

DESIGN SPECIFICATIONS

DESIGN CODE:
2020 FLORIDA BUILDING CODE – RESIDENTIAL
DESIGN IS VOID ONE YEAR AFTER THE DATE OF THE ORIGINAL PLANS.
UNLESS PLANS HAVE BEEN REVIEWED FOR CODE COMPLIANCE.

DESIGN LOADS: ACTUAL AND UNIFORM

ROOF		FLOOR	
LOAD	PSF	LOAD	PSF
ROOF DEAD LOAD	(cd=1.25)	FLOOR DEAD LOAD	(cd=1.00)
TOP CHORD LIVE LOAD	20 psf	TOP CHORD DEAD LOAD	10 psf
TOP CHORD DEAD LOAD	7 psf (ARCH SHINGLES)	BOTTOM CHORD LIVE LOAD	10 psf
BOTTOM CHORD LIVE LOAD	20 psf (TILE SHINGLES)	BOTTOM CHORD DEAD LOAD	5 psf
DEFLECTION CRITERIA:			
ROOF FRAMING: LIVE LOAD /240 TOTAL LOAD /180		FLOOR FRAMING: LIVE LOAD /360 + TOTAL LOAD /240	
0.75" MAX ANY CASE			

WIND LOADING:
ASCE 7/16 FOR WIND UPLIFT, TRUSSES SHALL BE DESIGNED WITH A MIN. DEAD LOAD CONDITION OF 5 PSF TOP CHORD AND 3 PSF BOTTOM CHORD REACTIONS CALCULATED FOR THE BEARING POINTS OF ROOF TRUSSES SHALL BE REDUCED. SPECIFICALLY, ATTIC FLOOR LIVE LOADS COMBINED WITH ROOF LIVE LOADS SHALL BE MULTIPLIED BY 0.75 WHEN COMBINED W/ DEAD LOAD.

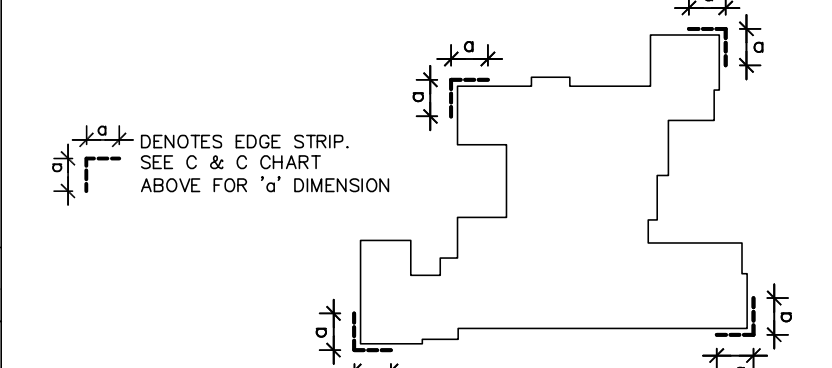
BASIC WIND SPEED (ASCE 7-16)	130 MPH
IMPORTANCE FACTOR	1.00
MEAN ROOF HEIGHT	20.0 FT
ROOF PITCH	7/12
BUILDING CATEGORY	II
EXPOSURE CATEGORY	ENCLOSED
INTERNAL PRESSURE COEFFICIENT	± .18

MATERIAL SPECIFICATIONS

HARDWARE AND ANCHORS:
ANCHOR BOLTS & THREADED ROD: SHALL BE IN ACCORDANCE WITH ASTM A307 OR ASTM F 1554 GRADE 36.
WASHERS: SHALL BE IN ACCORDANCE WITH ASTM A500 (GRADE B).
NUTS: SHALL BE IN ACCORDANCE WITH ASTM A 563 GRADE A HEX.
METAL CONNECTORS: ALL METAL CONNECTORS WHICH ARE EXPOSED TO EXTERIOR SHALL BE GALVANIZED.
REINFORCING BAR/ROD INSTALLATION: EMBEDMENT OF RODS OR REBAR DOWELS SHALL BE 12 BAR DIAMETER MINIMUM. HOLES SHALL BE 1/4" LARGER THAN REBAR SIX AND 1/8" LARGER THAN THREADED ROD SIZE. (U.O.N.)
ANCHORING ADHESIVE: SHALL BE ONE OF THE FOLLOWING PRODUCTS (DOAL CARTRIDGE INSTALLATION ONLY):
EPOXY: ITW RED HEAD A7
REINFORCING STEEL: SHALL BE ASTM A615, GRADE 60.
STRUCTURAL STEEL: SHALL BE ASTM A992, GRADE 50.
WELDED WIRE FABRIC (W/F): SHALL BE ASTM A185.
LAMINATED VENEER LUMBER (LVL): ALL LAMINATED VENEER LUMBER SHALL MEET OR EXCEED THE FOLLOWING DESIGN PROPERTIES – ELASTIC MODULUS (E):900ksi, BENDING STRESS (Fb) 2600psi

TRIBUTARY AREA (sf)	COMPONENTS & CLADDING ALLOWABLE DESIGN PRESSURES		GARAGE DOOR PRESSURES (PSF)
	INTERIOR (PSF)	EDGE STRIP (PSF): "d" = 4'-6"	
10	+25.6 –27.7	+25.6 –34.2	+22.9
50	+22.9 –25.0	+22.9 –28.8	+21.8
100	+21.8 –23.9	+21.8 –26.6	+23.9

- THE VALUES ABOVE ARE ALLOWABLE WIND PRESSURE VALUES (ASD). THE ABOVE WIND PRESSURES HAVE BEEN REDUCED BY 0.60 AS PERMITTED BY THE ALLOWABLE STRESS DESIGN METHODOLOGY. NO FURTHER REDUCTION SHALL BE PERMITTED.
- COMPONENT & CLADDING WALL ELEMENTS SHALL BE DESIGNED FOR BOTH POSITIVE AND NEGATIVE PRESSURES SHOWN IN TABLE ABOVE.
- LINEAR INTERPOLATION IS PERMISSIBLE.
- PLUS = PRESSURE AND MINUS = SUCTION.
- DESIGN OF WINDOWS/DOORS FASTENING TO THE WALL FRAMING IS THE RESPONSIBILITY OF THE WINDOW/DOOR MANUF./SUPPLIER & SHALL MEET THE ABOVE NOTED POSITIVE AND NEGATIVE PRESSURES.



SCOPE OF SERVICE

MEANS AND METHODS:
THE STRUCTURAL ENGINEER SHALL NOT HAVE CONTROL OR BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, PROCEDURES, OR SEQUENCES; FOR THE ACTS OR OMISSIONS OF THE CONTRACTOR OR ANY OTHER PERSONS PERFORMING THE WORK OR FOR THE FAILURE FOR ANY OF THEM TO CONSTRUCT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

LIMITS OF STRUCTURAL ENGINEERING DESIGN RESPONSIBILITIES:
THE ITEMS SPECIFICALLY DESIGNED BY THE STRUCTURAL ENGINEER ARE LIMITED TO THE FOLLOWING: CONTINUOUS LOAD PATH FOR WIND UPLIFT, WOOD PANEL SHEARWALLS, WALL FRAMING AND REQUIRED SHEATHING AND HEADERS DIRECTLY SUPPORTING ROOF FRAMING. ITEMS NOT DESIGNED PRE-ENGINEERED WOOD FLOOR AND ROOF TRUSSES, FLOOR FRAMING NOT SPECIFICALLY ADDRESSED, TRUSS-TO-TRUSS CONNECTION, AND ANY ARCHITECTURAL, MECHANICAL OR ELECTRICAL SYSTEM.

GENERAL NOTES & CONSTRUCTION SPECIFICATIONS

FLOOR SHEATHING SPECIFICATIONS

23/32" T&G OSB OR PLYWOOD SHEATHING, GLUE AND NAIL WITH 10d COMMON @ 6" O.C. EDGE & FIELD

ROOF SHEATHING SPECIFICATIONS:

SHINGLE – MIN. 15/32", 32/16, APA RATED OSB OR PLYWOOD SHEATHING, NAILED W/ 0.131x2 1/2" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).

TILE – MIN. 15/32" 32/16, APA RATED PLYWOOD SHEATHING, NAILED W/ 0.131x2 1/2" RING SHANK @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" O.C. WITHIN 4'-0" OF ROOF EDGE).

METAL – MIN. 1/2", 24/16, APA RATED PLYWOOD SHEATHING, NAILED W/ 0.131x2 1/2" RING SHANK NAILS @ 6" O.C. EDGE & 6" O.C. FIELD (AT GABLE ENDS DECREASE EDGE NAIL SPACING TO 4" WITHIN 4'-0" OF ROOF EDGE).

WALL SHEATHING SPECIFICATIONS:

FLEXIBLE FINISH – MIN. 7/8", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, FASTENED W/ 8d @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL EXTEND FULL HEIGHT FROM BOTTOM PLATE TO UPPER TOP PLATE. FLEXIBLE FINISH WALLS INCLUDE: WOOD, CEMENT, OR VINYL SIDING, HARDI PANEL & BRICK. ALL OTHER WALL SHALL BE CONSIDERED BRITTLE FINISH.

