1. WOOD CONSTRUCTION SHALL CONFORM TO THE NDS "NATIONAL DESIGN

2. ALL EXTERIOR WOOD STUD WALLS, BEARING WALLS, SHEAR WALLS AND

2 GRADE OR BETTER SHALL BE USED REGARDLESS OF SPECIES.

END BRACING) SHALL BE EITHER SOUTHERN PINE, OR S.P.F. NUMBER

3. ANY WOOD FRAME INTERIOR BEARING WALL STUDS THAT HAVE HOLES IN

SHIELDS FOR ALL HOLES OVER 1" IN DIA. FOR PLUMBING LINES, ETC.

SHALL BE REPAIRED WITH SIMPSON HSS2 STUD SHOES, TYP., U.N.O.

1. ALL PREFABRICATED WOOD TRUSSES SHALL BE SECURELY FASTENED

TO THEIR SUPPORTING WALLS OR BEAMS WITH HURRICANE CLIPS OR

2. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE

3. TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPORTIONED (WITH

4. BRIDGING FOR PRE-ENGINEERED TRUSSES SHALL BE AS REQUIRED BY

5. TRUSS ELEVATIONS AND SECTIONS ARE FOR GENERAL CONFIGURATION

OF TRUSSES ONLY. WEB MEMBERS ARE NOT SHOWN, BUT SHALL BE

DESIGNED BY THE TRUSS MANUFACTURER IN ACCORDANCE WITH THE

6. DESIGN SPECIFICATIONS FOR LIGHT WEIGHT METAL PLATE CONNECTED

GOVERNING CODES. SUBMITTALS SHALL INCLUDE TRUSS FRAMING

BRIDGING AS REQUIRED FOR ERECTION AND FOR THE PERMANENT

8. THE TRUSS MANUFACTURER SHALL DETERMINE ALL SPANS WORKING

POINTS, BEARING POINTS, AND SIMILAR CONDITIONS. TRUSS SHOP

DRAWINGS SHALL SHOW ALL TRUSSES, ALL BRACING MEMBERS, AND

ASSUMED ALLOWABLE SOIL BEARING PRESSURE AFTER COMPACTION:

2000 SF SEE SOILS REPORT AND SPECIFICATIONS FOR COMPACTION

CONTACT THE ENGINEER PRIOR TO FOUNDATION POUR FOR VERIFICATION

OF FOUNDATION DESIGN. SOIL TO BE COMPACTED TO AT LEAST 95% OF

REQUIREMENTS IF SOIL CONDITIONS IN THE PROJECT DO NOT MEET

OR EXCEED THE CAPACITY THE GENERAL CONTRACTOR SHALL

STRUCTURE. EACH SUBMITTAL SHALL BE SIGNED AND SEALED BY A

FLORIDA REGISTERED STRUCTURAL ENGINEER. SUBMIT 2 COPIES FOR

7. PRE-ENGINEERED WOOD TRUSSES SHALL BE DESIGNED BY THE

REVIEW AND APPROVAL PRIOR TO FABRICATION.

MANUFACTURER IN ACCORDANCE WITH SPECIFIED LOADS AND

WOOD TRUSSES PER THE TRUSS PLATE INSTITUTE TPI LATEST EDITION.

PLANS AND DETAILS SHOWING MEMBER SIZES, BRACING, ANCHORAGE,

CONNECTIONS, TRUSS LOCATIONS, AND PERMANENT BRACING AND/OR

THE TRUSS MANUFACTURER UNLESS NOTED ON THE PLANS.

A MAXIMUM ALLOWABLE STRESS INCREASE FOR LOAD DURATION OF

25%) TO WITHSTAND THE LIVE LOADS GIVEN IN THE NOTES AND TOTAL

WITH THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATION

FOR STRESS-GRADE LUMBER AND ITS FASTENERS" AS RECOMMENDED

THE CENTER OF THE STUD UP TO 1" DIA. SHALL HAVE STUD PROTECTION

4. FASTENERS FOR PRESSURE PRESERVATIVE AND FIRE-RETARDANT-TREATED

WOOD SHALL BE OF HOT-DIPPED GALVANIZED STEEL, STAINLESS STEEL,

MISC. STRUCTURAL WOOD FRAMING MEMBERS, (I.E. BLOCKING OR GABLE

SPECIFICATION FOR WOOD CONSTRUCTION", 2018 EDITION.

WOOD CONSTRUCTION

SILICON BRONZE OR COPPER.

FOLLOWING DESIGN LOADS:

ALL TRUSS TO TRUSS HANGERS.

