



FL REG# 278, Yoonhwak Kim, FL PE #86367

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Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 19-3530
Job Description: HOLMES	
Address: FL	

Job Engineering Criteria:	
Design Code: FBC 2017 RES	IntelliVIEW Version: 18.02.01B through 19.02.02B
	JRef #: 1WVY2150003
Wind Standard: ASCE 7-10 Wind Speed (mph): 130	Roof Load (psf): 20.00-10.00- 0.00-10.00
Building Type: Closed	Floor Load (psf): None

This package contains general notes pages, 34 truss drawing(s) and 6 detail(s).

ltem	Drawing Number	Truss	Item	Drawing Number	Truss
1	161.20.0804.38177	A01	2	161.20.0804.40300	A02
3	161.20.0804.44760	A03	4	161.20.0804.47223	A04
5	161.20.0804.49070	A05	6	161.20.0804.50970	A06
7	161.20.0805.03990	A07	8	161.20.0805.10160	A08
9	161.20.0805.12193	B01	10	161.20.0805.13823	C01
11	161.20.0805.15410	C02	12	161.20.0805.20240	C03
13	161.20.0805.22303	C04	14	161.20.0805.23880	C04A
15	161.20.0805.34130	C05	16	161.20.0806.40180	C06
17	161.20.0806.43173	C07	18	161.20.0806.45670	D01
19	161.20.0806.49247	G01	20	161.20.0806.51303	G02
21	161.20.0807.01720	P01	22	161.20.0807.07713	P02
23	161.20.0807.09440	V1	24	161.20.0807.10570	V10
25	161.20.0807.11537	V11	26	161.20.0807.12540	V12
27	161.20.0807.13537	V2	28	161.20.0807.15050	V3
29	161.20.0807.16283	V4	30	161.20.0807.17420	V5
31	161.20.0807.18980	V6	32	161.20.0807.20300	V7
33	161.20.0807.21907	V8	34	161.20.0807.31563	V9
35	BRCLBSUB0119		36	A14015ENC101014	
37	CNNAILSP1014		38	GBLLETIN0118	
39	A14030ENC101014		40	VAL160101014	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer. The Truss Design Engineer. The Truss Design Engineer on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc). -R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment. W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; www.iccsafe.org.
- 3. Alpine, a division of ITW Building Components Group Inc.: 13723 Riverport Drive, Suite 200, Maryland Heights, MO 63043; <u>www.alpineitw.com</u>.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcindustry.com.

