

MITEK PLATE APPROVAL #'S 2197.2-2197.4, BOISE EWP PRODUCT #'S LVL FL1644-R2, BCI JOISTS FL1392-R2

THE ARROW HEAD AT THE END OF THE TRUSS ON THE TRUSS PLACEMENT PLAN (LAYOUT) CORRESPONDS WITH THE LEFT SIDE OF THE INDIVIDUAL TRUSS DRAWING. USE THIS AS AN ORIENTATION GUIDE WHEN SETTING THE TRUSSES ON THE STRUCTURE.

General Notes:

- Per ANSI/TPI 1-2002 all " Truss to Wall" connections are the responsibility of the Building Designer, not the Truss Manufacturer.

- Use Manufacturer's specifications for all hanger connections unless noted otherwise.

- Trusses are to be 24" o.c. U.N.O.

- All hangers are to be Simpson or equivalent U.N.O.-Use 10d x 1 1/2" Nails in hanger connections to single ply girder trusses.

Trusses are not designed to support brick U.N.O. Dimensions are Feet-Inches- Sixteenths

Notes:

No back charges will be accepted by Builders FirstSource unless approved in writing first. 850-835-4541

ACQ lumber is corrisive to truss plates. Any ACQ lumber that comes in contact with truss plates (i.e. scabbed on tails) must have an approved barrier applied first.

Refer to BCSI-B1 Summary Sheet-Guide for handling, Installing and Bracing of Metal Plate Connected Wood Truss prior to and during truss installation.

It is the responsibility of the Contractor to ensure of the proper orientation of the truss placement plans as to the construction documents and field conditions of the structure orientation. If a reversed or flipped layout is required, it will be supplied at no extra cost by Builders FirstSource.

It is the responsibility of the Contractor to make sure the placement of trusses are adjusted for plumbing drops, can lights, ect..., so the trusses do not interfere with these type of items.

All common framed roof or floor systems must be designed as to NOT impose any loads on the floor trusses below. The floor trusses have not been designed to carry any additional loads from above.

This truss placement plan was not created by an engineer, but rather by the Builders FirstSource staff and is solely to be used as an installation guide and does not require a seal. Complete truss engineering and analysis can be found on the truss design drawings which may be sealed by the truss design engineer.

Gable end trusses require continuous bottom chord bearing. Refer to local codes for wall framing requirements.

Although all attempts have been made to do so, trusses may not be designed symmetrically. Please refer to the individual truss drawings and truss placement plans for proper orientation and placement.



Lake City PHONE: 386-755-6894 FAX: 386-755-7973

Jacksonville PHONE: 904-772-6100 FAX: 904-772-1973

Tallahassee PHONE: 850-576-5177

AMIRA BLDRS.

Welsh Res.

Custom

Legal Address:

1-26-22 Floor 1 Job# N/A

KLH Floor 2 Job#: N/A

Drawn By:

Original Ref #: 3056583 Roof Job #: 3056583

10-04-00



RE: 2718981 - DETAILS

Site Information:

Customer Info: DETAILS Project Name: N/A Model: N/A Lot/Block: N/A Subdivision: N/A Address: N/A, N/A City: N/A State: N/A

Name Address and License # of Structural Engineer of Record, If there is one, for the building. Name: License #: Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014 Wind Code: ASCE 7-16 Roof Load: 37.0 psf

Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 20 individual, General Truss Details and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Detail Name	Date	No.	Seal#	Detail Name	Date	
1	T23399806	MII-REP10	4/2/21	15	T23399820	MII-VALLEY HIGH WIND2	4/2/21	
2	T23399807	MII-T-BRACE 2	4/2/21	16	T23399821	MII-VALLEY SP	4/2/21	
3	T23399808	MII-SCAB-BRACE	4/2/21	17	T23399822	MII-VALLEY SP	4/2/21	
4	T23399809	MII-REP05	4/2/21	18	T23399823	MII-GE146-001	4/2/21	and the state of t
5	T23399810	MII-GE130-D-SP	4/2/21	19	T23399824	MII-REP13B	4/2/21	AN Buildin
<u>6</u>	T23399811	MII-GE130-SP	4/2/21	20	T23399825	MII-STRGBCK	4/2/21	Country
(T23399812	MII-GE140-001	4/2/21					
8	T23399813	MII-GE170-D-SP	4/2/21				1	D Plans
9	T23399814	MII-GE180-D-SP MII-GE180-D-SP	4/2/21					2 Reviewed
10 11	T23399815 T23399816	MII-GE 180-D-SF MII-PIGGY-ALT-7-16	4/2/21 4/2/21				9	
12	T23399817	MII-REP01A1	4/2/21				1	
13	T23399818	MII-TOENAIL SP	4/2/21					SCompliance
14	T23399819	MII-VALLEY HIGH WIND1						
	120000010							

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Jacksonville.

Truss Design Engineer's Name: ORegan, Philip

My license renewal date for the state of Florida is February 28, 2023.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



April 2,2021

ORegan, Philip

MiTek USA, Inc. 6904 Parke East Blvd. Tampa, FL 33610-4115

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

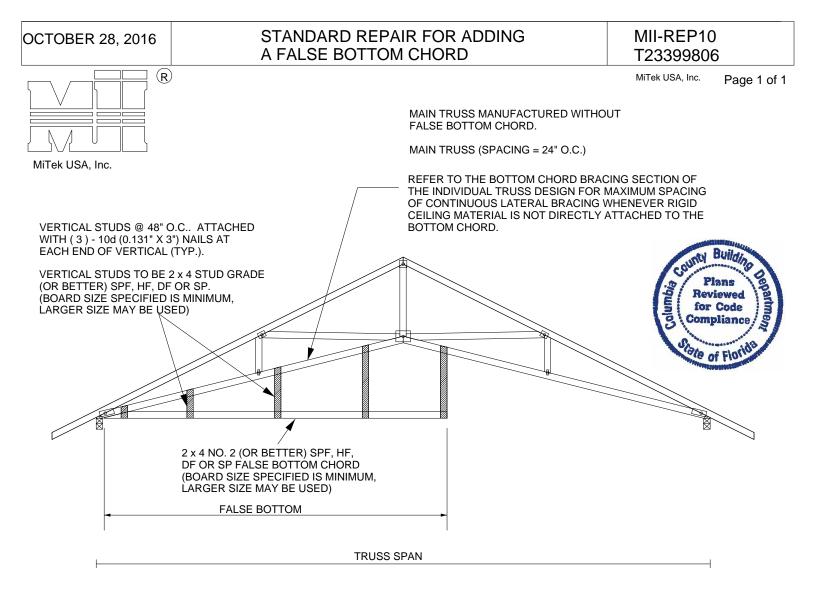


RE: \$JOBNAME - \$JOBDESC

MiTek USA, Inc. 6904 Parke East Blvd. Tampa, FL 33610-4115

Site Information:

Customer Info:\$SI_CUSTOMERProject Name:\$SI_JOBNAMEModel:\$SI_MODELLot/Block:\$SI_LOTNUMSubdivision:\$SI_SUBDIVAddress:\$SI_SITEADDRState:\$SI_SITESTATE



NOTES:

- 1. LOADING: TOP CHORD: (REFER TO THE MAIN TRUSS DESIGN FOR TOP CHORD LOADING). BOTTOM CHORD: LL = 0 PSF, DL = 10 PSF.
- 2. REFER TO THE MAIN TRUSS DESIGN FOR LUMBER AND PLATING REQUIREMENTS.
- 3. MAXIMUM BOTTOM CHORD PITCH = 6/12.
- 4. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID SPLITTING OF THE WOOD.
- 5. FALSE BOTTOM CHORD ONLY DESIGNED TO CARRY VERTICAL LOAD. NO LATERAL (SHEAR) LOAD ALLOWED.
- 6. FILLER MAY EXTEND FOR FULL LENGTH OF TRUSS.

This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss system. See **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

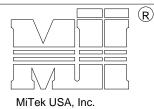
AUGUST 1, 2016

T-BRACE / I-BRACE DETAIL WITH 2X BRACE ONLY

MII-T-BRACE 2 T23399807

Page 1 of 1

MiTek USA, Inc.

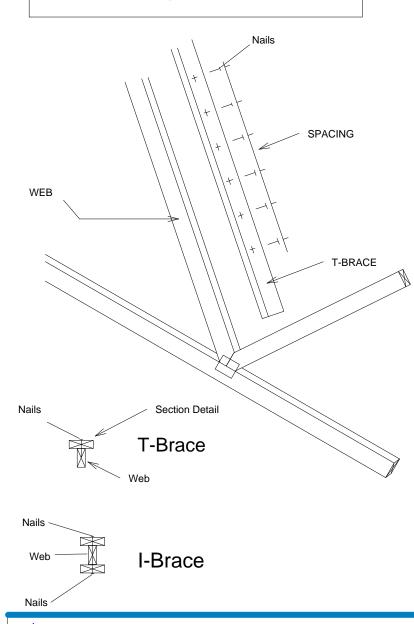


Note: T-Bracing / I-Bracing to be used when continuous lateral bracing is impractical. T-Brace / I-Brace must cover 90% of web length.

Note: This detail NOT to be used to convert T-Brace / I-Brace webs to continuous lateral braced webs.

lailing Pattern	
	1
Nail Size	Nail Spacing
10d (0 131" X 3")	6" o.c.
100 (0:101 7.0)	0 0.0.
entire length of T-Br	ace / I-Brace

(On Two-Ply's Nail to Both Plies)



	Brace for One-P	
	Specified Co Rows of Late	
Web Size	1	2
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace
2x6	2x6 T-Brace	2x6 I-Brace
2x8	2x8 T-Brace	2x8 I-Brace

	Brace for Two-F				
	Specified Continuous Rows of Lateral Bracing				
Web Size	1	2			
2x3 or 2x4	2x4 T-Brace	2x4 I-Brace			
2x6	2x6 T-Brace	2x6 I-Brace			
2x8	2x8 T-Brace	2x8 I-Brace			

T-Brace / I-Brace must be same species and grade (or better) as web member.

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Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021

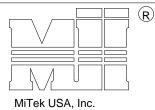


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AUGUST 1, 2016

SCAB-BRACE DETAIL

MII-SCAB-BRACE T23399808



MiTek USA, Inc.

Page 1 of 1

Note: Scab-Bracing to be used when continuous lateral bracing at midpoint (or T-Brace) is impractical. Scab must cover full length of web +/- 6".

*** THIS DETAIL IS NOT APLICABLE WHEN BRACING IS *** REQUIRED AT 1/3 POINTS OR I-BRACE IS SPECIFIED.

2 ROWS OF 10d SCAB MUST BE	CAB TO ONE FAC (0.131" X 3") NAI THE SAME GRA ETTER) AS THE V	CE OF WEB WITH ILS SPACED 6" O.C. DE, SIZE AND WEB.
SCAB BRACE		MAXIMUM WEB AXIAL FORCE = 2500 lb MAXIMUM WEB LENGTH = 12'-0" 2x4 MINIMUM WEB SIZE MINIMUM WEB GRADE OF #3
		M
Nails	Scab-Bra	on Detail
	🦳 Web	

Scab-Brace must be same species grade (or better) as web member.

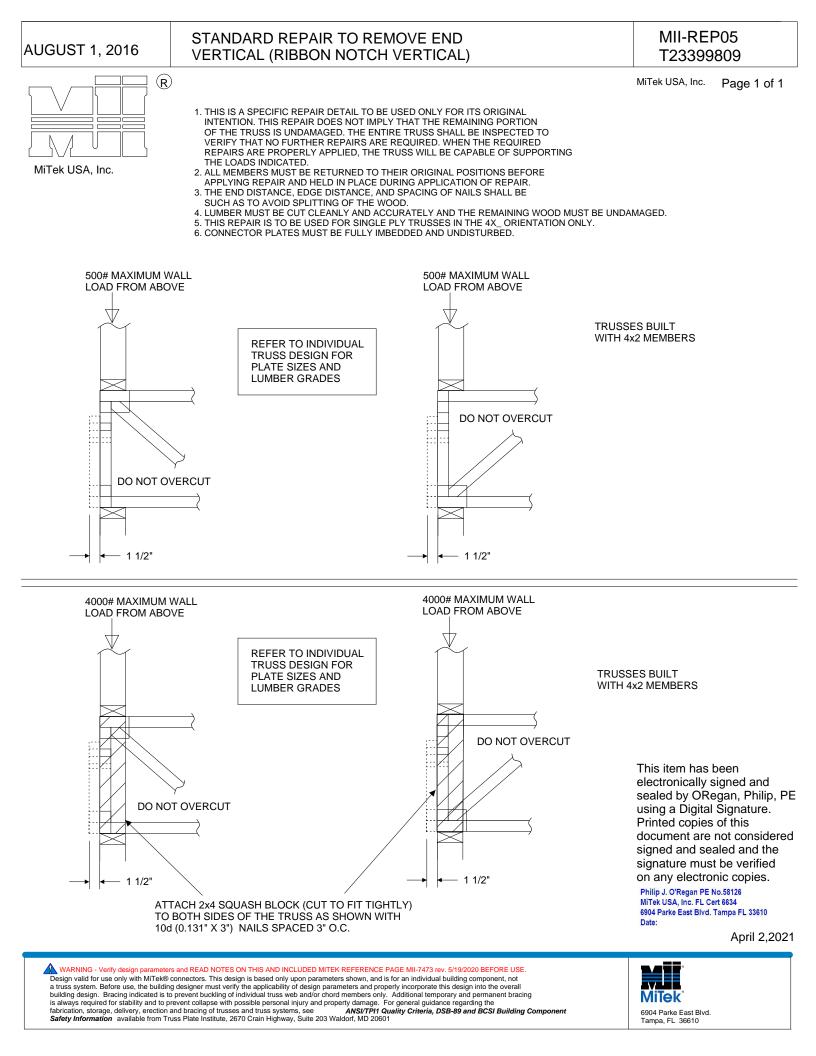
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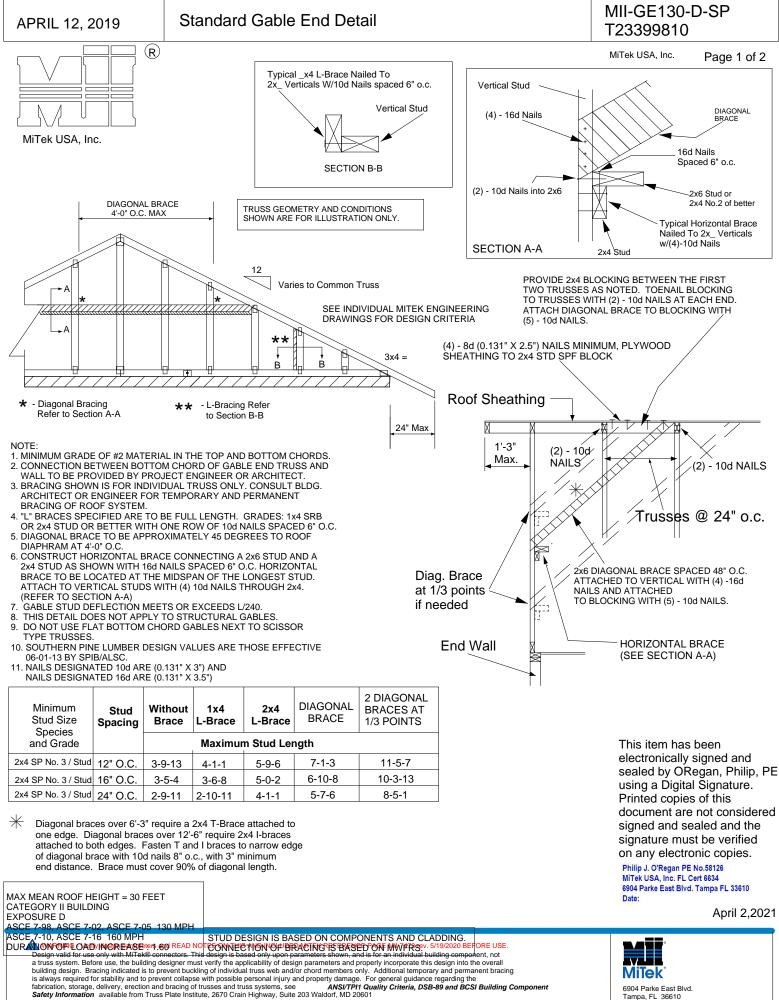
Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

