

RE: 6241953 - 1820-B Brick

Site Information:

Lot/Block: 140 Address: SW Silver Palm Dr, . City: Lake City

MiTek, Inc. 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Customer Info: Adams Homes-Gainesville Project Name: The Preserve at Laurel Lake414911200del: 1820-B Brick Subdivision: The Preserve at Laurel Lake

State: fl

Name Address and License # of Structural Engineer of Record, If there is one, for the building. Name: License #: Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2023/TPI2014 Wind Code: ASCE 7-22 Roof Load: 40.0 psf

Design Program: MiTek 20/20 8.7 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 24 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	T34578771	A01	7/30/24	23	T34578793	G02	7/30/24
2	T34578772	A02	7/30/24	24	T34578794	H7V	7/30/24
3	T34578773	A03	7/30/24				
4	134578774	A04	7/30/24				
5	1345/8//5	A05	7/30/24				
6	134578776	A06	7/30/24				
(1345/8///	AU7	7/30/24				
8	1345/8//8	AU6	7/30/24				
9	134578779	AU9	7/30/24				
10	T24570701	A10 A11	7/30/24				
11	1343/0/01 T24570702		7/30/24				
12	T34578783		7/30/24				
1/	T3/57878/	C5V	7/30/24				
15	T34578785	E01	7/30/24				
16	T34578786	F01X	7/30/24				
17	T34578787	E02	7/30/24				
18	T34578788	Ē03	7/30/24				
19	T34578789	ĒŽĂ	7/30/24				
20	T34578790	E7V	7/30/24				
21	T34578791	G01	7/30/24				
22	T34578792	G01X	7/30/24				

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Tibbetts Lumber Co., LLC.

Truss Design Engineer's Name: Velez, Joaquin

My license renewal date for the state of Florida is February 28, 2025.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.





Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 30,2024



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oullapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality** Criteria and **DSE-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)



8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=128, 10=128.

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Scale = 1:71.8



3.00 12

1	8-4-0	12-4-12	17-0-0	21-8-0	26-3-4	30-4-0	38-8-0	1
1	8-4-0	4-0-12	4-7-5	4-8-0	4-7-5	4-0-12	8-4-0	
Plate Offsets (X,Y)	[5:0-2-8,0-3-0], [6:0-2-8,0-	2-4], [7:0-6-0,0-	-2-8], [8:0-2-8,0-3-0],	[13:0-6-0,0-3-4], [1	9:0-6-0,0-3-4]			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.25 1.25 YES	CSI. TC 0.65 BC 0.97 WB 0.75	DEFL. Vert(LL) Vert(CT) Horz(CT	in (loc) -0.44 15 -0.88 14-15 0.61 11	l/defl L/d >999 360 >522 240 n/a n/a	PLATES MT20 MT20HS	GRIP 244/190 187/143
BCDL 10.0	Code FBC2023/TF	12014	Matrix-S	Wind(LL)	0.24 15	>999 240	Weight: 219 lb	FT = 20%
LUMBER- TOP CHORD 2x4 S	P No.2			BRACING TOP CHC	i- RD Structu	Iral wood sheathing	directly applied or 2-2-0 o	c purlins.

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 2-2-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing, Except: 2-2-0 oc bracing: 18-19,13-14.

REACTIONS. (size) 2=0-4-0, 11=0-4-0 Max Horz 2=156(LC 11) Max Uplift 2=-128(LC 12), 11=-128(LC 12) Max Grav 2=1663(LC 1), 11=1663(LC 1)

2x4 SP No.2 *Except*

2x4 SP No.2

2-19,11-13: 2x4 SP M 31 or 2x4 SP SS

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD
 2-3=-5161/376, 3-4=-4987/288, 4-5=-3409/257, 5-6=-2512/235, 6-7=-2207/235, 7-8=-2510/239, 8-9=-3410/263, 9-10=-4986/300, 10-11=-5161/387

 BOT CHORD
 2-19=-280/4632, 18-19=-141/4348, 16-18=-62/2988, 15-16=0/2205, 14-15=-80/2989, 13-14=-163/4348, 11-13=-302/4632

 WERS
 4-18=-163/4348, 11-13=-302/4632
- WEBS 4-19=0/1239, 4-18=-1555/91, 5-18=0/868, 5-16=-1084/112, 6-16=-18/827, 7-15=-22/825, 8-15=-1087/115, 8-14=-1/870, 9-14=-1554/96, 9-13=0/1239

NOTES-

BOT CHORD

WEBS

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=5ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 17-0-0, Zone2 17-0-0 to 21-2-15, Zone1 21-2-15 to 21-8-0, Zone2 21-8-0 to 26-2-5, Zone1 26-2-5 to 40-8-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) All plates are MT20 plates unless otherwise indicated.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Bearing at joint(s) 2, 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=128, 11=128.



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a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall
building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/ITPH Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org)
and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbscscomponents.com)



1	8-4-0		1	23-8-0	1	30-4-0	38-8-0	1
ſ	8-4-0	6-8-0	T	8-8-0	T	6-8-0	8-4-0	
Plate Offsets (X,Y)	[2:0-0-10,Edge], [6:0-2-8,0)-2-4], [8:0-2-8,0-2-4	4], [12:0-0-10,Edge], [14:0-5-4,0-3-0], [18	8:0-5-4,0-3-0			
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2023/TF	2-0-0 1.25 1.25 YES Pl2014	CSI. TC 0.69 BC 0.72 WB 0.53 Matrix-S	DEFL. Vert(LL) - Vert(CT) - Horz(CT) Wind(LL)	in (loc) 0.54 15-17 1.01 15-17 0.63 12 0.23 14-15	l/defl L/4 >844 360 >456 240 n/a n/3 >999 240	d PLATES 0 MT20 0 MT20HS a 0 Weight: 202 lb	GRIP 244/190 187/143 FT = 20%
LUMBER- TOP CHORD 2x4 S 4-6,8- BOT CHORD 2x4 S WEBS 2x4 S	P No.2 *Except* 10: 2x4 SP M 31 or 2x4 SP P M 31 or 2x4 SP SS P No.2	SS		BRACING- TOP CHORD BOT CHORD WEBS	Structu Rigid ce 1 Row a	ral wood sheat eiling directly a at midpt	hing directly applied or 2-1-15 pplied or 10-0-0 oc bracing. 5-17, 9-15	oc purlins.