SEQN: 347761	COMN	Ply: 1	Job Numb	er: 19-3530						C	ust: R 215	JRef:1WV	/Y215000)3 Т34 [°]
FROM: CDM		Qty: 3	HOLMES Truss Lab	el: A01							rwNo: 161 / YK	1.20.0804.: Of	38177 6/09/202(0
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	A	10'1"12	*			34'10'	4							
	1'6"	10'1"12	-+-	9'1"15		6'8"6	6'8"6	+-	9'1"8 41'0"	-	3'2"2			
		10112		193 11		20	3200		419	14	45	1'6" ,		
											-	· •		
Loading Criteria (psf)	Win	d Criteria		Snow Criteria (Pg,P	f in PSF)	Defl/CSI Crite	eria		▲ Maxir	num Rea	ctions (It	os)		
TCLL: 20.00	Wine	d Std: ASCE 7-1 ed: 130 mph	0	Pg: NA Ct: NA (CAT: NA	PP Deflection	in loc L/de	flL/# 0.240	Loc R+	Gravity	/Rh	/ Rw	n-Gravit	ly / RL
BCLL: 0.00	Encl	losure: Closed		Lu: NA Cs: NA	20. INA	VERT(CL): 0.	132 G 99	9 240 9 180	B 416	/-	/-	/279	/51	/276
BCDL: 10.00	Risk	Category: II P: C Kzt: NA		Snow Duration: NA		HORZ(LL): 0.	029 L -	-	S 237	8 /-	/-	/1207	/384 /	/-
Des Ld: 40.00	Mea	n Height: 15.00 ft		Building Code:		Creep Factor:	2.0	-	Wind re	actions b	/- ased on M	/922 /WFRS	/209 /	/-
Soffit: 2.00	BCD	DL: 5.0 pst DL: 5.0 psf		FBC 2017 RES		Max TC CSI:	0.676		B Brg	Width =	3.5	Min Rec	1 = 1.5	
Load Duration: 1.25	MW	FRS Parallel Dist	: 0 to h/2	TPI Std: 2014		Max BC CSI: Max Web CSI	0.995 · 0.851		L Brg	Width =	3.5 3.5	Min Rec	i = 2.4 i = 2.1	
Spacing: 24.0	Loc.	from endwall: An	y	FT/RT:20(0)/10(0)			. 0.001		Bearing	s B, S, & rs not list	L are a rig	jid surfac	e. than 37	75 #
		GCpi: 0.18		Plate Type(s):				0.45	Maximu	Im Top C	chord For	ces Per I	Ply (lbs))
Lumber	Wine	d Duration: 1.60		WAVE		VIEW Ver: 19	.02.028.012	2.15	Chords	Tens.Co	omp. C	Chords	Tens. C	Comp.
Top chord: 2x4 SP #2;									D - E E - F	408 601 -	-112 (1372 H)-H ⊣-I	717 · 715	- 1499 - 1681
Bot chord: 2x4 SP #2; Webs: 2x4 SP #3:									F-G	717 -	1499 I	- J	463 ·	- 1144
Bracing									Maximu	um Bot C	hord For	ces Per F	۷v (lbs)	
(a) Continuous lateral r	restrain	t equally spaced	on						Chords	Tens.Co	omp. C	Chords	Tens. C	Comp.
member.									S-R	498	-116 F	2-0	1422	- 361
Loading									Q-P	496 1148	-245 N	л-м л-м	1293	- 430
Truss passed check for chord live load in areas	r 20 ps s with 4	f additional bottor 2"-high x 24"-wid	n Ə		100	WWA.	11111		Massian		D		-)	
clearance.					0,01	20000	King	L.	Webs	Tens.Co	orces re	Vebs	5) Tens. C	Comp.
Wind						* JCENS	N	10	C-S	298	- 410 0	3 - P	195	- 432
Wind loads based on N member design.	IWFR	S with additional (C&C	618		No 8636	57 .		S-E	771 - 0/2	1935 I 195 M	- M	370 1425	- 847 - 419
Right end vertical not e	exposed	d to wind pressure	Э.	Ē.					F-P	624	- 253 J	/-5 J-L	736	- 1805
Additional Notes						utina 🙀 edita	() () ()							
WARNING: Furnish a contractor	copy of	this DWG to the	(en		5	STATE O	F							
during handling, shippi	ng and	installation of tru	sses.		Q	Alania	A The							
The overall height of th	below.	s excluding overh	ang is		S.S.	- CURIU	(C)	s C	/					
10-0-0.		o onceaning or one	<u>g</u>		111	ONAL	E Gara							
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				I	L REG	# 278, Yoonh	wak Kim, F	FL PE #	\$86367					
	**14/			OW ALL NOTED CO	06/09/	2020								
_ **IMPORTA	NT**	KNING** READ	AND FOLL	OW ALL NOTES ON O ALL CONTRACTO	RS INCI	LUDING THE IN	STALLERS	S						
Component Safety Info	e care i	in tabricating, har by TPI and SBC	aling, shipp A) for safe	oing, installing and bra y practices prior to pe	cing. Re	eter to and follo these functions	w the latest	edition shall p	ot BCSI	(Building				
attached rigid ceiling. Lo	ocation	is shown for perm	anent later	ave property attached al restraint of webs sha on as shown above a	all have l	bracing installed	d per BCSI s unless note	ections	B3, B7, Mise	or B10,				ты
drawings 160A-Z for sta	andard	plate positions. R	efer to job's	General Notes page	for addit	ional informatio	n.					AL	PIL	NE
truss in conformance willisting this drawing indi	th ANS	SI/TPI 1, or for h	andling, sh	ipping, installation and	bracing	of trusses. A	seal on this	drawin	g or cove	r page	(6750 Foru	m Drive	I ITW COMPANY
drawing for any structur	e is the	e responsibility of	the Building	Designer per ANSI/T	PI 1 Sec	2.2.			safe ora		5	Suite 305 Orlando Fl	L, 32821	
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Suite 305 Orlando FL, 32821