STUCCO FINISH – MIN. 7/8", 24/16, APA RATED OSB OR PLYWOOD SHEATHING, FASTENED W/ 8d @ 6" O.C. EDGE AND 6" O.C. FIELD. SHEATHING SHALL ORIENTED WITH THE LONG DIMENSION PERPENDICULAR TO THE STUDS. CONTRACTOR MAY USE 7/8" STRUCTURAL 1-GIN SHEATHING OR 19/32 OSB SHEATHING AND ORIENT THE PANELS VERTICALLY.

MASONRY SPECIFICATIONS:

MASONRY HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 530-05, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI530.1-05. GROUT SHALL BE IN ACCORDANCE WITH ASTM C476 WITH A MINIMUM OF 28 DAY COMPRESSIVE STRENGTH OF 2000 PSI PER ASTM C1019. GROUT SHALL HAVE A MAXIMUM COURSE AGGREGATE SIZE OF 3/4" PLACED AT AN 8" TO 11" SLUMP. MORTAR SHALL CONFORM TO ASTM C270 AND TYPE M OR S. TYPE N MORTAR MAY BE USED IN BRICK VENEER. CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL FLASHING.

CONCRETE MASONRY UNITS (CMU):

CMU SHALL BE IN ACCORDANCE WITH ASTM C90-75, HOLLOW LOAD-BEARING (CMU), TYPE 1, GRADE N-1, NORMAL WEIGHT, WITH A MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI (f'm=1500 PSI). GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT IN 5'-0" MAXIMUM LIFTS PROVIDE CLEANOUTS PER ACI 530.1-02 IN THE BOTTOM OF COURSE OF MASONRY WHEN THE WALL HEIGHT EXCEEDS 5'-0".

MASONRY STEMWALLS: ALL CONCRETE MASONRY UNITS SHALL BE COMPOSED OF ASTM C90E, E GRADE N-1 HOLLOW CONCRETE MASONRY UNITS WITH TYPE "S" MORTAR. WALL COURSING SHALL BE RUNNING BONDS, STACK BOND SHALL NOT BE USED. GROUT ALL CELLS CONTAINING VERTICAL REINFORCEMENT WITH 3000 PSI PEA ROCK CONCRETE GROUT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS. ALL EXTERIOR WALLS SHALL BE REINFORCED FULL HEIGHT WITH #4 @ 4'-0" O.C. MAX. AND AT EACH CORNER, WALL END, AND WALL INTERSECTIONS. PROVIDE CONTINUITY OF PERPENDICULAR MASONRY ELEMENTS BY INSTALLING CORNER BARS. MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. AT STEMWALL CONSTRUCTED OF 5 OR MORE COURSES, PROVIDE HORIZONTAL JOINT REINFORCEMENT AT 16" O.C. VERTICALLY, (EVERY OTHER COURSE), AND VERTICAL REINFORCING SHALL BE INCREASED AS NOTED ON 1/S1.0. UNLESS NOTED OTHERWISE, LAP JOINT REINFORCING SHALL BE A MINIMUM OF 6".

CONCRETE SPECIFICATIONS:

ALL CONCRETE HAS BEEN DESIGNED IN ACCORDANCE WITH ACI 318-08, AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 301. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS CONCRETE AT GARAGE AND PORCH SLABS SHALL HAVE A COMPRESSIVE STRENGTH OF 3000 PSI.

GENERAL NOTES:

FOOTING AND FOUNDATIONS:

FOOTINGS AND FOUNDATIONS SHALL BE IN ACCORDANCE WITH LOCAL BUILDING CODES. FOOTING HAVE BEEN DESIGNED WITH A SOIL BEARING (DESIGN MAXIMUM) OF 2000 PSF. A SOILS INVESTIGATION REPORT IS RECOMMENDED TO VERIFY SUITABLE SUBSURFACE CONDITIONS. IF THE FOOTING ELEVATIONS SHOW OCCUR IN A DISTURBED OR UNSTABLE SOIL, THE ENGINEER SHALL BE NOTIFIED. SOIL SHALL BE FREE OF ORGANIC MATERIAL AND COHESIVE (CLAY) SOILS. SOIL COMPACTION AND FILL SHALL BE COMPACTED TO A MIN. OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D 1557.

FOUNDATION PLAN ONLY CONVEYS STRUCTURAL INFORMATION. FOR GENERAL FEATURES, CONDUITS, ELECTRICAL EMBEDS, STEP HEIGHTS, ETC., SEE ARCHITECTURAL PLANS. DO NOT SCALE FOOTING DIMENSIONS AND LOCATION FROM THE FOUNDATION PLAN SHOWN ON S1.0. DO NOT DETERMINE FOOTING LOCATION BASED ON EITHER THE ARCHITECTURAL PLAN OR FRAMING PLAN, BUT BY DIMENSIONS PROVIDED ON FOUNDATION PLAN. IF FOOTING SIZE OR LOCATION IS NOT DETERMINED ON PLAN THEN CONTACT ENGINEER OF RECORD (EOR).

UNLESS OTHERWISE NOTED ON DRAWINGS, MINIMUM CONCRETE COVER FOR REINFORCING SHALL BE 3" IN FOOTINGS AND MESH SHALL BE CENTERED IN SLAB ON GRADE. IN ALL CONTINUOUS FOOTINGS PROVIDE #3 @ 48" O.C. OR ROD CHAIRS. PROVIDE CONTINUITY OF REINFORCING AT INTERSECTIONS OF PERPENDICULAR CONCRETE ELEMENTS BY INSTALLING CORNER BARS. MINIMUM OF 40 BAR DIAMETERS INTO EACH ELEMENT. SPLICES IN REINFORCING, WHERE PERMITTED, SHALL BE 48 BAR DIAMETERS

CONCRETE SLABS ON GRADE:

SLAB SHALL BE INSTALLED OVER MINIMUM 6 MIL POLYETHYLENE VAPO RETARDER WITH JOINTS LAPPED 6" AND SEALED OVER CLEAN, COMPACTED EARTH OR FILL WITH APPROVED CHEMICAL TREATMENT FOR PREVENTION OF SUBTERRANEAN TERMITES. SAWCUTS: FOR CONTROLLED CRACKING CUT A 1" SAWCUT INTO SLAB IN A 12"x12" GRID WITHIN 12 HOURS OF CONCRETE PLACEMENT, PROVIDE SAWCUTS THROUGH OUT SLAB CALL EOR FOR ALTERNATIVE METHODS.

WOOD FRAMING SPECIFICATIONS:

WOOD FRAMING HAS BEEN DESIGNED IN ACCORDANCE WITH NATIONAL DESIGN SPECIFICATIONS (NDS) FOR WOOD CONSTRUCTION, LATEST EDITION. ALL WOOD MEMBERS EXPOSED TO WEATHER OR IN CONTACT WITH MASONRY, CONCRETE OR SOIL SHALL BE PRESSURE-TREATED. IF, AQO OR NON-DOT BORATE PRESERVATIVE TREATMENT IS USED, ALL ATTACHED FASTENERS SHALL BE HOT DIPPED GALVANIZED; IF ACZA PRESERVATIVE IS USED, ALL ATTACHED FASTENERS SHALL BE STAINLESS STEEL.

PRE-ENGINEERED WOOD TRUSSES:

SHALL BEAR THE SEAL OF AN ENGINEER IN THE STATE WHERE PROJECT IS BEING BUILT AND SHALL COMPLY WITH NFPA, TPI, AND AITC 100. CONTRACTOR SHALL VERIFY THAT ADEQUATE TRUSS BRACING IS INSTALLED AT ALL TRUSSES AS INDICATED IN THE TRUSS SHOP DRAWINGS. ALL TRUSS-TO-TRUSS CONNECTIONS AND TRUSS PROFILES ARE THE RESPONSIBILITY OF THE DELEGATED TRUSS ENGINEER. ALL TRUSSES SHALL HAVE TEMPORARY BRACING PER 'COMMENTARY' AND RECOMMENDATION FOR HANDLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES, HB-91." AT MULTIPLE STRAP CONNECTIONS, SPREAD STRAPS TO AVOID NAILING CONFLICTS THROUGH TRUSS. WHEN USING (2) STRAPS ON SINGLE PLY TRUSSES, PLACE STRAPS DIAGONALLY ACROSS DBL. TOP PLATE FROM EA. OTHER.

ROOF COVERING SPECIFICATIONS:

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF THE ROOF COVERING SYSTEM. ASPHALT SHINGS SHALL COMPLY WITH ASCE D3161 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. CLAY AND TILE ROOFS SHALL BE INSTALLED PER THE "CONCRETE AND CLAY ROOF TILE INSTALLATION MANUAL" AND THE MANUFACTURER'S REQUIREMENTS. STANDING SEAM METAL ROOFS SHALL COMPLY WITH ASTM E1514 AND BE INSTALLED ACCORDING TO THE MANUFACTURER'S REQUIREMENTS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN AND INSTALLATION OF ALL METAL FLASHING AND VALLEY MATERIALS.