SOIL BEARING VALUE:

ANCHORS.

DEAD LOAD.

PREFABRICATED WOOD TRUSSES

BY THE NATIONAL FOREST PRODUCTS ASSOCIATION.

ACI 318-19 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE

ACI 530-19 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES

ACI 301-19 SPECIFICATIONS FOR STRUCTURAL CONCRETE BUILDINGS

2018 NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION

2020 FLORIDA BUILDING CODE RESIDENTIAL

2018 WOOD FRAMED CONSTRUCTION MANUAL

ASCE/SEI 7-16 AMERICAN SOCIETY OF CIVIL ENGINEERS

RESIDENTIAL FLOOR, UNLESS OTHERWISE INDICATED

ALL CONCRETE UNLESS OTHERWISE INDICATED

WELDED WIRE FABRIC SHALL CONFORM TO

POLYPROPYLENE FIBERS FOR SLABS ON GRADE

ALL REINFORCING BARS

ALL STIRRUPS AND TIES

MORTAR TYPE "S" 1800 PSI

CONCRETE GROUT 3000 PSI

DESIGN LOADS:

PEA GRAVEL CONCRETE FOR MASONRY CELLS ONLY

(DO NOT USE FOR CONCRETE COLUMNS OR TIE BEAMS)

ASTM C90-01, STANDARD WEIGHT UNITS, fm=1500 PSI

2020 FLORIDA FIRE PREVENTION CODE

APA PLYWOOD DESIGN SPECIFICATION

LIGHT PARTITIONS (DEAD LOAD), U.N.O.

2020 FLORIDA ACCESSIBILITY CODE

NEC NFPA 70 & FBCEB

ROOF

BALCONIES

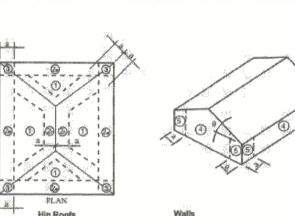
STAIRS

COVER SHEET

M D

GROUP R-3

-24.4



13.6

THIS DRAWING AND DESIGN IS VALID FOR 12 MONTHS AFTER THE DATE IT IS SIGNED AND SEALED OR WHILE **CURRENT CODE IS VALID**

IT IS THE INTENT OF THIS DESIGNER THAT THESE PLANS ARE ACCURATE AND ARE CLEAR ENOUGH FOR THE LICENSED PROFESS-ONAL TO CONSTRUCT THIS PROJECT. IN THE EVENT THAT SOMETHING IS UNCLEAR OR NEEDS CLARIFICATION..STOP..AND CALL THE DESIGNER LISTED IN THIS TITLE PAGE. IT IS THE RESPONSIBILITY OF THE LICENSED PROFESSIONAL THAT IS CONSTRUCTING THIS PROJECT TO FULLY REVIEW THESE DOCUMENTS BEFORE CONSTRUCTION BEGINS AND ANY AND ALL CORRECTIONS, IF NEEDED, TO BE MADE BEFORE ANY WORK IS DONE. *DO NOT SCALE DRAWINGS FOR CRITICAL DIMENSIONS

FLOOR PLAN FOUNDATION PLAN **ELEVATIONS** TRUSS LAYOUT

ELECTRICAL PLAN DETAILS

MINIMUM 1.5 LBS. OF FIBERS PER CUBIC YARD

BUILDING TYPE

SHT NO:

S-1

20 PSF (REDUCIBLE)

10 PSF ATTIC L.L.

ASTM A1064/A1064M

ASTM A615-40 40,000 PS

ASTM A615-40 40,000 PS

40 PSF

60 PSF

40 PSF

20 PSF

2500 PSI

3000 PSI

ALL STRUCTURAL AND MISCELLANEOUS STEEL A36 36,000 PSI, U.N.O. SHOP AND FIELD WELDS: E70XX ELECTRODES ALL BOLTS CAST IN CONCRETE: ASTM A36 OR ASTM A-307

CONTINUOUS MASONRY INSPECTION IS REQUIRED DURING CONSTRUCTION

SHINGLE ROOF:

BEAMS, RAFTERS, JOIST, PLATES, ETC. U.N.O. NO. 2 SOUTHERN YELLOW PINE (19% M.C.) ROOF DECK: PLYWOOD C-C/C-D, EXTERIOR, or OSB FLOOR SHEATHING: T&G A-C GROUP 1 APA RATED (48/24) WALL SHEATHING: PLYWOOD C-C/C-D, EXTERIOR OR OSB VERSA LAM BEAM Fb = 2900 PSI (2.0E) WOOD COLS. PARALLAM 2.0E U.N.O.

