

**TYPICAL WALL SECTION**  
SCALE: 1" = 1'-0"

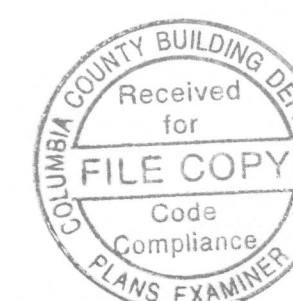


**REAR ELEVATION**  
SCALE: 1/4" = 1'-0"



**FRONT ELEVATION**  
SCALE: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS



WMLC 7m

REVISIONS	DATE
February 15, 2022	

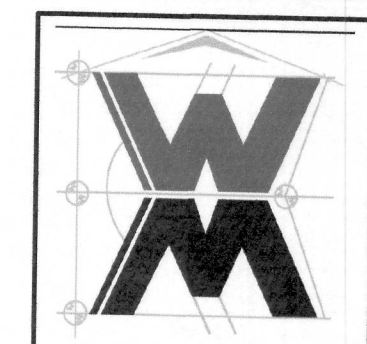
SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE

**FRONT & REAR ELEVATIONS**  
SCALE: 1/4" = 1'-0"

**TYPICAL WALL SECTION**  
SCALE: 1" = 1'-0"

MODEL 1880 FOR: Lot 50, Emerald Cove  
**Glen and Tavia Weatherly**  
Property Address: 265 SW Fieldstone Court, Lake City, Florida 32025  
**GIBALTAR CONTRACTING, LLC.**  
LIC# 1259633 HIGH SPRINGS, FLORIDA

© WML DESIGN & ASSOCIATES, INC.  
426 SW COMMERCE DR. STE 130  
LAKE CITY, FL 32025  
(386) 758-8406  
wml@wmlmyers.net