WATERPROOFING:

THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN/INSTALLATION OF ALL WATER PROOFING.

WOOD FASTENING SCHEDULE

MEMBERS	CONNECTION TYPE	FASTENER
TOP PLATE TO TOP PLATE	FACE NAIL	2-GUN NAILS @ 12" STAG.
TOP PLATE, LAP/INTERSECTION	FACE NAIL	(2-16d) 3-GUN NAILS
DBL. TOP PLATE TO STUD	FACE NAIL	(2-16d) 3-GUN NAILS
RIM JOIST TO TOP PLATE	TOE NAIL	(8d @ 6") GUN NAIL @ 6"
CELING JOIST TO TOP PLATE	TOE NAIL	(3-8d) 5-GUN NAILS
CELING JOIST, OVER PARTITIONS	FACE NAIL	(3-16d) 4-GUN NAILS
CELING JOIST TO ROOF RAFTER	FACE NAIL	(6-16d) 8-GUN NAILS
JOIST/TRUSS TO PLATE	TOE NAIL	(2-16d) 3-GUN NAILS
RAFTER TO PLATE	TOE NAIL	(3-8d) 3-GUN NAILS
JACK RAFTER TO HIP	TOE NAIL	(3-10d) 4-GUN NAILS
ROOF RAFTER TO 2x... RIDGE BM.	TOE NAIL	(2-16d) 3-GUN NAILS
CONT. HEADER, TWO PIECES	FACE NAIL	16d @ 16" O.C. @ EDGE
CONT. HEADER TO STUD	TOE NAIL	(3-16d) 4-GUN NAILS
STUD TO SOLE PLATE	TOE NAIL	(3-16d) 4-GUN NAILS
SOLE PLATE TO JOIST/BLOCKING	FACE NAIL	(16d @ 16") GUN NAIL @ 8"

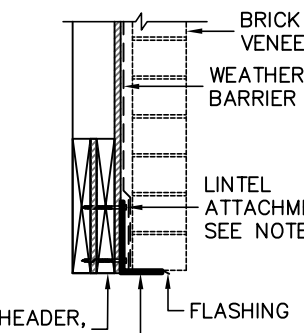
BRICK NOTES / LINTEL SCHD

LINTEL DIMENSION	MIN. BRG.	MAX. SPAN
L3 1/2 x 3 1/2 x 1/4	4"	6'-0"
L4 x 3 1/2 x 1/4	6"	8'-0"
L5 x 3 1/2 x 1/4	6"	10'-0"
L6 x 3 1/2 x 1/4	6"	12'-0"
(3-8d) 5-GUN NAILS	6"	16'-0"

1. STEEL LINTELS TO BE MINIMAL 36" LINTEL MUST HAVE CORROSION RESISTANT COATING OF EPOXY BASED PAINT.

2. LINTEL MORE THAN 8'-0", SHOULD BE LATERALLY SUPPORTED NOT TO EXCEED 6 FT. O.C. W/ 2-1/4"x3" WD. SCREWS INTO HEADER PROVIDE A 1/2" VERTICAL SLOTTED HOLE FOR SCREWS.

3. BRICK VENEER ATTACHMENT: HORIZONTAL TIES @ 24" O.C. VERT. TIES @ 12" O.C. (FOR 11mhp WIND-ZONE VERT. TIES @ 16" O.C.) AT ALL OPENINGS SPACE TIES WITHIN 12" OF OPENINGS. PROVIDE 3/8" WEEP HOLES @ 33" O.C. IMMEDIATELY ABOVE FLASHING.



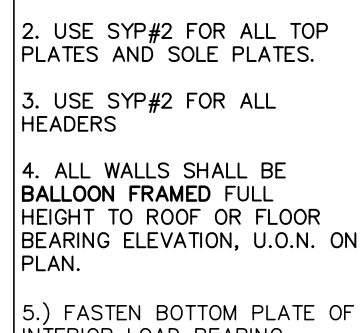
USP CONNECTORS

CONNECTOR	UPLIFT		FASTENERS	FL# CODE
	SYP	SPF		
USP A35	450	450	(9)10d1 1/2"	
USP R17	585	495	(5)8d EA. END	
USP R18A	775	650	(5)10d1 1/2" EA. END	
USP HTW12	1195	860	(7)10d1 1/2" EA. END	
USP HTW20	1450	1245	(12)10d1 1/2" EA. END	
USP MSTA24	1640	1455	(9)10d EA. END	
USP MSTA36	2065	2065	(13)10d EA. END	
USP LST20B	1105	1105	1/2" ROD TO FTG.	
USP JUS28	1305	1305	(6)10d TO HEADER	
USP HTT16	4290	4290	3/8" ROD TO FTG.	
USP HTT22	5370	5370	1/2" ROD TO FTG.	
USP PAU44	2535		3/8" ROD W/ (12)16d	
USP PAU66	2535		3/8" ROD W/ (12)16d	
USP MSTM24	1545	1455	(5)1/4"x2-1/2" TAPCONS	

SIMPSON CONNECTORS

CONNECTOR	UPLIFT		FASTENERS	FL# CODE
	SYP	SPF		
A35	450	450	12-8d1 1/2"	10446.4
H2.5T	600	520	5-8d EA. END	11478.3
HTS16	1150	1085	16-10d EA. END	10456.6
HTS12	1000	860	7-10d1 1/2" EA. END	10456.3
HTS20	1450	1245	24-10d1 1/2" EA. END	13872.3
MSTA24	1765	1270	9-10d EA. END	13872.4
MSTA36	2050	1870	13-10d EA. END	13872.8
HTT4	3480	3080	18-16d TO TRUSS/BEAM 1-3/8" ROD TO FTG.	11496.2
HTT5	5250	4670	32-16d TO TRUSS/BEAM 1-3/8" ROD TO FTG.	11496.2
LUS28	930	780	6-10d TO HEADER 4-10d TO JOIST	10655.113
HU410	905	785	14-16d TO HEADER 6-16d TO JOIST	10531.36
ABU44	2200		3/4" ROD EPOKID 6" MIN	10849.6
ABU66	2300		3/4" ROD EPOKID 6" MIN	10849.6
SET	N/A	N/A	SIMPSON EPOXY-TIE	11506.4
LTT20B	1675	1675	10-16d TO STUD/BEAM/POST 1-3/8" ROD TO FTG.	11496.3
LSTA12	805	695	10-10d	13872.5
CS16	1705	1705	13-8d	10852.1

TYPICAL WALL FRAMING NOTES:
1. USE SPF#2 OR BETTER FOR ALL WALL STUDS.
2. USE SYP#2 FOR ALL TOP PLATES AND SOLE PLATES.
3. USE SYP#2 FOR ALL HEADERS.
4. ALL WALLS SHALL BE BALLOON FRAMED FULL HEIGHT TO ROOF OR FLOOR BEARING ELEVATION, U.O.N. ON PLAN.
5. FASTEN BOTTOM PLATE OF INTERIOR LOAD BEARING WALLS TO CONCRETE SLAB W/10d MASONRY COT NAILS @ 48" O.C. MINIMUM. SEE FOUNDATION PLAN ADDITIONAL ANCHORS AT SHEARWALLS



INTERIOR BEARING WALL: SEE PLAN FOR STUD SIZE AND O.C. SPACING.

CONTINUOUS BLOCKING WITHIN FLOOR SYSTEM WHERE POST IS ABOVE.

SEE 5/SO.0.

SEE 1/SO.1.