TOP CHORD LIVE LOAD: 20 PSF 10 PSF TOP CHORD DEAD LOAD: BOTTOM CHORD DEAD LOAD:

BOTTOM CHORD ATTIC LIVE LOAD: 10 PSF SEE DRAWINGS FOR SPECIAL CONCENTRATED LOADS. DESIGN FOR NEW WIND UPLIFT AS PER SPECIFIED CODES, DEDUCTING A MAXIMUM OF 5 P.S.F. DEAD LOAD, BUT NOT EXCEEDING ACTUAL DEAD LOAD.

CODES:

LIVE LOADS:

CONCRETE

STRENGTH

@ 28 DAYS

CONCRETE

MASONRY

STRUCTURAL

WOOD ROOF

TRUSSES:

WOOD FRAMING:

UNITS:

STEEL:

REINFORCING:

FROM THE HOLE BY BRUSHING AND AND USING COMPRESSED AIR PRIOR TO APPLYING THE EPOXY. ALLOW THE EPOXY TO CURE TO MANUFACTURER'S SPECIFICATIONS, THEN FILL THE CELL IN THE NORMAL WAY DURING BOND BEAM POUR. 3. FOR MORTER JOINTS LESS THAN 1/4", PROVIDE (1) #5 VERT. IN CONC.

FILLED CELL EACH SIDE OF THE JOINT (BAR DOES NOT HAVE TO BE CONT. TO FOOTING)

4. MISSED LINTEL STRAPS FOR MASONRY CONSTRUCTION MAY BE SUBSTITUTED WITH (1) SIMPSON MTSM16 TWIST STRAP W/ (4) 1/4" X 21/4" TITENS TO MASONRY AND (7)-10d NAILS TO TRUSS FOR UPLIFTS LESS THAN 860 LBS (USE (2) MTSM16 FOR UPLIFTS LESS THAN 1720#). NO MORE THAN 10 STRAPS MAY BE SUBSTITUTED OR NO MORE THAN 3 IN A ROW. IF GIRDER TRUSS CONNECTIONS ARE MISSED CONTACT

TERMITE SPECIFICATIONS:

SECTION R318 PROTECTION AGAINST TERMITES

TERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOIL APPLIED PESTICIDES, BAITING SYSTEMS, AND PESTICIDES APPLIED TO WOOD, OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE A PREVENTIVE TREATMENT TO NEW CONSTRUCTION (SEE SECTION 202, REGISTERED TERMITICIDE). UPON COMPLETION OF THE APPLICATION OF THE TERMITE PROTECTIVE TREATMENT, A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. TREATMENT IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES."

WHERE PROJECT IS TO BE LOCATED IN KNOWN RADON GAS PREVALENT AREAS. APPENDIX "F" OF THE 2017 FLORIDA RESIDENTIAL BUILDING CODE IS TO BE IMPLEMENTED. PLANS THAT INDICATE 2500 PSI SHALL BE REPLACED WITH

3. COARSE GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM

FTGS, WALLS, COLUMNS, BEAMS, SLABS:

MASONRY WALL CONST.

CAST IN PLACE CONCRETE

STEEL CONFORMING TO ASTM A-615 GRADE 40.

CORNER BARS WITH A 2'-1" LAP PROVIDED

AND 4" FOR SLABS

WITHIN THE 6".

BAR DIAMETERS TYP.

REINFORCING STEEL

INDICATED ON THE DRAWINGS:

FILLED CELL REINFORCING:

WELDED WIRE MESH:

TEMPERATURE REINFORCING:

OF BEAMS.

1. ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT

28 DAYS OF 2500 PSI, A SLUMP OF 3" FOR FOOTINGS/FOUNDATIONS

2. ALL REINFORCING STEEL SHALL BE NEW DOMESTIC DEFORMED BILLET

3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185. WWF SHALL

5. HORIZONTAL FOOTING BARS SHALL HAVE 1'-0" HOOK LENGTH OR

7. CONCRETE COVER MIN. 3" WHEN EXPOSED TO EARTH OR 1 1/2" TO

BE LAPPED AT LEAST 6" AND CONTAIN AT LEAST ONE CROSS WIRE

4. HOOKS SHALL BE PROVIDED AT DISCONTINUOUS ENDS OF ALL TOP BARS

6. MINIMUM LAP SPLICES ON ALL REINFORCING BAR SPLICES SHALL BE 40

ALL REINFORCING STEEL SHALL BE NEW DEFORMED BARS FREE FROM RUST,

SCALE & OIL & SHALL MEET ASTM A-615 REINFORCING FOR FOOTING SHALL

BE SUPPORTED ON PRE-CAST CONCRETE PADS, TOP REINFORCING SHALL BE

POSITIVELY SUPPORTED BY TEMPORARY STRINGERS. DOWELS FOR COLUMNS

WHERE PERMITTED SHALL BE THE FOLLOWING MINIMUM, UNLESS OTHERWISE

36 DIA. OR 2'-0" MIN.

40 DIA. OR 2'-1" MIN.

20 DIA. OR 1'-0" MIN.

8" LAP

& FILLED CELLS SHALL BE SECURED IN PLACE BY USING ADDITIONAL CROSS-

REINFORCING TIED TO FOOTING REINFORCING. SPLICES IN REINFORCING

1. HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N. TYPE 2. CONFORMING TO ASTM C90. WITH A MINIMUM NET COMPRESSIVE STRENGTH OF 1900 PSI (f'm = 1500 PSI) 2. MORTAR SHALL BE TYPE "S", CONFORMING TO ASTM C270.

AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH

AT 28 DAYS OF 3000 PSI SLUMP 8" TO 11" 4. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS

WITH THE CELLS FILLED WITH COARSE GROUT. 5. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 192 BAR DIAMETERS. REINFORCEMENT SHALL BE PLACED IN THE CENTER OF THE MASONRY CELL TYPICAL UNLESS OTHERWISE NOTED.

6. REINFORCING STEEL SHALL BE LAPPED A MINIMUM OF 40 BAR DIAMETERS, UNLESS OTHERWISE NOTED ON THE DRAWINGS.

7. GROUT STOPS SHALL BE PROVIDED BELOW BOND BEAM. PLASTIC SCREEN, METAL LATH STRIP OR CAVITY CAPS MAY BE USED TO PREVENT THE FLOW GROUT INTO CELLS BELOW. THE USE OF FELT PAPER AS A STOP IS PROHIBITED.

