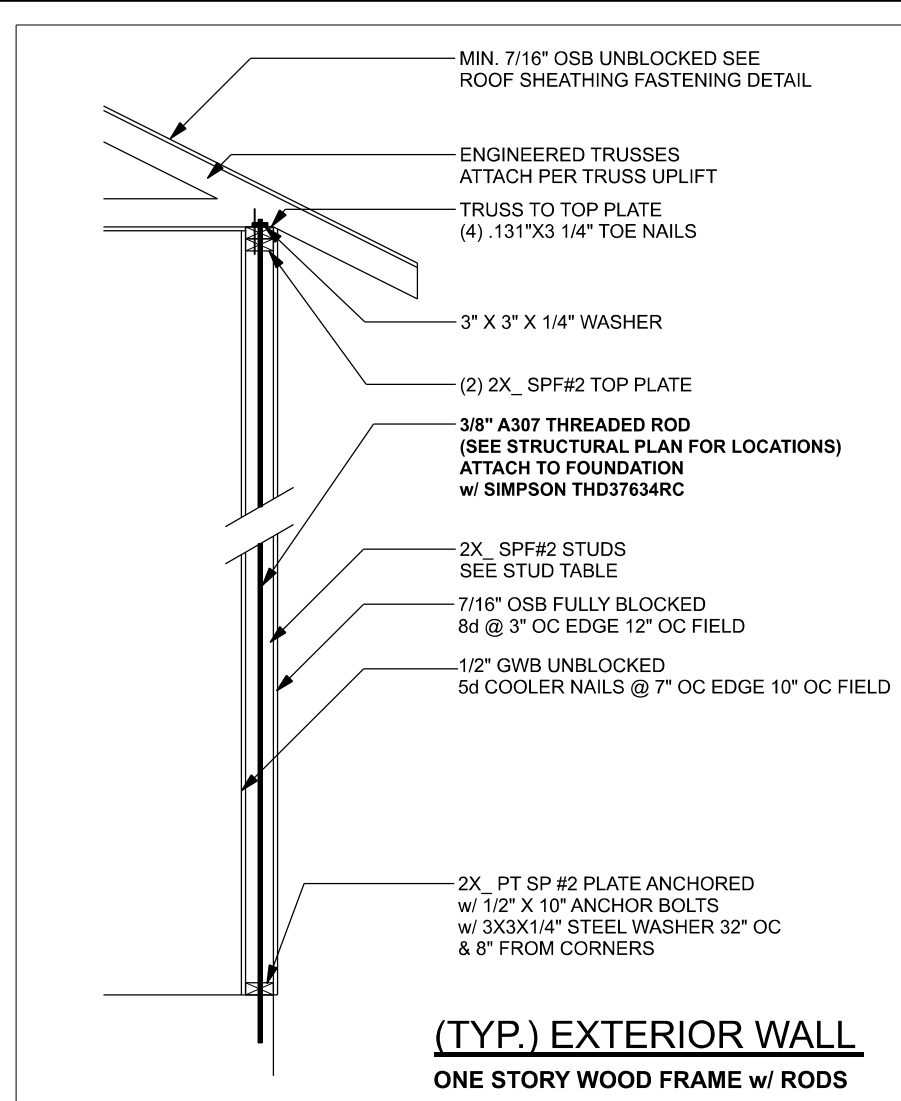
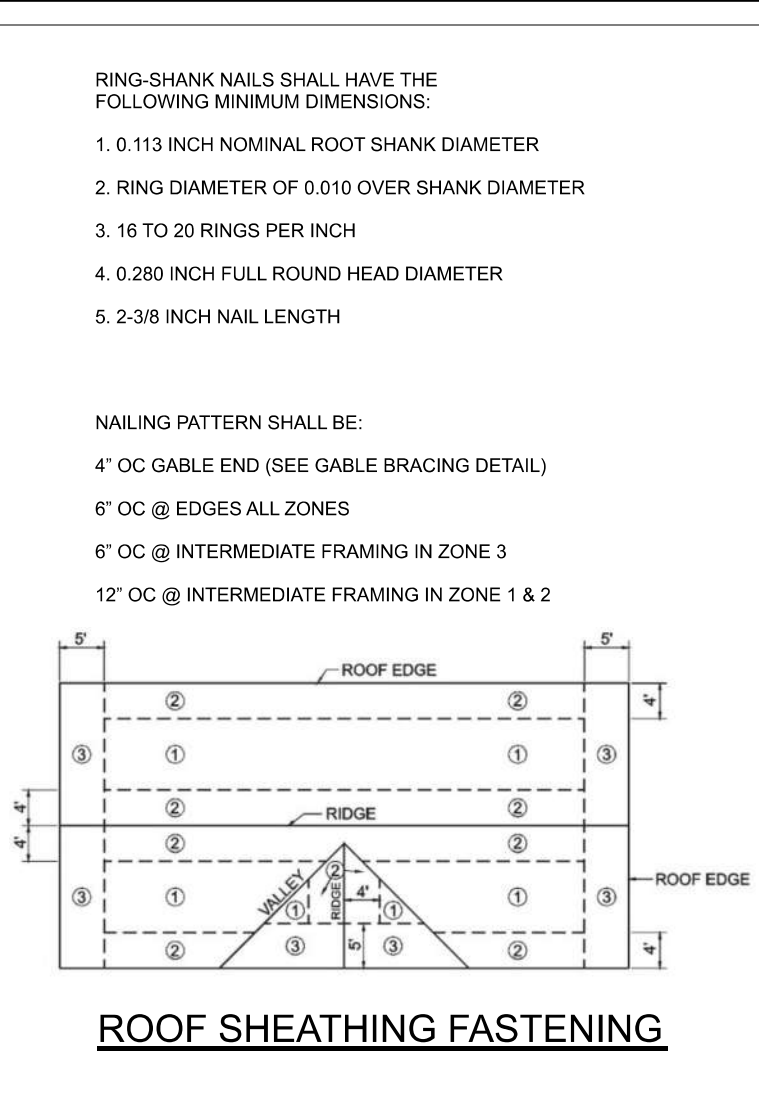
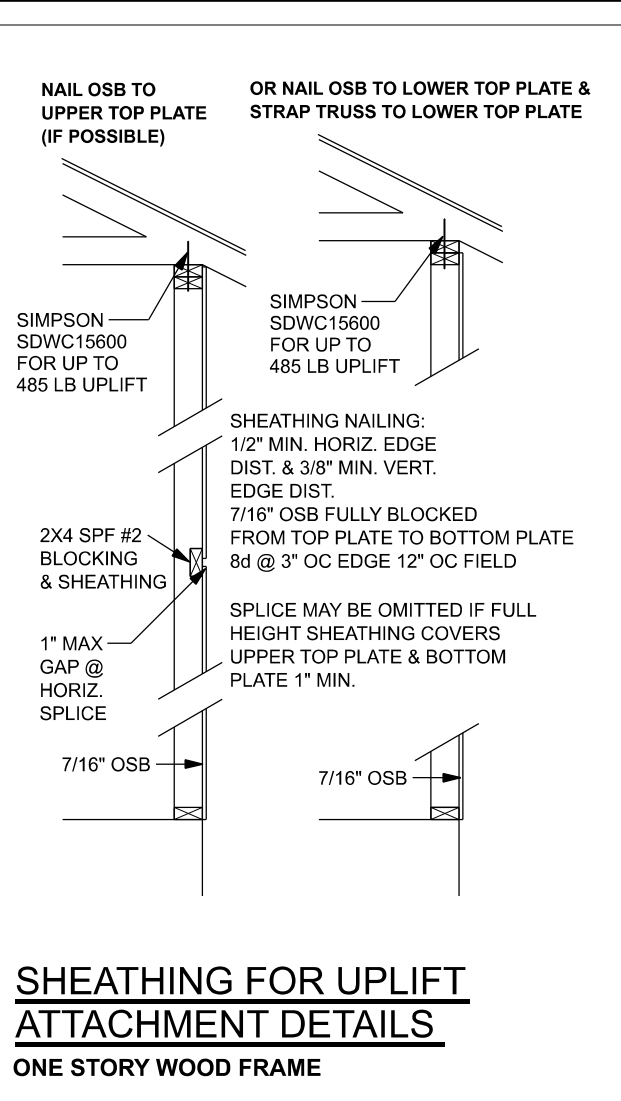