REACTIONS. (size) 2=0-4-0, 12=0-4-0 Max Horz 2=-140(LC 10) Max Uplift 2=-128(LC 12), 12=-128(LC 12) Max Grav 2=1808(LC 17), 12=1808(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-5688/359, 3-5=-5621/306, 5-6=-3182/221, 6-7=-2815/230, 7-8=-2777/234,

8-9=-3146/225. 9-11=-5514/319. 11-12=-5589/370

 BOT CHORD
 2-18-263/5232, 17-18-164/5000, 15-17=-40/2910, 14-15=-188/4803, 12-14=-287/5035

 WEBS
 5-18=0/1498, 5-17=-2280/174, 6-17=0/1095, 7-17=-309/56, 7-15=-309/56, 8-15=0/1108, 9-15=-2222/181, 9-14=0/1451

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=5ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 15-0-0, Zone2 15-0-0 to 19-4-0, Zone1 19-4-0 to 23-8-0, Zone2 23-8-0 to 27-10-15, Zone1 27-10-15 to 40-8-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Provide adequate drainage to prevent water ponding.

5) All plates are MT20 plates unless otherwise indicated.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) Bearing at joint(s) 2, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=128, 12=128.



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and BCSI Building Component Safety Information
available from the Structural Building Component Association (www.sbcscomponents.com)





L	<u>8-4-0 13-0-0</u> 8-4-0 4-8-0	19-4-0	<u>25-8-0</u> 6-4-0	<u>30-4-0</u> <u>4-8-0</u>	<u>38-8-0</u> 8-4-0	
Plate Offsets (X,Y)	[6:0-6-0,0-2-8], [8:0-6-0,0-2-8], [14:0-6-	0,0-3-4], [16:0-4-0,0-3-0], [[18:0-6-0,0-3-4]	4-0-0	0.4.0	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2023/TPI2014	CSI. TC 0.73 BC 1.00 WB 0.77 Matrix-S	DEFL. in (loc Vert(LL) -0.47 1 Vert(CT) -0.96 16-1' Horz(CT) 0.64 1 Wind(LL) 0.26 1	:) l/defl L/d 6 >970 360 7 >481 240 2 n/a n/a 6 >999 240	PLATES MT20 MT20HS Weight: 201 lb	GRIP 244/190 187/143 FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF 2×18,12 WEBS 2x4 SF	' No.2 ' No.2 *Except* 2-14: 2x4 SP M 31 or 2x4 SP SS ' No.2		BRACING- TOP CHORD Struc BOT CHORD Rigid	tural wood sheathing dire ceiling directly applied o	ectly applied or 2-2-0 of r 2-2-0 oc bracing.	; purlins.
REACTIONS. (size Max H Max U Max G	 2=0-4-0, 12=0-4-0 orz 2=124(LC 11) plift 2=-128(LC 12), 12=-128(LC 12) irav 2=1663(LC 1), 12=1663(LC 1) 					
FORCES. (ib) Hax. TOP CHORD 2-3=- 8-9=- BOT CHORD 2-18- 12-14 WEBS 5-18- 8-15	Comp./Max. Ten All forces 250 (lb) o 5157/374, 3-5=-4992/298, 5-6=-3252/2 3252/245, 9-11=-4992/310, 11-12=-515 =-278/4628, 17-18=-153/4355, 16-17=-5 4=-300/4628 =0/1238, 5-17=-1672/133, 6-17=0/853, 1 =0/853, 9-15=-1672/139, 9-14=0/1238	less except when shown. 39, 6-7=-3259/268, 7-8=-3 6/386 8/2880, 15-16=-50/2880, 3-16=-46/617, 7-16=-426/1	259/268, 14-15=-175/4355, 25, 8-16=-46/617,			
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-22; V II; Exp B; Encl., GCp Zone1 17-2-15 to 25 and forces & MWFR 3) Building Designer / I to the use of this true 4) Provide adequate di 5) All plates are MT20 0) This true here here	 loads have been considered for this de 'ult=130mph (3-second gust) Vasd=101 >i=0.18; MWFRS (directional) and C-C 2 i=8-0, Zone2 25-8-0 to 30-2-4, Zone1 30 S for reactions shown; Lumber DOL=1. Project engineer responsible for verifyin ss component. 'ainage to prevent water ponding. plates unless otherwise indicated. 	asign. mph; TCDL=4.2psf; BCDL Zone3 -2-0-0 to 1-0-0, Zon -2-4 to 40-8-0 zone; cantil 60 plate grip DOL=1.60 g applied roof live load sho	=6.0psf; h=15ft; B=45ft; L=24ft e1 1-0-0 to 13-0-0, Zone2 13-0 ever left and right exposed ;C-1 own covers rain loading require	t; eave=5ft; Cat. I-0 to 17-2-15, C for members Iments specific	No 68	VELEX VSE 182

- is truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Bearing at joint(s) 2, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=128, 12=128.