SEQN: 347797 0	GABL Ply: 1	Job Number: 19-3530		Cust: R 215 JRef:1WVY2150003 T6
Page 1 of 2	Giy. 1	Truss Label: A07		/ YK 06/09/2020
	+-	55" + 103'8 + 14'9'9 + 193'11 + 2 55" + 4'10'8 + 4'6'1 + 4'6'1 + 4	34 <u>*13 276" + - 320"9 + - 3810"3</u> "1"3 ^{+ -} 41"3 + - 46"9 + - 69"10 + -	42119) 45 3336 2107
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	1 ¹⁵	13 ■ ■5x6 N	8x8 ≡6x6 P R T 101 A A A A	Ŧ
		12 5X6	*3X5	≈ 3X4
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		\$5X6 E W (a)	BI BN BR BV CD	
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	≡2.5X6(<i>I</i>	A1) ≡8X12 12 s7X8 ≡4	xii 8x10 \\$3x4 or ≥3x6 2x4(**) ₹5x6 =3x6	
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	F 7-	5'5" 10'3"8 13'6" 15'6" 17'6" 19'6"	23'6" 25'6" 27'6" 29'6" 31'6" 33'6" 33'6" 37'6" 38'	
		' 19'3'1121'6	" "32'0"9' 35'6" ' 38'10'	"3 41'6" '45' (NNL)
	I			<u>₩</u> 46°
Loading Criteria (psf)	Wind Criteria Wind Std: ASCE 7-	Snow Criteria (Pg,Pf in PSF Pg: NA Ct: NA CAT: N	Defl/CSI Criteria	A Maximum Reactions (Ibs), or *=PLF Gravity Non-Gravity
TCDL: 10.00	Speed: 130 mph	Pf: NA Ce: NA	VERT(LL): 0.111 AF 999 240	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00 BCDL: 10.00	Risk Category: II	Lu: NA Cs: NA Snow Duration: NA	VERT(CL): 0.260 AF 999 180 HORZ(LL): 0.035 AF	B 441 /- /- /333 /56 /456 AF 3559 /- /- /1581 /624 /-
Des Ld: 40.00	EXP: C Kzt: NA Mean Height: 15.00 f	it During of	HORZ(TL): 0.081 AF	AE*542 /- /0 /208 /11 /0
NCBCLL: 10.00 Soffit: 2.00	TCDL: 5.0 psf	FBC 2017 RES	Max TC CSI: 0.716	B Brg Width = 3.5 Min Req = 1.5
Load Duration: 1.25	MWFRS Parallel Dis	t: h to 2h TPI Std: 2014	Max BC CSI: 0.762	AF Brg Width = 3.5 Min Req = - AE Brg Width = 63.5 Min Req = -
Spacing: 24.0 "	C&C Dist a: 4.50 ft Loc. from endwall: no	ot in 13.25 ft FT/RT:20(0)/10(0)		Bearings B, AF, & CY are a rigid surface. Members not listed have forces less than 375#
	GCpi: 0.18 Wind Duration: 1.60	Plate Type(s): WAVE	VIEW Ver: 19.02.02B.0122.15	Maximum Top Chord Forces Per Ply (lbs)
Lumber	Wind Buldaon. 1.00	Bearing Block(s)	NEW VOI: 10.02.020.0122.10	E-H 840 - 297 R-T 651 - 2241
Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2:		Brg blocks:0.128"x3", min bra x-loc #blocks lena	. nails th/blk #nails/blk wall plate	H-K 872 -186 T-X 604 -2167
Webs: 2x4 SP #3; W4,V Filler 2x4 SP #2; M3 2x4	V22 2x4 SP #2; 4 SP M-31;	2 10.000' 1 12 Bearing block to be same	" 5 Rigid Surface size and species as (Filler)	N-P 529 - 1445 AA-AC 214 - 1082
Stack Chord: SC1 2x4 S	SP #2;	Bot chd. Refer to drawing	CNNAILSP1014 for more info.	P - R 650 - 2240
Bracing	etraint equally spaced	on		Maximum Web Forces Per Ply (Ibs) Webs Tens.Comp. Webs Tens. Comp.
member.	Shann equally spaced			E -AF 221 - 717 BS-BW 1419 - 294
Plating Notes			NHWAK	H-AF 137 - 461 R-BV 105 - 384 AF-AR 161 - 663 BV-CD 434 - 65
All plates are 2X4 except (**) 3 plate(s) require sp	ot as noted. lecial positioning. Refe	r to	O ENG AL	AF-AS 1418 - 294 BV-T 648 - 250
scaled plate plot details requirements.	for special positioning		- TUENSA	AR-AW 158 - 649 BZ-CE 1419 - 294
			No. 86367 , 👔	K-BF 999 - 181 CG-X 939 - 238
Truss designed to suppo	ort 1-6-0 top chord out	lookers	- <u>+</u> -	AW-BB 162 - 663 CH-CK 1419 - 294 AX-BC 1419 - 294 CK-CP 1419 - 294
and cladding load not to and 24.0" span opposite	face. Top chord must	not be	STATE OF	BB-BF 150 - 635 CL-CX 207 - 430 BC-BG 1418 - 294 CP-CU 1419 - 293
cut or notched, unless s	pecified otherwise.	-0,	STATE OF WS	BF-BI 556 -131 CU-CY 1418 -293
Purlins	l" oc in lieu of rigid ceili	ing.	CORIUS	BG-BJ 1419 - 294 CX-DA 331 - 1634
Laterally brace BC abov	re filler at 24" oc.	-"	ONAL ELEMAN	BI-BN 581 - 135 CX-AC 1014 - 155 BJ-BM 1419 - 294 CY-DA 180 - 908
sloping TC @ 24" oc; al	I flat TC @ 0" oc.	all		BM-BO 1419 - 294 CY-DD 2886 - 595 P-BV 451 - 67 DA-DC 298 - 1439
Wind)	BN-BR 561 - 132 DC-AE 309 - 1491 BO-BS 1419 - 294 DD-AE 1441 - 298
Wind loads based on M member design.	WFRS with additional	C&C		BR-BV 580 - 135 AC-AE 134 - 1138
Right end vertical not ex	posed to wind pressur	re. FL RE 06/0	G# 278, Yoonhwak Kim, FL PE 9/2020	#86367 Maximum Gable Forces Per Ply (Ibs)
IMDODTAN	**WARNING READ		RAWING!	
Trusses require extreme Component Safety Inform	care in fabricating, ha	ndling, shipping, installing and bracing. CA) for safety practices prior to performin	Refer to and follow the latest edition g these functions. Installers shall p	of BCSI (Building
bracing per BCSI. Unless attached rigid ceiling. Lo	s noted otherwise, top cations shown for perm	chord shall have properly attached struct nanent lateral restraint of webs shall have so and position as shown above and an	ural sheathing and bottom chord sha bracing installed per BCSI sections	all have a properly s B3, B7, or B10, wise After to
drawings 160A-Z for star	ndard plate positions. F	Refer to job's General Notes page for add	itional information.	ALPINE