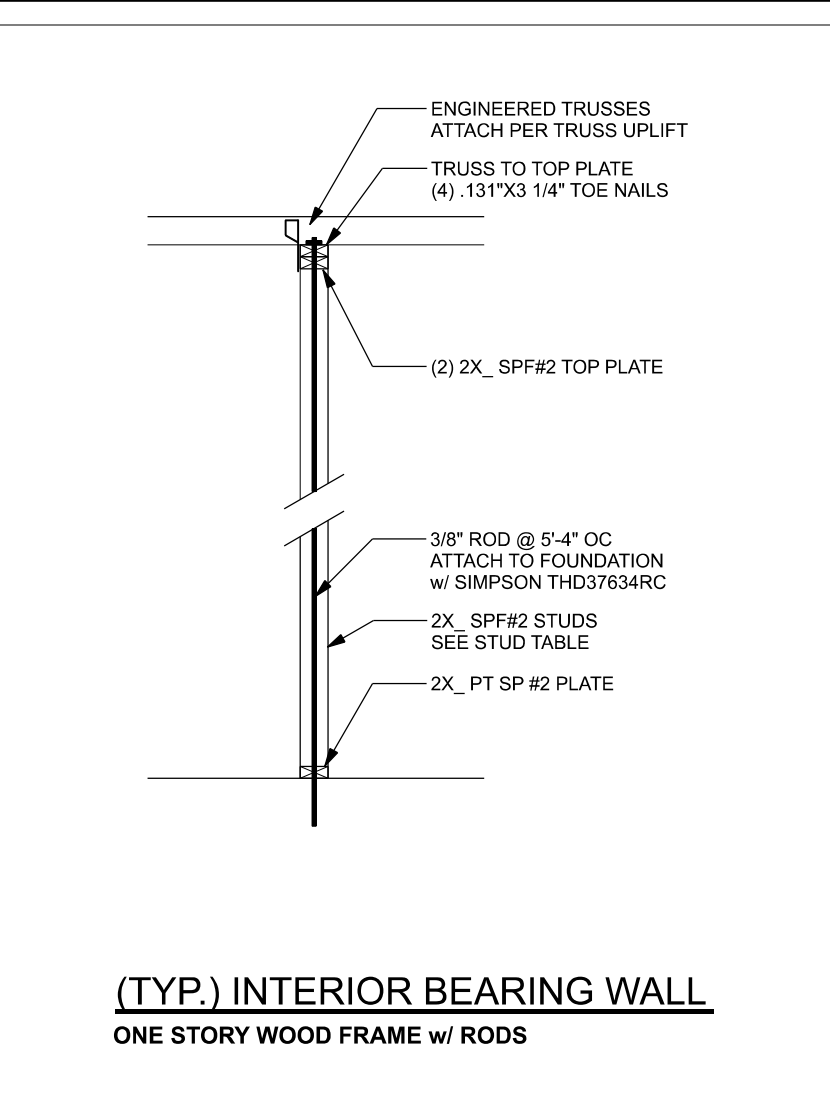
(TYP.) EXTERIOR WALL
ONE STORY WOOD FRAME w/ RODS