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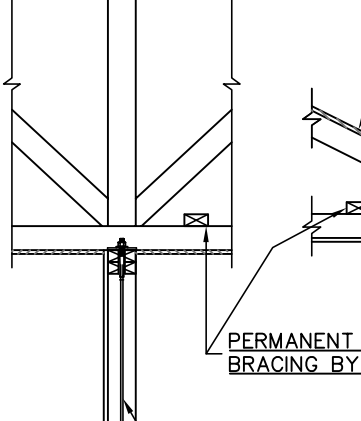
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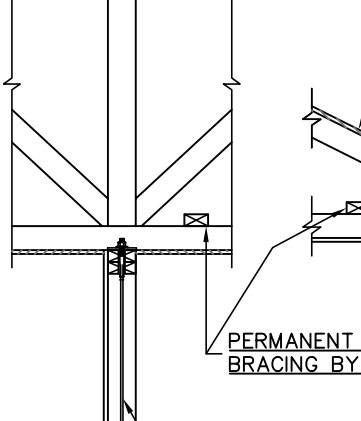
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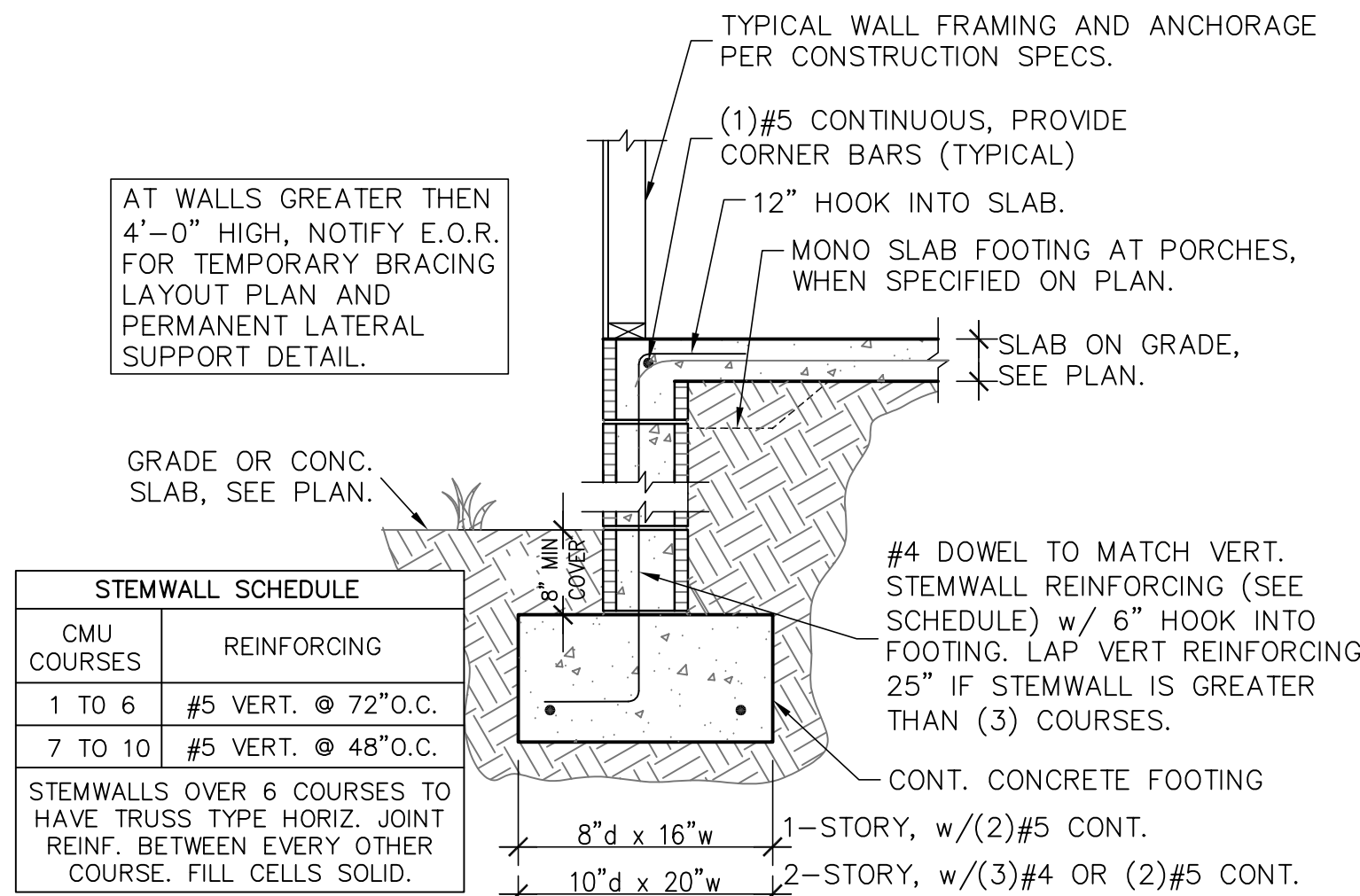
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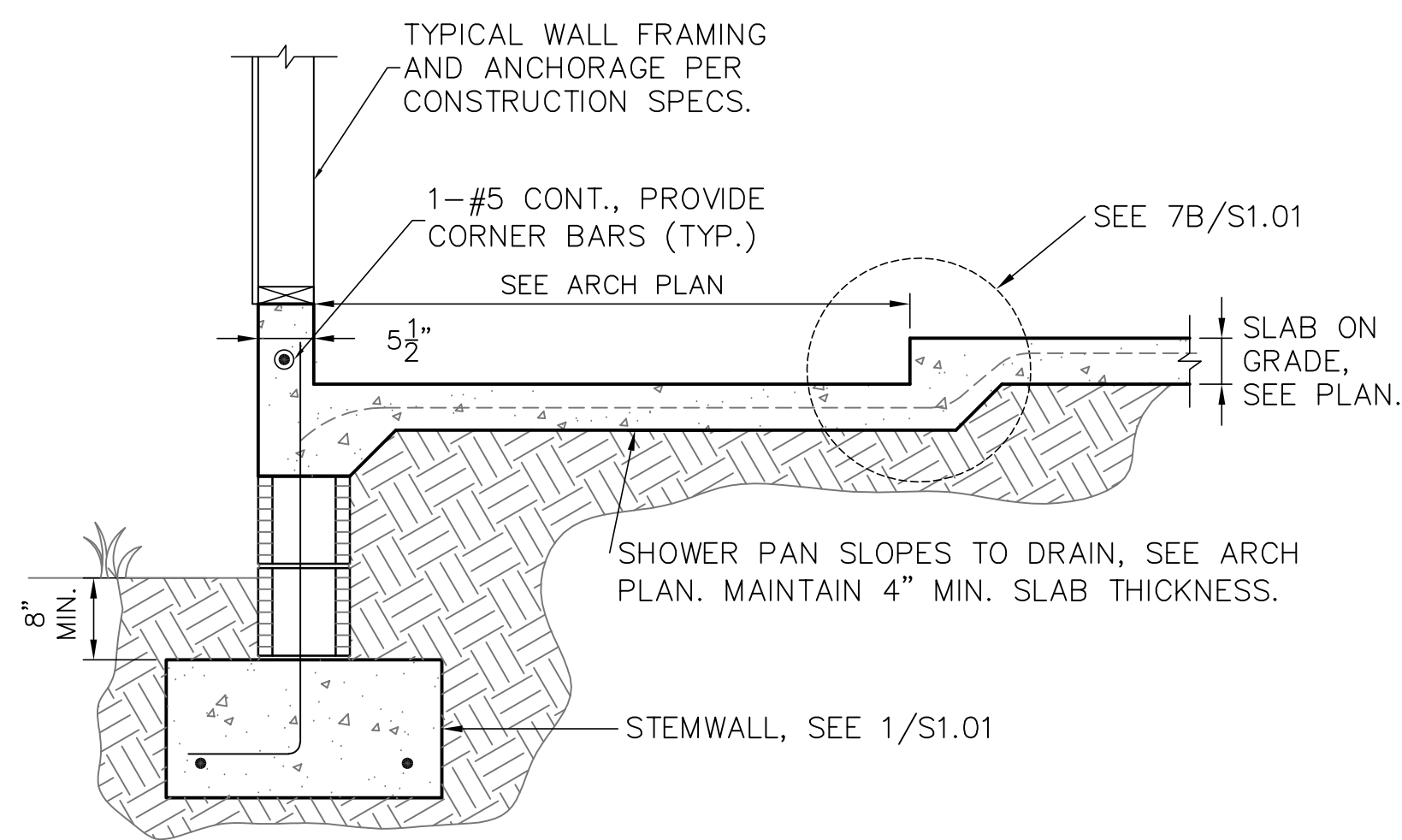
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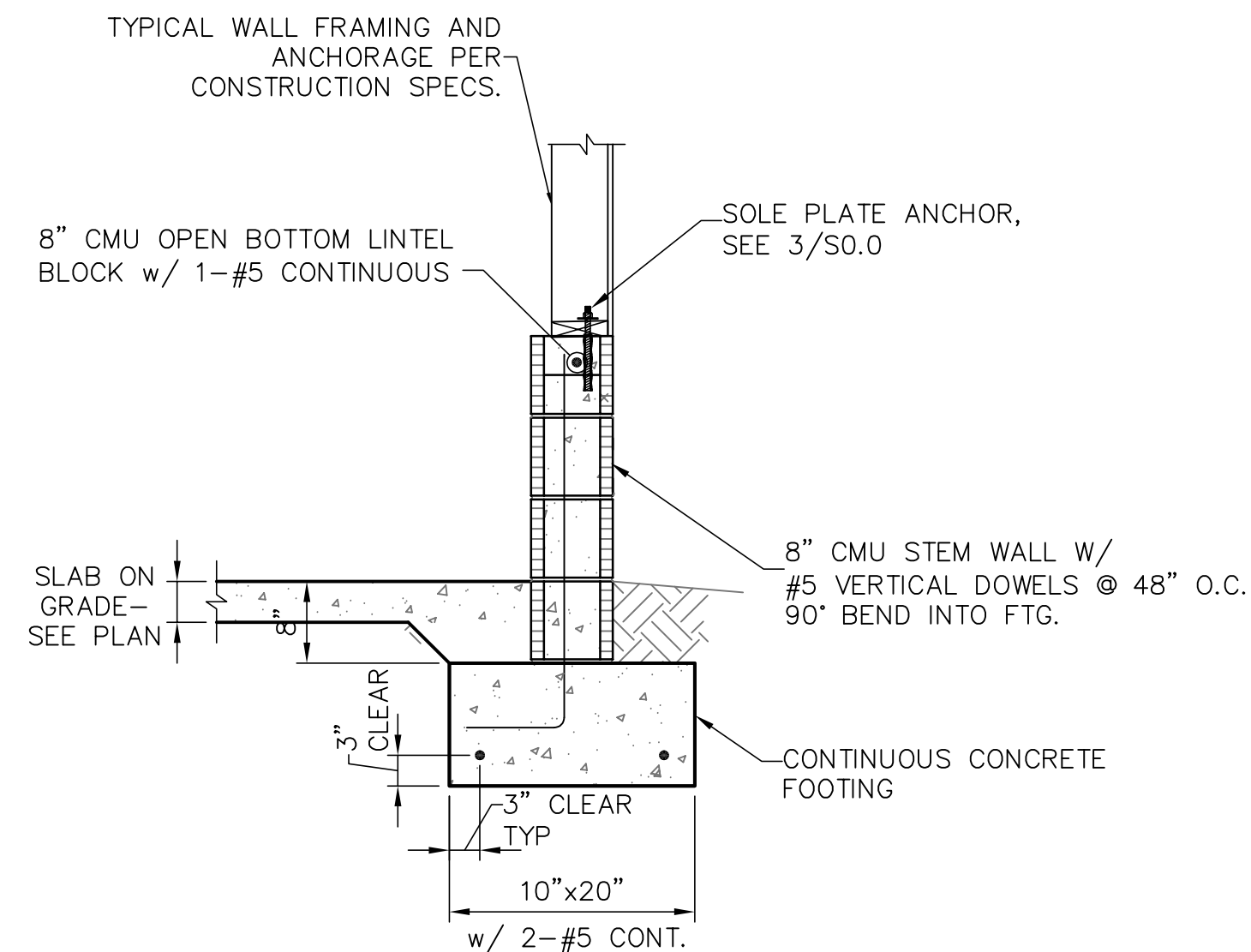
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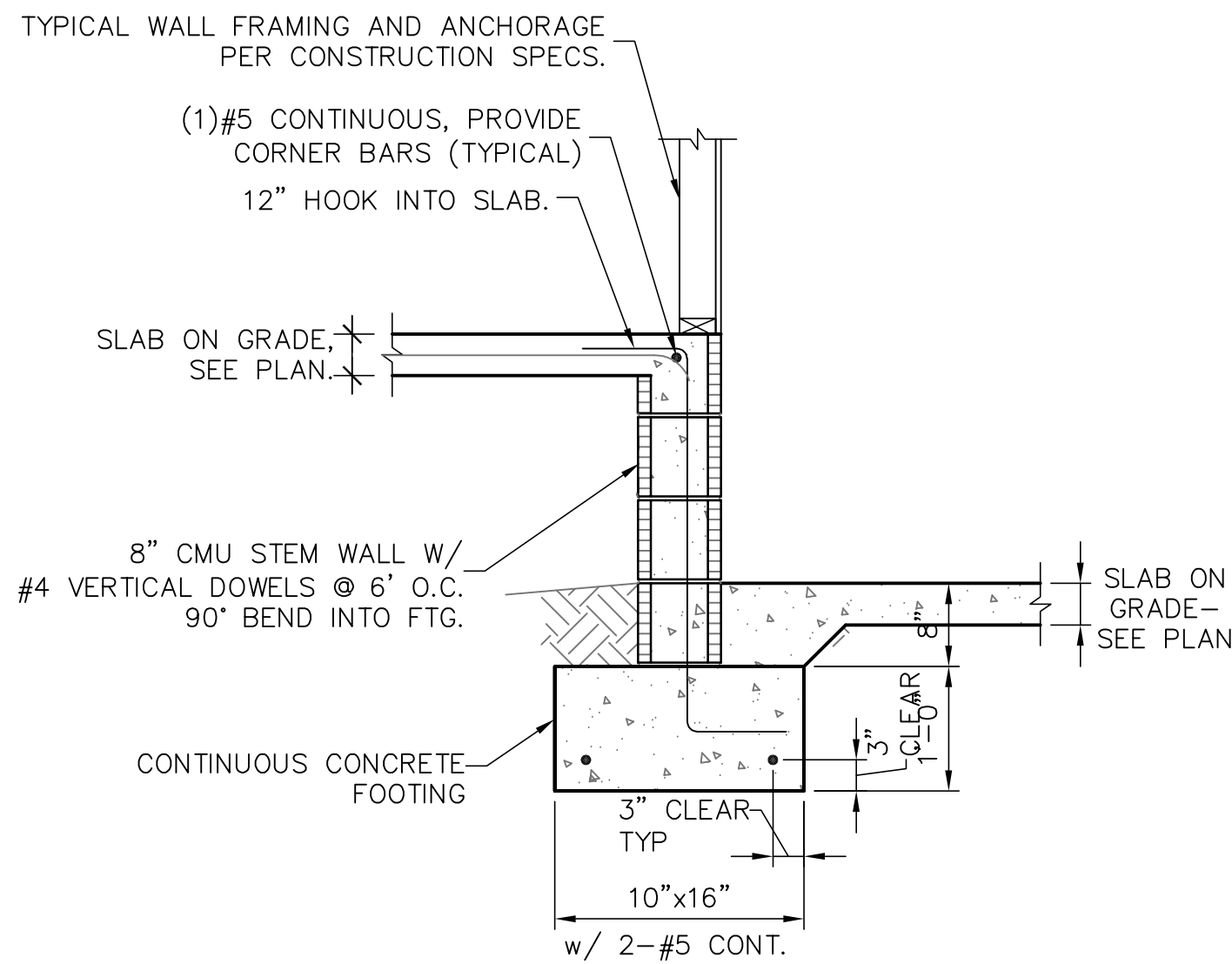
1 STEMWALL FOOTING
S1.01 SCALE: 3/4" = 1'-0"