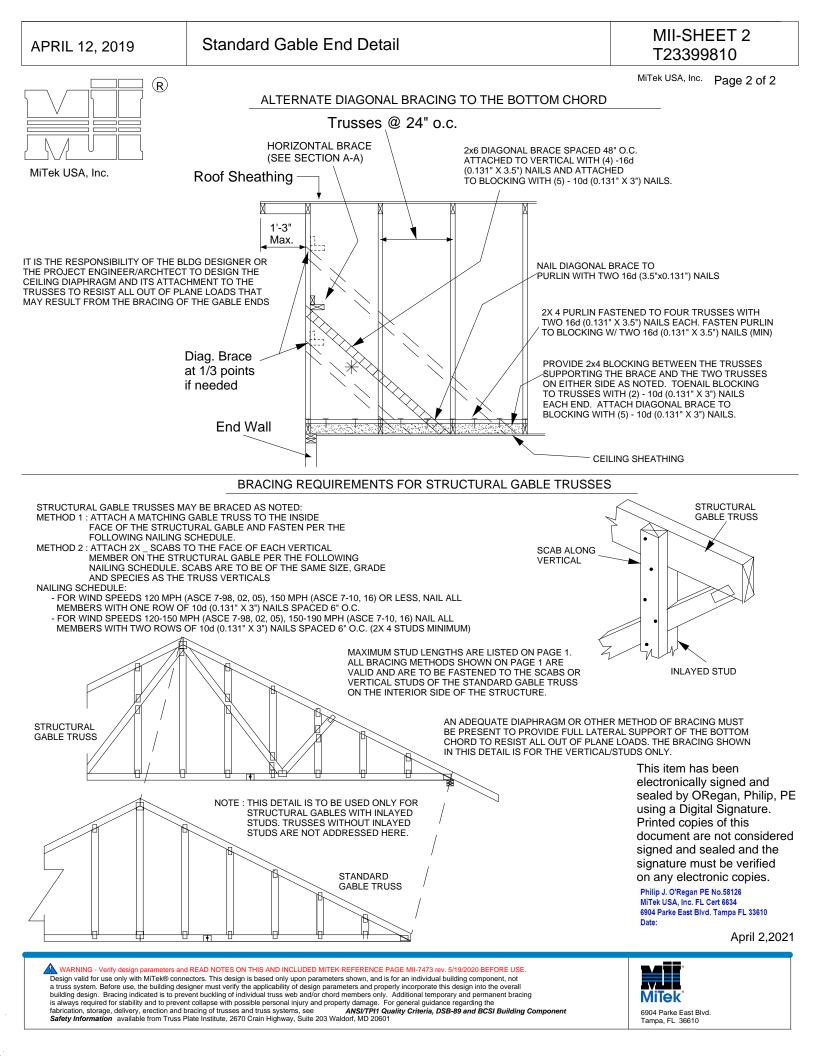
April 2,2021

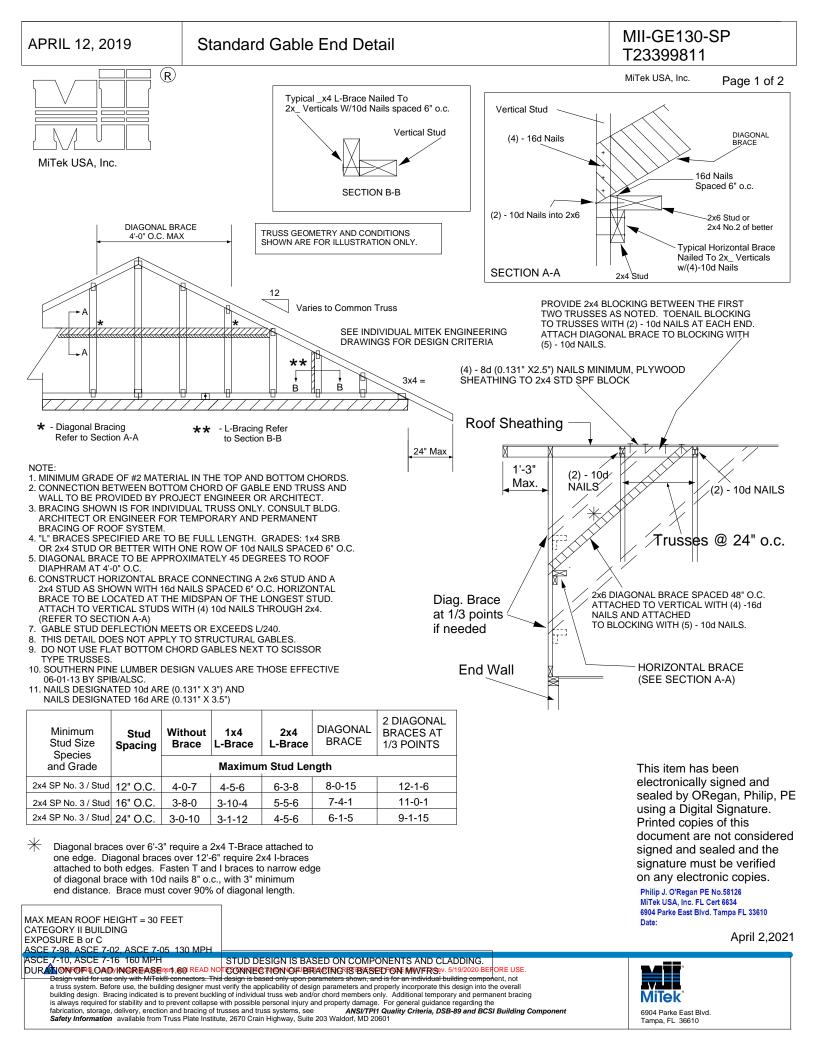


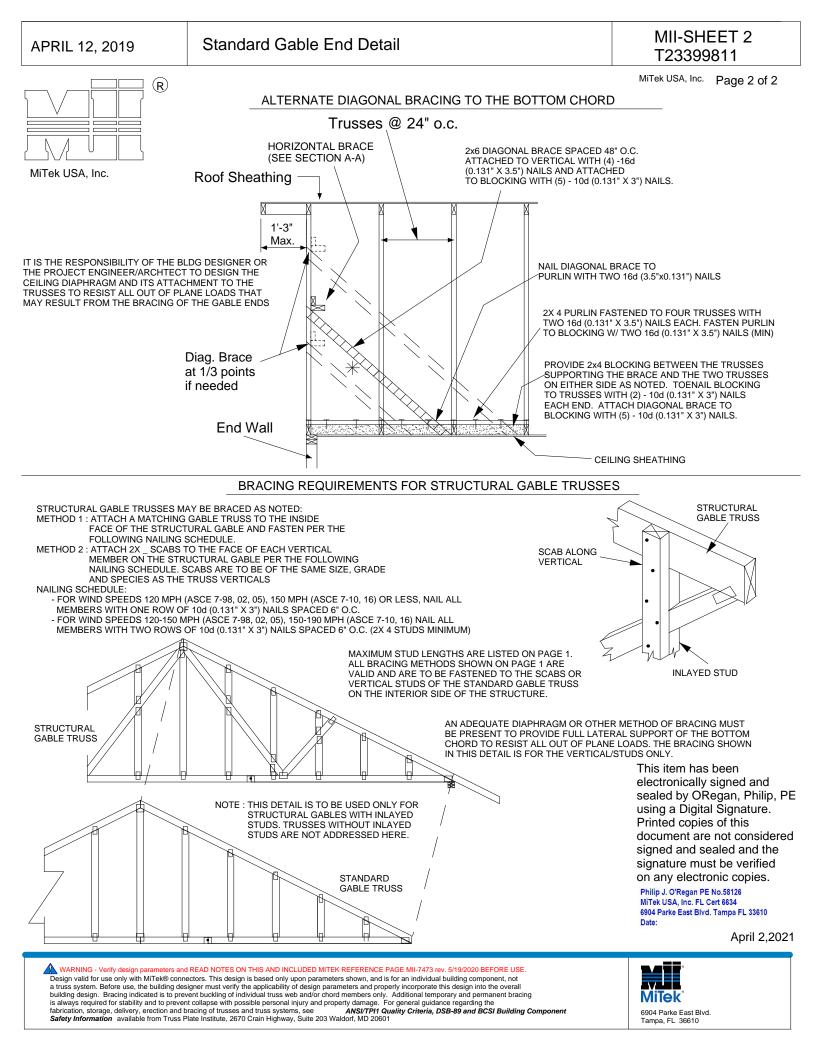
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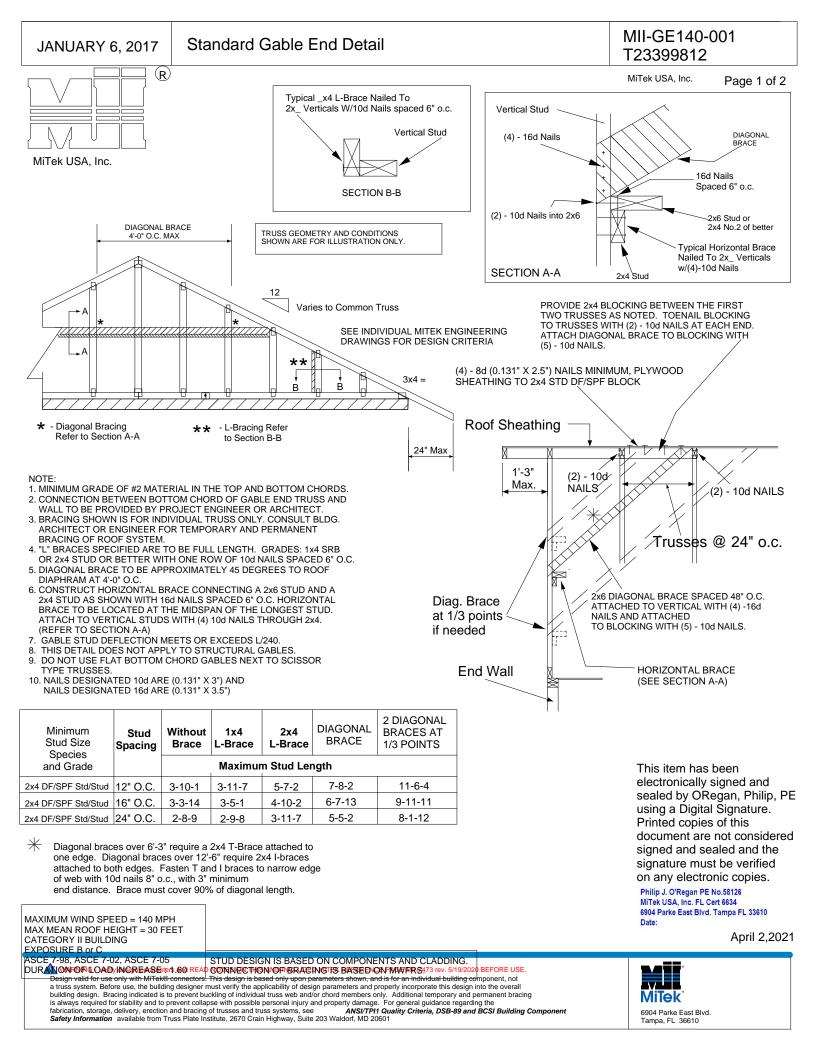