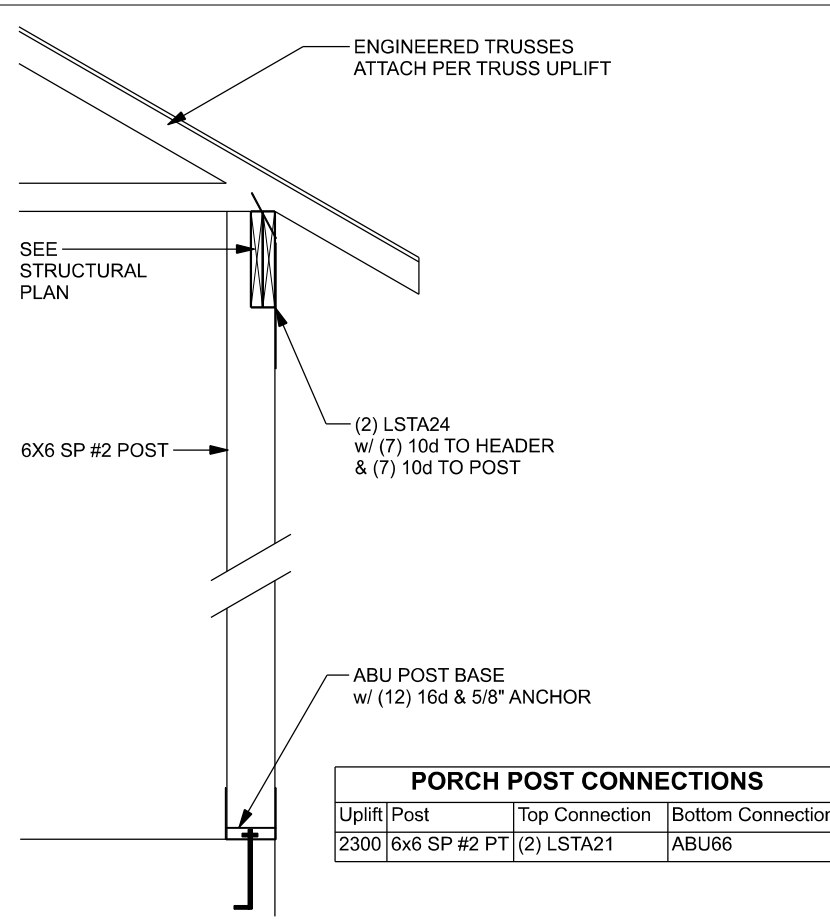
ROOF SHEATHING FASTENING



SHEATHING FOR UPLIFT
ATTACHMENT DETAILS
ONE STORY WOOD FRAME



(TYP.) INTERIOR BEARING WALL
ONE STORY WOOD FRAME w/ RODS



(TYP.) PORCH POST
ONE STORY WOOD

CONNECTOR TABLE					
Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter	
615	485	SDWC15600	4-8d x 1 1/2"	4-8d x 1 1/2"	
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"	
575	485	H2.5A	5-8d x 1 1/2"	5-8d x 1 1/2"	
1340	1015	H16A	9-10d1 1/2"	9-10d1 1/2"	
720	620	LTS12-20	6-10d1 1/2"	6-10d1 1/2"	
1000	860	MTS12-30	7-10d1 1/2"	7-10d1 1/2"	
1450	1245	HTS20-30	12-10d1 1/2"	12-10d1 1/2"	
Uplift SP	Uplift SPF	Strap Ties	To One Member	To Other Member	
1235	1235	LSTA1	9-10d	9-10d	
1640	1455	MSTA24	9-10d	9-10d	
1030	1030	CS20	7-10d	7-10d	
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate	
585	535	SP1	6-10d	4-10d	
1065	865	SP2	6-10d	6-10d	
771	771	LSTA24	10-10d	wrap under or over plate	
1235	1235	LSTA24	14-10d	wrap under or over plate	
Uplift SP	Uplift SPF	Holdowns @ Stenwall	To Stud / Post	Anchor	
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD	
4235	3640	HTT4	18-16d x 1 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor	
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x6" Titen HD	
4235	3640	HTT4	18-16d x 1 1/2"	1/2"x12" Titen HD	
Uplift SP	Uplift SPF	Post Bases @ Stenwall	To Post	Anchor	
2200		ABU44	12-16d	5/8"x12" Drill & Epoxy	
2300		ABU66	12-16d	5/8"x12" Drill & Epoxy	
Uplift SP	Uplift SPF	Post Bases @ Mono	To Post	Anchor	
2200		ABU44	12-16d	5/8"x7" Drill & Epoxy	
2300		ABU66	12-16d	5/8"x7" Drill & Epoxy	

EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:

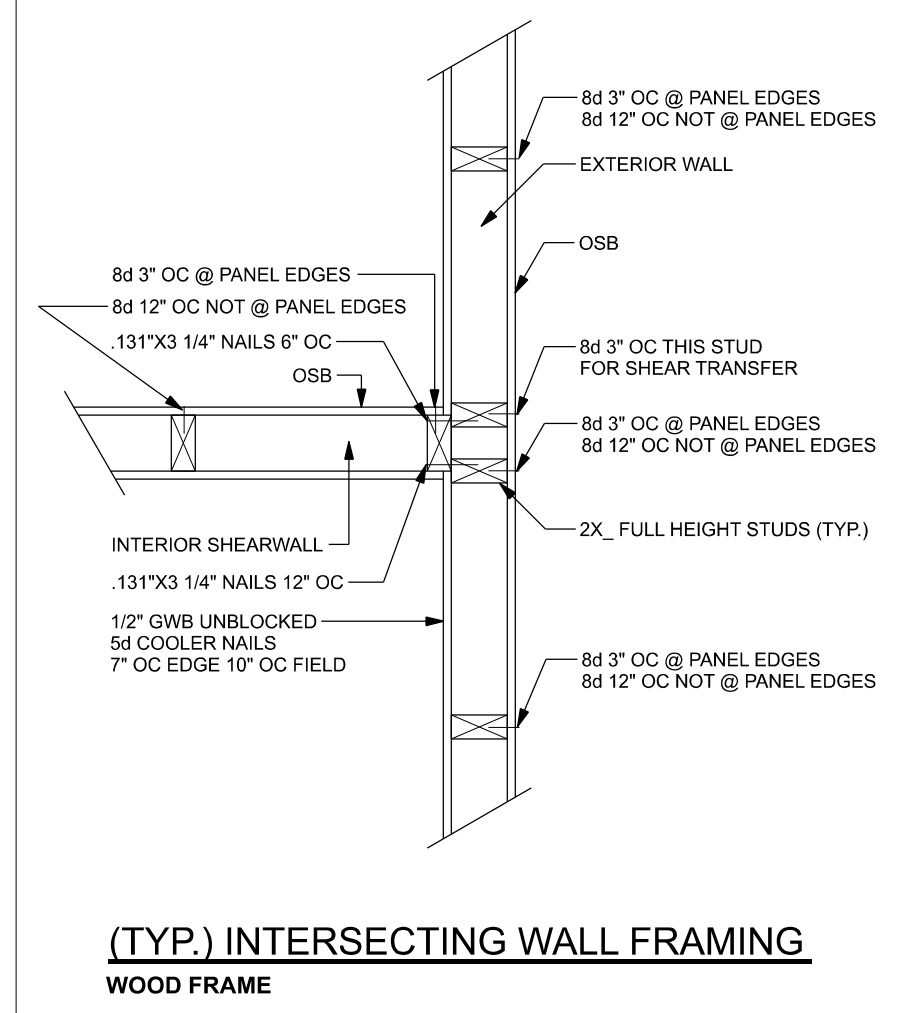
THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

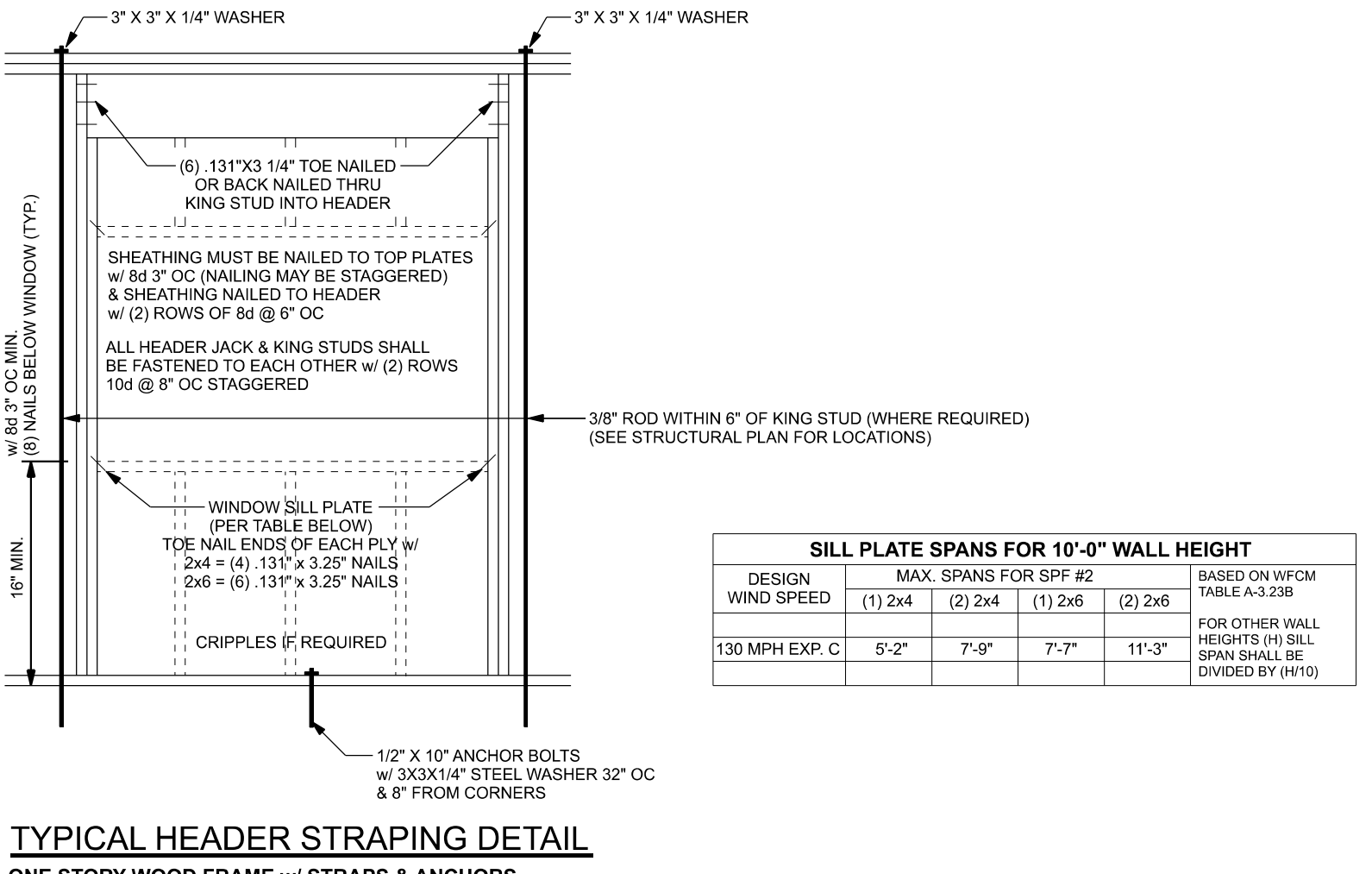
		Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