MAX. DRY DENSITY AS DETERMINED BY ASTM - D1557

USP AND SIMPSON CONNECTOR CROSS REFERENCE CHART

REF. NO./No. DE REF.	USP	REF. NO./No. DE REF.	USP	REF. NO./NO. DE REF.	USP	REF, NO NO DE REF.	USP	REF. NO.NO. DE REF.	USP	REF. NO./No. DE REF.	USP
MANAGEMENT PROPERTY AND ADDRESS OF THE PARTY	N10C-5	CBSQ66-SDS2HDG	THE RESIDENCE OF THE PARTY OF T	HH6	HH66	LSTA18	LSTA18	MSTAM36	MSTAM36		TTC42-TZ
TAXABLE PROPERTY AND ADDRESS OF THE PROPERTY O	N10C-1	CCQ44SDS2.5	KCCQ44	HHUS410	THD410	LSTA21	LSTA21	MTS12	MTW12	RTF2Z	TTF22-TZ
		THE RESERVE THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NAMED IN COLUMN TWO I	KCCQ46	HL33	KHL33	LSTA24	LSTA24	Contract of the Contract of th	MTW12-TZ	RTR	TTR-TZ
	LH12-HDG	CCQ46SDS2.5	THE RESERVE THE PERSON NAMED IN COLUMN TWO IS NOT THE OWNER.	A DESCRIPTION OF THE PERSON NAMED IN COLUMN 2 IS NOT THE OWNER, WHEN PERSON NA	KHL33-HDG	LSTA30	LSTA30		MTW16		TTU2-TZ
	TH12-HDG	CCQ66SDS2.5	KCCQ66	HL33HDG				A STATE OF THE PARTY OF THE PAR	MTW16-TZ		SB12
12L	L12	CPS4	CPB44	HL35	KHL35	LSTA36	LSTA36		Company of the Compan	A STATE OF THE PARTY OF THE PAR	LL915R50
12T	T1212	CPS5	CPB55	HL35HDG	KHL35-HDG	LSTA9	LSTA9		MTW18		
	T12	CPS6	CPB66	HL55HDG	KHL55-HDG	LSTHD8	LSTAD8		MTW20		LL930R50
	N16C-5	CS14-R	RS14-R	HRS12	HRS12	LSU26	LSSH15-TZ	MTS20Z	MTW20-TZ		LL915R50
THE RESIDENCE OF THE PERSON NAMED IN COLUMN 2 IS NOT THE PERSON NA	N16C-1	CS16	R\$150	HRS416Z	HRS416-TZ	LSU26Z	LSSH15-TZ	MTSM16	HTWM16	SD9112	LL915R50
		CS16-R	RS16-R	HRS6	HRS6	LTB20	N16	N10D5HDG	NA9D-5MC	SD9212	LL930R50
	L6	Mark of the second control of the second con		HRS8	HRS8	LTP4	MP4F		NA9D-1MC	SDS25112	WS15-BP
CHARLES AND ADDRESS OF THE PARTY OF THE PART	T6	CS18-R	R\$18-R	The same of the sa			MP6F		NA16D		WS15-GCR25
	L8	CS20	RS250	HS24	RT6	LTP5	Annual Street,	THE RESIDENCE OF THE PARTY OF T	NA11-5MC	THE RESERVE THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NAMED IN COLUMN TWO I	SPT22
21	JA1	CS22-R	RS22-R	HST2	KHST2	LTS12	LTW12		Company of the Compan		
21Z	JA1-TZ	DJT14Z	SDJT14-TZ	HST2HDG	KHST2-HDG	LTS16	LTW16		NA11-1MC	The second secon	SPT22-TZ
23	A3	DPPC4BK	PCP44	HST5	KHST5	LTT20B	LTS20B	NCA2x10-16	MBG1016		SPT24
777	A3-TZ	DPPC6BK	PCP66	HTP37Z	HTP37-TZ	LU26	JL26	NS1	KNS1	SP4	SPT4
	TDL5	DPT5Z	SDPT5-TZ	HTS16	HTW16	LU28	JL28	NS2	PL4	SP4Z	SPT4-TZ
	The second secon		SDPT7-TZ	HT\$20	HTW20	LUC210Z	JL210IF-TZ	AND DESCRIPTION OF THE PERSON NAMED IN COLUMN 2 IN COL	PA18	SP6	SPT6
DESCRIPTION OF THE PROPERTY OF	TDL10	DPT7Z	NAME AND ADDRESS OF THE OWNER, WHEN PERSON O	THE RESIDENCE OF THE PARTY OF T	CHARLES THE PROPERTY OF THE PARTY OF THE PAR		The second secon		PA23		SPTH4
	BL3	DTC	TR2	HTT5	HTT45	LUC26Z	JL26IF-TZ		AND DESCRIPTION OF THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NAMED I	THE RESIDENCE OF THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NAMED IN COLUMN	THE RESERVE THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NAMED IN COLUMN TWO I
34	MP34	DTT2Z-SDS2.5	DTB-TZ	HTU26	THD26	LUC28Z	JL28IF-TZ	A CONTRACTOR OF THE PARTY OF TH	TA51		SPTH4-TZ
Maria de la companya del la companya de la companya	MP34-SS	EB-TY	DC50-TZ	HU210	HD210	LUS210	JUS210	PA51Z	TA51-TZ		SPTH6