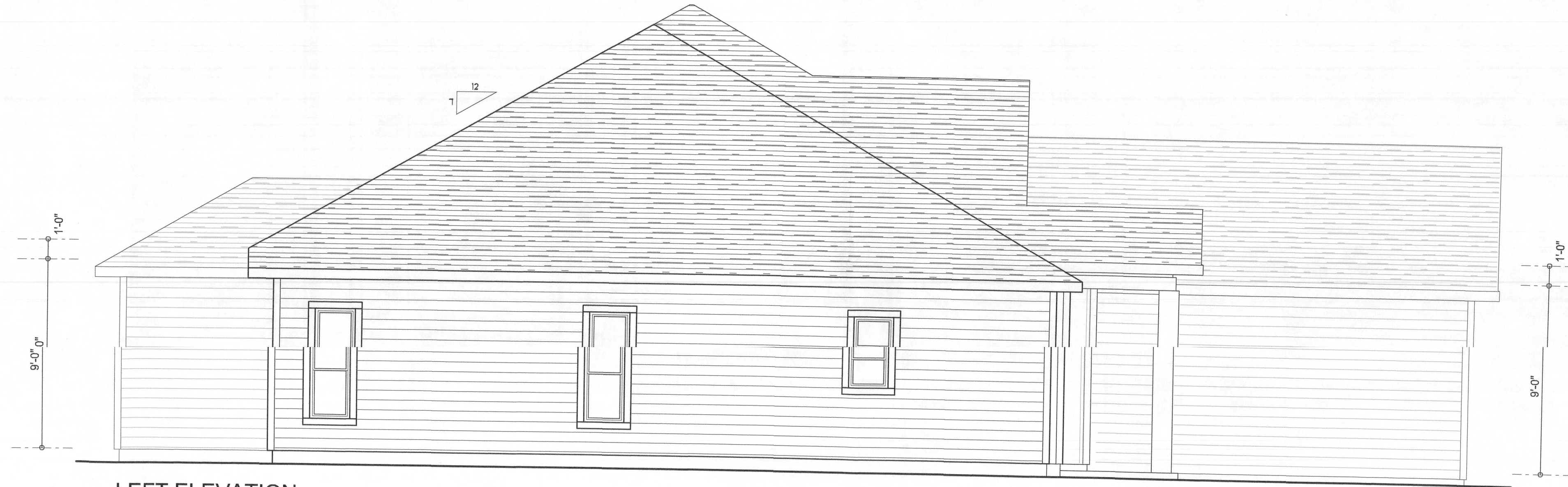


JOB NUMBER  
20220216

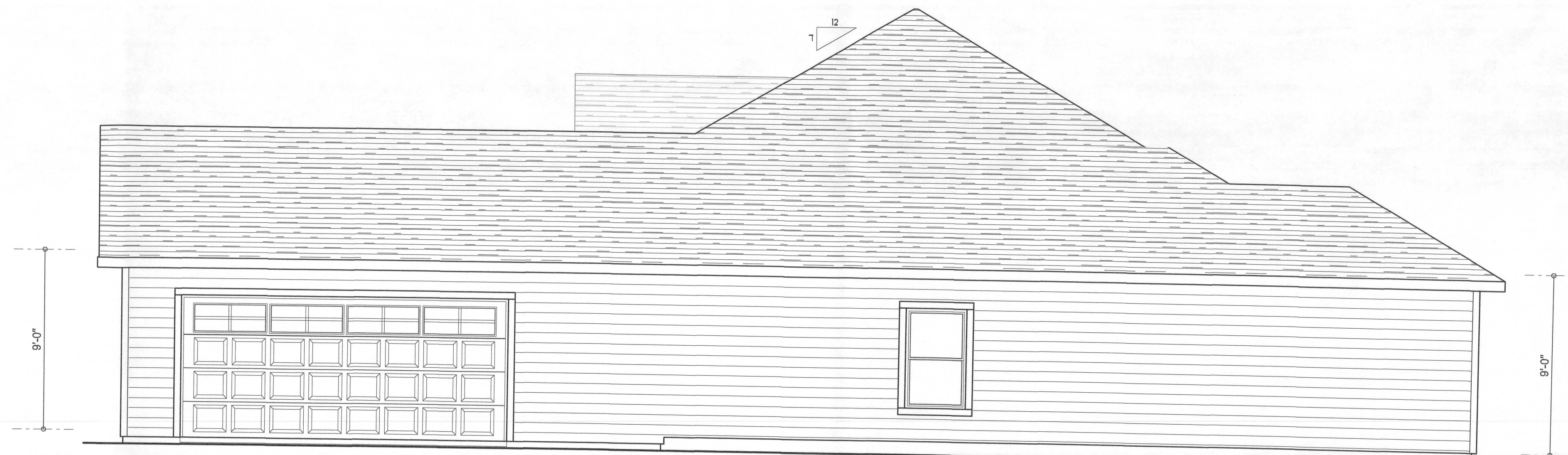
SHEET NUMBER

**A.1**  
OF 4 SHEETS





**LEFT ELEVATION**  
SCALE: 1/4" = 1'-0"



**RIGHT ELEVATION**  
SCALE: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

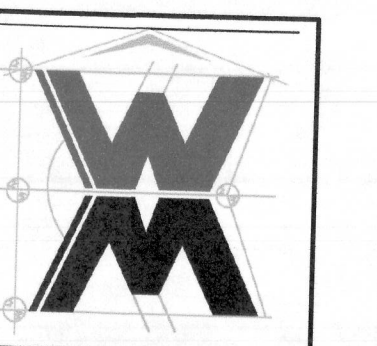
REVISIONS
February 15, 2022



**LEFT & RIGHT ELEVATIONS**  
SCALE: 1/4" = 1'-0"

MODEL 1880 FOR: 50, Emerald Cove  
**Glen and Tavia Weatherly**  
Property Address: 5 SW Fieldstone Court, Lake City Florida 32025  
**GIBRALTR CONTRACTING, LLC.**  
LIC# 1259633 HH SPRINGS, FLORIDA

© WM DESIGN & ASSOCIATES, INC.  
426 SW COMMERCE DR, STE 130  
LAKE CITY, FL 32025  
(386) 758-8406  
wm@gwillmiers.net