Uplift/Post	Top Connection	Bottom Connection
2300 6x6 SP #2 PT	(2) LSTA21	ABU66

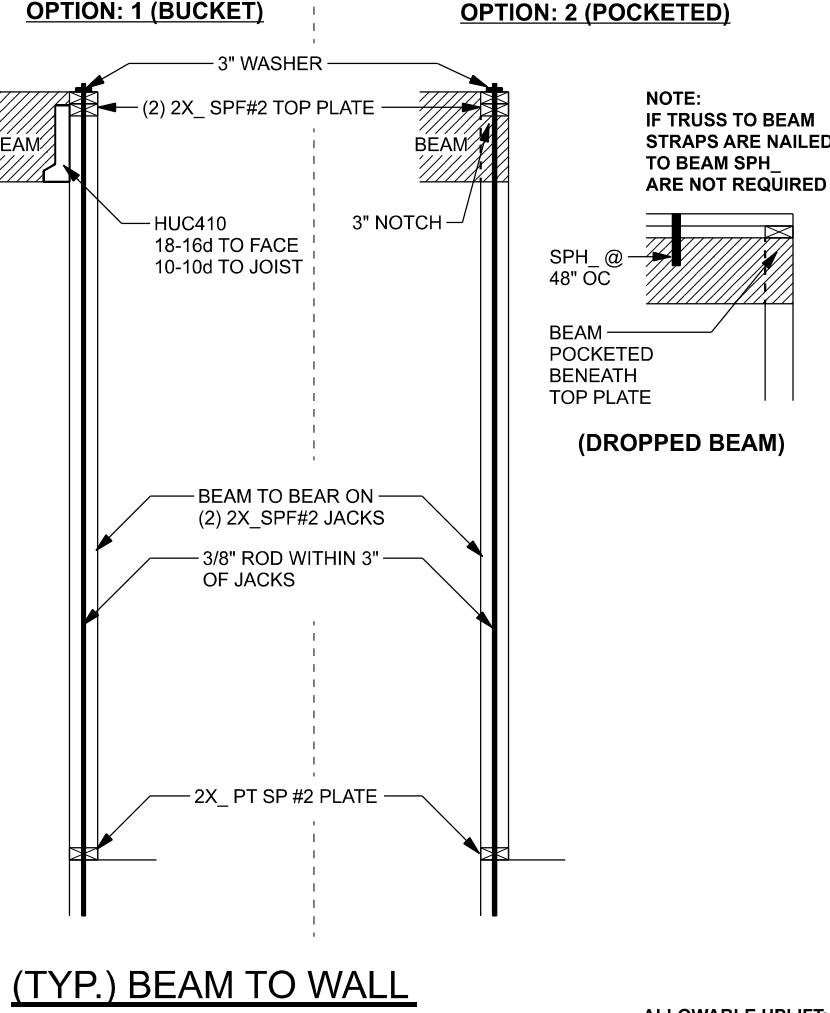
(TYP.) PORCH POST
ONE STORY WOOD



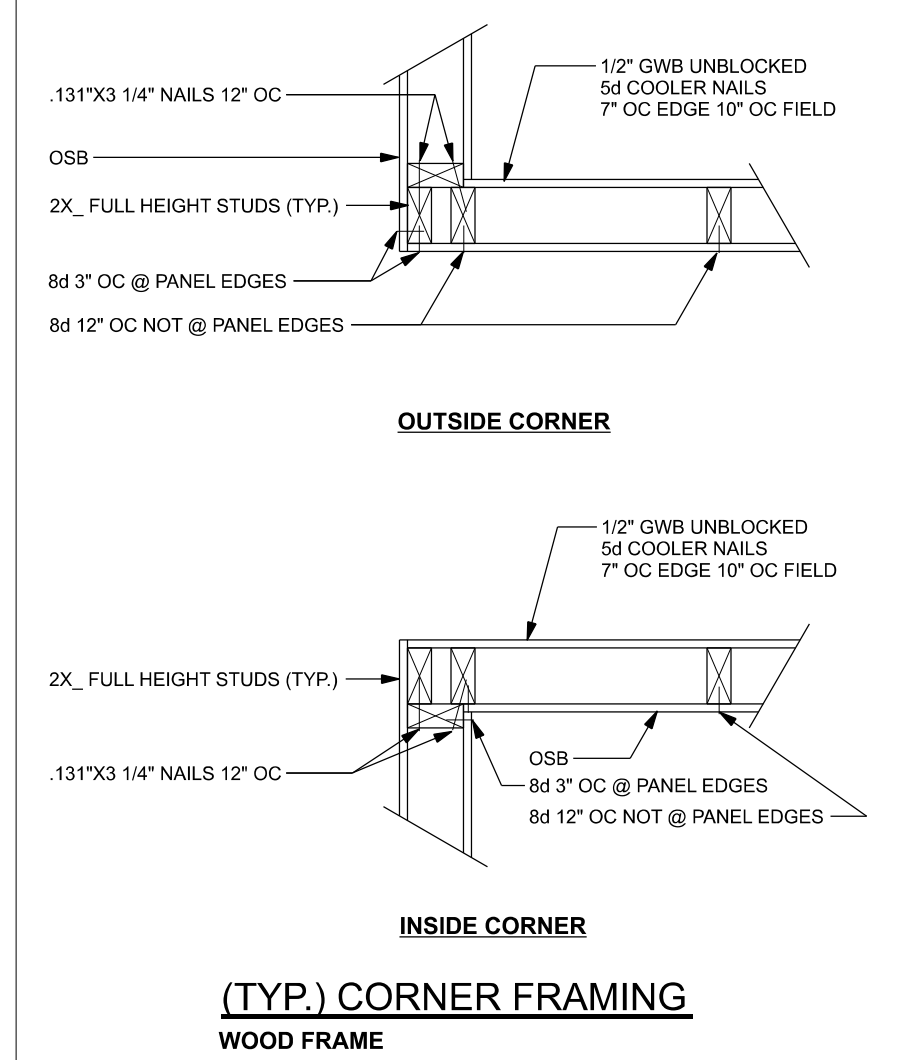
(TYP.) INTERSECTING WALL FRAMING
WOOD FRAME