Alphie, a division of it we building components Group inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing indicates acceptance www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org



[
SEQN: 347797 GABL	Ply: 1	Job Number:	19-3530						Cust: R 2	15 JRef: 1WVY2	2150003	T6
FROM: CDM	Qty: 1	HOLMES							DrwNo:	161.20.0805.03	990	
Page 2 of 2		Truss Label:	A07						/	YK 06/0	9/2020	
Additional Notes							Gables	Tens.(Comp.			
See DWGS A14015ENC1010	J14 & GBLLETINO	118 for					DB-DC	188	- 449			
Stacked ton chord must NOT	be notched or cut	in										
area (NNL). Dropped top cho	rd braced at 24" o	C										
intervals. Attach stacked top of	chord (SC) to drop	ped										
oc. Center plate on stacked/d	ropped chord inte	rface,										
plate length perpendicular to	chord length. Splic	ce top										
chord in notchable area using	JXD.											
installation contractor. Specia	l care must be tak	en										
during handling, shipping and	installation of trus	sses.										
See "WARNING" note below.												
Shim all supports to solid bea	iring.											
10-0-0	s excluding overha	ang is										
					11111111111	11100						
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				06/09/	2020							
	RNING** READ	AND FOLLOW	ALL NO	TES ON THIS DE								
Trusses require extreme care	in fabricating, han	dling, shippina	¬∟∟ ∪ON , installing	and bracing. R	efer to and follo	w the latest edition	of BCSI (Buildin	g			
Component Safety Information bracing per BCSI. Unless note	n, by TPI and SBC	A) för safety p hord shall hav	actices p	ior to performing attached structur	these functions al sheathing an	 Installers shall p nd bottom chord shall 	rovide ten III have a	porary	r V			
attached rigid ceiling. Location	s shown for perm	anent lateral re	straint of	webs shall have b	pracing installed	per BCSI sections	B3, B7, c	br B10, fer to	,			TM
drawings 160A-Z for standard	plate positions. R	efer to job's Ge	eneral Not	es page for additi	onal information	n.					N	JE
Alpine, a division of ITW Build	ing Components C	Group Inc. shal	l not be re ng. instal	sponsible for any ation and bracing	deviation from	this drawing, any fa	ailure to b	uild the	•		AN ITW	COMPANY
listing this drawing, indicates a	acceptance of prof	essional engine	eering res	ponsibility solely	for the design s	hown. The suitabilit	y and use	of this	;	6750 Forum Suite 305	Drive	
For more information see these w	veb sites: Alpine: w	w.alpineitw.con	n; TPI: www	v.tpinst.org; SBCA:	www.sbcindustr	y.com; ICC: www.icc	safe.org			Orlando FL, 3	32821	
	· · ·						U					



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6750 Forum Drive Suite 305 Orlando FL, 32821