	MP34-TZ	ECC66	KECC66	HU26	HD26	LUS210-2	JUS210-2	PA68Z	TA71-TZ		SPTH6-TZ
	MPA1	ECCQ44SDS2.5	KECCQ44	HU410	HD410	LUS210-2Z	JUS210-2TZ	PB44	WE44	SS1.5	STS1
	The same of the sa	CANADA MARIA DE CANADA DE		HU410	HD410	LUS210-3	JUS210-3		WE44-TZ	SSPZ	RSPT6-TZ
Name and Address of the Owner, where the Park of the Owner, where the Owner, while the Owne	MPA1-SS	ECCQ46SDS2.5	KECCQ46	AND DESCRIPTION OF THE PARTY OF	AND DESCRIPTION OF THE PARTY OF		JUS210-3TZ		WE66-TZ		STB16-10
35Z	MPA1-TZ	ECCQ66SDS2.5	KECCQ66	HU412	HD412	LUS210-3Z	THE RESERVE OF THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NAMED IN COLUMN TW		NAME OF TAXABLE PARTY OF TAXABLE PARTY.		STB16-HDG1
44	BL4	EPB44	EPB4408	HU412	HD412	LUS210SS	JUS210-SS		WAS44		
NAME OF TAXABLE PARTY OF TAXABLE PARTY.	B66	EPB44A	EBG44-TZ	HUC410	HD410IF	LUS210Z	JUS210-TZ	The second secon	WAS46		STB20-10
	B88	EPB44HDG	EPB4408-HDG	HUC412	HD412IF	LUS24	JUS24	PC44-16	PCM4416		STB24-HDG1
	PA44-TZ or PAU44-TZ	EPB66	EPB6608	HUC44	HD44IF	LUS24Z	JUS24-TZ	PC46-16	PCM4616	ST12	ST12
	The state of the s	70.75		HUC46	HD46IF	LUS26	JUS26	PC66-16	PCM6616	ST18	ST18
	PA44E-TZ	EPC44-16	EPCM4416					PF24	JPF24		ST22
BA46Z	PA46E-TZ	EPC46-16	EPCM4616	HUC66	HD66IF	LUS26-2	JUS26-2		THE RESIDENCE OF THE PERSON NAMED IN COLUMN TWO IS NOT THE OWNER, THE PERSON NAMED IN COLUMN TWO IS NOT THE OWNER, THE PERSON NAMED IN COLUMN TWO IS NOT THE OWNER, THE PERSON NAMED IN COLUMN TWO IS NOT THE OWNER, THE OWN		K\$T216
BA66R	PA66ER-TZ	EPC64-16	EPCM6416	HUS1,81/10	HUS179	LUS26-2Z	JUS26-2TZ	PF24Z	JPF24-TZ		NAME AND ADDRESS OF THE OWNER, WHEN PERSONS ASSESSED.
	PA66E-TZ	EPC66-16	EPCM6616	HUS26	HUS26	LUS26SS	JUS26-SS	PF26	JPF26	THE RESIDENCE OF THE PERSON NAMED IN COLUMN 2 IS NOT THE OWNER, THE PERSON NAMED IN COLUMN 2 IS NOT THE OWNER.	KST224
	PAU44	FB24Z	FC24-TZ	HUS28	HUS28	LUS26Z	JUS26-TZ	PF26Z	JPF26-TZ	ST6236	KST234
	Contract of the Contract of th	FB26	FB26-TZ	ITS1.81/11.88	THO17118	LUS28	JUS28	PFD26	JDS26	ST9	ST9
	PAU44-TZ				TFL23118	LUS28-2	JUS28-2		PRT15-TZ		TR1
Application of the latest and the la	PAU46-TZ	FPBB44	BD	ITS2.37/11.88	THE RESIDENCE AND ADDRESS OF THE PARTY OF TH		JUS28-2TZ		PRT2H-TZ	and the second s	STAD10
BU66	PAU66	FPBM44E	SMP	ITS2.56/11.88	TFL25118	LUS28-2Z	THE RESIDENCE OF THE PERSON OF		AND DESCRIPTION OF THE PERSON NAMED IN COLUMN TWO IS NOT THE PERSON NAMED IN COLUMN TWO IS NAMED I	NAME AND ADDRESS OF THE OWNER, WHEN PERSON O	STAD14
BU66Z	PAU66-TZ	FPBS44	SFP30	IUS1.81/11.88	THF17112	LUS28-3	JUS28-3		PRT2-TZ		
C4	PBS44	GA1	A3	IUS2.37/11.88	THF23118	LUS28-3Z	JUS28-3TZ		PRT2H-TZ		SKH210L
	PBS44-TZ	GA2	A3	IUS2.56/11.88	THF25112	LUS28Z	JUS28-TZ	PGTIC2Z-R	PRTIC2-TZ		SKH210L-TZ
A CONTRACTOR OF THE PARTY OF TH		NAME AND ADDRESS OF THE OWNER, TH	RT15	IU32.56/16	THF25160	LUS410	JUS410	PS218	PS218-HDG	SUL26	SKH26L
C6	PBS66	H1	AND DESCRIPTION OF THE PERSON NAMED IN COLUMN TWO IS NOT THE OWNER.	A CONTRACTOR OF THE OWNER O	The same of the sa	LUS44	JUS44		PS418-HDG		SKH26L-TZ
C6Z	PBS66-TZ	H10-2	RT16-2	JB210	HL210	AND DESCRIPTION OF THE PERSON NAMED IN COLUMN 2 IS NOT THE PERSON	JUS44-TZ		PC12		SKH210R