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	8-4-0 11-0-0 8-4-0 2-8-0		19-4-0	27	7-8-0	30-4-0	38-8-0	
Plate Offsets (X,Y)	[5:0-2-8,0-2-4], [7:0-2-8,0	-3-0], [8:0-2-8,0	-2-4], [13:0-5-12,0-3-0], [15:0-3-0,Edge], [17	:0-5-12,0-3-0]	2-0-0	0-4-0	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2023/T	2-0-0 1.25 1.25 YES PI2014	CSI. TC 0.65 BC 0.61 WB 0.66 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.51 15 -1.05 15-16 0.63 11 0.28 15	l/defl L/d >895 360 >437 240 n/a n/a >999 240	PLATES MT20 MT20HS Weight: 195 lb	GRIP 244/190 187/143 FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF	P No.2 P M 31 or 2x4 SP SS P No.2			BRACING- TOP CHORD BOT CHORD	Structur Rigid ce	al wood sheathing iling directly applied	directly applied or 2-2-0 or d or 10-0-0 oc bracing.	e purlins.
REACTIONS. (size Max H Max U Max G	e) 2=0-4-0, 11=0-4-0 lorz 2=107(LC 11) lplift 2=-128(LC 12), 11=- irav 2=1663(LC 1), 11=16	128(LC 12) 663(LC 1)						
FORCES. (lb) - Max. TOP CHORD 2-3=- 7-8=-	Comp./Max. Ten All for 5163/376, 3-4=-4978/284 3379/243, 8-9=-3747/256	ces 250 (lb) or l 4, 4-5=-3747/248 5, 9-10=-4978/29	ess except when shown. 3, 5-6=-3380/236, 6-7=-4 96, 10-11=-5163/388	100/266,				
BOT CHORD 2-17= 11-1 WEBS 4-17=	=-279/4634, 16-17=-134/4 3=-303/4634 =-7/1206, 4-16=-1341/107	340, 15-16=-14 7, 5-16=-46/1432	0/4033, 14-15=-139/403 2, 6-16=-892/94, 7-14=-8	3, 13-14=-158/4340 92/94,	,			
 8-14: NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-22; V II; Exp B; Encl., GCr Zone1 15-2-15 to 27 members and forces 3) Building Designer / I to the use of this tru: 4) Provide adequate di 5) All plates are MT20 6) This truss has been 7) * This truss has been will fit between the b 8) Bearing at joint(s) 2, capacity of bearing a 9) Provide mechanical 2=128, 11=128. 	=-50/1432, 9-14=-1341/11 e loads have been consid- /ult=130mph (3-second gr pi=0.18; MWFRS (direction 7-8-0, Zone2 27-8-0 to 37 & MWFRS for reactions Project engineer responsi ss component. rainage to prevent water p plates unless otherwise in designed for a 10.0 psf b n designed for a 10.0 psf b n designed for a live load bottom chord and any other .11 considers parallel to g surface. connection (by others) of	2, 9-13=-13/120 ered for this des ust) Vasd=101m nal) and C-C Zc 10-15, Zone13 shown; Lumber ble for verifying vonding. ndicated. ottom chord live of 20.0psf on the er members. grain value using truss to bearing	ign. iph; TCDL=4.2psf; BCDL ne3 -2-0-0 to 1-0-0, Zom: 1-10-15 to 40-8-0 zone; (DOL=1.60 plate grip DC applied roof live load sho load nonconcurrent with te bottom chord in all area g ANSI/TPI 1 angle to gra plate capable of withsta	=6.0psf; h=15ft; B= e1 1-0-0 to 11-0-0, 2 cantilever left and rig L=1.60 wwn covers rain load any other live loads as where a rectang ain formula. Building nding 100 lb uplift a	45ft; L=24ft; e Zone2 11-0-0 ght exposed ;(ding requireme s. e 3-6-0 tall by g designer sho t joint(s) excep	ave=5ft; Cat. to 15-2-15, C-C for ents specific 2-0-0 wide puld verify pt (jt=lb)	DB STACE	VELEN 182 OF



July 30,2024



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Scale = 1:70.6



L	8-4-0 1	-2-0 19-4-0	24-6-0	30-4-0	38-8-0
1	8-4-0 5	10-0 5-2-0	5-2-0	5-10-0	8-4-0
Plate Offsets (X,Y)	[4:0-2-8,0-2-4], [6:0-4-0,0-3-0], [8:	2-8,0-2-4], [12:0-5-12,0-3-0],	[16:0-5-12,0-3-0]		
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2023/TPI2014	CSI. TC 0.97 BC 0.60 WB 0.34 Matrix-S	DEFL. in Vert(LL) -0.66 Vert(CT) -1.32 Horz(CT) 0.71 Wind(LL) 0.37	(loc) I/defl L/d 14 >698 360 14 >349 240 10 n/a n/a 14 >999 240	PLATES GRIP MT20 244/190 MT20HS 187/143 Weight: 187 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF 4-6,6-8 BOT CHORD 2x4 SF WEBS 2x4 SF	P No.2 *Except* 3: 2x4 SP M 31 or 2x4 SP SS P M 31 or 2x4 SP SS P No.2		BRACING- TOP CHORD BOT CHORD	Structural wood sheathing d Rigid ceiling directly applied	irectly applied. or 10-0-0 oc bracing.
REACTIONS. (siz Max H Max U Max G	e) 2=0-4-0, 10=0-4-0 lorz 2=91(LC 11) plift 2=-128(LC 12), 10=-128(LC 1 rav 2=1663(LC 1), 10=1663(LC 1)			
FORCES. (lb) - Max. TOP CHORD 2-3=- 7-8=- 7-8=- BOT CHORD 2-16: 10-1 10-1 WEBS 4-16: 8-13 8-13	Comp./Max. Ten All forces 250 -5178/390, 3-4=-4945/261, 4-5=-52 -5329/337, 8-9=-4945/273, 9-10=-5 -294/4649, 15-16=-109/4048, 14- 2=-317/4649 =0/1306, 4-15=-97/1527, 5-15=-32 =-97/1527, 8-12=0/1306	o) or less except when shown 9/339, 5-6=-5329/339, 6-7=-{ 78/402 5=-218/5733, 13-14=-218/573 98, 6-15=-534/27, 6-13=-534	5329/337, 13, 12-13=-129/4048, /27, 7-13=-327/98,		
 NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-22; V II; Exp B; Encl., GC; Zone1 13-2-15 to 22 members and forces 3) Building Designer / I to the use of this tru 4) Provide adequate di 5) All plates are MT200 6) This truss has been 7) * This truss has been 7) * This truss has been 8) Bearing at joint(s) 2, capacity of bearing 1 9) Provide mechanical 2=128, 10=128. 	e loads have been considered for t /ult=130mph (3-second gust) Vaso bi=0.18; MWFRS (directional) and 0-8-0, Zone2 29-8-0 to 33-10-15, Z & MWFRS for reactions shown; L Project engineer responsible for ver- ss component. rainage to prevent water ponding. plates unless otherwise indicated. designed for a 10.0 psf bottom ch- n designed for a live load of 20.0p- bottom chord and any other member 10 considers parallel to grain valu- surface. connection (by others) of truss to l	s design. 101mph; TCDL=4.2psf; BCDI -C Zone3 -2-0-0 to 1-0-0, Zor ne1 33-10-15 to 40-8-0 zone; mber DOL=1.60 plate grip DC fying applied roof live load sh d live load nonconcurrent with on the bottom chord in all are s. using ANSI/TPI 1 angle to gr paring plate capable of withsta	L=6.0psf; h=15ft; B=45ft; L le1 1-0-0 to 9-0-0, Zone2 9 cantilever left and right ex DL=1.60 own covers rain loading re n any other live loads. eas where a rectangle 3-6- ain formula. Building desi- anding 100 lb uplift at joint	=24ft; eave=5ft; Cat. 9-0-0 to 13-2-15, posed ;C-C for equirements specific 0 tall by 2-0-0 wide gner should verify (s) except (jt=lb)	PD STOCK OF UNIT