SEQN: 347846	COMN Ply: 1	Job Numb	er: 19-3530		Cust: R 215 JRef: 1WVY2150003 T15
FROM: CDM	Qty: 2	HOLMES			DrwNo: 161.20.0806.40180
		Truss Lab	el: C06		/ YK 06/09/2020
		ŀ	5'9"6 11'6" 5'9"6 5'8"10	17'2"10 - 20'1"13 5'8"10	+ 23' 2'10"3 +
	т			=474 E	Ŧ
			/		
			45X5	*5X5	
	727 -				о Ф
		1			N. E.
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		44X6			8 • 3X4
		в			
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		A =5X5(B2) S	R	≡5X6 K ≡3X	$\begin{array}{c} K \\ X4 \\ = 4 \times 5 (B2) \end{array}$
		k			
		r		2.5	X
		+ 1'6" + 2'3"8 2'3"8	-+- 3'5"14 5'8"10 5'9"6 11'6"	-+- 2'5" +- 3'1"14 +- 3'0"15 13'11" +- 17'0"14 +- 20'1"13 -	++
					- 1'6" -
	1		I		
Loading Criteria (psf)	Wind Criteria		Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 20.00	Speed: 130 mph	-10	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc R+ /R- /Rh /Rw /U /RL
BCIL: 0.00	Enclosure: Closed		Lu: NA Cs: NA	VERT(CL): 0.229 R 999 180) B 1096 /- /- /682 /163 /357
BCDL: 10.00	Risk Category: II		Snow Duration: NA	HORZ(LL): 0.104 K	I 1096 /- /- /682 /163 /-
Des Ld: 40.00	EXP: C Kzt: NA) ft		HORZ(TL): 0.219 K	Wind reactions based on MWFRS
NCBCLL: 10.00	TCDL: 5.0 psf	/ IC	Building Code:	Creep Factor: 2.0	B Brg Width = 3.5 Min Req = 1.5
Soffit: 2.00	BCDL: 5.0 psf		TPI Std: 2014	Max IC CSI: 0.472 Max BC CSI: 0.722	Bearings B & I are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel D	ist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.501	Members not listed have forces less than 375#
opuoling. 2 1.0	Loc. from endwall:	not in 4.50 ft	FT/RT:20(0)/10(0)		Maximum Top Chord Forces Per Ply (lbs) Chords Tens Comp Chords Tens Comp
	GCpi: 0.18		Plate Type(s):		
	Wind Duration: 1.6	0	WAVE	VIEW Ver: 19.02.02B.0122.15	C-D 238-1419 G-H 209-1168
					D-E 258-955 H-I 230-1222
Bot chord: 2x4 SP #2;	11 2x4 SP M-31;				E - F 250 - 943
Webs: 2x4 SP #3;					Maximum Bot Chord Forces Per Ply (lbs)
Rt Slider: 2x4 SP #3; bl	ock length = 1.456°				Chords Tens.Comp. Chords Tens. Comp.
	y				C - S 980 - 178 P - N 1051 - 49
All ploton are 2X4 even	at an noted				S-Q 1066 - 207 N-L 1038 - 46
All plates are 274 exce	pi as noted.				Q-P 1063-207 K-I 835-66
Wind			-	**************	Maximum Web Forces Ber Ply (lbs)
Wind loads based on N	IWFRS with additiona	al C&C	1111	NHWAK	Webs Tens.Comp. Webs Tens. Comp.
member design.			0,00		D-P 172 -532 P-F 171 -517
Additional Notes			5.	UCENSA .	E-P 696 -173 L-K 855 -66
1 he overall height of the 10-2-7.	s truss excluding ove	rhang is		No 86367 . 3	
Laterally brace top chor	d below filler and bot	tom chord	Ē, ·	110.00307	
above filler at 24" o.c., i	ncluding a lateral bra	ce at chord	T N N	- <u>+</u> -	
ends (If no rigid diaphra	igm exists at that poir	nt).	50.		
			E.R.	STATE OF	
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				/	
				/	
			FL REG	# 278, Yoonhwak Kim, FL PE =	#86367
IMPORTAN	NT FURNISH THIS	B DRAWING	TO ALL NOTES ON THIS DR	LUDING THE INSTALLERS	
Trusses require extreme Component Safety Infor	e care in fabricating, h	andling, ship BCA) for safe	bing, installing and bracing. R	efer to and follow the latest edition these functions. Installers shall n	n of BCSI (Building provide temporary
bracing per BCSI. Unles	s noted otherwise, to	p chórd shall rmanent later	have properly attached structure al restraint of webs shall have	ral sheathing and bottom chord sha	hall have a properly Is B3, B7, or B10.
as applicable. Apply plater	ates to each face of the	uss and posit	ion as shown above and on the General Notes page for addit	e Joint Details, unless noted other ional information.	
Alpine, a division of ITW	Building Component	s Group Inc.	shall not be responsible for any	/ deviation from this drawing, any f	failure to build the
truss in conformance wit	IN ANSI/TPL 1, or for ates acceptance of p	nandling, sh rofessional er	ipping, installation and bracing	g or trusses. A seal on this drawin for the design shown. The suitabili	ng or cover page 6750 Forum Drive
For more information see t	e is the responsibility hese web sites: Alpine:	or the Building www.alpineitw	uesigner per ANSI/TPI 1 Séc .com; TPI: www.tpinst.org; SBCA	3.∠. : www.sbcindustry.com; ICC: www.icc	csafe.org Orlando FL, 32821



