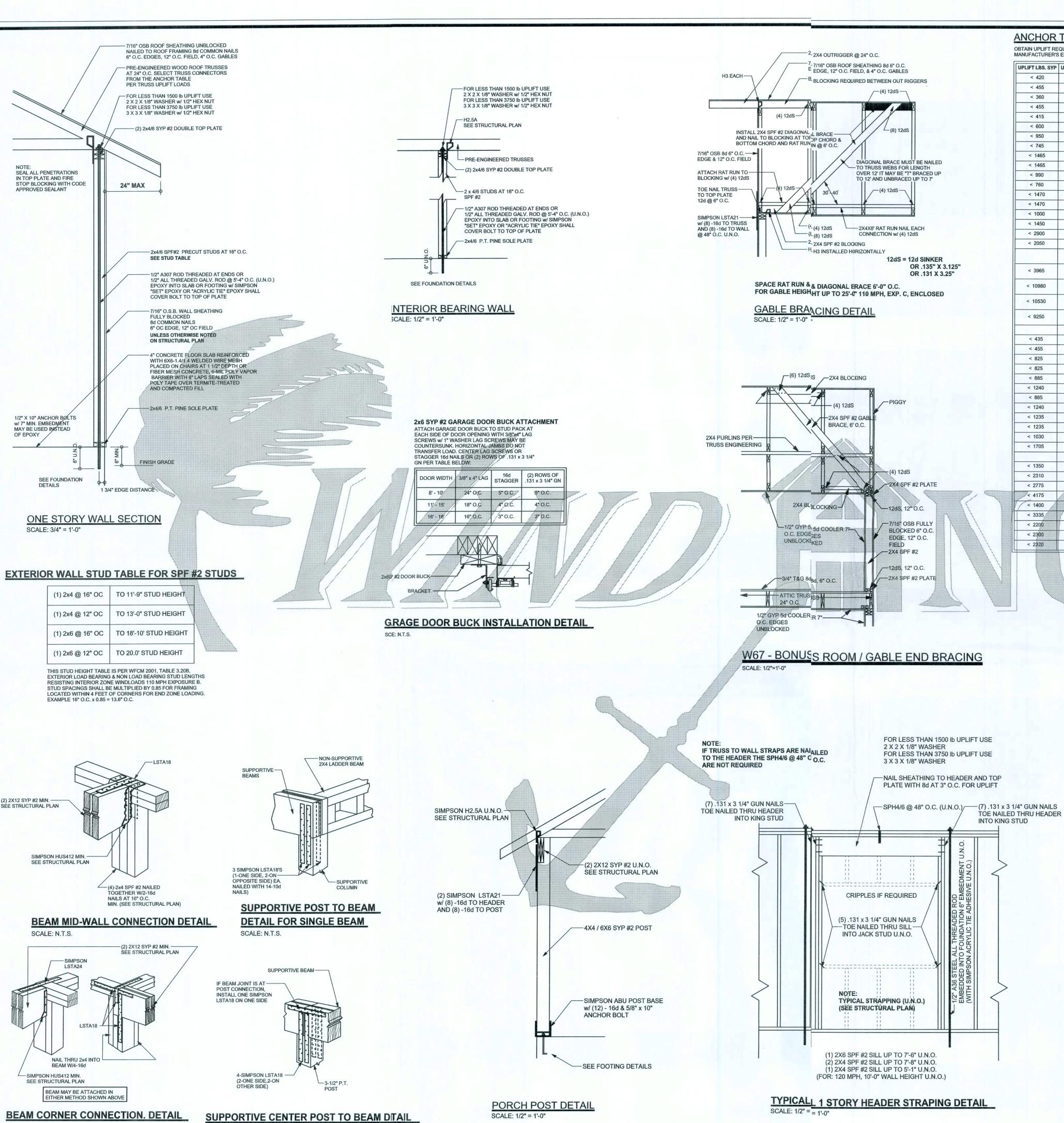


ELECTIRICAL PLAN NOST TO SCALE

SE MYRTIS DORTCH TERRACE LAKE CITY, FL

CHECK:

A-5				
FILE: 07-006	RAINES	SHEET: 5 OF 5		
TE: 1-5-08	RESIDENCE	CAD FILE: 07006		
AWN: TAD	PREPARED BY: TIM DELBENE Drafting + Technical Services	REV:		
TAD	192 SW Sagewood Gln., Lake City, FL 32024 Phone (386) 755-5891	REV:		



ANCHOR TABLE

< 2775

< 4175

< 1400

< 3335

< 2200

< 2300

< 2320

< 2570

< 3695

< 1400

< 2200

< 2300

< 2320

HD2A

PAHD42

HPAHD22

ABU44

ABU66

ABU88

2-5/8" BOLTS

18 - 16d

16-16d

16-16d

12-16d

18 - 16d

GRADE & SPECIES TABLE

SYP #2

SYP #2

SYP #2

24F-V3 SP

TIMBERSTRAND I

MICROLAM

PARALAM

DOUBLE 2x4 SPF TOP PLATE NAILED

TOGETHER W/2-16d NAILS AT 16" O.C.