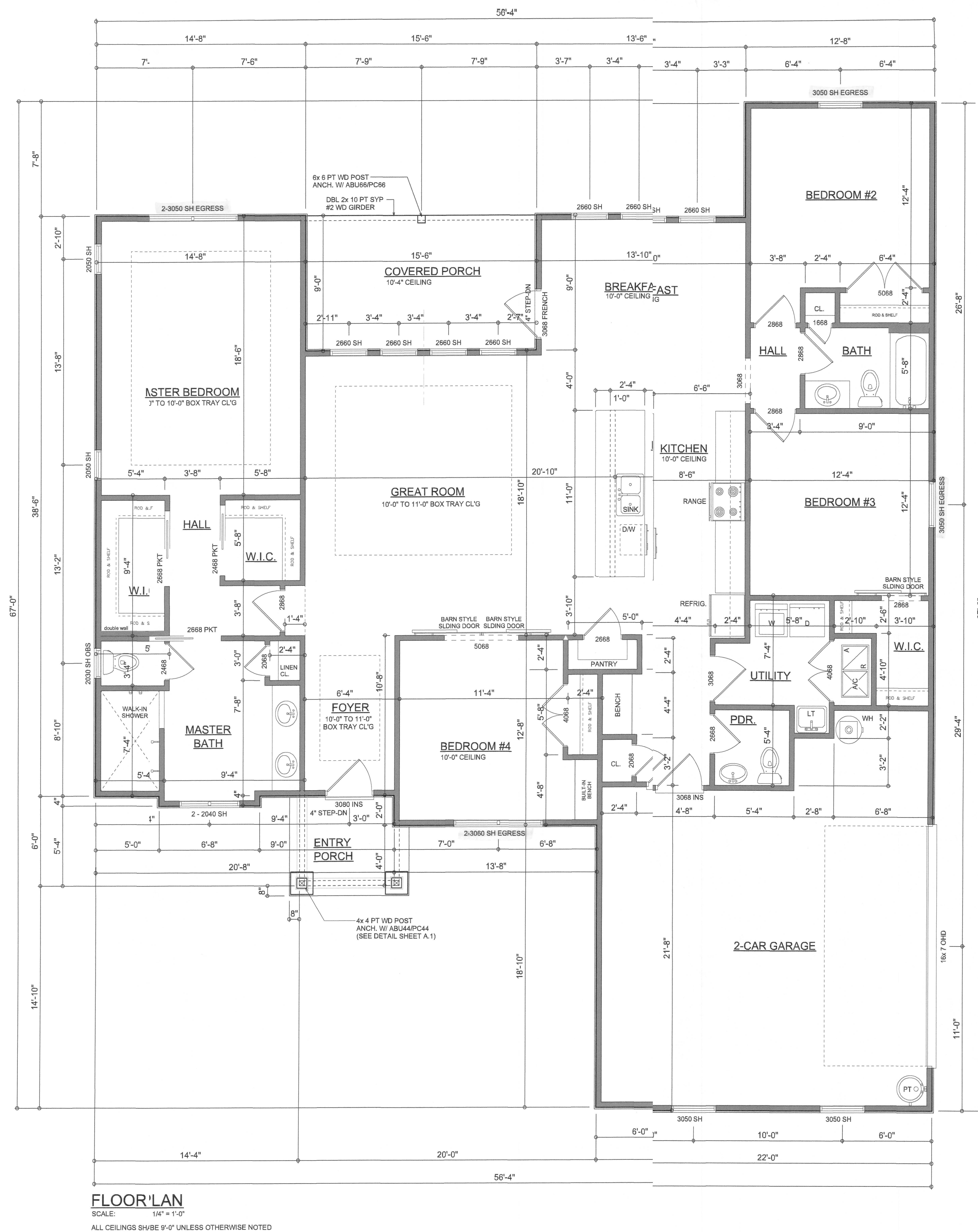
JOB NUMBER  
20220216

SHEET NUMBER

**A.2**  
OF 4 SHEETS

*Wm C. Gm*





# FLOOR PLAN

SCALE: 1/4" = 1'-0"

ALL CEILINGS SH/BE 9'-0" UNLESS OTHERWISE NOTED

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

## Garage fire separations shall comply with the following:

1. The private garage shall be separated from the dwelling unit and its attic area by means of a minimum 1/2-inch (12.7 mm) gypsum board applied to the garage side. Garages beneath habitable rooms shall be separated from all habitable rooms above by not less than 5/8-inch Type X gypsum board or equivalent. Door openings between a private garage and the dwelling unit shall be equipped with either solid wood doors, or solid or honeycomb core steel doors not less than 13/8 inches (34.9 mm) thick, or doors in compliance with Section 715.3.3. Openings from a private garage directly into a room used for sleeping purposes shall not be permitted.
2. Ducts in a private garage and ducts penetrating the walls or ceilings separating the dwelling unit from the garage shall be constructed of a minimum 0.019-inch (0.48 mm) sheet steel and shall have no openings into the garage.
3. A separation is not required between a Group R-3 and U carport provided the carport is entirely open on two or more sides and there are not enclosed areas above.
4. When installing an attic access and/or pull-down stair unit in the garage, devise shall have a minimum 20 min. fire rating.