Jonquin Velez PE No.68182 MiTek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 30,2024



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Continued on page 2

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Job	Truss	Truss Type	Qty	Ply	1820-B Brick	
6241953	A08	Hip Girder	1			T34578778
	100			2	Job Reference (optional)	
Tibbetts Lumber Co., LLC (C	Dcala, FL), Ocala, FL - 34	472,		8.730 s Ju	11 2024 MiTek Industries, Inc. Mon Jul 29 12:10:50 2024	Page 2

8.730 s Jul 11 2024 MiTek Industries, Inc. Mon Jul 29 12:10:50 2024 Page 2 ID:9677KBVwwjNKu0Wl9IYrcUzY81Q-tUJ9?XIc28uTks4MrS6gk8D_avvot3pk4cZaJFytCpZ

NOTES-

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 252 lb down and 172 lb up at 7-0-0, 122 lb down and 83 lb up at 9-0-12, 122 lb down and 83 lb up at 11-0-12, 127 lb down and 85 lb up at 13-0-12, 127 lb down and 85 lb up at 13-0-12, 127 lb down and 85 lb up at 19-0-12, 127 lb down and 85 lb up at 19-0-12, 127 lb down and 85 lb up at 19-7-4, 127 lb down and 85 lb up at 21-7-4, 127 lb down and 85 lb up at 23-7-4, 122 lb down and 85 lb up at 23-7-4, 122 lb down and 85 lb up at 23-7-4, 122 lb down and 85 lb up at 25-7-4, 122 lb down and 85 lb up at 29-7-4, and 252 lb down and 172 lb up at 31-8-0 on top chord, and 312 lb down at 7-3-0, 95 lb down at 9-0-12, 95 lb down at 13-0-12, 97 lb down at 15-0-12, 97 lb down at 19-0-12, 97 lb down at 19-7-4, 97 lb down at 23-7-4, 95 lb down at 25-7-4, 95 lb down at 27-7-4, and 95 lb down at 29-7-4, and 312 lb down at 31-5-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-60, 4-8=-60, 8-11=-60, 2-17=-20, 12-17=-20, 10-12=-20

Concentrated Loads (lb)

Vert: 4=-205(B) 8=-205(B) 17=-262(B) 12=-262(B) 6=-127(B) 14=-49(B) 7=-122(B) 13=-48(B) 18=-122(B) 20=-122(B) 21=-127(B) 22=-127(B) 23=-127(B) 24=-127(B) 25=-127(B) 25=-127(B) 26=-127(B) 27=-122(B) 29=-122(B) 30=-48(B) 31=-48(B) 32=-49(B) 33=-49(B) 35=-49(B) 35=-49(

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fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org)
and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcscomponents.com)





	8-4-0	11-0-0	19-4-0	27-8-0		30-4-0	38-8-0	—
Plate Offsets (X,Y)	[5:0-2-8,0-2-4], [7:0-2-8,0)-3-0], [8:0-2-8,0		15:0-3-0,Edge], [17:0-5-	12,0-3-0]	2-0-0	8-4-0	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2023/T	2-0-0 1.25 1.25 YES PI2014	CSI. TC 0.65 BC 0.61 WB 0.66 Matrix-S	DEFL. in Vert(LL) -0.51 Vert(CT) -1.05 Horz(CT) 0.63 Wind(LL) 0.28	n (loc) l/defl 15 >895 5 15-16 >437 3 11 n/a 3 15 >999	L/d 360 240 n/a 240	PLATES MT20 MT20HS Weight: 195 lb	GRIP 244/190 187/143 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP	No.2 M 31 or 2x4 SP SS No.2			BRACING- TOP CHORD BOT CHORD	Structural wood Rigid ceiling dir	I sheathing direc ectly applied or	tly applied or 2-2-0 oc 10-0-0 oc bracing.	; purlins.
REACTIONS. (size Max H Max U Max G	e) 2=0-4-0, 11=0-4-0 orz 2=107(LC 11) plift 2=-128(LC 12), 11=- rav 2=1663(LC 1), 11=1	128(LC 12) 663(LC 1)						
FORCES. (lb) - Max. TOP CHORD 2-3=- 7-8=- 7-8=- BOT CHORD 2-17= 11-13 4-17= 8-14= 8-14=	Comp./Max. Ten All fo 5163/376, 3-4=-4978/284 3380/243, 8-9=-3747/256 279/4634, 16-17=-134/4 3=-303/4634 7/1206, 4-16=-1341/107 50/1432, 9-14=-1341/12	rces 250 (lb) or l 4, 4-5=-3747/24 3, 9-10=-4978/2 4340, 15-16=-14 7, 5-16=-46/143 12, 9-13=-13/12	ess except when shown. 3, 5-6=-3380/236, 6-7=-4 96, 10-11=-5163/388 0/4033, 14-15=-139/4033 2, 6-16=-892/94, 7-14=-89 06	100/266, 3, 13-14=-158/4340, 92/94,				
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-22; V II; Exp B; Encl., GCp Zone1 15-2-15 to 27 members and forces 3) Building Designer / F to the use of this trus 4) Provide adequate dr 5) All plates are MT20 µ 6) This truss has been 7) * This truss has been will fit between the b 8) Bearing at joint(s) 2,	e loads have been consid fult=130mph (3-second g bi=0.18; MWFRS (directic -8-0, Zone2 27-8-0 to 31 s & MWFRS for reactions Project engineer responsi as component. ainage to prevent water p plates unless otherwise i designed for a 10.0 psf b n designed for a live load ottom chord and any othe 11 considers parallel to g	ered for this des ust) Vasd=101rr nal) and C-C Zo 10-15, Zone1 3 shown; Lumber ble for verifying ponding. ndicated. ottom chord live of 20.0psf on th er members. grain value using	ign. ph; TCDL=4.2psf; BCDL: one3 -2-0-0 to 1-0-0, Zone 1-10-15 to 40-8-0 zone; c DOL=1.60 plate grip DO applied roof live load sho cload nonconcurrent with the bottom chord in all area g ANSI/TPI 1 angle to gra	=6.0psf; h=15ft; B=45ft; a1 1-0-0 to 11-0-0, Zone antilever left and right e L=1.60 wn covers rain loading i any other live loads. as where a rectangle 3-6 in formula. Building des	L=24ft; eave=5ft 2 11-0-0 to 15-2 xposed ;C-C for equirements spe 3-0 tall by 2-0-0 v signer should ver	; Cat. 15, cific vide	No 68	VELEX 182 OF

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=128, 11=128.