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SEQN: 347816 (GABL	Ply: 1	Job Numb	er: 19-3530			Cust: R 215 JRef: 1WVY2150003 T17
FROM: CDM		Qty: 1	HOLMES Truss Lab	el: D01			DrwNo: 161.20.0806.45670
		1	т 905 1 1 1 1 1 1 1 1 1 1 1 1 1	$\begin{array}{c} + \frac{13^{2}14}{12^{2}11} \\ 13 \\ 13 \\ 13 \\ 10 \\ 10 \\ 10 \\ 10 \\ 10$	10272 + 114*13 + 10272 + 114*13 45*2 + 12*11 H4X4 F	SC2	
						∖ ⊥	
					11'6"		
			 ← 1'6'	• -+ +	11'6"	1'6"	
			 -	(NNL) 4'4"	(NNL) 	-	
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Spee Encle Risk EXP Mea TCD BCD MWI C&C Loc.	d Criteria d Std: ASCl ed: 130 mph losure: Close : Category: II 2: C Kzt: NA in Height: 15 1L: 5.0 psf DL: 5.0 psf FRS Parallel C Dist a: 3.00 from endwa GCpi: 0.*	E 7-10 ad l 5.00 ft l Dist: 0 to h/2 0 ft all: Any 18	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s):	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.001 L 999 240 VERT(CL): 0.003 L 999 180 HORZ(LL): 0.001 I HORZ(TL): 0.002 I Creep Factor: 2.0 Max BC CSI: 0.346 Max Web CSI: 0.055	▲ Maximum R Gravity Loc R+ /R- B* 139 /- Wind reactions B Brg Width Bearing B is a I Members not lis Maximum Top Chords Tens.0 B - C 550	eactions (lbs), or *=PLF / Non-Gravity / Rh / Rw / U / RL /- /75 /- /8 based on MWFRS = 137 Min Req = - rigid surface. sted have forces less than 375# 0 Chord Forces Per Ply (lbs) Comp. Chords Tens. Comp. -469 I - J 437 -441
	Wind	d Duration: 1	1.60	WAVE	VIEW Ver: 19.02.02B.0122.15]	
Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Stack Chord: SC1 2x4 S Stack Chord: SC2 2x4 S Plating Notes All plates are 2X4 excep	3P #2; 3P #2; ot as n	; ; noted.					
Truss designed to suppr and cladding load not to and 24.0" span opposite cut or notched, unless s	ort 1-4) excee e face. pecifie	I-0 top chord ed 2.00 PSF . Top chord r ed otherwise	l outlookers one face must not be	AND	CENSE 4		
Wind loads based on M member design.	WFRS	S with addition	onal C&C	*	No. 86367		
Additional Notes See DWGS A14015ENd gable wind bracing and Stacked top chord must area (NNL). Dropped top intervals. Attach stacked top chord in notchable a oc. Center plate on stac plate length perpendicul chord in notchable area The overall height of this 5-0-6.	C1010 other NOT p chor d top c irea us ked/di lar to c using s truss)14 & GBLLE requirement be notched rd braced at chord (SC) to sing 3x4 tie- ropped chord chord length 3x6. s excluding c	ETIN0118 for ts. or cut in 24" oc o dropped plates 24" d interface, . Splice top	FL REG 06/09.	STATE OF CORIDA SONAL EN Contraction G# 278, Yoonhwak Kim, FL PE /2020	#86367	
IMPORTAN Trusses require extreme Component Safety Inforr bracing per BCSI. Unless attached rigid ceiling. Lo as applicable. Apply pla drawings 160A-Z for star Alpine, a division of ITW truss in conformance with	**WAF IT F care in nation s note cation ates to ndard Buildi h ANS	RIING** R FURNISH TI in fabricating by TPI and dotherwise, s shown for each face c plate positio ing Compon SI/TPI 1, or	EAD AND FOLL HIS DRAWING T J, handling, shipp J SBCA) for safet top chord shall f permanent latera of truss and positi ns. Refer to job's ents Group Inc. a for handling, sh	OW ALL NOTES ON THIS DF O ALL CONTRACTORS INCI- ing, installing and bracing. R- y practices prior to performing have properly attached structural al restraint of webs shall have I ion as shown above and on the s General Notes page for additi shall not be responsible for any pping, installation and bracing	AWING! LUDING THE INSTALLERS efer to and follow the latest edition these functions. Installers shall p ral sheathing and bottom chord sh bracing installed per BCSI section e Joint Details, unless noted othe ional information. y deviation from this drawing, any ' g of trusses. A seal on this drawing	o of BCSI (Buildin provide temporary all have a proper s B3, B7, or B10, rwise. Refer to failure to build the ing or odvers of Bits	

listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see these web sites: Alpine: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org