## AREA SUMMARY

LIVING AREA	2,105	S.F.
GARAGE AREA	512	S.F.
ENTRY PORCH AREA	50	S.F.
COVERED PORCH AREA	140	S.F.
TOTAL AREA	2,807	S.F.

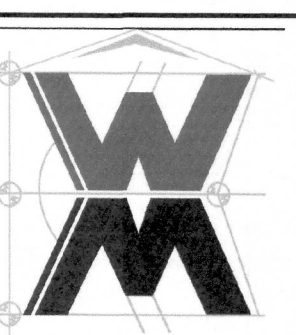
REVISIONS  
April 28, 2022

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE

DIMENSIONED FLOOR PLAN  
SCALE: 1/4" = 1'-0"

MODEL 1880 FOR: Lot 50, Emerald Cove  
**Glen and Tavia Weatherly**  
Property Address: 265 SW Fieldstone Court, Lake City, Florida 32025  
**GIBALTAR CONTRACTING, LLC.**  
LIC# 1259633 HIGH SPRINGS, FLORIDA

© W.M. DESIGN &  
ASSOCIATES, INC.  
426 SW COMMERCE DR., STE. 130  
LAKE CITY, FL 32025  
(386) 758-8406  
wm@willmyers.net



JOB NUMBER  
20220216

SHEET NUMBER

**A.3**  
OF 4 SHEETS

Will C. Myers



ELECTRICAL LEGEND	
	CEILING FAN (PRE-WIRE FOR LIGHT KIT)
	DOUBLE SECURITY LIGHT
	RECESSED CAN LIGHT
	BATH EXHAUST FAN
	LIGHT FIXTURE
	DUPLEX OUTLET (AFCI & TAMPER RESISTANT)
	220v OUTLET
	GFI DUPLEX OUTLET (PER NEC 406.8)
	TELEVISION JACK
	CIRCUIT FOR MINI-SPLIT A/C UNIT
	SMOKE / CARBON MONOXIDE DETECTOR (see note below)
	WALL SWITCH
	3 WAY WALL SWITCH
	WATER PROOF GFI OUTLET
	2 OR 4 TUB FLUORESCENT FIXTURE

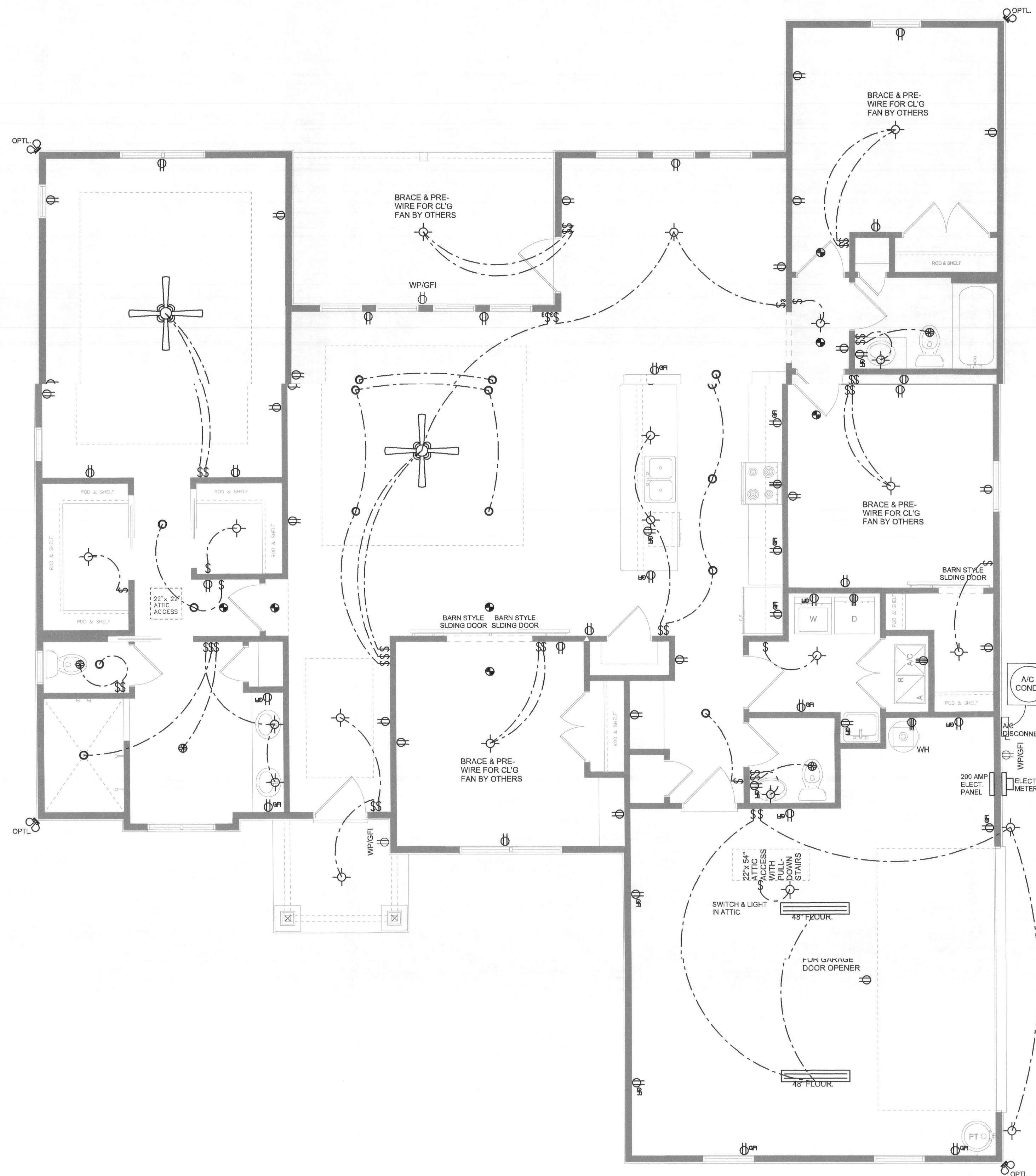
NOTE:  
ALL INTERIOR RECEPTACLES SHALL BE AFCI  
(ARC FAULT CIRCUIT INTERRUPT) PER NEC 210.12 & TAMPER RESISTANT PER  
NEC 406.11