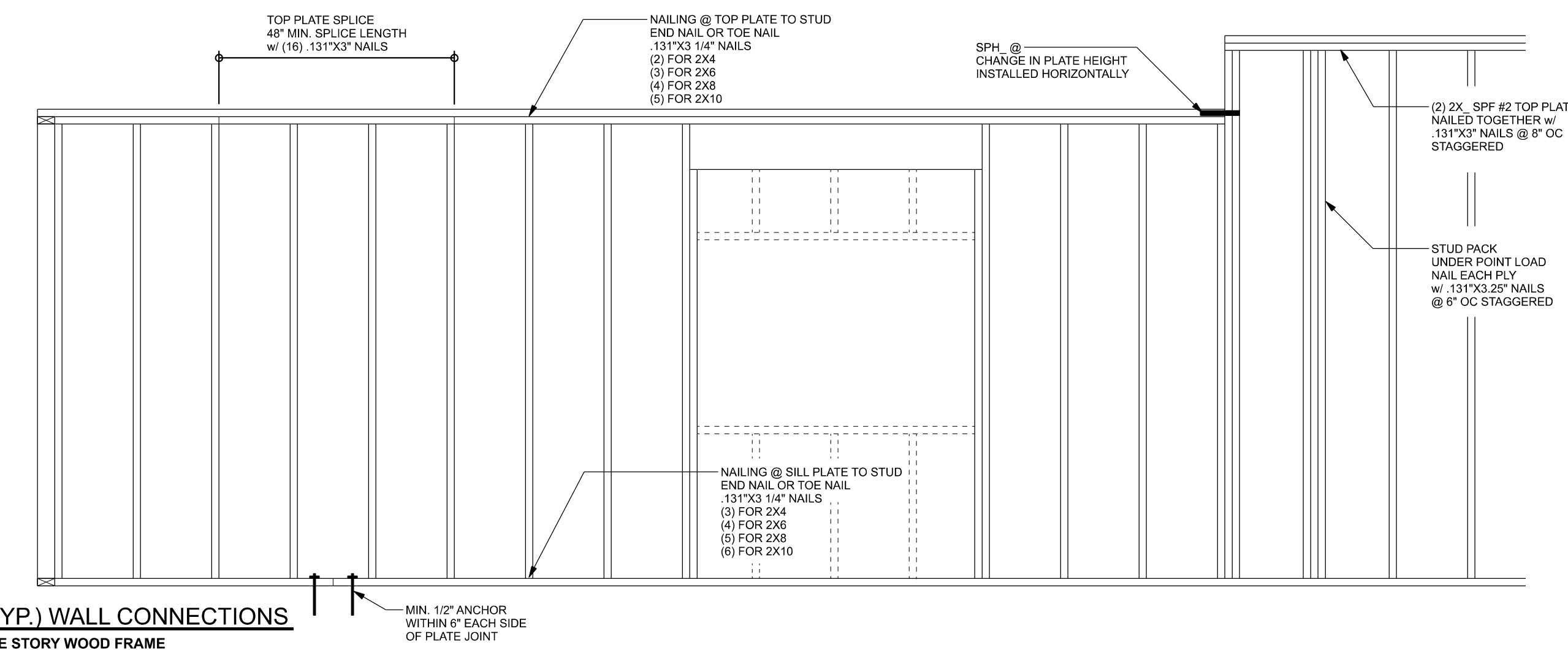
TYPICAL HEADER STRAPPING DETAIL
ONE STORY WOOD FRAME w/ STRAPS & ANCHORS



(TYP.) BEAM TO WALL
WOOD FRAME w/ RODS



(TYP.) CORNER FRAMING
WOOD FRAME



(TYP.) WALL CONNECTIONS
ONE STORY WOOD FRAME

GENERAL NOTES:

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBCR. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 415LB EACH END, 2X6 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, $F_c = 2500$ PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1 x W1.4, FB = 89KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB; SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACINGS NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. PLACEMENT: THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 12FT. DO NOT CUT WWM OR REINFORCING STEEL (RECOMMENDED) LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A 615, GRADE 40, DEFORMED BARS, $F_y = 40$ KSI, ALL LAP SPICES 40" DB (25" FOR #5 BARS); UNO. ALL REINFORCEMENT SHALL BE DETAIL AND PLACED IN ACCORDANCE WITH ACI 318-08, UNLESS OTHERWISE NOTED.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL DIAPHRAGMS; 7/16" OSB SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GROUTED CMU.

BUILDER'S RESPONSIBILITY:

THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK.

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBCR REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

PROVIDE A CONTINUOUS LOAD PATH FROM TRUSSES TO FOUNDATION. IF YOU BELIEVE THE PLAN OMTS A CONTINUOUS LOAD PATH CONNECTION, CALL THE WIND LOAD ENGINEER IMMEDIATELY.

VERIFY THE TRUSS MANUFACTURER'S SEALED ENGINEERING INCLUDES TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS.