Date:

July 30,2024



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Scale = 1:70.6



	8-4-0 14-2-0 8-4-0 5-10-0	19-4-0 5-2-0	24-6-0	30-4-0		38-8-0
Plate Offsets (X,Y)	[4:0-2-8,0-2-4], [6:0-4-0,0-3-0], [8:0-2-8,	0-2-4], [12:0-5-12,0-3-0], [[16:0-5-12,0-3-0]	0100		040
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2023/TPI2014	CSI. TC 0.97 BC 0.60 WB 0.34 Matrix-S	DEFL. in Vert(LL) -0.66 Vert(CT) -1.32 Horz(CT) 0.7' Wind(LL) 0.33	n (loc) l/defl 6 14 >698 2 14 >349 1 10 n/a 7 14 >999	L/d 360 240 n/a 240	PLATES GRIP MT20 244/190 MT20HS 187/143 Weight: 187 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF 4-6,6-8 BOT CHORD 2x4 SF WEBS 2x4 SF	P No.2 *Except* 8: 2x4 SP M 31 or 2x4 SP SS P M 31 or 2x4 SP SS P No.2		BRACING- TOP CHORD BOT CHORD	Structural wood sh Rigid ceiling direct	eathing directl	ly applied. 0-0-0 oc bracing.
REACTIONS. (siz Max H Max U Max G	e) 2=0-4-0, 10=0-4-0 lorz 2=91(LC 11) plift 2=-128(LC 12), 10=-128(LC 12) rav 2=1663(LC 1), 10=1663(LC 1)					
FORCES. (b) Hax. TOP CHORD 2-3= 7-8= 2-16: BOT CHORD 2-16: 10-1 10-1 WEBS 4-16: 8-13	Comp./Max. Ten All forces 250 (lb) or 5178/390, 3-4=-4945/261, 4-5=-5329/33 5329/337, 8-9=-4945/273, 9-10=-5177// =-294/4649, 15-16=-109/4048, 14-15=-2 2=-317/4649 =0/1306, 4-15=-97/1527, 5-15=-327/98, =-97/1527, 8-12=0/1306	less except when shown. 19, 5-6=-5329/339, 6-7=-5 102 18/5733, 13-14=-218/573 6-15=-534/27, 6-13=-534/	329/337, 3, 12-13=-129/4048, 27, 7-13=-327/98,			
NOTES- 1) Unbalanced roof live 2) Wind: ASCE 7-22; V II; Exp B; Encl., GC; Zone1 13-2-15 to 25 members and forces 3) Building Designer // to the use of this tru 4) Provide adequate di 5) All plates are MT20 6) This truss has been will fit between the b 8) Bearing at joint(s) 2, capacity of bearing 3 9) Provide mechanical 2=128, 10=128.	a loads have been considered for this de /ult=130mph (3-second gust) Vasd=101/ bi=0.18; MWFRS (directional) and C-C 2)-8-0, Zone2 29-8-0 to 33-10-15, Zone1 s & MWFRS for reactions shown; Lumbe Project engineer responsible for verifying ss component. rainage to prevent water ponding. plates unless otherwise indicated. designed for a 10.0 psf bottom chord liv n designed for a 10.0 psf bottom chord liv n designed for a live load of 20.0psf on to bottom chord and any other members. . 10 considers parallel to grain value usin surface. connection (by others) of truss to bearin	sign. mph; TCDL=4.2psf; BCDL ione3 -2-0-0 to 1-0-0, Zono 33-10-15 to 40-8-0 zone; o in DOL=1.60 plate grip DC g applied roof live load sho he bottom chord in all are ng ANSI/TPI 1 angle to gra g plate capable of withsta	=6.0psf; h=15ft; B=45ft; e1 1-0-0 to 9-0-0, Zone2 cantilever left and right e pL=1.60 own covers rain loading i any other live loads. as where a rectangle 3-6 ain formula. Building des nding 100 lb uplift at joir	L=24ft; eave=5ft; C 2 9-0-0 to 13-2-15, exposed ;C-C for requirements specifi 6-0 tall by 2-0-0 wide signer should verify nt(s) except (jt=lb)	at.	NO 68182 STITLE OF

Jonquin Velez PE No.68182 MiTek Inc. DBA MITek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 30,2024



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									21-5-4					
0 ₁ 4 ₁ 0	3-10-4		7-0-0	71418	11-6-0	12-0 ₁ 0	15-5-0	19-0-	0 19-4-0	25-8-0	28-8-0	32-3-5	38-8-0	1
0-4-0	3-6-4	1	3-1-12	048	4-1-8	0-6-0	3-5-0	3-7-0	0-4-0 2-1-4	4-2-12	3-0-0	3-7-5	6-4-11	7
Plate Offsets (X,Y)	4:0-2-8,0	-2-4],	, [16:0-4	-0,0-3-0)], [21:0-2	-4,0-2-1	2], [24:0-	2-12,0-2-4]					
						1								

LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrNOCodeFBC2023/TPI2014	CSI. TC 0.63 BC 0.66 WB 0.63 Matrix-S	DEFL. in Vert(LL) -0.53 Vert(CT) -1.08 Horz(CT) 0.38 Wind(LL) 0.31	(loc) l/defl 20 >864 20 >426 13 n/a 20 >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 666 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP 23-24: WEBS 2x4 SP REACTIONS. (size Max H Max U Max G	No.2 No.2 *Except* 2x6 SP No.2 No.2 e) 2=0-4-0, 13=0-4-0 orz 2=-106(LC 25) plift 2=-142(LC 8), 13=-133(LC 8) rav 2=2303(LC 1), 13=1838(LC 1)		BRACING- TOP CHORD BOT CHORD	Structural wood s Rigid ceiling direc	sheathing direc	:tly applied or 5-7-6 oc 10-0-0 oc bracing.	; purlins.
FORCES. (lb) - Max. TOP CHORD 2-3=- 7.8=- 12-13 BOT CHORD 2-25= 15-16 5-24= WEBS 5-24= 18-21 18-21 5-23= 18-21	Comp./Max. Ten All forces 250 (lb) or 7439/142, 3-4=-7655/205, 4-5=-7490/18 12767/439, 8-9=-11223/360, 9-10=-4913 3=-3260/121 -57/6654, 24-25=-64/6739, 23-24=-180/ 5=-20/2816, 13-15=-20/2816, 22-23=-33/ -1714/148, 8-21=-5663/228, 7-22=0/393 -241/6714, 6-23=-5261/256, 7-21=-285/ 1=-136/6554, 9-21=-171/7846, 12-16=-37 -15/1336	less except when shown. 9, 5-6=-9781/310, 6-7=-1 3/222, 10-11=-2883/155, 7 8762, 17-18=-75/4287, 16 5231, 21-22=-32/5224 3, 4-24=0/3035, 3-24=-81/ 8318, 9-18=-4964/158, 16 70/44, 11-16=-92/2361, 16	1442/403, 11-12=-2909/157, 6-17=-77/4285, /472, 0-18=-35/816, 0-16=-2689/126,			NULL QUIN	
NOTES- 1) 3-ply truss to be con Top chords connect Bottom chords connect Webs connected as 2) All loads are conside ply connections have 3) Unbalanced roof live 4) Wind: ASCE 7-22; V II; Exp B; Encl., GCF 5) Building Designer / F to the use of this trus 6) Provide adequate dr 7) This truss has been will fit between the b 9) Bearing at joint(s) 2 capacity of bearing s 10) Provide mechanica Contiented of agias	nected together with 10d (0.131"x3") nai ed as follows: 2x4 - 1 row at 0-9-0 oc. ected as follows: 2x4 - 1 row at 0-7-0 oc, follows: 2x4 - 1 row at 0-9-0 oc. ered equally applied to all plies, except if e been provided to distribute only loads r loads have been considered for this der (ult=130mph (3-second gust) Vasd=101n pi=0.18; MWFRS (directional); cantilever Project engineer responsible for verifying ss component. ainage to prevent water ponding. designed for a 10.0 psf bottom chord live n designed for a live load of 20.0psf on th ottom chord and any other members. considers parallel to grain value using Al surface.	Is as follows: 2x6 - 2 rows staggered a noted as front (F) or back toted as (F) or (B), unless sign. nph; TCDL=4.2psf; BCDL left and right exposed ; Lt applied roof live load sho e load nonconcurrent with ne bottom chord in all are: NSI/TPI 1 angle to grain fr ng plate capable of withst	at 0-9-0 oc. k (B) face in the LOAD CA s otherwise indicated. .=6.0psf; h=15ft; B=45ft; L umber DOL=1.60 plate gr own covers rain loading re n any other live loads. as where a rectangle 3-6- formula. Building designe tanding 100 lb uplift at joir	.SE(S) section. P .=24ft; eave=4ft; ip DOL=1.60 equirements spec 0 tall by 2-0-0 wi r should verify nt(s) except (jt=lb)	ly to Cat. ific de Joa Mii 160 Dat	NO 68 NO 68 STATE STATE OR SONA rquin Velez PE No.68182 Tek Inc. DBA MITEK US. 123 Swingley Ridge Rd. C te: July	A FL Cert 6634 Chesterfield, MO 63017 Y 30,2024
WARNING - Verify Design valid for use or a truss system. Before building design. Braci	design parameters and READ NOTES ON THIS AN hy with MITek® connectors. This design is based on use, the building designer must verify the applicab ng indicated is to prevent buckling of individual trus	ID INCLUDED MITEK REFEREN Ny upon parameters shown, and liity of design parameters and pr s web and/or chord members on	VCE PAGE MII-7473 rev. 1/2/2023 d is for an individual building com roperly incorporate this design int ly. Additional temporary and per	3 BEFORE USE. ponent, not to the overall rmanent bracing		Mi	Tek [®]

billioning design. Staching indicated is to prevent obligation buckling of individual rubs web and/or cliniter individual rubs web and/or



Job	Truss	Truss Type	Qty	Ply	1820-B Brick
					T34578781
6241953	A11	Roof Special Girder	1	2	
				5	Job Reference (optional)
Tibbetts Lumber Co., LLC (C	Dcala, FL), Ocala, FL - 34	472,		8.730 s Jul	11 2024 MiTek Industries, Inc. Mon Jul 29 12:10:52 2024 Page 2
		ID:96	77KBVwwj	NKu0WI9I	YrcUzY81Q-qtQwQDKtZm8BzAElzt88pZlL4iXrLww0Xw2hO8ytCpX

NOTES-

11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 302 lb down and 172 lb up at 7-0-0, and 162 lb down and 83 lb up at 9-0-12, and 162 lb down at 85 lb up at 11-0-12 on top chord, and 312 lb down at 7-0-0, and 95 lb down at 9-0-12, and 95 lb down at 11-4-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf) Vert: 1-4=-60, 4-6=-60, 6-7=-60, 7-8=-60, 8-10=-60, 10-11=-60, 11-14=-60, 2-24=-20, 21-24=-20, 13-20=-20

Concentrated Loads (lb)

Vert: 4=-205(F) 23=-48(F) 26=-122(F) 28=-122(F) 29=-262(F) 30=-48(F)

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							1	1.	-0-0			
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.28	Vert(LL)	0.00	2	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.20	Vert(CT)	0.00	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	4	n/a	n/a		
BCDL	10.0	Code FBC2023/TF	912014	Matri	k-P						Weight: 7 lb	FT = 20%

TOP CHORD

BOT CHORD

1-0-0

Structural wood sheathing directly applied or 1-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 2=0-4-0, 4=Mechanical

Max Horz 2=81(LC 12) Max Uplift 2=-218(LC 12), 4=-91(LC 1) Max Grav 2=290(LC 1), 4=94(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed ;C-C for members and
- forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=218.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 30,2024



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LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.33	Vert(LL)	-0.00	2-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.09	Vert(CT)	-0.01	2-4	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2023/TP	12014	Matri	x-P	Wind(LL)	0.00	2	****	240	Weight: 13 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-4-0, 4=Mechanical

Max Horz 2=71(LC 12)

Max Uplift 3=-14(LC 9), 2=-85(LC 12)

Max Grav 3=35(LC 17), 2=292(LC 1), 4=55(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 2-11-4 zone; cantilever left and

- right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



Structural wood sheathing directly applied or 3-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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LOADING	G (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.25	TC 0.31	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL 1.25	BC 0.28	Vert(CT)	-0.06	2-4	>894	240		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2023/TPI2014	Matrix-P	Wind(LL)	0.00	2	****	240	Weight: 20 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-4-0, 4=Mechanical

Max Horz 2=95(LC 12)

Max Uplift 3=-36(LC 12), 2=-70(LC 12)