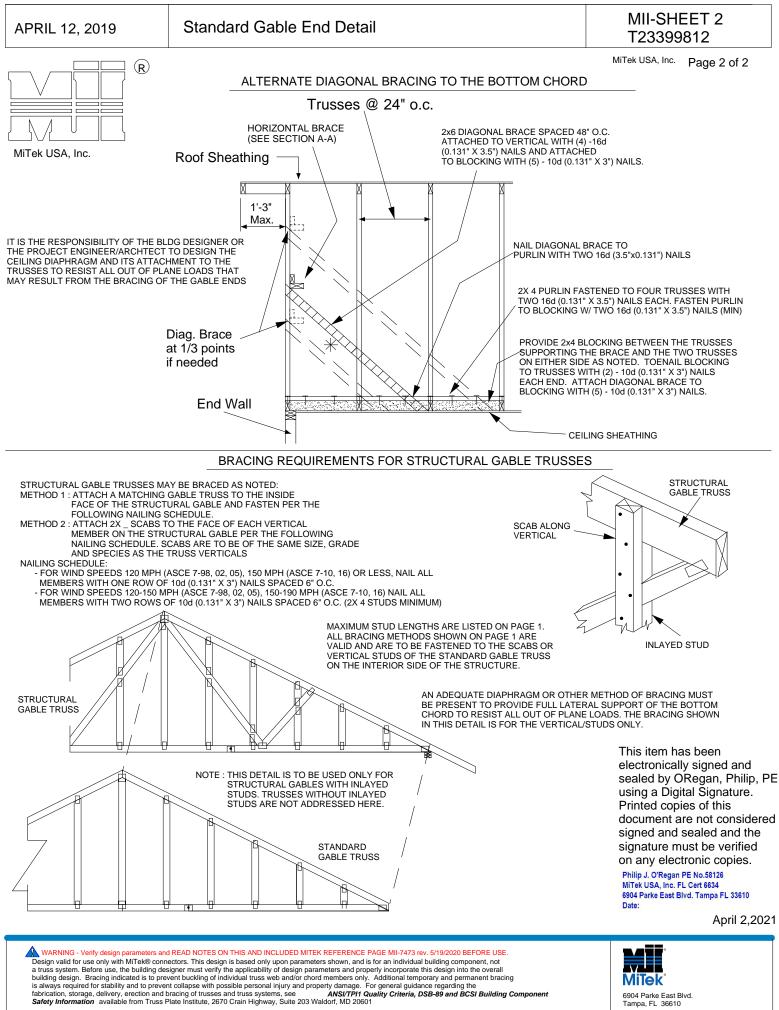




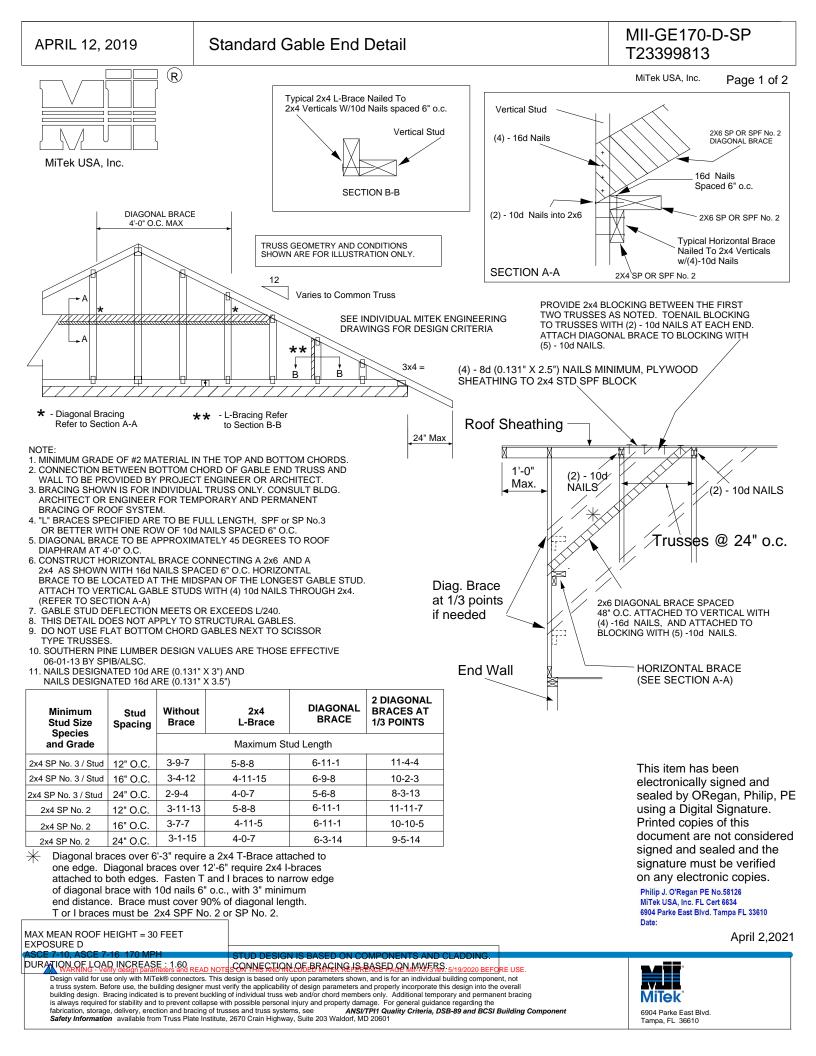


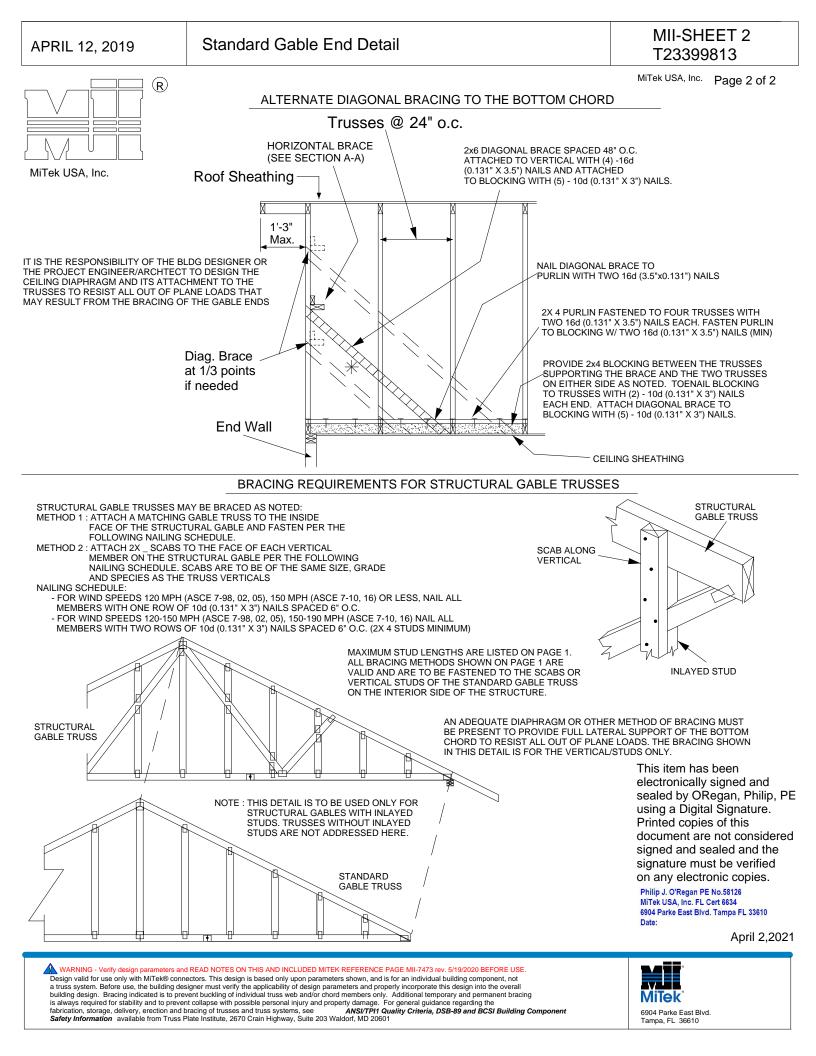


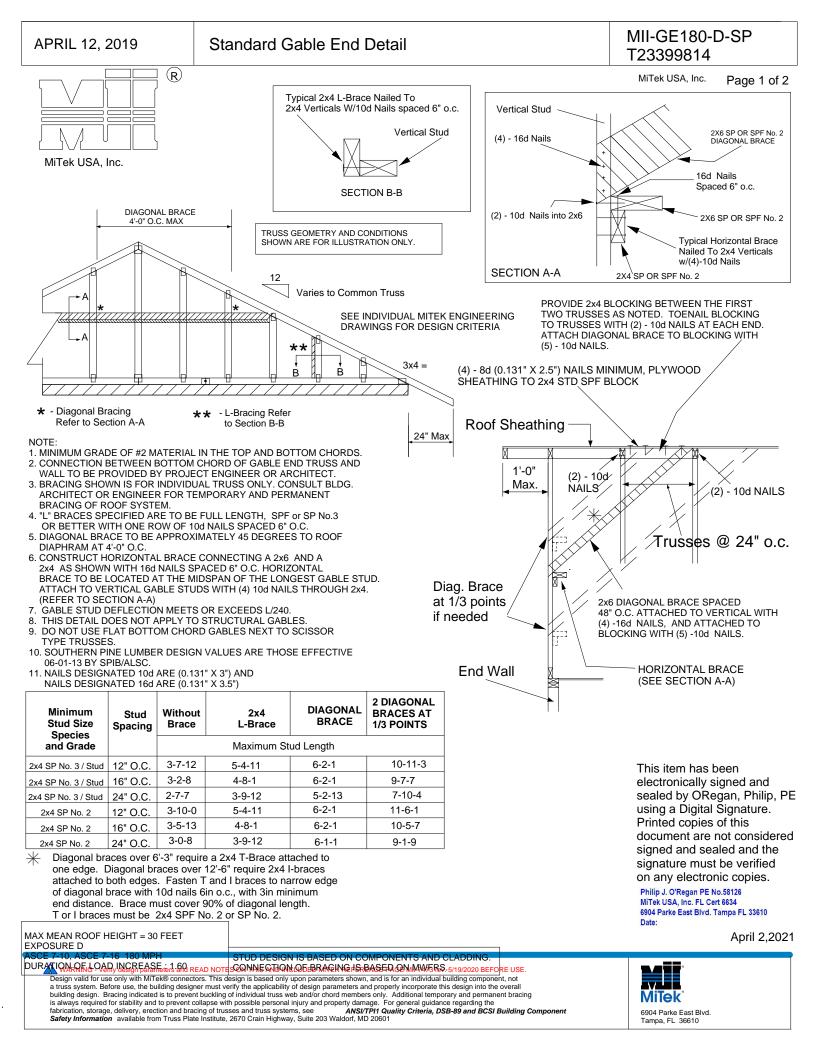


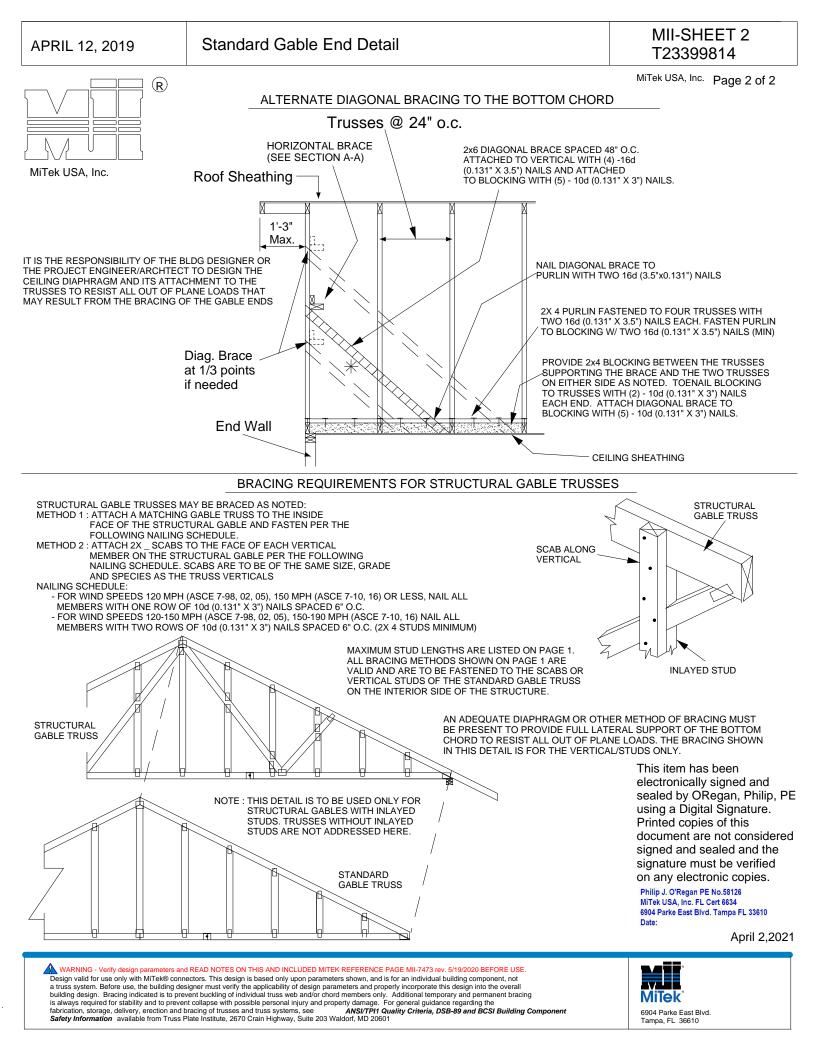


6904 Parke East Blvd Tampa, FL 36610









January 8, 2019

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

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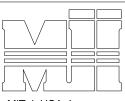
MII-PIGGY-7-16 T23399815

Page 1 of 1

MiTek USA, Inc.

D

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E





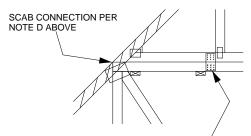
A - PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.

 (\mathbf{R})

- A PIGGBACK TRUSS, REFER TO WITEK TRUSS DESIGN DRAWING.
 SHALL BE CONNECTED TO EACH PURLIN WITH (2) (0.131" X 3.5") TOE-NAILED.
 B BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
 C PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C. UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING.
 CONNECT TO PAGE TRUGGE TRUGGINITH (0.4 1041 VG EWAND E GADILICATION)
- UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0:131" X 3.5") NAILS EACH. 2 X __ X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" O.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIPECTIONS AND: D-2X DIRECTIONS AND:
- 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 180 MPH WITH A MAXIMUM
- PIGGYBACK SPAN OF 12 ft. FOR WIND SPEEDS BETWEEN 116 AND 180 MPH, ATTACH MITEK NP37 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 72" O.C. W/ (4) (0.131" X 1.5") NAILS PER MEMBER. STAGGER NAILS FROM OPPOSING FACES. ENSURE 0.5" NAIL EDGE DISTANCE. (MIN. 2 PAIRS OF PLATES REQ. REGARDLESS OF SPAN)

WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH Nail-On PLATES AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



FOR ALL WIND SPEEDS, ATTACH MITEK NP37 20 GA Nail-On PLATES TO EACH FACE OF TRUSSES AT 48" O.C. W/ (4) (0.131" X 1.5") PER MEMBER. STAGGER NAILS FROM OPPOSING FACES ENSURE 0.5" NAIL EDGE DISTANCE.

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK

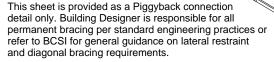
FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS 1) MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL
- 2) ATTACH 2 x _ x 4'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH
- VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM 3) CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.
 CONCENTRATED LOAD MUST BE APPLIED TO BOTH

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses sand truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

EXPOSURE B or C ENCLOSED BUILDING LOADING = 5 PSF TCDL ASCE 7-10, ASCE 7-16 **DURATION OF LOAD INCREASE : 1.60** DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED. Е

MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING



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This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date

April 2,2021



JANUARY 8, 2019

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

MII-PIGGY-ALT-7-16 T23399816 Page 1 of 1 MiTek USA, Inc.

MAXIMUM WIND SPEED = REFER TO NOTES D AND OR E

MAX MEAN ROOF HEIGHT = 30 FEET MAX TRUSS SPACING = 24 " O.C. CATEGORY II BUILDING

LOADING = 5 PSF TCDL MINIMUM ASCE 7-10, ASCE 7-16

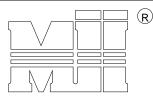
DURATION OF LOAD INCREASE : 1.60 DETAIL IS NOT APPLICABLE FOR TRUSSES TRANSFERING DRAG LOADS (SHEAR TRUSSES). ADDITIONAL CONSIDERATIONS BY BUILDING ENGINEER/DESIGNER ARE REQUIRED.