ROOF SYSTEM DESIGN:

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBCR, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBCR REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF SYSTEM AS A WHOLE AND TO PROVIDE RESTRAINT FOR ANY LATERAL BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER ALSO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

DESIGN CRITERIA & LOADS:		
BUILDING CODE	6TH EDITION FLORIDA BUILDING CODE RESIDENTIAL (2017)	
CODE FOR DESIGN LOADS	ASCE 7-10	
WINDLOADS		
BASIC WIND SPEED (ASCE 7-10, 3S GUST)	130 MPH	
WIND EXPOSURE (BUILDER MUST FIELD VERIFY)	C	
TOPOGRAPHIC FACTOR (BUILDER MUST FIELD VERIFY)	I	
RISK CATEGORY	II	
ENCLOSURE CLASSIFICATION	ENCLOSED	
INTERNAL PRESSURE COEFFICIENT	0.18	
ROOF ANGLE	7-45 DEGREES	
MEAN ROOF HEIGHT	30 FT	
C&C DESIGN PRESSURES		
SEE TABLE		
FLOOR LOADING		
ROOMS OTHER THAN SLEEPING ROOM	40 PSF LIVE LOAD	
SLEEPING ROOMS	30 PSF LIVE LOAD	
ROOF LOADING		
FLAT OR < 4:12	20 PSF LIVE LOAD	
4:12 TO < 12:12	16 PSF LIVE LOAD	
12:12 & GREATER	12 PSF LIVE LOAD	
SOIL BEARING CAPACITY	1500 PSF	
FLOOD ZONE	THIS BUILDING IS NOT IN THE FLOOD ZONE	

COMPONENT & CLADDING DESIGN PRESSURES 130 MPH (EXP C) (Vult)			
EFFECTIVE WIND AREA (F2)	ZONE 4 INTERIOR	ZONE 5 END 4' FROM ALL OUTSIDE CORNER	ZONE 5 END 4' FROM ALL OUTSIDE CORNER
0'-20'	+42.6	-46.2	+42.6
GARAGE DOOR DESIGN PRESSURES 130 MPH (EXP C) (ASD)			
6x7 GARAGE DOOR	+22.8	-25.5	
16x7 GARAGE DOOR	+21.7	-24.1	

Blake Construction
Josephine Abbate Res.
(Mother - In - Law suite Addition)

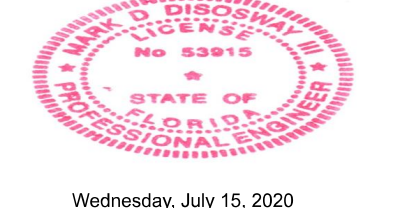
DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disoway, P.E. for resolution. Do not proceed without clarification.

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CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 6th Edition Florida Building Code Residential (2017) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOWAY P.E. 53915
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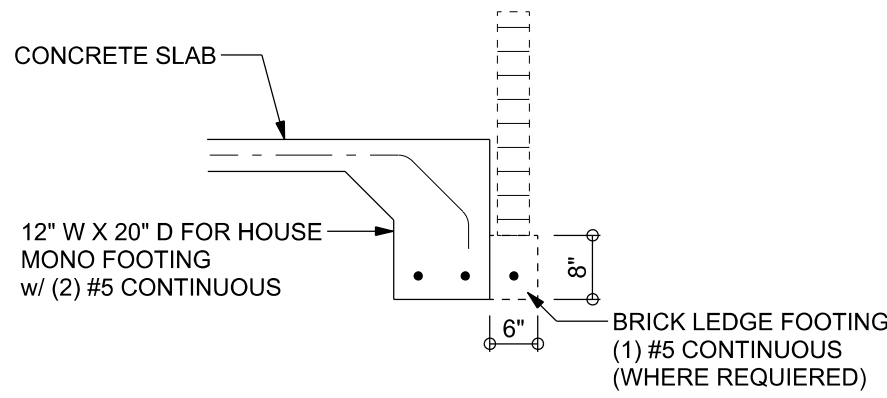


Wednesday, July 15, 2020

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disowaydesign@gmail.com

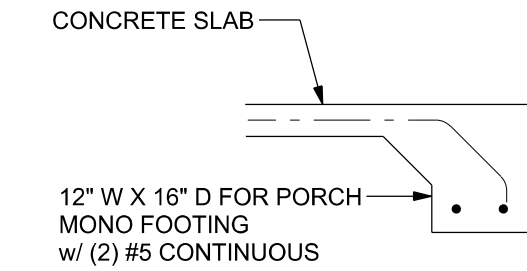
JOB NUMBER:
200118

S-1
OF 5 SHEETS

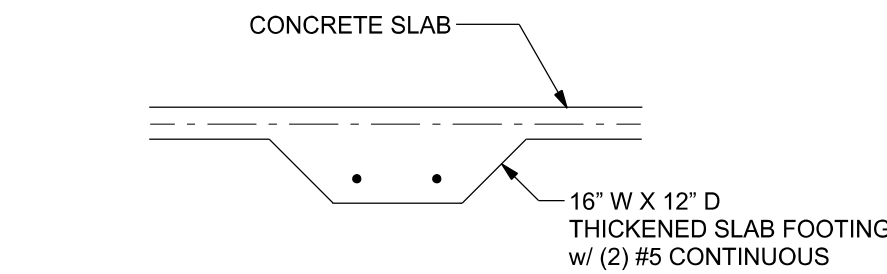


F1
S-2 **MONOLITHIC FOOTING @ HOUSE**
SCALE: 1/2" = 1'-0"