Max Grav 3=114(LC 1), 2=350(LC 1), 4=95(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 4-11-4 zone; cantilever left and

right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



Structural wood sheathing directly applied or 5-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbscsomponents.com)



	+ 0-7-2 + 0-7-2 +	4-3-0 3-7-14		7	7-9-12 3-6-12		+ 8-6-0 0-8-4	<u> </u>	
Plate Offsets (X,Y)	[2:0-1-4,Edge], [2:0-1-4,0-9-11], [4:0-1-4	"Edgej, [4:0-1-4,0-9-11]							
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2023/TPI2014	CSI. TC 0.42 BC 0.59 WB 0.05 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.01 -0.02 0.00 0.01	(loc) 2-6 2-6 4 2-6	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 38 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 S BOT CHORD 2x4 S WEBS 2x4 S WEDGE Left: 2x4 SP No.2 , Rig	P No.2 P No.2 P No.2 ght: 2x4 SP No.2	BRACING- TOP CHORI BOT CHORI	D S D R	Structur Rigid ce	al wood : iling dire	sheathing direct ctly applied or 1	ly applied or 6-0-0 0-0-0 oc bracing.	oc purlins.	
REACTIONS. (siz Max H Max I Max 0									
FORCES. (lb) - Max TOP CHORD 2-3= BOT CHORD 2-6=	less except when shown.								

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 4-3-0, Zone2 4-3-0 to 8-4-4, Zone1 8-4-4 to 10-6-0 zone; cantilever left and right exposed ; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 5) will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=145, 4=145.



Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

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Plate Offsets (X,Y)	0-7-2 1-9 0-7-2 1-2 [2:0-3-8,Edge], [2:0-2-8,Edge], [6:0	-7 1-11-1 4-3-0 -6 0-1-9 2-3-15)-3-8,Edge], [6:0-2-8,Edge]	6-6-15 6-8-19 7-10-0 8-6-0 2-3-15 0-1-9 1-1-7 0-8-0
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2023/TPI2014	CSI. TC 0.35 BC 0.60 WB 0.05 Matrix-P	DEFL. in (loc) l/defl L/d Vert(LL) -0.01 2-8 >999 360 MT20 244/190 Vert(CT) -0.02 2-8 >999 240 MT20 244/190 Horz(CT) 0.00 6 n/a n/a Wind(LL) 0.01 2-8 >999 240 Weight: 42 lb FT = 20%
LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x4 SF WEBS 2x4 SF OTHERS 2x4 SF	P No.2 P No.2 P No.2 P No.2		BRACING-TOP CHORDStructural wood sheathing directly applied or 6-0-0 oc purlins.BOT CHORDRigid ceiling directly applied or 10-0-0 oc bracing.
REACTIONS. (siz Max H Max U Max C	e) 2=0-3-8, 6=0-3-8 lorz 2=46(LC 11) Jplift 2=-145(LC 12), 6=-145(LC 12 Jrav 2=457(LC 1), 6=457(LC 1))	

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-393/212, 4-6=-393/212

BOT CHORD 2-8=-73/308, 6-8=-73/308

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 4-3-0, Zone2 4-3-0 to 8-4-4, Zone1 8-4-4 to 10-6-0 zone; cantilever left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

5) Gable studs spaced at 2-0-0 oc.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=145, 6=145.



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and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbscscomponents.com)



Plate Offsets (X,Y)	0-10-0 0-10-0 [2:0-1-4,Edge], [2:0-1-4,0-9-11], [4:0-1-4	4-3-0 3-5-0 ,Edge], [4:0-1-4,0-9-11]		7-9-11 3-6-11		+ 8-6-0 0-8-5	1	
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2023/TPI2014	CSI. TC 0.46 BC 0.65 WB 0.05 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (loc) -0.01 4-6 -0.02 4-6 0.00 4 0.01 4-6	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 38 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP BOT CHORD 2x4 SP WEBS 2x4 SP WEDGE Left: 2x4 SP No.2 , Rig	No.2 No.2 No.2 ht: 2x4 SP No.2		BRACING- TOP CHOR BOT CHOR	D Structu D Rigid o	ural wood sh ceiling direct	heathing directly tly applied or 10-	applied or 6-0-0 -0-0 oc bracing.	oc purlins.
REACTIONS. (size Max H Max U Max G	e) 2=0-4-0, 4=0-3-8 orz 2=51(LC 11) plift 2=-146(LC 12), 4=-145(LC 12) rav 2=458(LC 1), 4=456(LC 1)							
FORCES. (lb) - Max. TOP CHORD 2-3=- BOT CHORD 2-6=-	Comp./Max. Ten All forces 250 (lb) or 370/198, 3-4=-369/198 38/262, 4-6=-38/262	less except when shown.						

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 4-3-0, Zone2 4-3-0 to 8-4-4, Zone1 8-4-4 to 10-6-0 zone; cantilever left and right exposed ; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 5) will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=146, 4=145.



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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 0-2-0 to 3-2-0, Zone1 3-2-0 to 4-3-0, Zone3 4-3-0 to 8-4-4 zone; cantilever left and right exposed ; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



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and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbscscomponents.com)



			0-2-	-0								
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.74	Vert(LL)	-0.14	2-4	>603	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.62	Vert(CT)	-0.27	2-4	>301	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2023/TF	PI2014	Matri	x-P	Wind(LL)	0.00	2	****	240	Weight: 26 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 4=Mechanical, 2=0-2-0

Max Horz 2=118(LC 12)

Max Uplift 3=-64(LC 12), 2=-60(LC 12)

Max Grav 3=187(LC 1), 4=137(LC 3), 2=418(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 6-11-4 zone; cantilever left and

right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

0_T2_T0

- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 2.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

Joaquin Velez PE No.68182 MiTek Inc. DBA MiTek USA FL Cert 6634 16023 Swingley Ridge Rd. Chesterfield, MO 63017 Date:

July 30,2024



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and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbscsomponents.com)



LOADING	G (psf)	SPACING- 2	-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.71	Vert(LL)	-0.13	2-4	>625	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.25	BC	0.60	Vert(CT)	-0.26	2-4	>313	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2023/TPI20)14	Matri	k-P	Wind(LL)	0.00	2	****	240	Weight: 26 lb	FT = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2

REACTIONS. (size) 3=Mechanical, 2=0-4-0, 4=Mechanical

Max Horz 2=118(LC 12)

Max Uplift 3=-63(LC 12), 2=-63(LC 12)