EXPOSURE B or C ENCLOSED BUILDING

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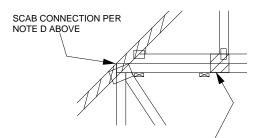




- A PIGGBACK TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
- BIGBACK TRUSS, REFER TO WITEK TRUSS DESIGN DRAWING.
 SHALL BE CONNECTED TO EACH PURLIN
 WITH (2) 0(0.131" X 3.5") TOE-NAILED.
 BASE TRUSS, REFER TO MITEK TRUSS DESIGN DRAWING.
 PURLINS AT EACH BASE TRUSS JOINT AND A MAXIMUM 24" O.C.
 UNLESS SPECIFIED CLOSER ON MITEK TRUSS DESIGN DRAWING. C
- ONLESS SPECIFIED CLOSER ON MITTER TRUSS DESIGN DRAWING. CONNECT TO BASE TRUSS WITH (2) (0.131" X 3.5") NAILS EACH. 2 X __ X 4'-0" SCAB, SIZE TO MATCH TOP CHORD OF PIGGYBACK TRUSS, MIN GRADE #2, ATTACHED TO ONE FACE, CENTERED ON INTERSECTION, WITH (2) ROWS OF (0.131" X 3") NAILS @ 4" 0.C. SCAB MAY BE OMITTED PROVIDED THE TOP CHORD SHEATHING _____ D - 2 X IS CONTINUOUS OVER INTERSECTION AT LEAST 1 FT. IN BOTH DIRECTIONS AND:
- 1. WIND SPEED OF 115 MPH OR LESS FOR ANY PIGGYBACK SPAN, OR 2. WIND SPEED OF 116 MPH TO 180 MPH WITH A MAXIMUM
- PIGGYBACK SPAN OF 12 ft. E FOR WIND SPEED IN THE RANGE 116 MPH 180 MPH ADD 9" x 9" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 48" O.C. OR LESS. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

WHEN NO GAP BETWEEN PIGGYBACK AND BASE TRUSS EXISTS:

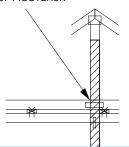
REPLACE TOE NAILING OF PIGGYBACK TRUSS TO PURLINS WITH PLYWOOD GUSSETS AS SHOWN, AND INSTALL PURLINS TO BOTTOM EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS MITEK DESIGN DRAWING.



7" x 7" x 1/2" PLYWOOD (or 7/16" OSB) GUSSET EACH SIDE AT 24" O.C. ATTACH WITH 3 - 6d (0.113" X 2") NAILS INTO EACH CHORD FROM EACH SIDE (TOTAL - 12 NAILS)

This sheet is provided as a Piggyback connection detail only. Building Designer is responsible for all permanent bracing per standard engineering practices or refer to BCSI for general guidance on lateral restraint and diagonal bracing requirements.

VERTICAL WEB TO EXTEND THROUGH BOTTOM CHORD OF PIGGYBACK



FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:

- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL
- ATTACH 2 x ___ x 4-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 4" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH ATTACH 2 x 2) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- THIS CONNECTION IS ONLY VALID FOR A MAXIMUM 3) CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS. 4) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS,
- NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS. 5) CONCENTRATED LOAD MUST BE APPLIED TO BOTH E PIGGYBACK AND THE BASE TRUSS DESIGN

🗼 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-7473 rev. 5/19/2/02/ BEFORE USE. Design valid for use only with MITER® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601 This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date

April 2,2021

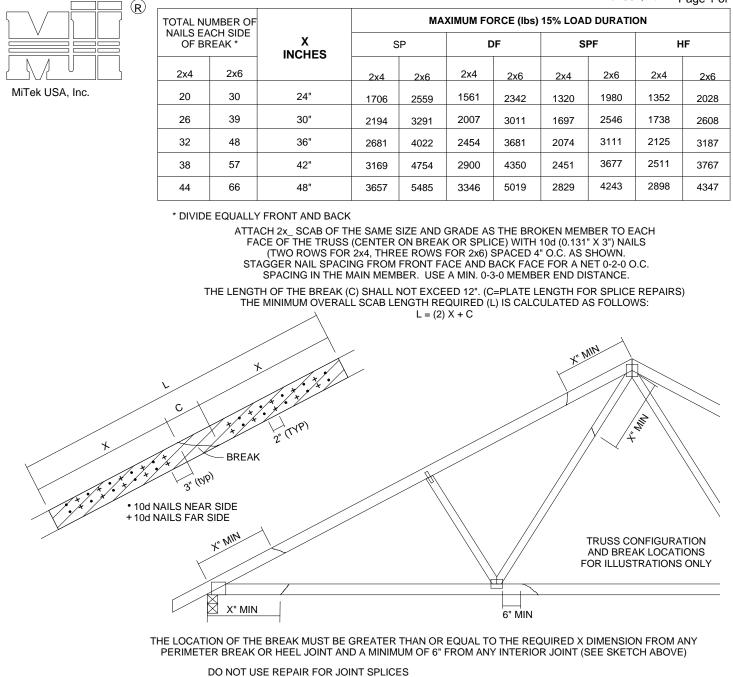


AUGUST 1, 2016

STANDARD REPAIR DETAIL FOR BROKEN CHORDS, WEBS AND DAMAGED OR MISSING CHORD SPLICE PLATES

MII-REP01A1 T23399817

MiTek USA, Inc. Page 1 of 1



NOTES 1.

- THIS REPAIR DETAIL IS TO BE USED ONLY FOR THE APPLICATION SHOWN. THIS REPAIR DOES NOT IMPLY THAT THE REMAINING PORTION OF THE TRUSS IS UNDAMAGED. THE ENTIRE TRUSS SHALL BE INSPECTED TO VERIFY THAT NO FURTHER REPAIRS ARE REQUIRED. WHEN THE REQUIRED REPAIRS ARE PROPERLY APPLIED, THE TRUSS WILL BE CAPABLE OF SUPPORTING THE LOADS INDICATED.
- ALL MEMBERS MUST BE RETURNED TO THEIR ORIGINAL POSITIONS BEFORE APPLING REPAIR
- AND HELD IN PLACE DURING APPLICATION OF REPAIR
- THE END DISTANCE, EDGE DISTANCE AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD. 3
- WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES. THIS REPAIR IS TO BE USED FOR SINGLE PLY TRUSSES IN THE 2X_ ORIENTATION ONLY.
- 5
- 6. THIS REPAIR IS LIMITED TO TRUSSES WITH NO MORE THAN THREE BROKEN MEMBERS.

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April 2,2021



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MAY 7, 2019

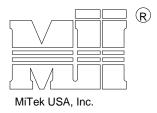
LATERAL TOE-NAIL DETAIL

MII-TOENAIL SP T23399818

Tampa, FL 36610

Page 1 of 1

MiTek USA. Inc.



- NOTES:
- 1. TOE-NAILS SHALL BE DRIVEN AT AN ANGLE OF 45 DEGREES WITH THE MEMBER AND MUST HAVE FULL WOOD SUPPORT. (NAIL MUST BE DRIVEN THROUGH AND EXIT AT THE BACK CORNER OF THE MEMBER END AS SHOWN.
- 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- 3. ALLOWABLE VALUE SHALL BE THE LESSER VALUE OF THE TWO SPECIES FOR MEMBERS OF DIFFERENT SPECIES.

	TOE-NAIL	SINGLE S	HEAR VAL	UES PER	NDS 2018	(lb/nail)
	DIAM.	SP	DF	HF	SPF	SPF-S
G	.131	88.0	80.6	69.9	68.4	59.7
LONG	.135	93.5	85.6	74.2	72.6	63.4
.5" L	.162	108.8	99.6	86.4	84.5	73.8
ы.						
ġ	.128	74.2	67.9	58.9	57.6	50.3
LONG	.131	75.9	69.5	60.3	59.0	51.1
3.25"	.148	81.4	74.5	64.6	63.2	52.5
ŝ						

VALUES SHOWN ARE CAPACITY PER TOE-NAIL

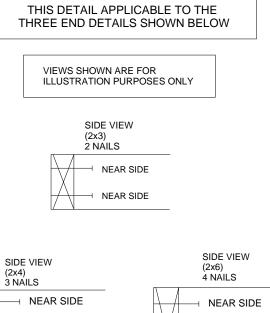
APPLICABLE DURATION OF LOAD INCREASES MAY BE APPLIED.

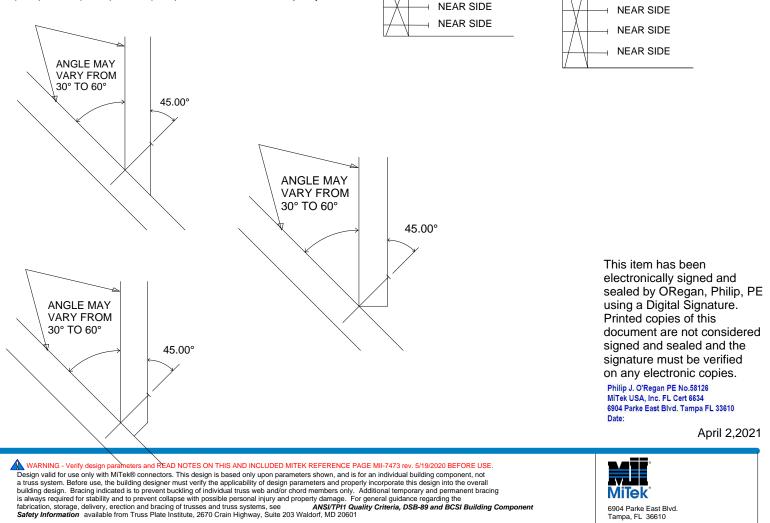
EXAMPLE:

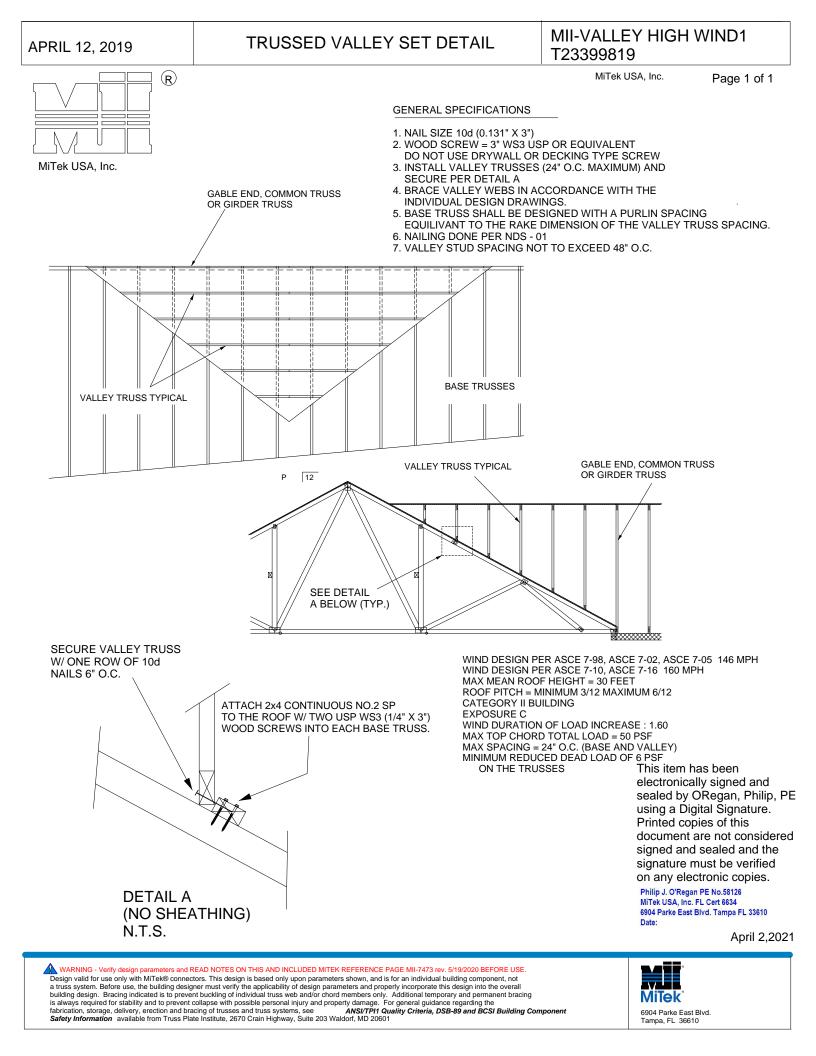
(3) - 16d (0.162" X 3.5") NAILS WITH SPF SPECIES BOTTOM CHORD

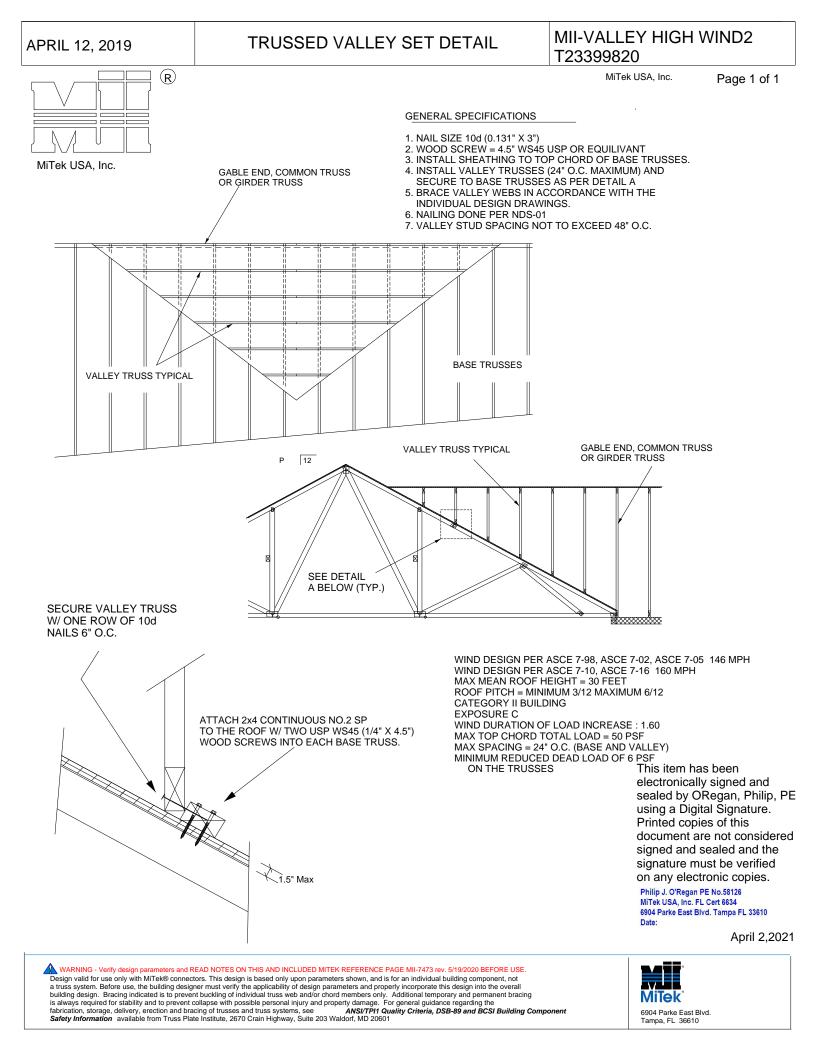
For load duration increase of 1.15:

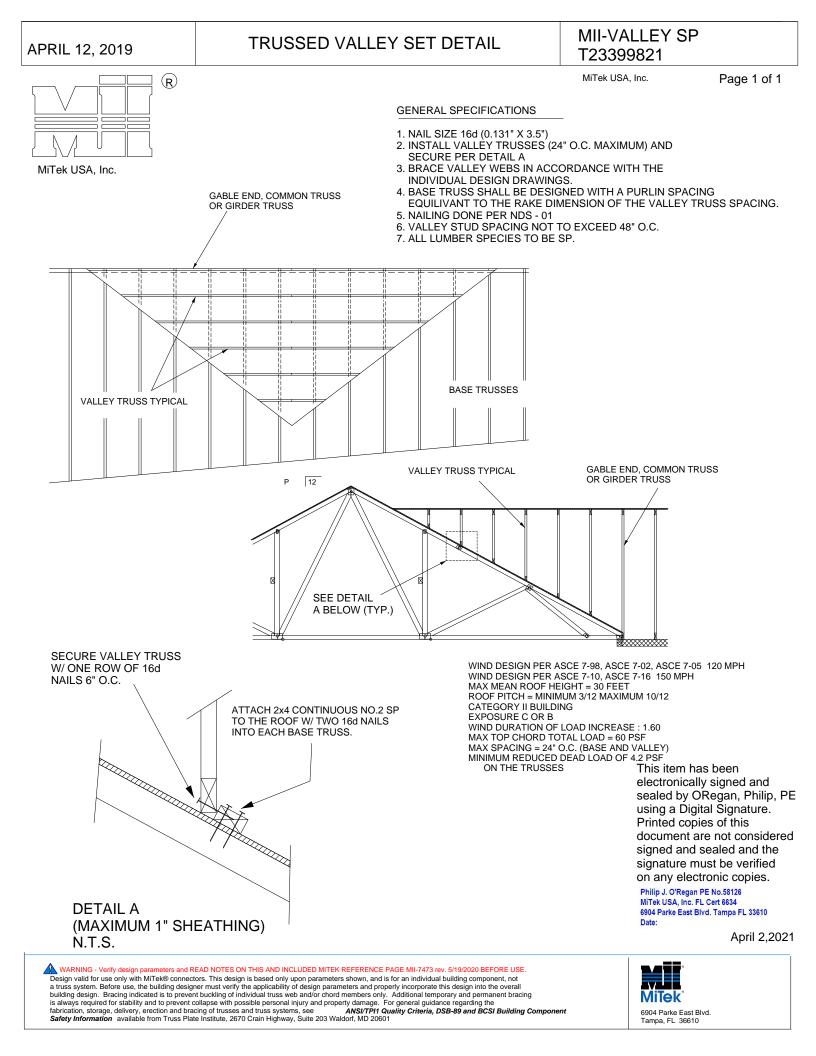
3 (nails) X 84.5 (lb/nail) X 1.15 (DOL) = 291.5 lb Maximum Capacity

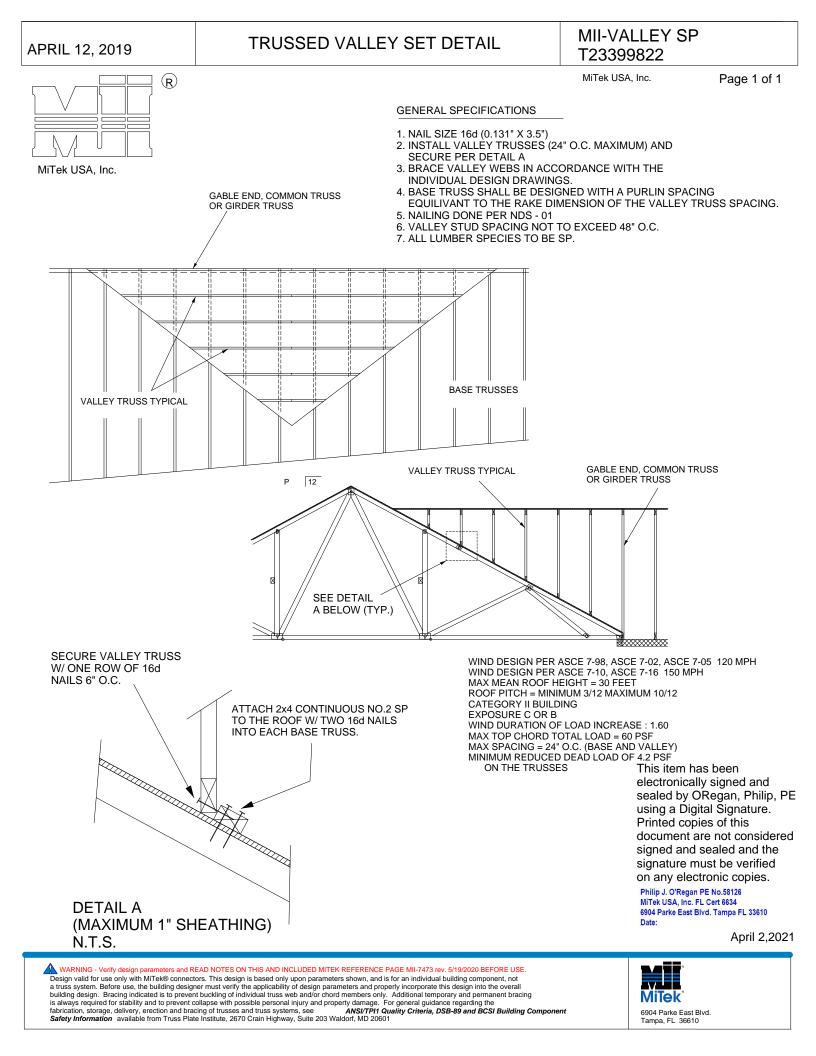


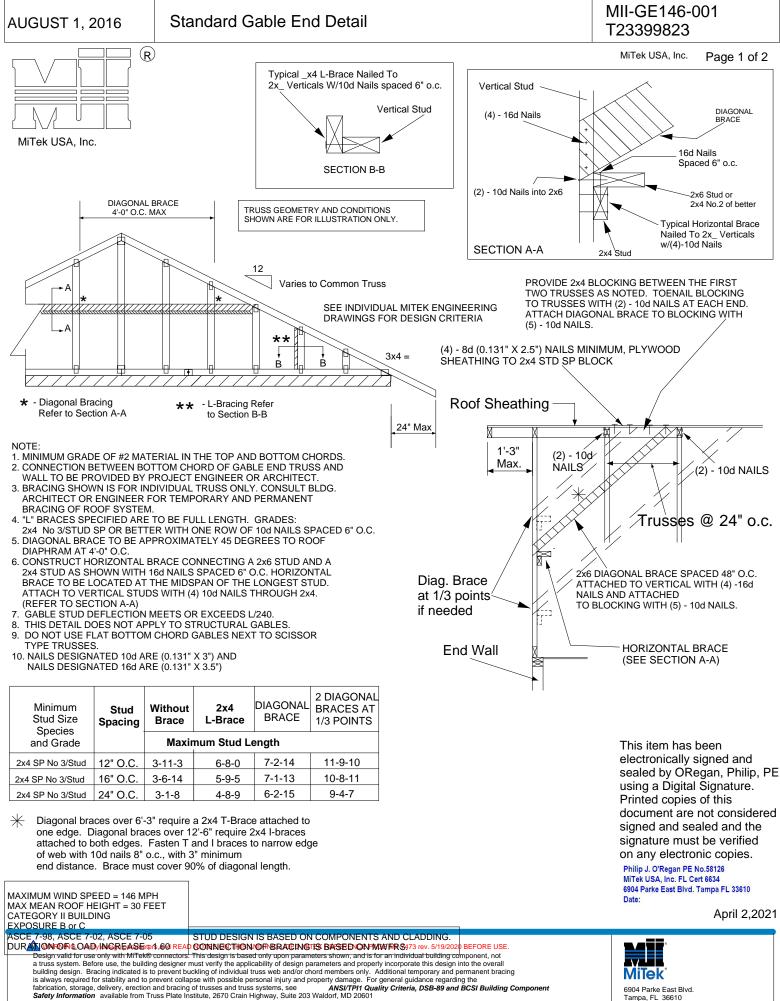




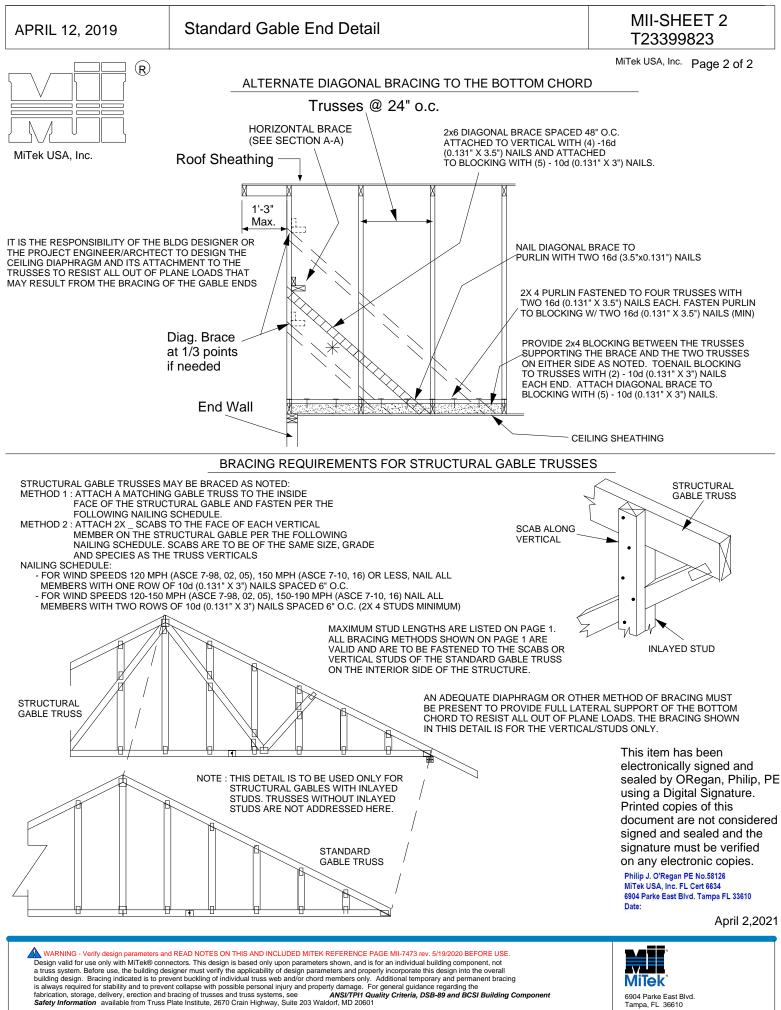




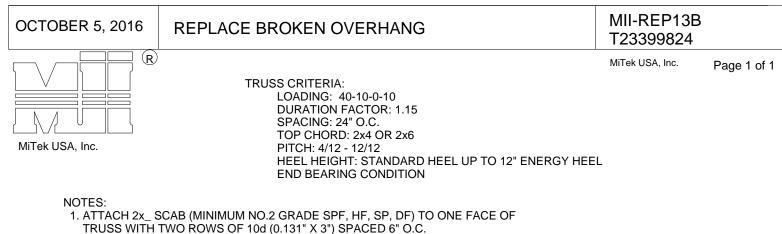




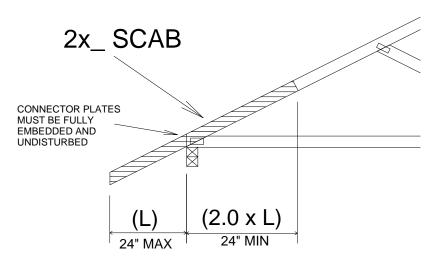
6904 Parke East Blvd Tampa, FL 36610



6904 Parke East Blvd Tampa, FL 36610



- 2. THE END DISTANCE, EDGE DISTANCE, AND SPACING OF NAILS SHALL BE SUCH AS TO AVOID UNUSUAL SPLITTING OF THE WOOD.
- 3. WHEN NAILING THE SCABS, THE USE OF A BACKUP WEIGHT IS RECOMMENDED TO AVOID LOOSENING OF THE CONNECTOR PLATES AT THE JOINTS OR SPLICES.



IMPORTANT

This detail to be used only with trusses (spans less than 40') spaced 24" o.c. maximum and having pitches between 4/12 and 12/12 and total top chord loads not exceeding 50 psf. Trusses not fitting these criteria should be examined individually.

REFER TO INDIVIDUAL TRUSS DESIGN FOR PLATE SIZES AND LUMBER GRADES This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021



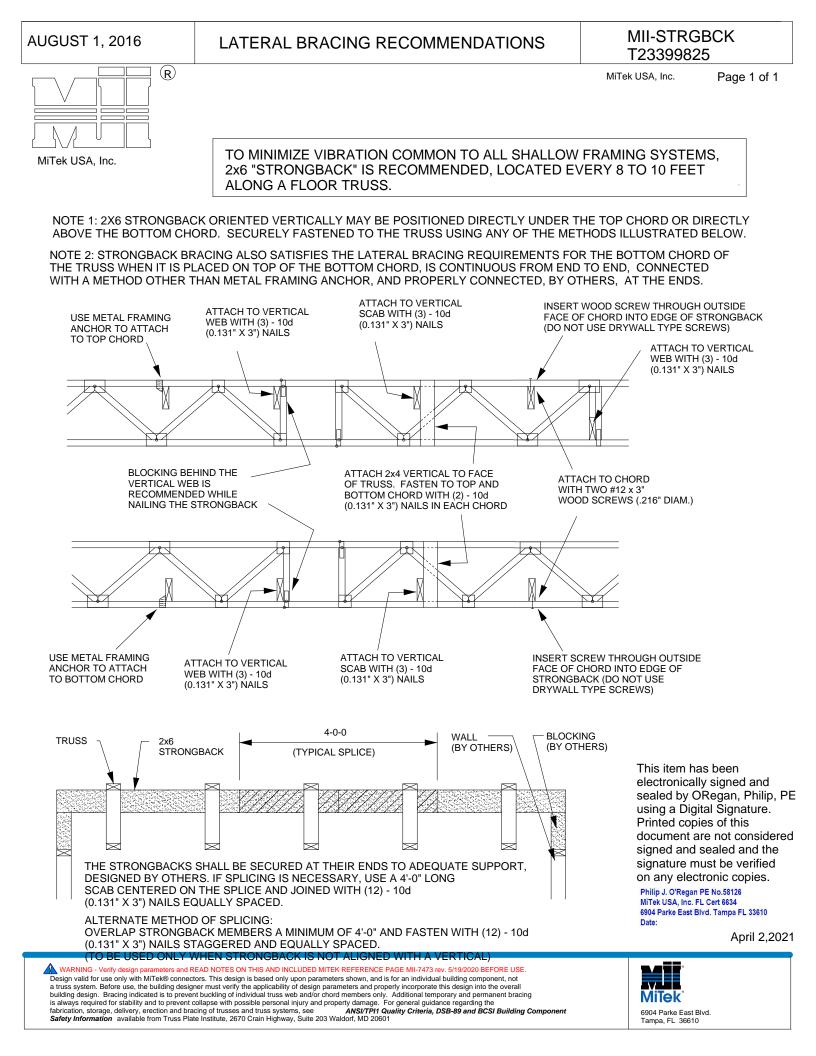
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

April 2,2021











RE: 3056583 - AMIRA BLDRS - WELSH RES.