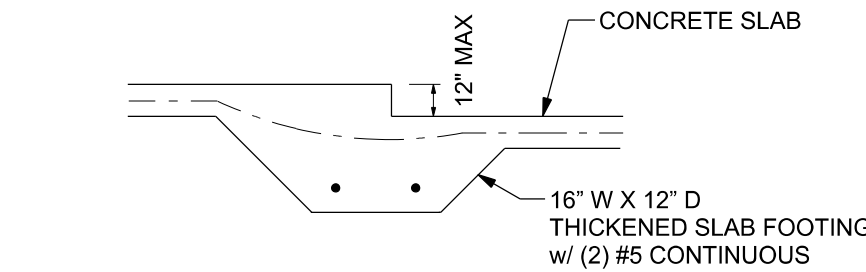
BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF
12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL
PER FBC 2017-RES. SECTION R403.1.4



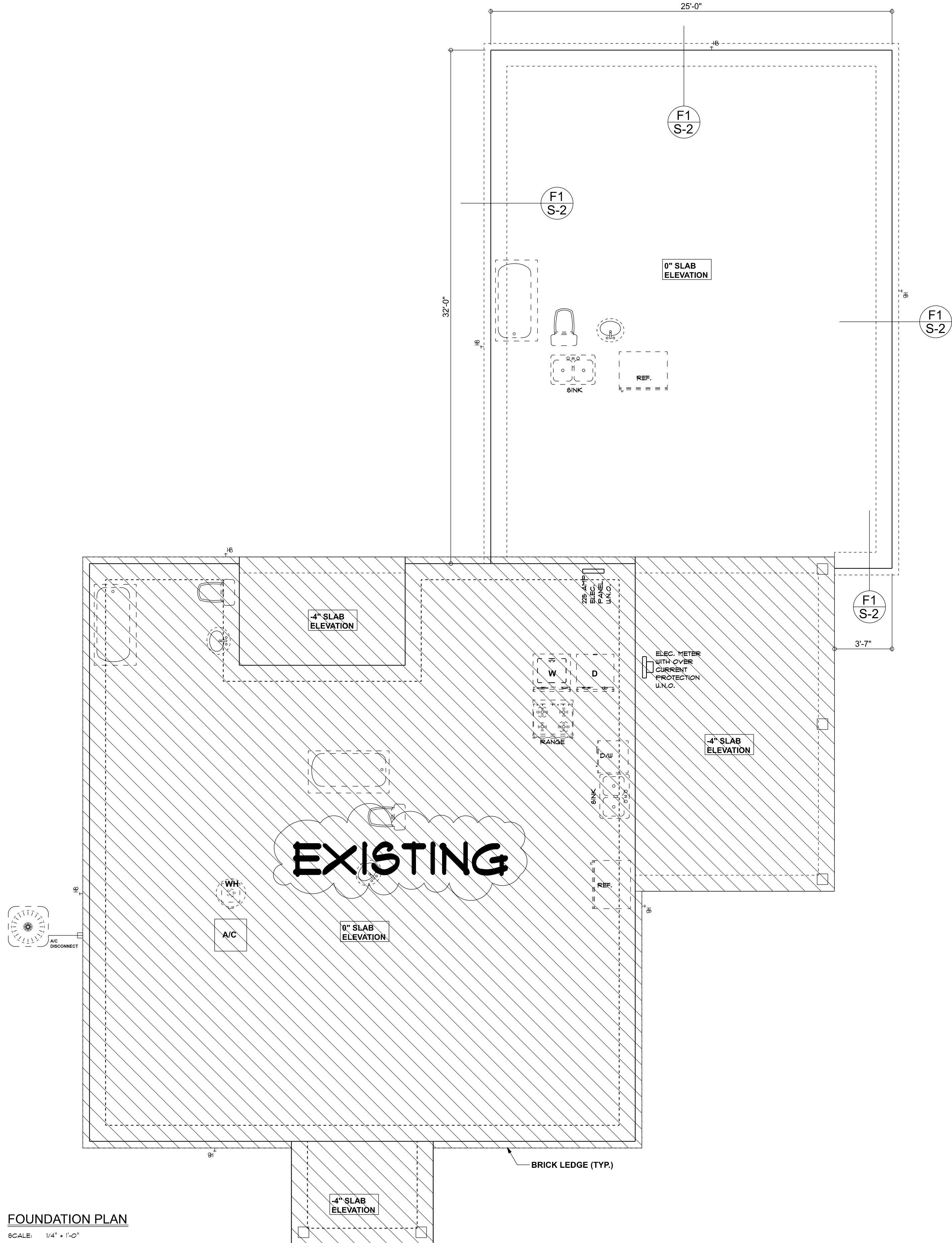
F2
S-2 **MONOLITHIC FOOTING @ PORCH**
SCALE: 1/2" = 1'-0"



F3
S-2 **INTERIOR BEARING FOOTING**
SCALE: 1/2" = 1'-0"



F4
S-2 **INTERIOR BEARING STEP FOOTING**
SCALE: 1/2" = 1'-0"



FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

FOUNDATION NOTES

- FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS. RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- FN - 2 CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
- FN - 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED W/ 6X8-14/14 WELDED WIRE MESH PLACED ON CHAIRS @ 11/2" DEPTH OR FIBER MESH CONCRETE, 5-MIL POLY VAPOR BARRIER W/ 8" LAPS SEALED W/ POLY TAPE OVER TERMITES TREATED & COMPACTED FILL (ALSO, ANY OTHER CODE APPROVED TERMITES TREATMENT METHOD CAN BE USED INSTEAD).

Blake Construction

Josephine Abbate Res.
(Mother - In - Law suite Addition)

PROJECT ADDRESS:
Lot 9 Southern Approaches
Columbia County, FL

DIMENSIONS:
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

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JOB NUMBER:
200118

S-2
OF 5 SHEETS