4' MIN. LAP w/ (12) - 16d OR 4" LAP w/

CS20 w/ (4) - 16d &(14) - 10d

SPECIFIED ON FLOOR PLAN

ALL STUDS TO BE 2x4 ----

CONTINUOUS FRAME TO

CEILING DIAPHRAGM DETAIL

AND BOTTOM PLATES

WITH 2-16d NAILS

SCALE: N.T.S.

INTERIOR CEILING AS

CONTINUOUS FRAME

BOTTOM CHORD OF TRUSS

TO TOP PLATE AT

Fb (psi) E (10⁶ psi)

1.6

1200

1050

975

2400

1700

2900

2900

OBTAIN UPLIFT REQUIREMENTS FROM TRUSS

JPLIFT LBS. SYP	UPLIFT LBS. SPF	TRUSS CONNECTOR*	TO PLATES	TO RAFTER/TRUSS	TO STUDS
< 420	< 245	H5A	3-8d	3-8d	
< 455	< 265	H5	4-8d	4-8d	
< 360	< 235	H4	4-8d	4-8d	
< 455	< 320	H3	4-8d	4-8d	
< 415	< 365	H2.5	5-8d	5-8d	
< 600	< 535	H2.5A	5-8d	5-8d	
< 950	< 820	H6	8-8d	8-8d	
< 745	< 565	H8	5-10d, 1 1/2"	5-10d, 1 1/2"	
< 1465	< 1050	H14-1	13-8d	12-8d, 1 1/2"	
< 1465	< 1050	H14-2	15-8d	12-8d, 1 1/2"	
< 990	< 850	H10-1	8-8d, 1 1/2"	8-8d, 1 1/2"	
< 760	< 655	H10-2	6-10d	6-10d	
< 1470	< 1265	H16-1	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1470	< 1265	H16-2	10-10d, 1 1/2"	2-10d, 1 1/2"	
< 1000	< 860	MTS24C	7-10d 1 1/2"	7-10d 1 1/2"	
< 1450	< 1245	HTS24	12-10d 1 1/2"	12-10d 1 1/2"	
< 2900	< 2490	2 - HTS24			
< 2050	< 1785	LGT2	14 -16d	14 -16d	
		HEAVY GIRDER TIEDOWNS*			TO FOUNDATION
< 3965	< 3330	MGT		22 -10d	1-5/8" THREADED ROD 12" EMBEDMENT
< 10980	< 6485	HGT-2		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 10530	< 9035	HGT-3		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
< 9250	< 9250	HGT-4		16 -10d	2-5/8" THREADED ROD 12" EMBEDMENT
		STUD STRAP CONNECTOR*			TO STUDS
< 435	< 435	SSP DOUBLE TOP PLATE	3 -10d		4 -10d
< 455	< 420	SSP SINGLE SILL PLATE	1 -10d		4 -10d
< 825 < 825 DSP D		DSP DOUBLE TOP PLATE	6 -10d		8 -10d
< 825	< 600	DSP SINGLE SILL PLATE	2 -10d		8 -10d
< 885	< 760	SP4			6-10d, 1 1/2"
< 1240	< 1065	SPH4			10-10d, 1 1/2"
< 885 < 760 SP6				6-10d, 1 1/2"	
< 1240	< 1065	SPH6			10-10d, 1 1/2"
< 1235	< 1165	LSTA18	14-10d		
< 1235	< 1235	LSTA21	16-10d		
< 1030	< 1030	CS20	18-8d		
< 1705	< 1705	CS16	28-8d		
		STUD ANCHORS*	TO STUDS		TO FOUNDATION
< 1350	< 1305	LTT19	8-16d		1/2" AB
< 2310	< 2310	LTTI31	18-10d, 1 1/2"		1/2" AB

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR 2004. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS, BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN UPLIFT CONNECTION 415LB EACH END; 2X8 RAFTERS 700 LB EACH END.

REVISIONS

SOFTPLAN

/INDLOAD ENGINER: Mark Disosway, PE No.53915, POB 86, Lake City, FL

Stated dimensions suprcede scaled

limensions. Refer all destions to

Do not proceed withou clarification.

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Mark Disosway, P.E. hreby expressly reserv s common law copyrights and property right i

hese instruments of sirvice. This document is

not to be reproduced, Itered or copied in any form or manner withou first the express writte permission and consett of Mark Disosway.