Site Information:

Customer Info: Amira Bldrs. Project Name: Welsh Res. Model: Custom Lot/Block: N/A Subdivision: N/A Address: TBD, N/A City: Alachua Cty State: FL

Name Address and License # of Structural Engineer of Record, If there is one, for the building. Name: License #: Address:

City:

State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: FBC2020/TPI2014 Wind Code: ASCE 7-16 Roof Load: 37.0 psf Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 18 individual, Truss Design Drawings and 0 Additional Drawings. With my seal affixed to this sheet, I hereby certify that I am the Truss Design Engineer and this index sheet conforms to 61G15-31.003, section 5 of the Florida Board of Professional Engineers Rules.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	T26650739	T01	1/27/22	15	T26650753	V05	1/27/22
2	T26650740	T01G	1/27/22	16	T26650754	V06	1/27/22
3	T26650741	T02G	1/27/22	17	T26650755	V07	1/27/22
4	T26650742	T03	1/27/22	18	T26650756	V08	1/27/22
5 6	T26650743	T03G	1/27/22				
6	T26650744	T04G	1/27/22				
7	T26650745	T05	1/27/22				
8 9	T26650746	T05G	1/27/22				
	T26650747	T06	1/27/22				
10	T26650748	T06G	1/27/22				
11	T26650749	V01	1/27/22				
12	T26650750	V02	1/27/22				
13	T26650751	V03	1/27/22				
14	T26650752	V04	1/27/22				

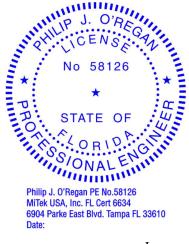
This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies

The truss drawing(s) referenced above have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Builders FirstSource-Lake City, FL.

Truss Design Engineer's Name: ORegan, Philip

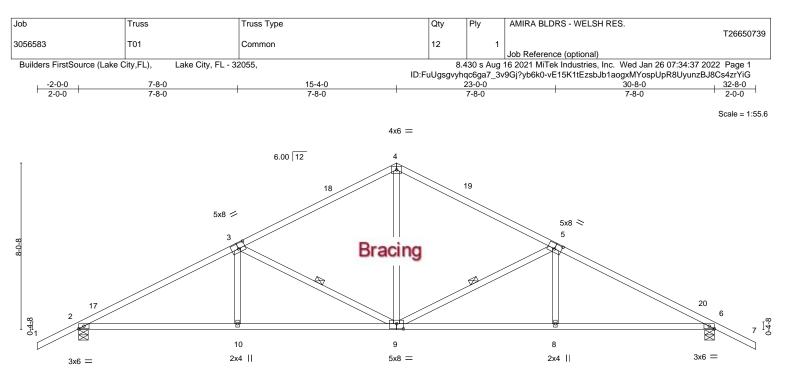
My license renewal date for the state of Florida is February 28, 2023.

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



January 27,2022

MiTek USA, Inc. 6904 Parke East Blvd. Tampa, FL 33610-4115



	7-8-0 7-8-0	<u> </u>	23-0-0	<u> </u>
Plate Offsets (X,Y)	[3:0-4-0,0-3-0], [5:0-4-0,0-3-0]	[6:0-2-15,Edge], [9:0-4-0,0-3-0]		
LOADING(psf)TCLL20.0TCDL7.0BCLL0.0BCDL10.0	Plate Grip DOL 1 Lumber DOL 1	0-0 CSI. 25 TC 0.64 25 BC 0.67 ES WB 0.29 4 Matrix-MS	DEFL. in (loc) l/defl L/d Vert(LL) 0.11 10-13 >999 240 Vert(CT) -0.22 8-9 >999 180 Horz(CT) 0.08 6 n/a n/a	PLATES GRIP MT20 244/190 Weight: 150 lb FT = 20%
LUMBER- TOP CHORD 2x4 SI BOT CHORD 2x4 SI WEBS 2x4 SI	P No.2		BOT CHORD Rigid ceiling directly applied	lirectly applied or 3-6-8 oc purlins. I or 6-10-9 oc bracing. 5-9, 3-9

REACTIONS. (size) 2=0-5-8, 6=0-5-8 Max Horz 2=186(LC 16) Max Uplift 2=-509(LC 12), 6=-509(LC 13) Max Grav 2=1243(LC 1), 6=1243(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2030/741, 3-4=-1378/579, 4-5=-1378/579, 5-6=-2030/741

BOT CHORD 2-10=-702/1749, 9-10=-702/1750, 8-9=-525/1750, 6-8=-525/1749

WEBS 4-9=-246/766, 5-9=-698/464, 5-8=0/317, 3-9=-698/463, 3-10=0/317

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 15-4-0, Exterior(2R) 15-4-0 to 18-4-0, Interior(1) 18-4-0 to 32-8-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=509, 6=509.

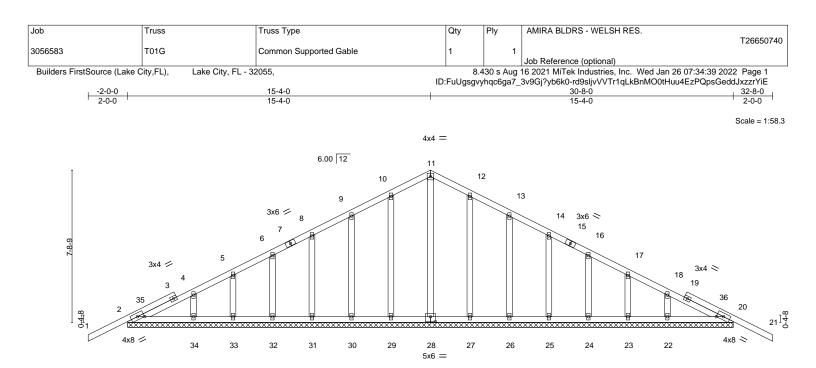
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January 27,2022



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.OADING (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.25	TC	0.34	Vert(LL)	-0.02	21	n/r	120	MT20	244/190
TCDL 7.0	Lumber DOL	1.25	BC	0.07	Vert(CT)	-0.02	21	n/r	120		
BCLL 0.0 *	Rep Stress Incr	YES	WB	0.19	Horz(CT)	0.01	20	n/a	n/a		
3CDL 10.0	Code FBC2020/T	PI2014	Matrix	<-S						Weight: 186 lb	FT = 20%

REACTIONS. All bearings 30-8-0.

(lb) - Max Horz 2=179(LC 16)

2x4 SP No.3

Max Uplift All uplift 100 lb or less at joint(s) 2, 29, 30, 31, 32, 33, 34, 27, 26, 25, 24, 23, 22 except 20=-109(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 28, 29, 30, 31, 32, 33, 34, 27, 26, 25, 24, 23, 22 except 2=256(LC 1), 20=256(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 9-10=-86/257, 10-11=-107/315, 11-12=-107/315, 12-13=-86/257

NOTES-

OTHERS

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Corner(3E) -2-0-0 to 1-0-0, Exterior(2N) 1-0-0 to 15-4-0, Corner(3R) 15-4-0 to 18-4-0, Exterior(2N) 18-4-0 to 32-8-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 29, 30, 31, 32, 33, 34, 27, 26, 25, 24, 23, 22 except (jt=lb) 20=109.

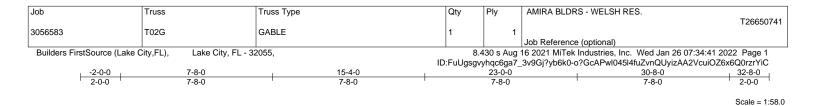
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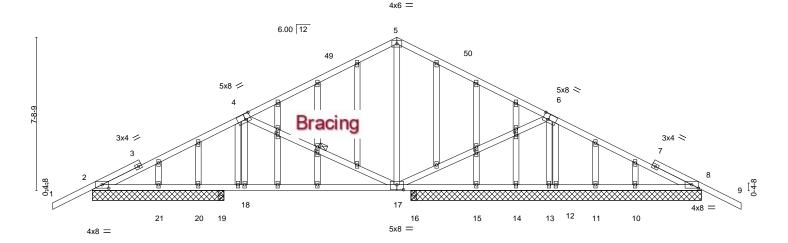
Philip J. O'Regan PE No.58126 MiTek USA, Inc. FL Cert 6634 6904 Parke East Blvd. Tampa FL 33610 Date:

January 27,2022



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	6-4-0	7-8-0	15-4-0	16-4-0	23-0-0	30-8-0	
	6-4-0	' 1-4-0 '	7-8-0	'1-0-0'	6-8-0	7-8-0	1
Plate Offsets (X,Y)	[2:0-4-0,0-2-1], [4:0-3-4,0)-3-4], [6:0-3-4,0-	3-4], [8:0-4-0,0-2-1], [17:	0-4-0,0-3-0], [24:0-1-1	4,0-1-0], [27:0-1-14,0-1-0	0], [37:0-1-14,0-1-0], [39:0-1-14,0-1	-0]
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc) l/defl L/d	d PLATES GR	IP
TCLL 20.0	Plate Grip DOL	1.25	TC 0.62	Vert(LL) -0.1	0 17-18 >999 240	0 MT20 244	1/190
TCDL 7.0	Lumber DOL	1.25	BC 0.66	Vert(CT) -0.2	2 17-18 >535 180	0	
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.25	Horz(CT) 0.0)1 16 n/a n/a	a	
BCDL 10.0	Code FBC2020/T	PI2014	Matrix-MS			Weight: 216 lb F	T = 20%
LUMBER-		1	1	BRACING-			
TOP CHORD 2x4 S	SP No.2			TOP CHORD	Structural wood sheat	hing directly applied or 6-0-0 oc pur	lins.
BOT CHORD 2x4 S	SP No.2			BOT CHORD		pplied or 6-0-0 oc bracing.	
WEBS 2x4 S	SP No.3			WEBS	1 Row at midpt	4-17	
OTHERS 2x4 S	SP No.3						

REACTIONS. All bearings 14-7-8 except (jt=length) 2=6-7-8, 20=6-7-8, 21=6-7-8, 19=0-3-8, 16=0-3-8, 2=6-7-8.

(lb) -Max Horz 2=179(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 15, 14, 12, 11, 10 except 2=-205(LC 12), 13=-400(LC 13), 8=-117(LC 13), 20=-373(LC 1), 21=-134(LC 12), 19=-323(LC 12), 16=-133(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 8, 20, 15, 14, 12, 11, 10, 16, 8 except 2=504(LC 1),

13=836(LC 1), 21=285(LC 1), 19=547(LC 1), 2=504(LC 1)

- FORCES. (lb) Max. Comp./Max. Ten. All forces 250 (lb) or less except when shown.
- TOP CHORD 2-4=-696/235, 4-5=-402/215, 5-6=-402/222, 6-8=-103/308
- BOT CHORD 2-21=-215/579, 20-21=-215/579, 19-20=-215/579, 18-19=-215/579, 17-18=-214/578
- WEBS 6-17=-135/518, 6-13=-876/432, 4-17=-369/267

NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl.,
- GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-1-11. Interior(1) 1-1-11 to 15-4-0. Exterior(2R) 15-4-0 to 18-4-0, Interior(1) 18-4-0 to 32-8-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 14, 12, 11, 10 except (jt=lb) 2=205, 13=400, 8=117, 20=373, 21=134, 19=323, 16=133, 2=205, 8=117.

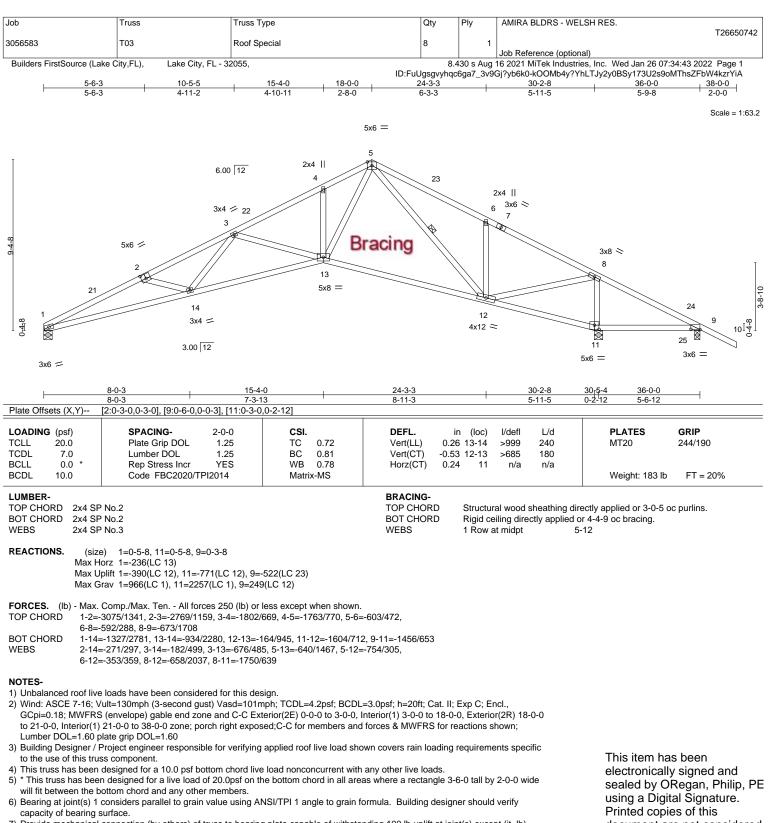
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January 27,2022



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to preven tbuckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP11 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=390, 11=771, 9=522.

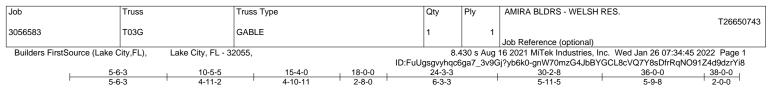
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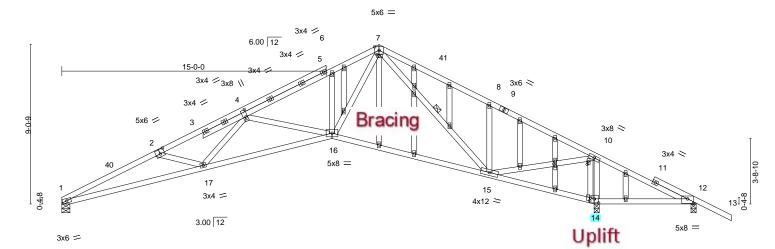
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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss system. See **MSIVTPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601



Scale = 1:65.4



24-3-3

8-11-3

30-2-8

5-11-5

36-0-0

5-9-8

Plate Offsets (X,Y)--[2:0-3-0,0-3-0], [4:0-5-0,0-1-4], [7:0-1-8,0-0-13], [10:0-2-0,0-0-8], [12:0-4-0,0-3-1], [14:0-3-0,0-2-12], [30:0-1-9,0-1-0], [31:0-2-0,0-0-8] LOADING (psf) SPACING-2-0-0 CSI. DEFL. in (loc) l/defl L/d PLATES GRIP TCLL 20.0 Plate Grip DOL 1.25 тс 0.62 Vert(LL) 0.25 16-17 >999 240 MT20 244/190 TCDL 7.0 Lumber DOL 1.25 BC 0.80 Vert(CT) -0.53 15-16 >687 180 WB BCLL 0.0 Rep Stress Incr YES 0.83 Horz(CT) 0.24 14 n/a n/a BCDL 10.0 Code FBC2020/TPI2014 Matrix-MS Weight: 231 lb FT = 20% LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 2-11-11 oc purlins. BOT CHORD 2x4 SP No 2 BOT CHORD Rigid ceiling directly applied or 4-4-8 oc bracing. 2x4 SP No 3 WFBS WFBS 1 Row at midpt 7-15 OTHERS 2x4 SP No.3 REACTIONS. (size) 1=0-5-8, 12=0-3-8, 14=0-3-8 Max Horz 1=-230(LC 17) Max Uplift 1=-401(LC 12), 12=-443(LC 23), 14=-716(LC 12) Max Grav 1=984(LC 1), 12=206(LC 12), 14=2152(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

 TOP CHORD
 1-2=-3120/1366, 2-4=-2882/1215, 4-6=-1982/741, 6-7=-1983/844, 7-8=-741/495, 8-10=-726/309, 10-12=-638/1680

 BOT CHORD
 1-17=-1341/2819, 16-17=-1014/2451, 15-16=-216/1058, 14-15=-1610/687, 12-14=-1456/627

 WEBS
 4-17=-144/453, 4-16=-643/477, 6-16=-254/211, 7-16=-674/1594, 7-15=-693/276, 8-15=-362/363, 10-15=-693/2171, 10-14=-1673/673

NOTES-

1) Unbalanced roof live loads have been considered for this design.