CE4	PBES44	H10-2Z	RT16-2TZ	JB28	HL28	LUS44Z	The same of the sa		CONTRACTOR OF THE PROPERTY OF		SKH210R-TZ
CE6	PBES66	H10A	RT16A	L30	A3	LUS46	JUS46	The state of the s	PC12-BMC		Var. 14 a 8 1 Mars - 1 (9 cc - 1 dec
C4	C44	H10AZ	RT16A-TZ	L50	AC5	MAB15	ST1-TZ		PC1532		SKH26R
A STATE OF THE PARTY OF THE PAR	D44-TZ	H1Z	RT15-TZ	L50Z	AC5-TZ	MAB15Z	ST1-TZ	PSCL15/32-R50	PC1532-BMC	SUR26Z	SKH26R-TZ
	D44-TZ	H2.5A	RT7	L70	AC7	MAB23Z	ST2-TZ		PC1932	TA10	SCA10-TZ
C40Z				L70Z	AC7-TZ	MASA	FA3		PC1932-BMC		SCA10-TZ
C46	C46	H2.5A	RT7A	the same of the sa	THE RESIDENCE OF THE PROPERTY		FA3-TZ		PC58		SCA9-TZ
C4Z	C44-TZ	H2.5A33	RT7-35	L90	AC9	MASAZ	CONTRACTOR OF THE PROPERTY OF				MSH213
C6	C66	H2.5AZ	RT7A-TZ	L90Z	AC9-TZ	META20	HTA20-18		PC58-BMC		
C60Z	D66-TZ	H2.5AZ	RT7-TZ	LBP1/2	LBP12-TZ	MIT411.88	THO17118-2		PC716		MSH218
C82-2/4	BC400-TZ	H2A	RT10	LBP5/8Z	LBP58-TZ	ML24Z	ML24-TZ	The second secon	PC716-BMC	AND DESCRIPTION OF THE PARTY OF	MSH29
C82-2/4Z	BC400-TZ	H3	RT3A	LBPS1/2Z	LBPS12-TZ	ML26Z	ML26-TZ	PSPN516Z	ICPL516-TZ		MSH422
	CONTRACTOR OF THE PARTY OF THE	H3Z	RT3A-TZ	LBPS5/8	LBPS58-TZ	MP14	TPP14		ICPL58-TZ	THD50600HMG-RP	7750SD
P1/2-3HDG	HBPS12-HDG		The second state of the second			MP24	TPP24		RBC		7766SD
P5/8	BP583	H4	RT4	LBPS5/8Z	LBPS58-TZ				THR125-HDGBMC		NP15
P5/8-2HDG	BP582-HDGBC	H5	RT5	LCB44	CBE44	MP36	TPP36				
P5/8-3HDG	BP583-HDG	H6	LFTA6	LCE4	PBES44	MST27	KST227		THR5812-HDGBMC		NP35
P5/8HDG	BP583-HDG	H8	RT8A	LGT2	LUGT2	MST37	KST237		THR5816-HDGBMC	THE RESIDENCE AND ADDRESS OF THE PARTY OF TH	NP37
PS1/2-3	HBPS12	H8Z	RT8A-TZ	LPC4Z	PB44-6TZ	MST48	KST248	RFB#5X5	THR585-HDG		NP39
NAME AND ADDRESS OF THE OWNER, WHEN PERSON ADDRESS OF THE OWNER, WHEN PERSON AND ADDRESS OF THE OWNER, WHEN	HBPS58	HCP2	HHCP2	LPC6Z	PB66-6TZ	MST60	KST260		THR585-HDG		NP45
P\$5/8-3					THE RESIDENCE OF THE PERSON NAMED IN COLUMN TWO IS NOT THE OWNER, THE PERSON NAMED IN COLUMN TWO IS NAMED IN COLUMN TWO IS NAMED IN THE OWNER, THE PERSON NAMED IN THE OWNER, THE PERSON NAMED IN THE OWNER, THE	MSTA12	MSTA12	RFB#5X8	THR588-HDG	TP47	NP47
P\$5/8-3HDG	HBPS58-HDG	HD12	TDX14	L\$30	MP3					TP57	NP57
	WT22-BMC	HD3B	TDX2-TZ	LS50	MP5	MSTA12Z	MSTA12-TZ		KRPS18		
B44	KCB44	HD5B	TDX6	LS50Z	MP5-TZ	MSTA15Z	MSTA15-TZ		KRPS22	TSBR2-24	31-24
B44HDG	KCB44-HDG	HD7B	TDX10	LS70	MP7	MSTA18	MSTA18		KRPS22-TZ	TSP	LFTA6
B46	KCB46	HDU2-SDS2.5	PHD2A	LS70Z	MP7-TZ	MSTA18Z	MSTA18-TZ		KRPS28-TZ	TWB10	WBT10
						MSTA24	MSTA24	RR	RR	TWB12	WBT12
NAME OF TAXABLE PARTY.	KCB66	HDU4-SDS2.5	PHD4A	LS90	MP9					U210-2	SUH210-2
	KCB66-HDG	HDU5-SDS2.5	PHD5A	LSCZ	CSH-TZ	MSTA24Z	MSTA24-TZ	RSP4	RSPT4		
BSQ44-SDS2	CBSQ44-TZ	HDU8-SDS2.5	PHD8	LSSU210	LSSH210	MSTA30	MSTA30	RTA12	TTA12-TZ		SRC
		HETA20	HTA20	LSSU28	LSSH15-TZ	MSTA30Z	MSTA30-TZ	RTA2Z	TTA2-TZ	W1	WG
RSOME SUSSEMENT	The entire of all all a property	AND DESCRIPTION OF THE PROPERTY OF THE PROPERT		LSSUI25	LSSH179	MSTA36	MSTA36	RCWB12	WBC12	WB126	WB126
BSQ44-SDS2HDG	CDCOAR TT						TOTAL PROPERTY.		B - E - OF -		
THE RESERVE OF THE PERSON NAMED IN COLUMN 2 IS NOT THE OWNER.	CBSQ46-TZ	HGA10KT HGAM10KTA	HGA10KT HGAM10KT	LSTA12	LSTA12	MSTA36Z	MSTA36-TZ	RTB22	TTB22-TZ	WB126C	RWB114