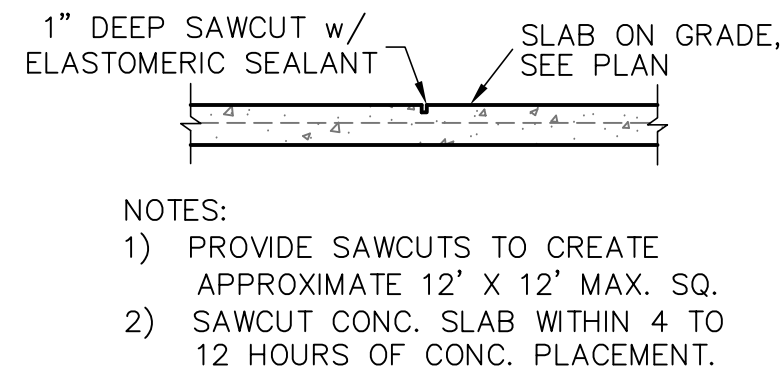
2 FOOTING W/ SHOWER RECESS
S1.01 SCALE: 3/4" = 1'-0"



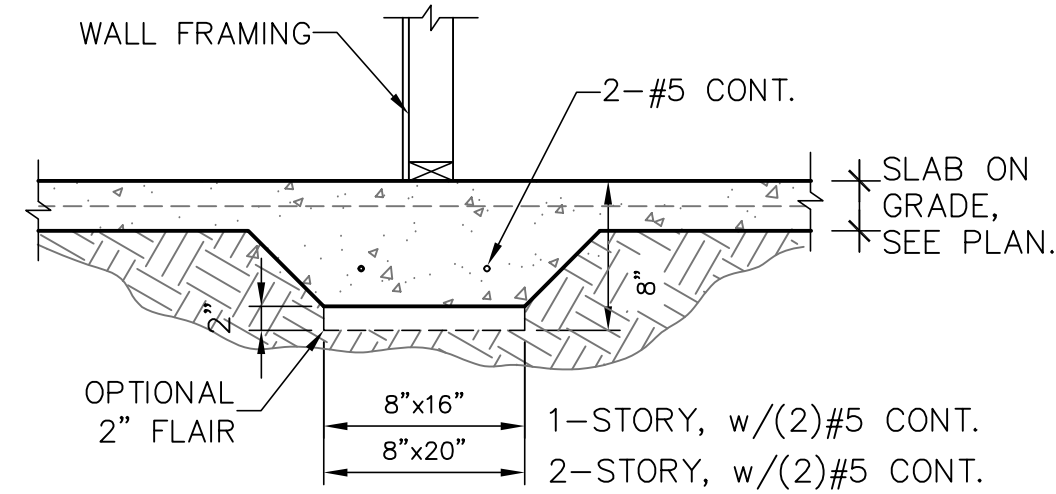
3 GARAGE STEM WALL
S1.01 SCALE: 3/4" = 1'-0"