8-0-3

8-0-3

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 18-0-0, Exterior(2R) 18-0-0 to 21-0-0, Interior(1) 21-0-0 to 38-0-0 zone; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

15-4-0

7-3-13

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

9) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

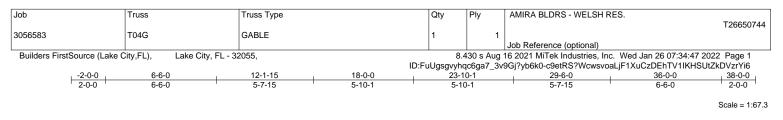
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=401, 12=443, 14=716.

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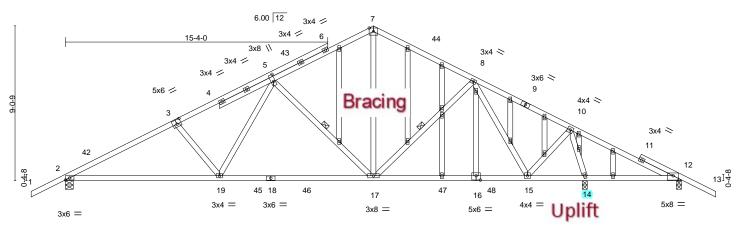
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January 27,2022





5x6 =



	9-0-0	<u>18-0-0</u> 9-0-0		<u>27-0-0</u> 9-0-0		<u>30-6-0 36-0</u> 3-6-0 5-6	
Plate Offsets (X,Y)	[3:0-3-0,0-3-0], [5:0-5-0,0-1-4], [12:0-4-0			9-0-0		3-0-0 3-0	-0
LOADING (psf) TCLL 20.0 TCDL 7.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code FBC2020/TPI2014	CSI. TC 0.49 BC 0.92 WB 0.61 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.19 15-17 >999 -0.34 15-17 >999 0.06 14 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 251 lb	GRIP 244/190 FT = 20%
		-	BRACING- TOP CHORI BOT CHORI WEBS		ectly applied o	ectly applied or 3-5-5 (or 2-2-0 oc bracing. -17, 5-17	oc purlins.
Max F Max L	te) 12=0-3-8, 2=0-5-8, 14=0-3-8 Horz 2=-208(LC 17) Jplift 12=-148(LC 10), 2=-504(LC 12), 14 Grav 12=90(LC 24), 2=1258(LC 2), 14=1	()					
TOP CHORD 2-3= 10-1	. Comp./Max. Ten All forces 250 (lb) or -2036/738, 3-5=-1888/708, 5-7=-1108/46 2=-397/933	31, 7-8=-1125/476, 8-10=-					
WEBS 7-17	=-729/1827, 17-19=-489/1426, 15-17=-1 =-233/683, 8-17=-32/276, 8-15=-710/335 =-177/586, 3-19=-256/257, 10-14=-1859	5, 10-15=-283/1154, 5-17=	=-631/437,				
2) Wind: ASCE 7-16; GCpi=0.18; MWFR	e loads have been considered for this de Vult=130mph (3-second gust) Vasd=101i S (envelope) gable end zone and C-C Ex) 21-0-0 to 38-0-0 zone; porch right expo	nph; TCDL=4.2psf; BCDL terior(2E) -2-0-0 to 1-0-0,	Interior(1) 1-0-0 to	18-0-0, Exterior(2R) 1	8-0-0		

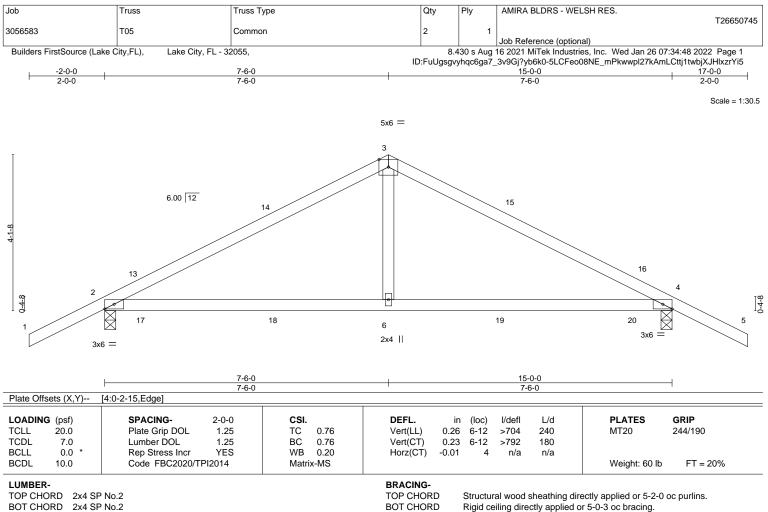
- to 21-0-0, Interior(1) 21-0-0 to 38-0-0 zone; porch right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=148, 2=504, 14=619.

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 BOT CHORD
 2x4 SP No.2

 WEBS
 2x4 SP No.3

 REACTIONS.
 (size)
 2=0-3-8, 4=0-3-8

Max Horz 2=-102(LC 13) Max Uplift 2=-289(LC 9), 4=-289(LC 8) Max Grav 2=663(LC 1), 4=663(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-758/1221, 3-4=-758/1222

BOT CHORD 2-6=-914/604, 4-6=-914/604

WEBS 3-6=-684/342

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 7-6-0, Exterior(2R) 7-6-0 to 10-6-0, Interior(1) 10-6-0 to 17-0-0 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

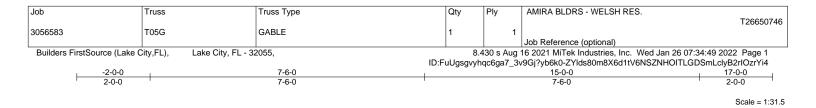
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=289, 4=289.

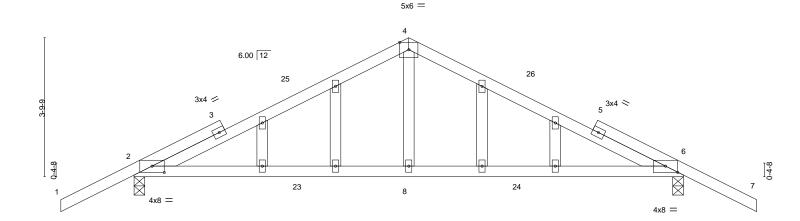
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		-6-0	15-0-0
Plate Offsets (X,Y) [2	2:0-4-0,0-2-1], [6:0-4-0,0-2-1]	-6-0	7-6-0
LOADING (psf) ICLL 20.0 ICDL 7.0 3CLL 0.0 3CDL 10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode FBC2020/TPI2014	CSI. TC 0.93 BC 0.73 WB 0.17 Matrix-MS	DEFL. in (loc) l/defl L/d Vert(LL) 0.28 8-22 >635 240 MT20 244/190 Vert(CT) 0.24 8-22 >724 180 MT20 244/190 Horz(CT) -0.02 2 n/a n/a Weight: 76 lb FT = 20%
Max Ho	No.2 No.3 No.3) 2=0-3-8, 6=0-3-8 rz 2=-95(LC 17)		BRACING- TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins. BOT CHORD Rigid ceiling directly applied or 4-10-0 oc bracing.
Max Gr ORCES. (Ib) - Max. C OP CHORD 2-4=-7 30T CHORD 2-8=-9	lift 2=-289(LC 12), 6=-289(LC 13) av 2=660(LC 1), 6=660(LC 1) Comp./Max. Ten All forces 250 (lb) o '60/1267, 4-6=-760/1267 '83/637, 6-8=-983/637 ;79/334	r less except when shown.	
2) Wind: ASCE 7-16; Vu GCpi=0.18; MWFRS to 10-6-0, Interior(1) 1 shown; Lumber DOL=	10-6-0 to 17-0-0 zone; porch left and ri =1.60 plate grip DOL=1.60	mph; TCDL=4.2psf; BCDL xterior(2E) -2-0-0 to 1-1-11 ght exposed;C-C for memb	=3.0psf; h=20ft; Cat. II; Exp C; Encl., Interior(1) 1-1-11 to 7-6-0, Exterior(2R) 7-6-0 ers and forces & MWFRS for reactions

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

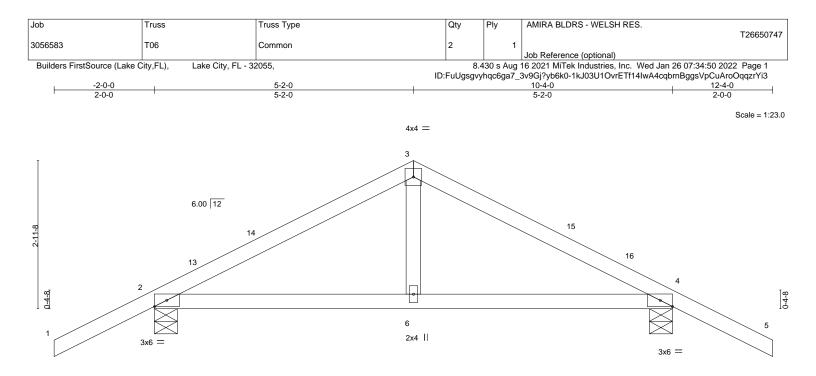
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=289, 6=289. This item has been electronically signed and sealed by ORegan, Philip, PE using a Digital Signature. Printed copies of this document are not considered signed and sealed and the signature must be verified on any electronic copies.

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TCDL 7.0 Lumber DOL 1.25 BC 0.27 Vert(CT) -0.03 6-12 >999 180 BCLL 0.0 * Rep Stress Incr YES WB 0.09 Horz(CT) 0.00 4 n/a n/a BCDL 10.0 Code FBC2020/TPI2014 Matrix-MS Weight: 43 lb F				10-4-0 5-2-0					-	5-2	0-2-15,Edge]	sets (X,Y) [4:0	Plate Offs
LUMBER-	FT = 20%	MT20	240 180	>999 >999	6-12 6-12	0.02 -0.03	Vert(LL) Vert(CT)	0.27 0.09	TC BC WB	1.25 1.25 YES	Plate Grip DOL Lumber DOL Rep Stress Incr	20.0 7.0 0.0 *	LOADING TCLL TCDL BCLL BCDL
TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc pur BOT CHORD 2x4 SP No.2 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS 2x4 SP No.3 2x4 SP No.3 Structural wood sheathing directly applied or 10-0-0 oc bracing.	purlins.										0.2	ORD 2x4 SP No ORD 2x4 SP No	TOP CHO BOT CHO

Max Horz 2=77(LC 16) Max Uplift 2=-221(LC 12), 4=-221(LC 13) Max Grav 2=490(LC 1), 4=490(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-483/323. 3-4=-483/323

2-6=-104/381, 4-6=-104/381

BOT CHORD

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) -2-0-0 to 1-0-0, Interior(1) 1-0-0 to 5-2-0, Exterior(2R) 5-2-0 to 8-2-0, Interior(1) 8-2-0 to 12-4-0 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

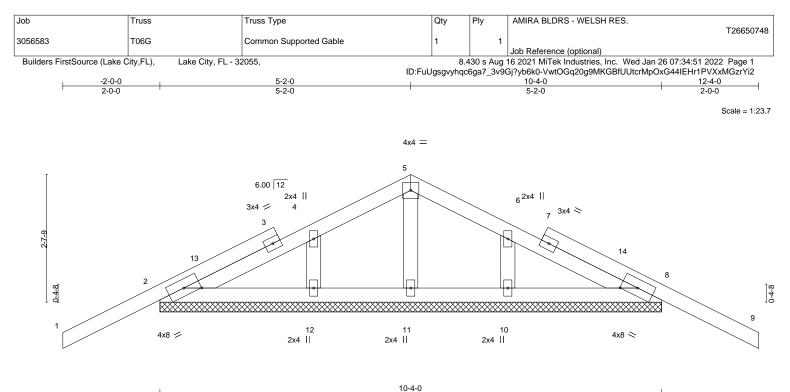
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=221. 4=221.

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OADING (psf) SPACING-	2-0-0	CSI.	DEFL. i	n (loc)	l/defl	L/d	PLATES	GRIP
CLL 20.0) Plate Grip DOI	L 1.25	TC 0.39	Vert(LL) -0.02	9	n/r	120	MT20	244/190
CDL 7.0) Lumber DOL	1.25	BC 0.06	Vert(CT) -0.03	9	n/r	120		
CLL 0.0) * Rep Stress Inc	r YES	WB 0.06	Horz(CT) 0.0	8 (n/a	n/a		
CDL 10.0	Code FBC202	20/TPI2014	Matrix-S					Weight: 52 lb	FT = 20%

TOP CHORD	2X4 3P INU.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

REACTIONS. All bearings 10-4-0.