AND ANCHOR BOLTS ARE ONLY REQUIRED ON MEMBERS IN WALLS THAT ARE EXPOSED TO UPLIFT FORCES. INTERIOR LOAD BEARING WALLS ARE NOT ALWAYS EXPOSED TO UPLIFT FORCES. THE MEMBERS OF THESE WALLS WOULD NOT NEED TO HAVE CONNECTORS APPLIED. PLEASE CONSULT THE TRUSS ENGINEERING FOR THE LOCATION OF THESE WALLS. FIELD REPAIR NOTES

1. UPLIFT CONNECTORS SUCH AS HURRICANE CLIPS, TRUSS ANCHORS

UPLIFT CONNECTORS

MISSED "J" BOLTS FOR WOOD BEARING WALLS MAY BE SUBSTITUTED W/ 1/2" DIA. EPOXY ANCHORS WITH 6" EMBEDMENT. SIMPSON "SET" EPOXY ADHESIVE BINDER FOLLOWING ALL MANUFACTURERS RECOMMENDATIONS. SEE PLAN FOR EMBEDMENT DEPTH AT FLOOR

2. FOR MISSED VERT. DOWELS DRILL A 3/4" DIAMETER HOLE 6" DEEP AT THE LOCATION OF THE OMITTED REBAR, AND INSTALL A 32" LONG #5 BAR INTO THE EPOXY FILLED HOLE. USE A TWO PART EMBEDMENT EPOXY (SIMPSON "SET", EPOXY), MIXED PER MANUFACTURER'S INSTRUCTIONS ASSURE THAT ALL DUST AND DEBRIS FROM DRILLING ARE REMOVED

ENGINEER OF RECORD FOR SUBSTITUTION.

CONCRETE STRENGTH IN THESE AREAS ARE TO BE A MINIMUM OF 3000 P.S.I.. THEREFORE, ANY AND ALL NOTES ON THESE 3000 P.S.I. FOR THE CONCRETE STRENGTH.

OCCUPANCY

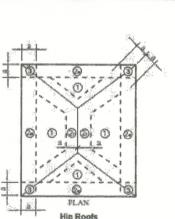
BUILDING INFO

WIND LOADING CRITERIA WIND SPEED (ULTIMATE) 130 MPH 101 MPH WIND SPEED (ALLOWABLE) **EXPOSURE CATEGORY BUILDING CATEGORY**

ENCLOSURE CLASSIFICATION ENCLOSED INTERNAL PRESSURE COEFFICIENT +/- 0.18

CON	/PONENT	& CLADI	DING LOAD			
HIP ROOF > 27 TO 45 DEGREES						
ZONE	WIND AREA	Poc	Nog			

	ZONE	WIND AREA	Pos	Neg				
	1	10	13.0	-26.0				
	1	20	11.3	-23.0				
_	1	50	10.0	-19.2				
	1	100	10.0	-16.2				
	2e	10	13.0	-30.9				
	2e	20	11.3	-24.4				
	2e	50	10.0	-15.1				
	2e	100	10.0	-15.1				
	2r	10	13.3	-39.1				
	2r	20	11.3	-32.8				
	2r	50	10.0	-24.5				
	2r	100	10.0	-18.2				
	3	10	13.0	-41.7				
	3	20	11.3	-31.4				
	3	50	10.0	-18.2				
	3	100	10.0	-18.2				
	WALLS							
	ZONE	WIND AREA	Pos	Neg				
	4	10	18.2	-19.8				
	4	20	17.4	-19.0				
	4	50	16.3	-17.9				
	4	100	15.5	-17.1				



NOTICE TO BUILDER

INSTEAD CALL THE DESIGNER LISTED IN TITLE PAGE*

SHEET NO.

allister