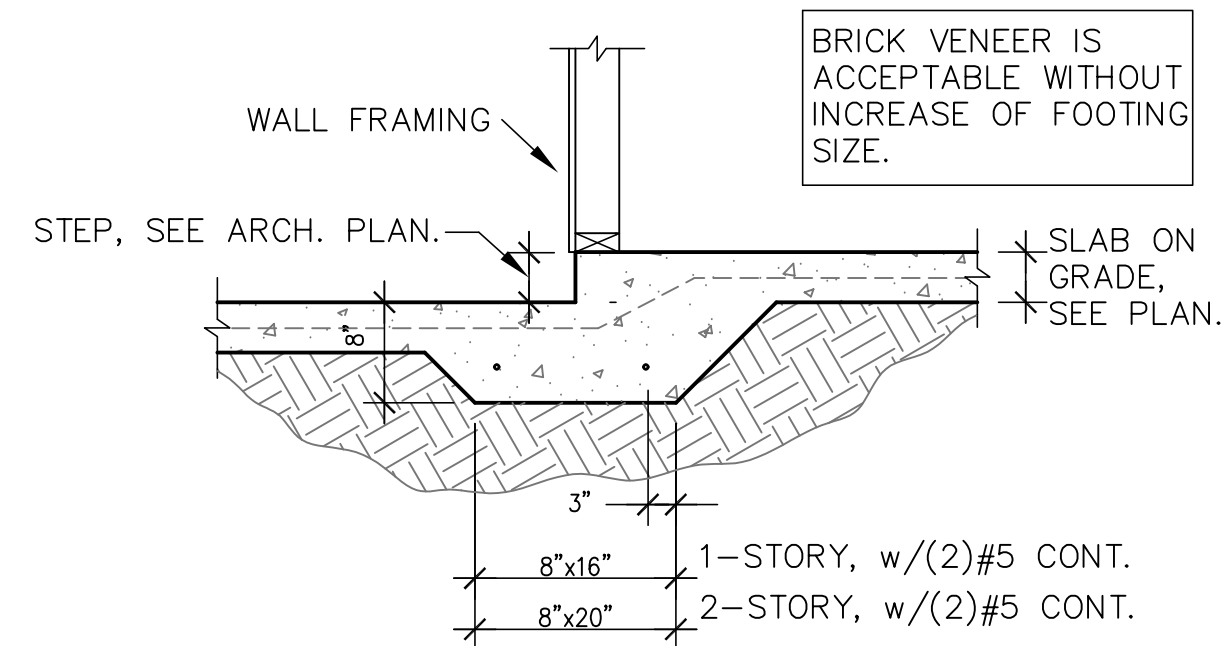
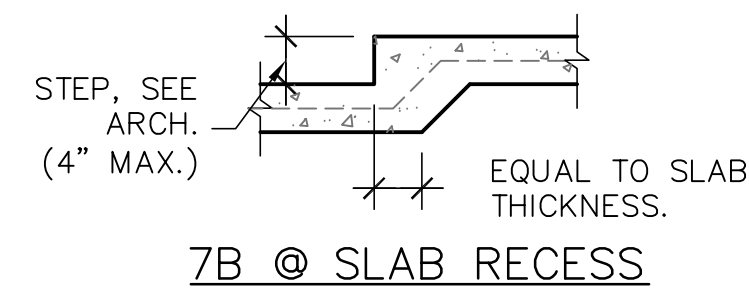
4 STEMWALL AT GARAGE
S1.01 SCALE: 3/4" = 1'-0"



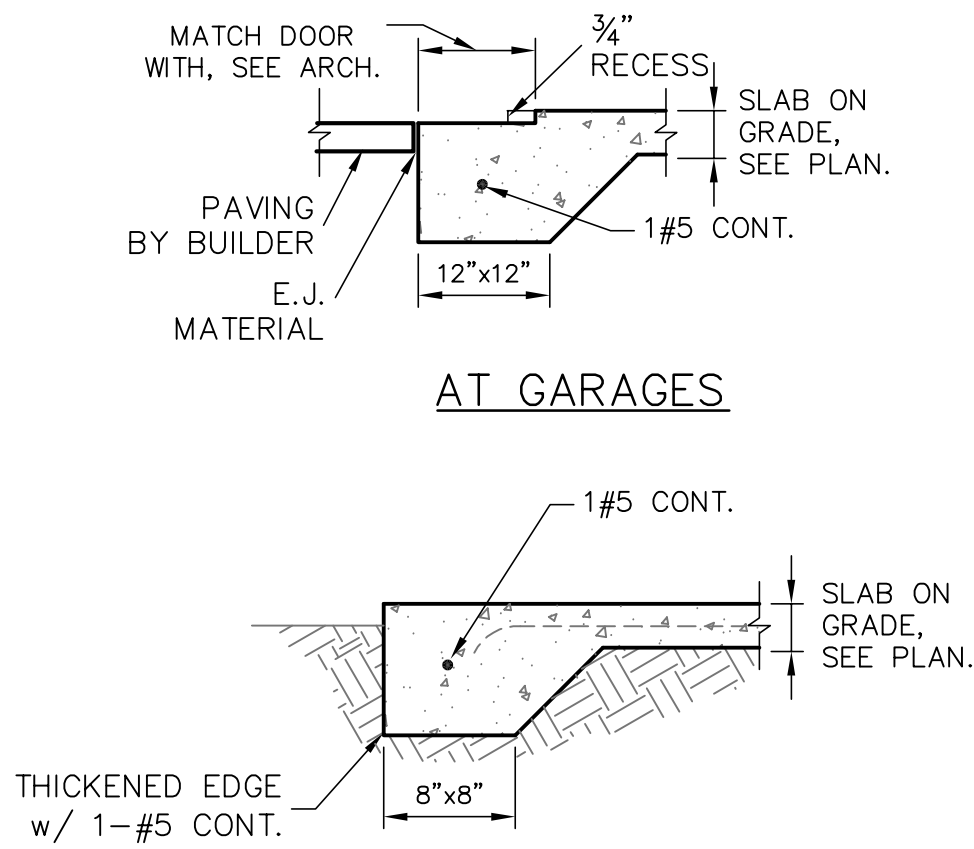
5 SAW CUT DETAIL
S1.01 SCALE: 3/4" = 1'-0"



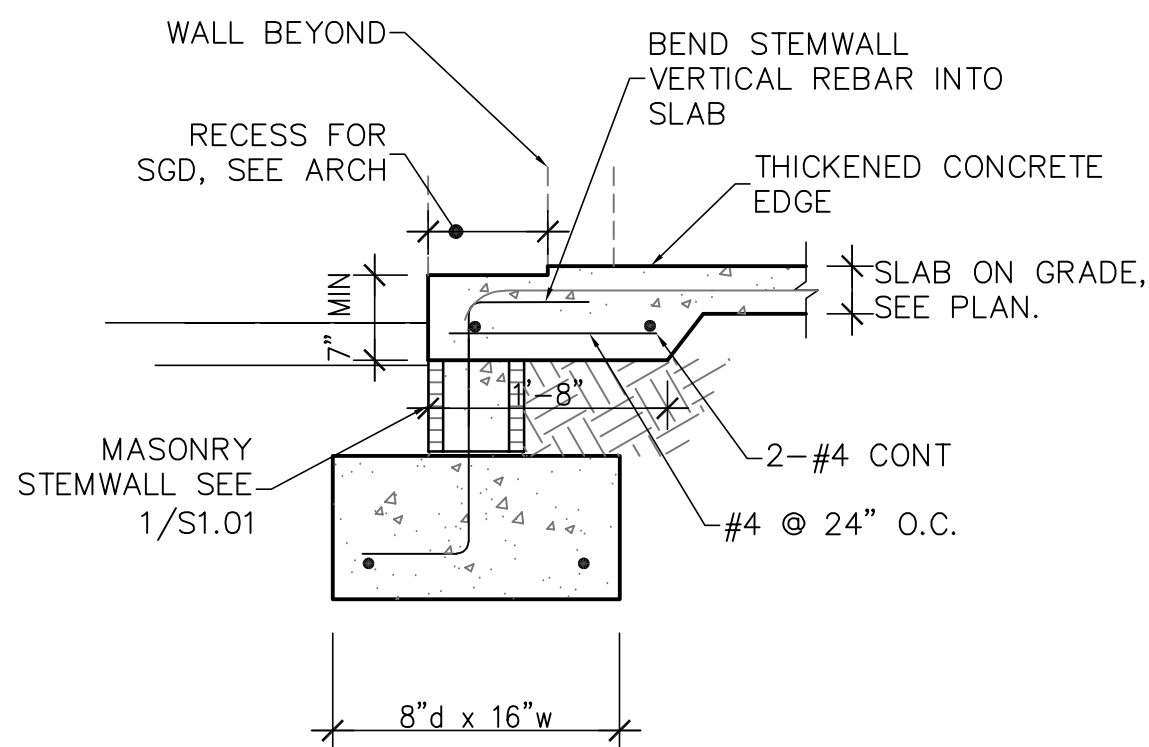
6 BEARING AT INTERIOR
S1.01 SCALE: 3/4" = 1'-0"