CERTIFICATION: I heaby certify that I have xamined this plan, and that the applicable

ortions of the plan, reating to wind engine

code residential 2004, o the best of my

IMITATION: This desgn is valid for one

P.E.53915

Norto₁ Home

<u>Improvements</u>

Raines Residence

ADDRESS:

SE Myrtis Errtch Terrace

Lake Ciy, Florida

Mark Dis₀sway P.E. P.O. Eox 868

Lake City, Florida 32056 Phone: (386) 754 - 5419

Fax: (386)269 - 4871

PRINTED DATE:

January 3, 2008

CHECKED BY:

ouilding, at specified loation.

comply with section R01.2.1, florida building

Mark Disosway, P.E. fr resolution

32056, 386-754-5419

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1000 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, F'c = 3000 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185; LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT. FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. FIBERS TO COMPLY WITH ASTM C 1116. SUPPLIER TO PROVIDE ASTM C 1116 CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB PLACEMENT. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL. (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 60, DEFORMED BARS, FY = 60 KSI. ALL LAP SPLICES 40 * DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 315-96, U.N.O.

GLULAM BEAMS: GLULAM BEAM, GLB, 24F-V3SP, Fb = 2.4ksi, E = 1800ksi; UNO. SUPPLIER MAY SUPPLY AN ALTERNATE BEAM WITH EQUAL PROPERTIES OR MAY SUBMIT THEIR OWN SIZING CALCS.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED, FASTENED WITH 8d COMMON NAILS (.131), 6"OC PANEL EDGES, 12"OC INTERMEDIATE MEMBERS, GABLE ENDS AND DIAPHRAGM BOUNDARY; 4"OC, UNO.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 15" IN GROUTED CMU.

WASHERS: WASHERS USED WITH 1/2" BOLTS TO BE 2" x 2" x 9/64"; WITH 5/8" BOLTS TO BE 3" x 3" x 9/64"; WITH 3/4" BOLTS TO BE 3" x 3" x 9/64"; WITH 7/8" BOLTS TO BE 3" x 3" x 5/16"; UNO.

NAILS: ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE SPECIFIED OR ACCEPTED BY FBC TEST REPORTS AS HAVING EQUAL STRUCTURAL VALUES.

BUILDER'S RESPONSIBILITY

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR 2004 REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMITS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS

ROOF SYSTEM DESIGN

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR 2004, SECTION R301.2.1 IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR 2004 REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN DATA

5/8" AB

1/2" AB

2-5/8" AB

5/8" AB

WIND LOADS PER FLORIDA BUILDING CODE 2004 RESIDENTIAL, SECTION R301.2.1 (ENCLOSED SIMPLE DIAPHRAGM BUILDINGS WITH FLAT, HIPPED, OR GABLE ROOFS) MEAN ROOF HEIGHT NOT EXCEEDING LEAST HORIZONTAL DIMENSION OR 60 FT; NOT ON UPPER HALF OF HILL OR ESCARPMENT 60FT IN EXP. B, 30FT IN EXP. C AND >10% SLOPE AND UNOBSTRUCTED UPWIND FOR 50x HEIGHT OR 1 MILE WHICHEVER IS LESS.

BUILDING IS NOT IN THE HIGH VELOCITY HURRICANE ZONE BUILDING IS NOT IN THE WIND-BORNE DEBRIS REGION

1.) BASIC WIND SPEED = 110 MPH

2.) WIND EXPOSURE = B

3.) WIND IMPORTANCE FACTOR = 1.0

4.) BUILDING CATEGORY = II

5.) ROOF ANGLE = 10-45 DEGREES

6.) MEAN ROOF HEIGHT = <30 FT

7.) INTERNAL PRESSURE COEFFICIENT = N/A (ENCLOSED BUILDING)

8.) COMPONENTS AND CLADDING DESIGN WIND PRESSURES (TABLE R301.2(2))

Zone	Zone Effective Wind Area (ft2)			
	10		100	
1	19.9	-21.8	18.1	-18.1
2	19.9	-25.5	18.1	-21.8
2 O'hg		-40.6		-40.6
3	19.9	-25.5	18.1	-21.8
3 O'hg		-68.3		-42.4
4	21.8	-23.6	18.5	-20.4
5	21.8	-29.1	18.5	-22.6
Doors & Windows Worst Case			21.8	-29.1
(Zone 5, 10 ft2) 8x7 Garage Door 16x7 Garage Door				
			19.5	-22.9
			18.5	-21.0

DESIGN LOADS

FLOOR 40 PSF (ALL OTHER DWELLING ROOMS)

30 PSF (SLEEPING ROOMS)

30 PSF (ATTICS WITH STORAGE)

10 PSF (ATTICS WITHOUT STORAGE, <3:12) | ROOF 20 PSF (FLAT OR <4:12)

16 PSF (4:12 TO <12:12) 12 PSF (12:12 AND GREATER)

NOT IN FLOOD ZONE (BUILDER TO VERIFY

STAIRS 40 PSF (ONE & TWO FAMILY DWELLINGS) SOIL BEARING CAPACITY 1000PSF

FINALS DATE: 13 / Jan / 08

DRAWN BY:

David Disosway

JOB NUMBER: 801111 DRAWING NUMBER

OF 3 SHEETS