Max Grav 3=182(LC 1), 2=422(LC 1), 4=135(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 6-11-4 zone; cantilever left and

right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

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- 2) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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	6-3-11		13-8-5				-		20-0-0	
	6-3-11		1	7-4-10			1		6-3-11	
Plate Offsets (X,Y)-	[8:0-2-8,0-3-0]									
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.25 1.25 YES	CSI. TC 0.51 BC 0.51 WB 0.10	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.06 -0.16 0.03	(loc) 8-9 8-9 6	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20	GRIP 244/190
BCDL 10.0	Code FBC2023/11	712014	Matrix-S	Wind(LL)	0.03	8-9	>999	240	vveight: 97 lb	FI = 20%

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.2

REACTIONS. (size) 2=0-4-0, 6=0-4-0 Max Horz 2=-99(LC 10) Max Uplift 2=-95(LC 12), 6=-95(LC 12) Max Grav 2=917(LC 1), 6=917(LC 1)

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-3=-1394/180, 3-4=-1175/140, 4-5=-1175/140, 5-6=-1394/180
- BOT CHORD 2-9=-88/1191, 8-9=0/752, 6-8=-109/1191

WEBS 4-8=-2/422, 5-8=-293/149, 4-9=-2/422, 3-9=-293/149

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 -2-0-0 to 1-0-0, Zone1 1-0-0 to 10-0-0, Zone2 10-0-0 to 14-2-15, Zone1 14-2-15 to 22-0-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6.



Structural wood sheathing directly applied or 4-7-4 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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	L				20-0-0							
					20-0-0							
Plate Offsets (X,Y)	[2:0-4-0,0-2-1], [12:0-4-0,	,0-2-1], [17:0-2	2-8,0-3-0]									_
LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code FBC2023/T	2-0-0 1.25 1.25 YES Pl2014	CSI. TC BC WB Matrix	0.26 0.10 0.05 -S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.02 0.00	(loc) 13 13 12	l/defl n/r n/r n/a	L/d 120 120 n/a	PLATES MT20 Weight: 103 lb	GRIP 244/190 FT = 20%	_
LUMBER-					BRACING-							

TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.2

REACTIONS. All bearings 20-0-0.

Max Horz 2=93(LC 11) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 12, 18, 19, 16, 15

Max Grav All reactions 250 lb or less at joint(s) 17, 18, 19, 16, 15 except 2=285(LC 23), 12=285(LC 24), 20=258(LC 23), 14=258(LC 24)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult gualified building designer as per ANSI/TPI 1.

4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable requires continuous bottom chord bearing.

7) Gable studs spaced at 2-0-0 oc.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 12, 18, 19, 16, 15.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

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F	6-3-11 6-3-11				1	3-8-5 7-4-10					<u>20-0-0</u> 6-3-11		
Plate Offs	ets (X,Y)	[7:0-2-8,0-3-0]											
LOADING TCLL	3 (psf) 20.0	SPACING- Plate Grip DOL	2-0-0 1.25 1.25	CSI. TC BC	0.51	DEFL. Vert(LL) Vert(CT)	in -0.06 -0.15	(loc) 7-8 7-8	l/defl >999	L/d 360 240	PLATES MT20	GRIP 244/190	

Horz(CT)

Wind(LL)

BRACING-

TOP CHORD

BOT CHORD

5

7-8

n/a

>999

n/a

240

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 4-6-12 oc purlins.

Weight: 94 lb

FT = 20%

0.03

0.03

L	I.	I٨	л	R	F	P	_	

BCLL

BCDL

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.2

0.0

10.0

REACTIONS. (size) 1=0-4-0, 5=0-4-0 Max Horz 1=-95(LC 10) Max Uplift 1=-30(LC 12), 5=-99(LC 12) Max Grav 1=780(LC 1), 5=924(LC 1)

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 1-2=-1440/226, 2-3=-1217/181, 3-4=-1189/161, 4-5=-1409/202

Rep Stress Incr

Code FBC2023/TPI2014

- BOT CHORD 1-8=-144/1255, 7-8=-10/766, 5-7=-129/1204
- WEBS 3-7=-0/421, 4-7=-293/150, 3-8=-14/435, 2-8=-326/161

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional) and C-C Zone3 0-2-0 to 3-2-0, Zone1 3-2-0 to 10-0-0, Zone2 10-0-0 to 14-2-15, Zone1 14-2-15 to 22-0-0 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

WB 0.10

Matrix-S

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

YES

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5.



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	<u> 4-5-1</u> 4-5-1		<u>9-10-1</u> 5-5-0	I
LOADING (psf)SPACING-TCLL 20.0Plate Grip DOLTCDL 10.0Lumber DOLBCLL 0.0 *Rep Stress IncrBCDL 10.0Code FBC2023/TP	2-0-0 CSI. 1.25 TC 0.71 1.25 BC 0.79 NO WB 0.56 I2014 Matrix-S	DEFL. i Vert(LL) -0.0' Vert(CT) -0.1' Horz(CT) 0.0: Wind(LL) -0.0'	in (loc) l/defl L/d 17 6-7 >999 360 7 6-7 >666 240 12 5 n/a n/a 17 2-7 >999 240	PLATES GRIP MT20 244/190 Weight: 43 lb FT = 20%

LUMBER-

TOP CHORD2x4 SP No.2BOT CHORD2x4 SP No.2WEBS2x4 SP No.2

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 5-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 4=Mechanical, 2=0-4-15, 5=Mechanical Max Horz 2=118(LC 8) Max Uplift 4=-54(LC 8), 2=-186(LC 8) Max Grav 4=168(LC 1), 2=628(LC 31), 5=273(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-1292/72

BOT CHORD 2-7=-118/1186, 6-7=-123/1184

WEBS 3-6=-1159/113

NOTES-

1) Wind: ASCE 7-22; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=6.0psf; h=15ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Encl., GCpi=0.18; MWFRS (directional); cantilever left and right exposed ; Lumber DOL=1.60 plate grip DOL=1.60

Exp 5, Encl., SCPI=0.10, MWFKS (directional), calified internal right exposed, cumber DOL=1.00 plate grip DOL=1.00
 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=186.

8) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.

9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-4=-60, 2-5=-20

Concentrated Loads (lb)

Vert: 8=-58(F=-29, B=-29) 9=100(F=50, B=50) 10=-39(F=-19, B=-19)



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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- 1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor1 bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- 12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.