Suite 305 Orlando FL, 32821



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Orlando FL, 32821





SEQN: 652303 FROM: CDM	VAL	Ply: 1 Qty: 1	Job Numb HOLMES Truss Lab	er: 19-3530 el: V11			Cust: R 215 JRef: 1WVY2150003 T28 DrwNo: 161.20.0807.11537 / YK 06/09/2020
				- 2'4"13 2'4"13	- - 4'9"10 2'4"13		
				$\begin{array}{c c} & 12 \\ 10 \\ \hline \\ = 3X4(D1) \\ \hline \\$	=4X4 B =3X4(D1) C U U 2X4		
				<u>-</u> 2'//"13			
				24 13	4'9"10	-	
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.25 Spacing: 24.0 "	Win Spe Encl Risk EXP Mea TCD BCD MW C&C Loc.	d Criteria d Std: ASCE 7-1 ed: 130 mph losure: Closed c Category: II 2: C Kzt: NA in Height: 19.08 ft DL: 5.0 psf DL: 5.0 psf FRS Parallel Dist C Dist a: 3.00 ft from endwall: no GCpi: 0.18	0 : h/2 to h t in 9.00 ft	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: N/ Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 2017 RES TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):	Defl/CSI Criteria A PP Deflection in loc L/defl L/# VERT(LL): 0.001 D 999 240 VERT(CL): 0.003 D 999 180 HORZ(LL): -0.001 D - HORZ(LL): 0.002 D - Creep Factor: 2.0 Max TC CSI: 0.070 Max Web CSI: 0.033	▲ Maximum R Gravity Loc R+ / R- O C* 83 /- Wind reactions C Brg Width Bearing A is a Members not li	eactions (Ibs), or *=PLF / Non-Gravity / / Rh / Rw / U / RL /- /44 /9 /10 based on MWFRS = 57.6 Min Req = - rigid surface. sted have forces less than 375#
Lumber	Win	d Duration: 1.60		WAVE	VIEW Ver: 18.02.01B.0321.08		
Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Wind Wind loads based on M	WFR	S with additional (C&C				
member design. Additional Notes See DWG VAL1601010 The overall height of thi 2-0-4.)14 foi s truss	r valley details. s excluding overh	ang is		No. 86367		
					STATE OF ZON	/ /	
				FL RE 06/0	G# 278, Yoonhwak Kim, FL PF 9/2020	E #86367	
IMPORTAN Trusses require extreme Component Safety Inforn bracing per BCSI. Unles attached rigid ceiling. Lo as applicable. Apply pla drawings 160A-2 for stat Alpine, a division of ITW truss in conformance wit listing this drawing, indic	**WAI NT e care matior s note cation ates to ndard ' Build th ANS ates, a	RNING** READ FURNISH THIS L in fabricating, har i, by TPI and SBC d otherwise, top c s shown for permo each face of trus plate positions. R ing Components SI/TPI 1, or for ha coceptance of prop	AND FOLL RAWING 1 colling, shipp colling, shall I anent later: s and positi efer to job's Group Inc. s andling, sh essional er	OW ALL NOTES ON THIS D TO ALL CONTRACTORS INC bing, installing and bracing. If y practices prior to performing have properly attached structi al restraint of webs shall have ion as shown above and on th s General Notes page for add shall not be responsible for an ipping, installation and bracin genering responsibility, solet)	RAWING! LUDING THE INSTALLERS Refer to and follow the latest edition g these functions. Installers shall iral sheathing and bottom chord sr bracing installed per BCSI section e Joint Details, unless noted othe itional information. y deviation from this drawing, any g of trusses. A seal on this drawing log the design shown. The suitabi	n of BCSI (Buildin provide temporar) all have a proper is B3, B7, or B10, rwise. Refer to failure to build the ng or cover page lify and use of this	G750 Forum Drive