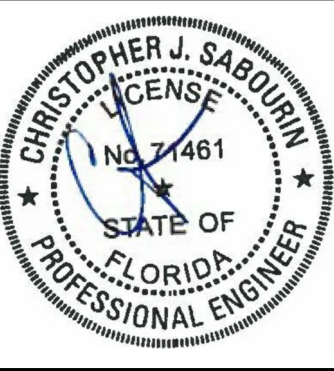
7 MONO. FOOTING AT STEP-DOWN
S1.01 SCALE: 3/4" = 1'-0"



8 THICKENED SLAB
S1.01 SCALE: 3/4" = 1'-0"



9 STEMWALL FOOTING AT SLIDER
S1.01 SCALE: 3/4" = 1'-0"



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PLAN NAME	BZEC
SSE No.	21-0514

ISSUE	DATE
PERMIT	10.06.21
REVISIONS	DATE

STRUCTURAL ENGINEERING FOR
THE SHELLEY RESIDENCE

FIELD ALTERATION
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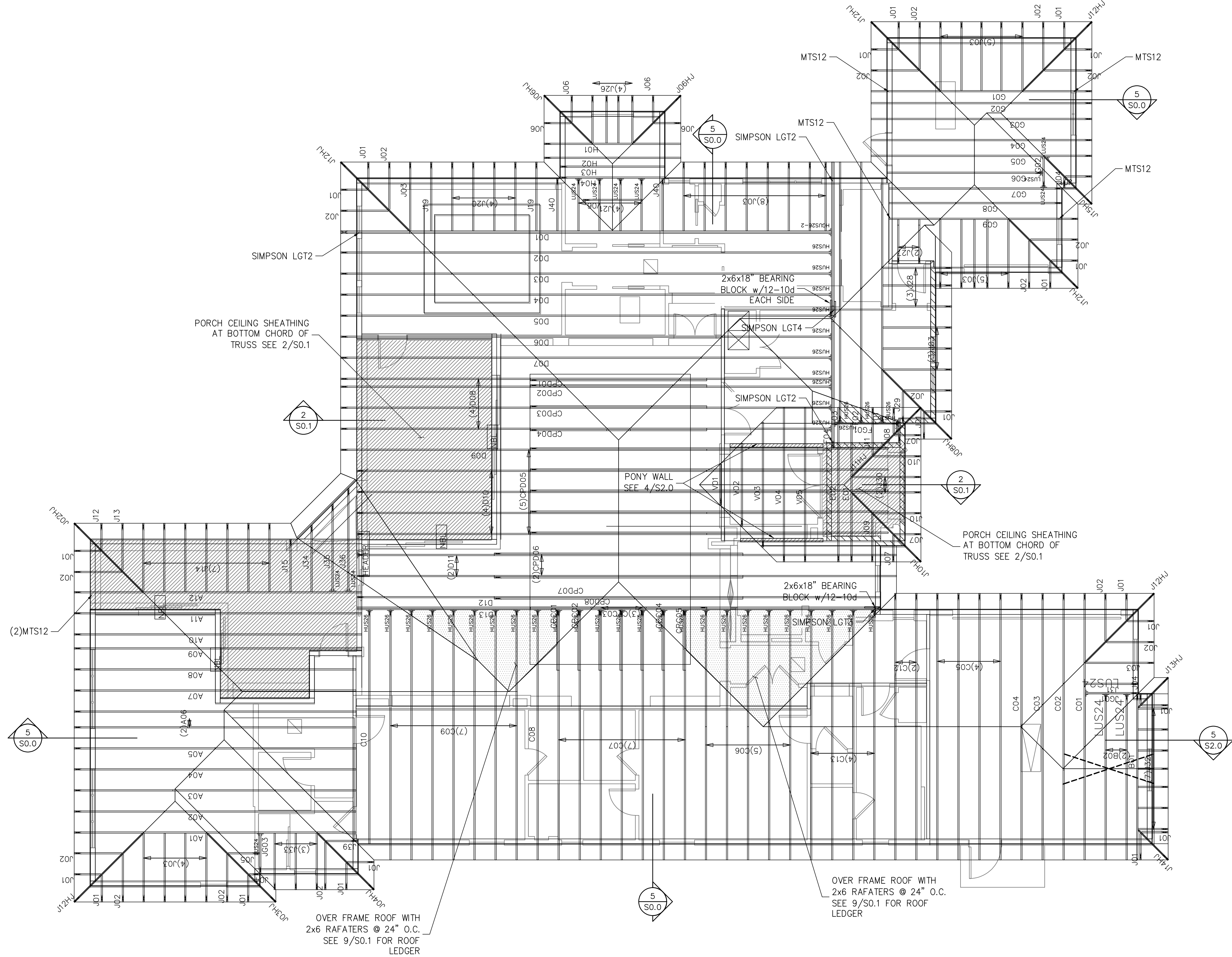
SCALING
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FOUNDATION
DETAILS

SHEET
S1.01
SHEET 4 OF 7



SHEET
S1.1
SHEET 5 OF 7



FIRST LEVEL TRUSS FRAMING PLAN
SCALE: 3/16" = 1'-0"

SYMBOLS LEGEND

HTS16

DESIGNATES UPLIFT CONNECTION.

FRAMING PLAN NOTES:

1. FOR TYPICAL ROOF SHEATHING AND FRAMING, SEE SHEET S0.0.
2. FOR SPECIFIC UPLIFT CONNECTORS, SEE PLAN MIN. (3)SDWC CONNECTOR.
3. FOR GENERAL DESIGN SPECIFICATIONS SEE SHEET S0.0.
4. WHEN USING (2)H25T CLIPS ON 1 1/2" WIDE LUMBER, PLACE CLIPS DIAGONALLY ACROSS DOUBLE TOP PLATE FROM EACH OTHER.

TRUSS FASTENING DETAILS

STUD DIRECTLY BELOW TRUSS

SDWC15600

TOP PLATE TO STUD SDWC15600

TRUSS TIE DOWN WITH SIMPSON SDWC

Rafter to Top Plate shown
Truss to Top Plate similar

Optimal 22 1/2°

STUD DIRECTLY BELOW TRUSS

SDWC15600

TOP PLATE TO STUD SDWC15600

Note: 1. Sloped-roof rafters may be sloped up to and including a 12:12 pitch and must be "birdsmouth" cut.
2. Reference detail 4 for installation instructions.

SIMPSON SDWC INSTALLATION RANGE

STUD NOT DIRECTLY BELOW TRUSS

SDWC15600

Note: Reference detail 2a for installation angle limit

SDWC INSTALLATION

Rafter to Top Plate shown
(Truss to Top Plate similar)

12" max

STUD NOT DIRECTLY BELOW TRUSS

SDWC15600

Overhang

1 1/2" MIN

2" MAX

SDWC INSTALLATION RANGE

Rafter or Truss

1/2" minimum edge distance for full values (with or without a plate splice)

Splice may be in upper or lower plate

STUD NOT DIRECTLY BELOW TRUSS

SDWC AT TOP PLATE SPLICE

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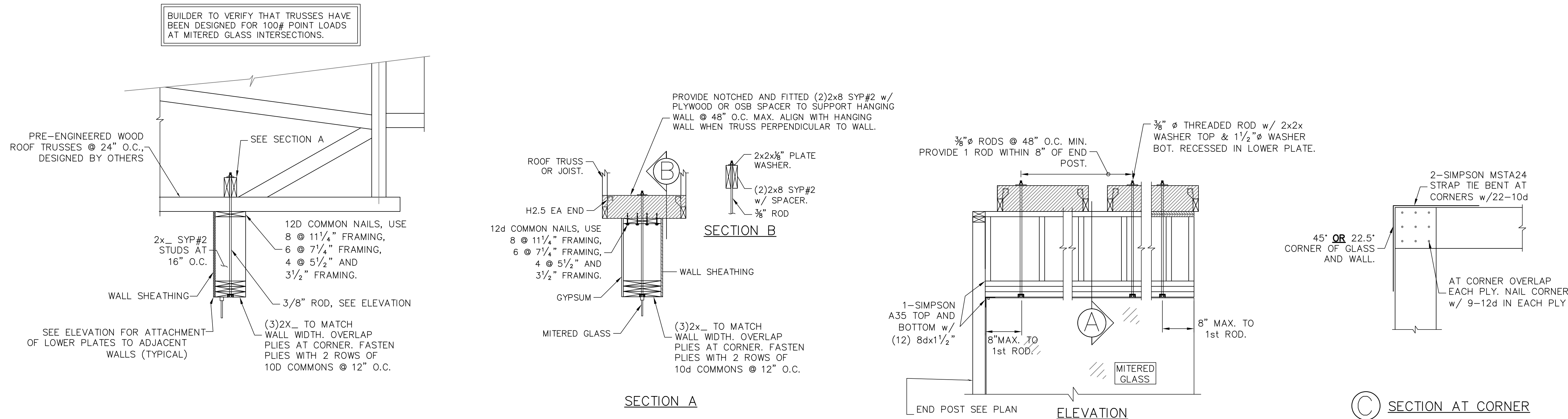
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ROOF TRUSS
PLACEMENT
PLAN