(lb) -Max Horz 2=70(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 11, 12, 10 except 2=-139(LC 12), 8=-152(LC 13) Max Grav All reactions 250 lb or less at joint(s) 11, 12, 10 except 2=252(LC 1), 8=252(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

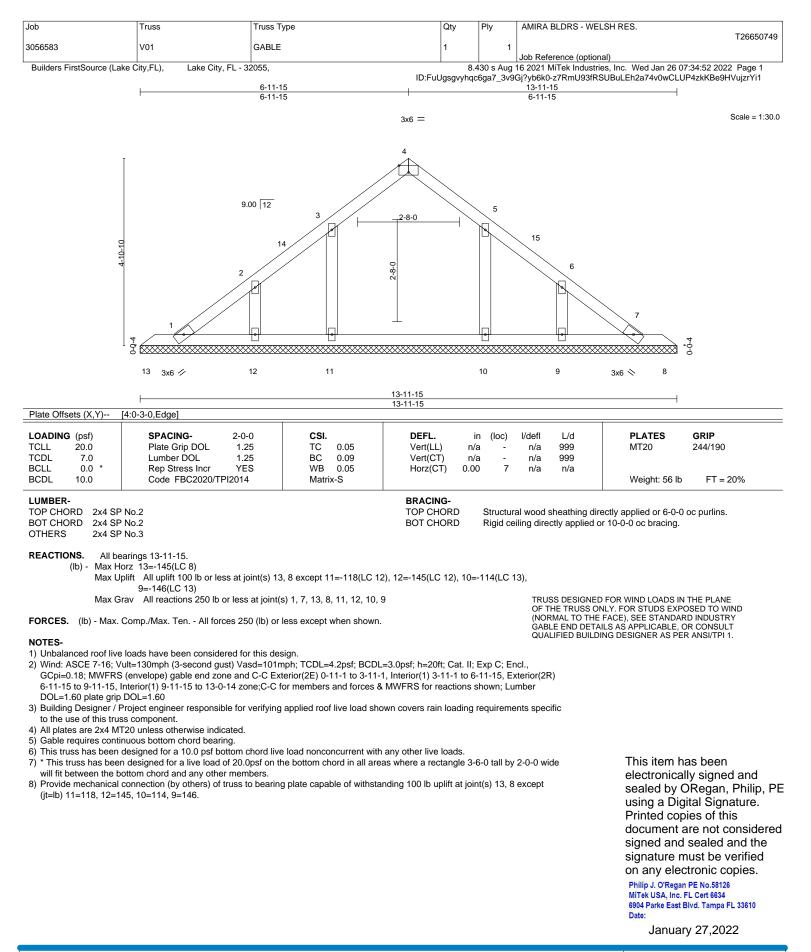
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl., GCpi=0.18; MWFRS (envelope) gable end zone and C-C Corner(3E) -2-0-0 to 1-0-0, Exterior(2N) 1-0-0 to 5-2-0, Corner(3R) 5-2-0 to 8-2-0, Exterior(2N) 8-2-0 to 12-4-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 12, 10 except (jt=lb) 2=139, 8=152.

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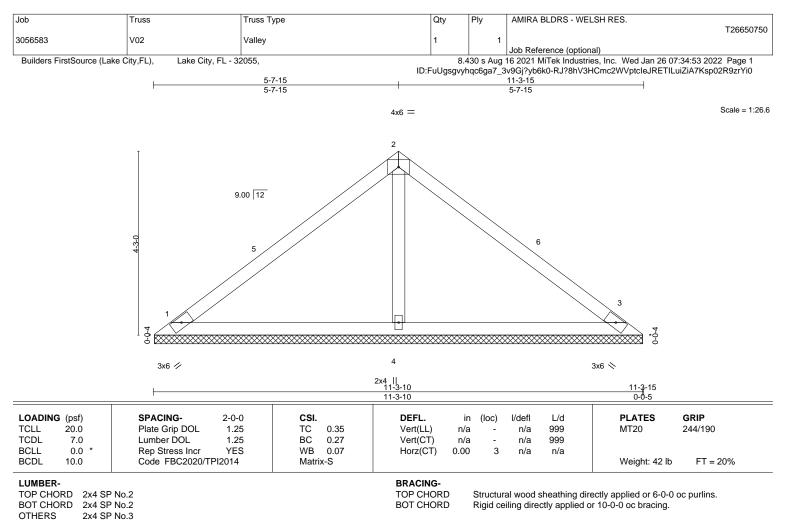
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REACTIONS. (size) 1=11-3-4, 3=11-3-4, 4=11-3-4

Max Horz 1=-125(LC 8)

Max Holz 1=-125(LC 8) Max Uplift 1=-88(LC 12), 3=-104(LC 13), 4=-107(LC 12) Max Grav 1=196(LC 1), 3=196(LC 1), 4=381(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-5-4 to 3-5-4, Interior(1) 3-5-4 to 5-7-15, Exterior(2R) 5-7-15 to 8-7-15, Interior(1) 8-7-15 to 10-10-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

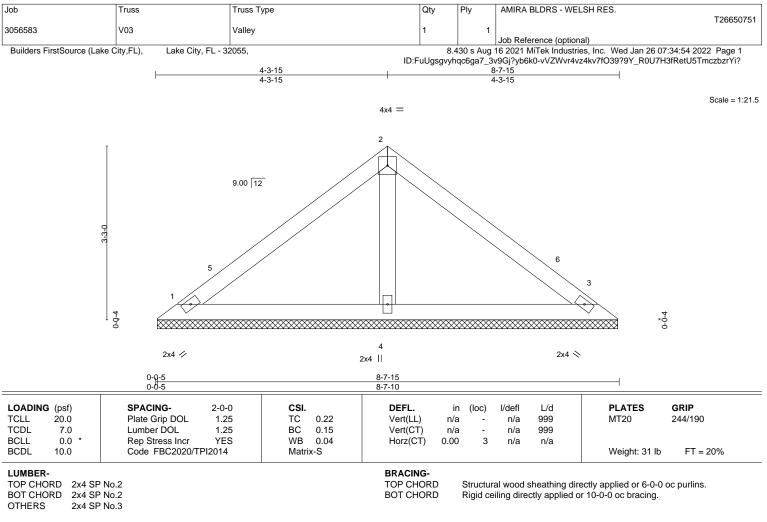
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 3=104, 4=107.

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January 27,2022





REACTIONS. (size) 1=8-7-4, 3=8-7-4, 4=8-7-4

Max Horz 1=-93(LC 8)

Max Uplift 1=-65(LC 12), 3=-77(LC 13), 4=-79(LC 12) Max Grav 1=146(LC 1), 3=146(LC 1), 4=284(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-5-4 to 3-5-4, Interior(1) 3-5-4 to 4-3-15, Exterior(2R) 4-3-15 to 7-3-15, Interior(1) 7-3-15 to 8-2-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

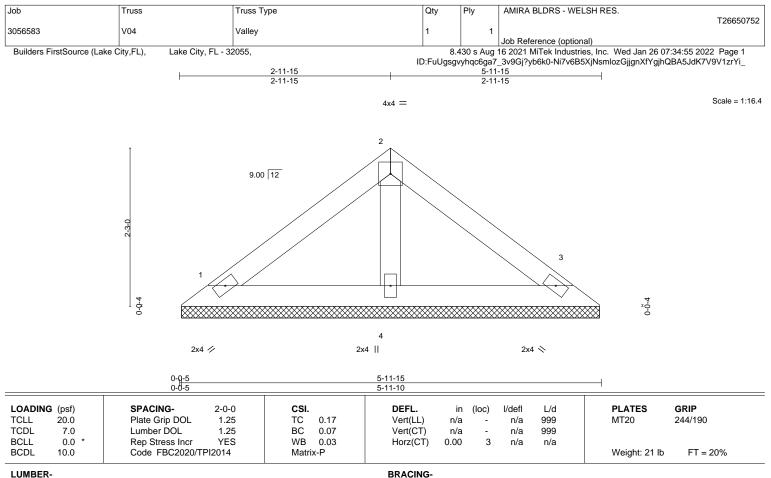
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

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TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 2x4 SP No.3 OTHERS

REACTIONS. (size) 1=5-11-4, 3=5-11-4, 4=5-11-4

Max Horz 1=61(LC 9) Max Uplift 1=-52(LC 12), 3=-60(LC 13), 4=-34(LC 12)

Max Grav 1=105(LC 1), 3=105(LC 1), 4=170(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

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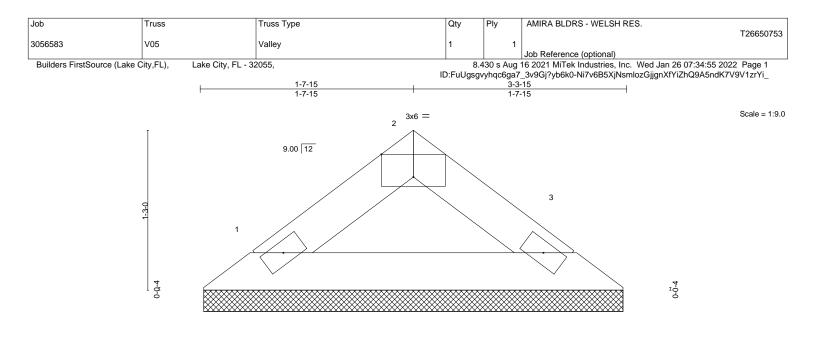
Structural wood sheathing directly applied or 5-11-15 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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2x4 1/

2x4 📎

Plate Offse	215 (A, T) [2	2:0-3-0,Edge]		1							1	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.25	TC	0.05	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	7.0	Lumber DOL	1.25	BC	0.07	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCDL	10.0	Code FBC2020/T	PI2014	Matri	x-P						Weight: 10 lb	FT = 20%
LUMBER-	I			1		BRACING-						
TOP CHOP	RD 2x4 SP I	No.2				TOP CHOR	D S	Structur	al wood :	sheathing dir	ectly applied or 3-3-15	oc purlins.
BOT CHOR	RD 2x4 SP I	No 2				BOT CHOR					r 10-0-0 oc bracing.	

REACTIONS. (size) 1=3-3-4, 3=3-3-4

Max Horz 1=29(LC 11) Max Uplift 1=-33(LC 12), 3=-33(LC 13)

Max Grav 1=91(LC 1), 3=91(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

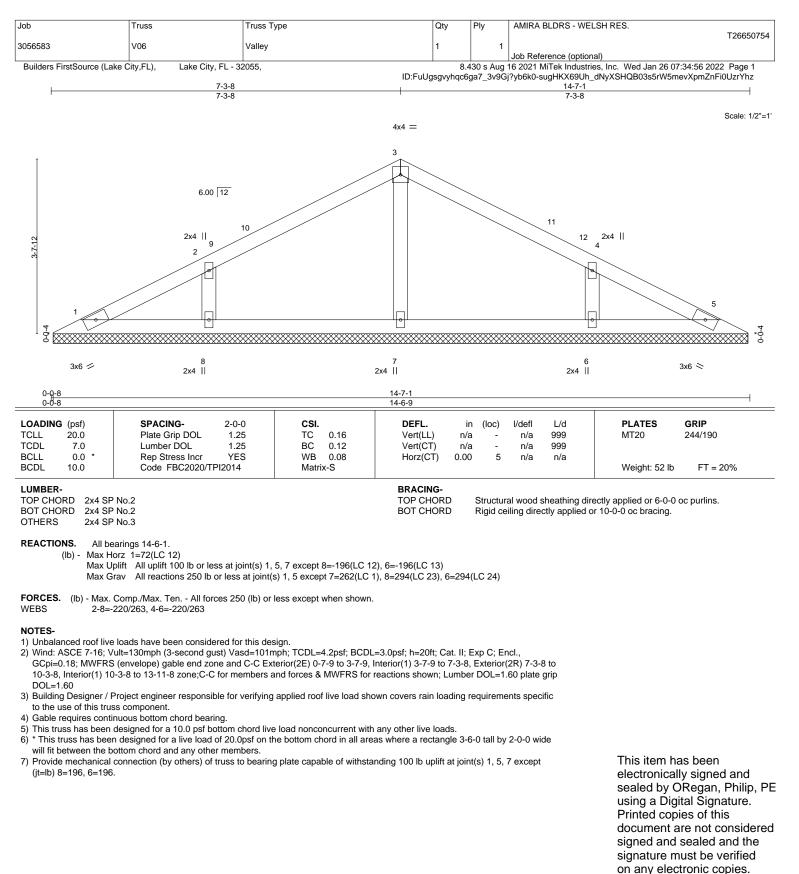
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

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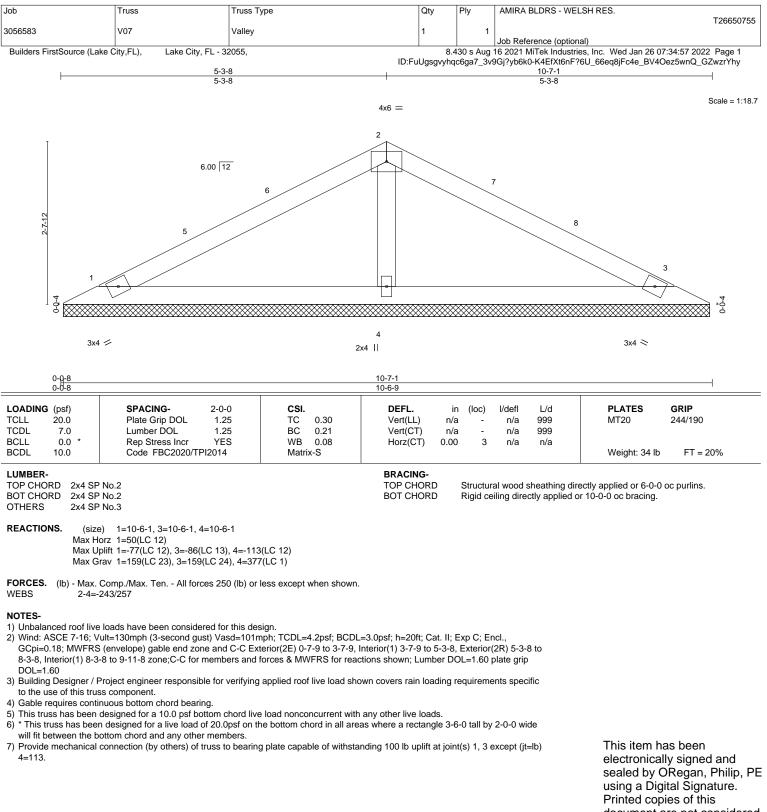




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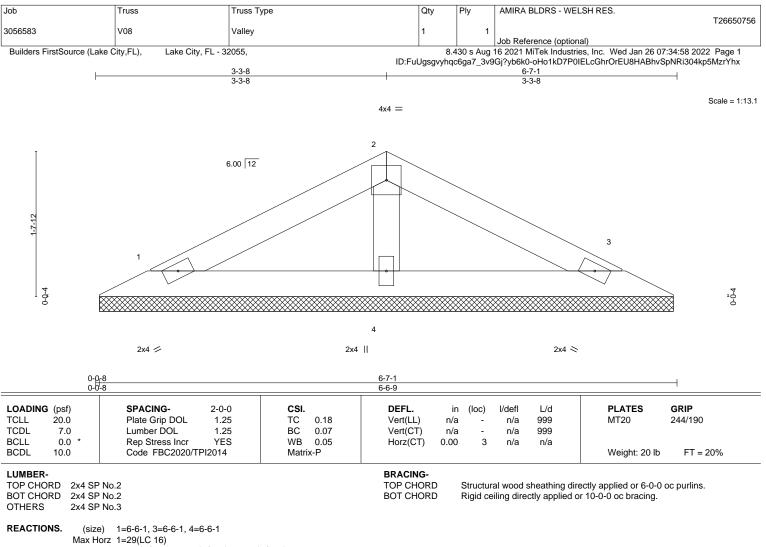


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Max Horz 1=29(LC 16) Max Uplift 1=-52(LC 12), 3=-57(LC 13), 4=-49(LC 12) Max Grav 1=99(LC 1), 3=99(LC 1), 4=196(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=101mph; TCDL=4.2psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Encl.,

GCpi=0.18; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Building Designer / Project engineer responsible for verifying applied roof live load shown covers rain loading requirements specific to the use of this truss component.

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