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SEQN: 652279	VAL	Ply: 1 Qtv: 1	Job Numb	er: 19-3530		Cust: R 215 JRef: 1WVY2150003 T29 DrwNo: 161.20.0807.12540
		~.y	Truss Lab	el: V12		/ YK 06/09/2020
				∝ 1 1	'2"7 2'4"14 '2"7 1'2"7 ►	
					= 3X4 4(D1) $= 3X4(D1)$ C C C C C C C C C C C C C C C C C C C	
Loading Criteria (psf)	Win	d Criteria		Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (Ibs), or *=PLF
TCLL: 20.00 TCDL: 10.00	Wine Spee	d Std: ASCE 7-1 ed: 130 mph	0	Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA	PP Deflection in loc L/defl L/# VERT(LL): 0.001 999 240	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00 BCDL: 10.00	Encl Risk	osure: Closed Category: II		Lu: NA Cs: NA Snow Duration: NA	VERT(CL): 0.001 999 180 HORZ(LL): -0.000	C* 80 /- /- /39 /6 /8 Wind reactions based on MWFRS
Des Ld: 40.00 NCBCLL: 10.00	Mea	n Height: 19.58 ft		Building Code:	HORZ(TL): 0.001 Creep Factor: 2.0	C Brg Width = 28.8 Min Req = - Bearing A is a rigid surface.
Soffit: 2.00 Load Duration: 1.25	BCD	L: 5.0 psf ERS Parallel Dist	h/2 to h	FBC 2017 RES TPI Std: 2014	Max TC CSI: 0.023 Max BC CSI: 0.039	Members not listed have forces less than 375#
Spacing: 24.0 "	C&C Loc.	Dist a: 3.00 ft from endwall: not	t in 9.00 ft	Rep Fac: Yes FT/RT:20(0)/10(0)	Max Web CSI: 0.000	
	Wine	GCpi: 0.18 d Duration: 1.60		Plate Type(s): WAVE	VIEW Ver: 18.02.01B.0321.08	
Lumber						-
Bot chord: 2x4 SP #2;						
Wind Wind loads based on M member design.	WFR	S with additional C	C&C			
Additional Notes						
See DWG VAL1601010 The overall height of this)14 for s truss	valley details. excluding overha	ang is	191	NATHINI A LOUGH	
1-0-4.				anna C	ONFINAL FILE	
				11 Lag.		
					NO. 8030/	
				RO	STATE OF WE	
				and S	SORIU	7

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				FL RE0 06/09	G# 278, Yoonhwak Kim, FL PE 0/2020	#86367
	WAF IT	RNING** READ FURNISH THIS D	AND FOLL	OW ALL NOTES ON THIS DE	RAWING! LUDING THE INSTALLERS	
Component Safety Inform bracing per BCSI. Unles	care i mation s note	in rapricating, han , by TPI and SBC d otherwise, top c	aling, shipp A) for safe chord shall	bing, installing and bracing. R by practices prior to performing have properly attached structure a restructure works about here	erer to and tollow the latest edition these functions. Installers shall p ral sheathing and bottom chord sh bracing installed por PCSI action	I OF BUSI (Building provide temporary all have a property B3 B7 or B10
drawings 160A-Z for star	ates to ndard	each face of trus plate positions. R	efer to job's	on as shown above and on the General Notes page for addit	e Joint Details, unless noted other ional information.	
Alpine, a division of ITW truss in conformance wit listing this drawing, indic	Buildi h ANS ates a	Ing Components (SI/TPI 1, or for hat cceptance of prof	Foup Inc. s andling, sh essional er	shall not be responsible for any ipping, installation and bracing gineering responsibility solely Designer per ANSUTTEL 1	y deviation from this drawing, any f g of trusses. A seal on this drawin of the design shown. The suitabil	allure to build the ig or cover page ity and use of this Suite 305

drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Séc.2. For more information see these web sites: Alpine: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcindustry.com; ICC: www.iccsafe.org









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Suite 305 Orlando FL, 32821









CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member	Specified CLR	Alternative Reir	iforecement
Size	Restraint	T- or L- Reinf.	Scab Reinf,
2x3 or 2x4	1 row	2×4	1-2×4
2x3 or 2x4	2 rows	2×6	2-2×4
2×6	1 row	2×4	1-2×6
2×6	2 rows	2×6	2-2×4(X)
2×8	1 row	2×6	1-2×8
2×8	2 rows	2×6	2-2×6(X)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

AN ITW COMPANY

13723 Riverport Drive Suite 200

Maryland Heights, MO 63043



SPACING

96/09/2020 FL REC# 278, Yoonhwak Kim, FL PE #86367



^{66/09/2020,} Yoonhwak Kim, FL PE #86367



06/09/2020 FL REG# 278, Yoonhwak Kim, FL PE #86367





66/09/2020 FL REG# 278, Yoonhwak Kim, FL PE #86367

Valley Detail - ASCE 7-10: 160 mph, 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better. Bot Chord 2x4 SP #2N or SPF #1/#2 or better. Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are ITW BCG Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with: properly attached, rated sheathing applied prior to valley truss installation. Dr

Purlins at 24" o.c. or as otherwise specified on engineer's sealed design Dr

By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

- *** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.
- ++ Larger spans may be built as long as the vertical height does not exceed 14'-0''.



06/09/2020 FL REG# 278, Yoonhwak Kim, FL PE #86367