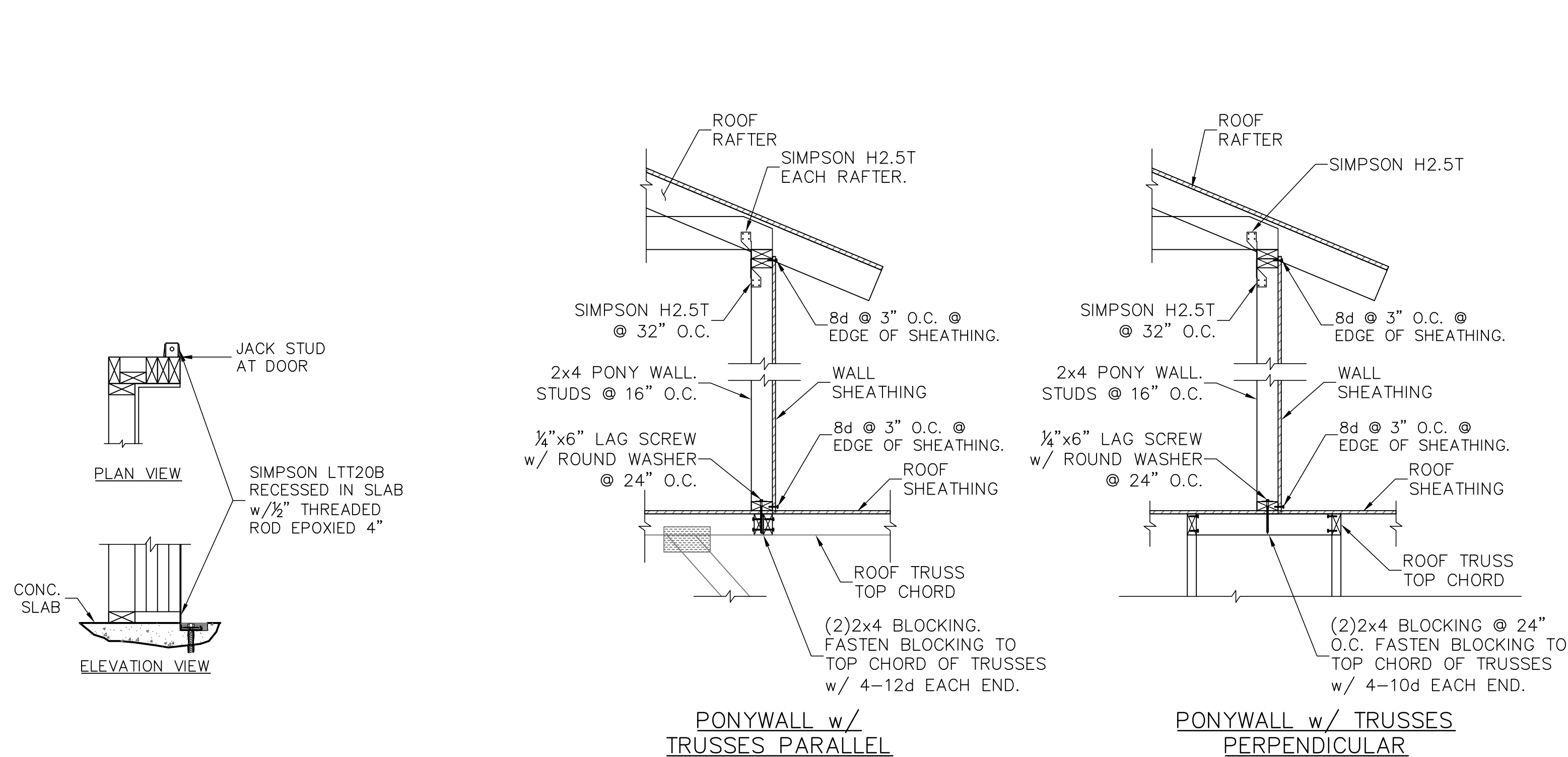
SHEET
S1.2

SHEET 6 OF 7



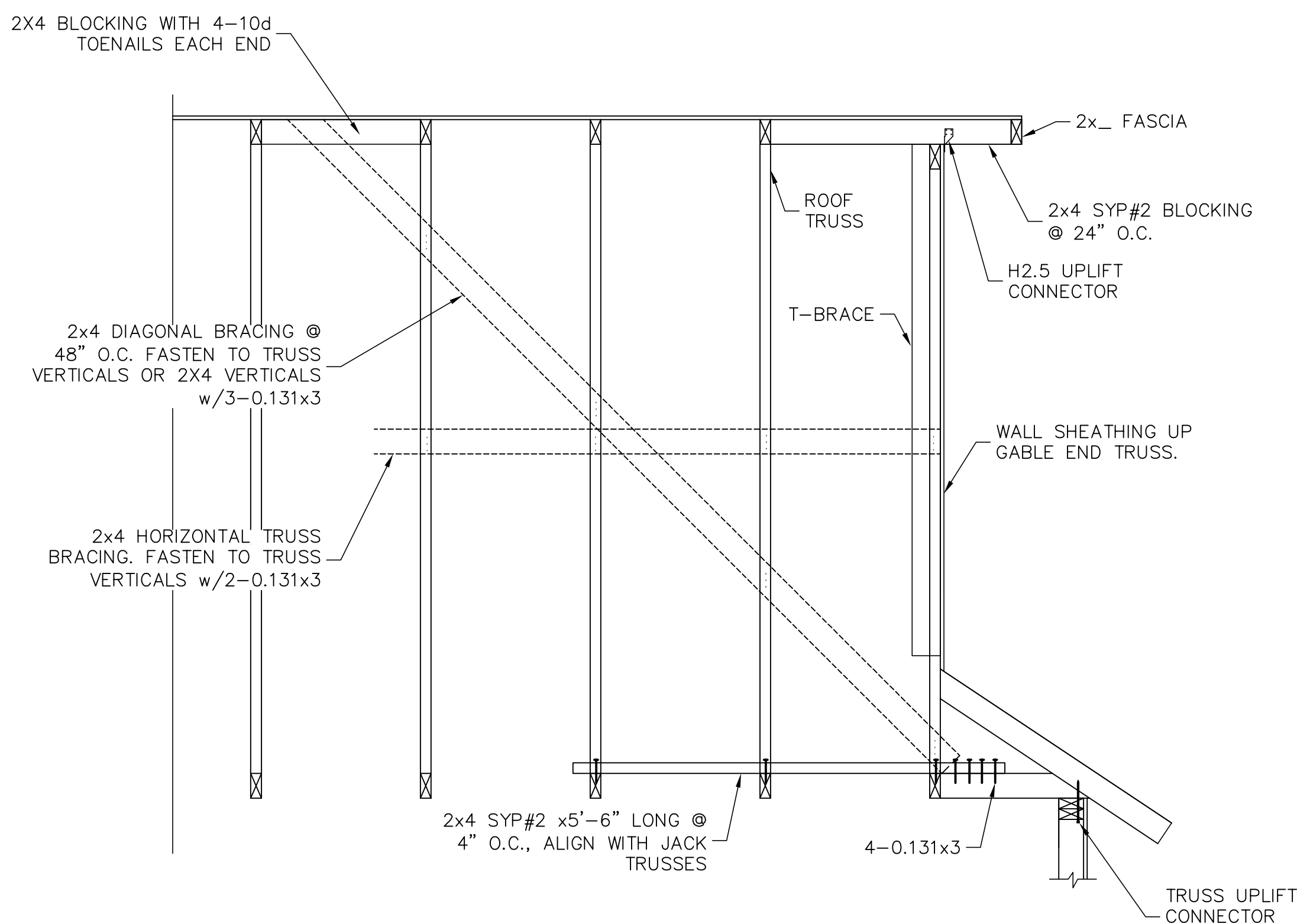
MITERED WINDOW HEAD FRAMING

SCALE: N.T.S.



PONYWALL DETAIL

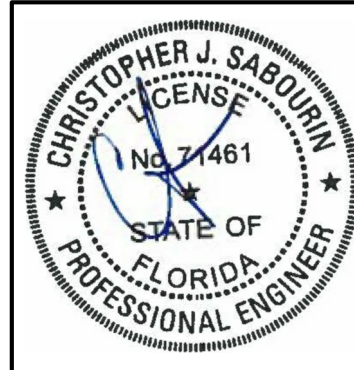
SCALE: N.T.S.



GABLE END BRACE DETAIL

DOOR JAMB FASTENING

THIS DETAIL ONLY APPLIES WHEN NOTED ON PLAN



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MISC
DETAILS

SHEET
S2.0
SHEET 7 OF 7