ALL INTERIOR & EXTERIOR LIGHTING SHALL MEET OR EXCEED THE MIN. 75% HIGH-EFFICIENCY  
LIGHTING PER FBC-ENERGY CONSERVATION R404.

ALL SMOKE DETECTORS BE A COMBO SMOKE & CARBON MONOXIDE DETECTOR  
AND SHALL HAVE BATTERY BACKUP POWER  
AND ALL WIRED TOGETHER SO IF ANY ONE UNIT IS ACTUATED THEY  
ALL ACTIVATE.

THE ELECTRICAL SERVICE OVERCURRENT PROTECTION DEVICE SHALL BE  
INSTALLED ON THE EXTERIOR OF STRUCTURES TO SERVE AS A DISCONNECT MEANS.  
CONDUCTORS USED FROM THE EXTERIOR DISCONNECTING MEANS TO A PANEL OR SUB  
PANEL SHALL HAVE FOUR-WIRE CONDUCTORS, OF WHICH ONE CONDUCTOR  
SHALL BE USED AS AN EQUIPMENT GROUND.

IT IS THE LICENSED ELECTRICAL CONTRACTORS RESPONSIBILITY TO INSURE THAT ALL  
WORK PERFORMED AND EQUIPMENT INSTALLED MEETS OR EXCEEDS THE 2017 (NFPA-70) NATIONAL  
ELECTRIC CODE AND ALL OTHER LOCAL CODES AND ORDINANCES.



ELECTRICAL PLAN  
SCALE: 1/4" = 1'-0"

NOTE: ALL DRAWINGS NOT TO BE SCALED, WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS

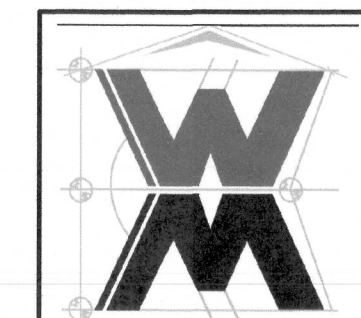
REVISIONS
April 28, 2022

SOFTPLAN  
ARCHITECTURAL DESIGN SOFTWARE

ELECTRICAL PLAN  
SCALE: 1/4" = 1'-0"

MODELER FOR: Lot 50, Emerald Cove  
Glin and Tavia Weatherly  
Project Address: 265 SW Fieldstone Court, Lake City, Florida 32025  
GIRALTAR CONTRACTING, LLC.  
LIC# 39633 HIGH SPRINGS, FLORIDA

© WM DESIGN & ASSOCIATES, INC.  
426 SW COMMERCE DR., STE 130  
LAKE CITY, FL 32025  
(386) 758-8406  
wm@wmdesigners.net

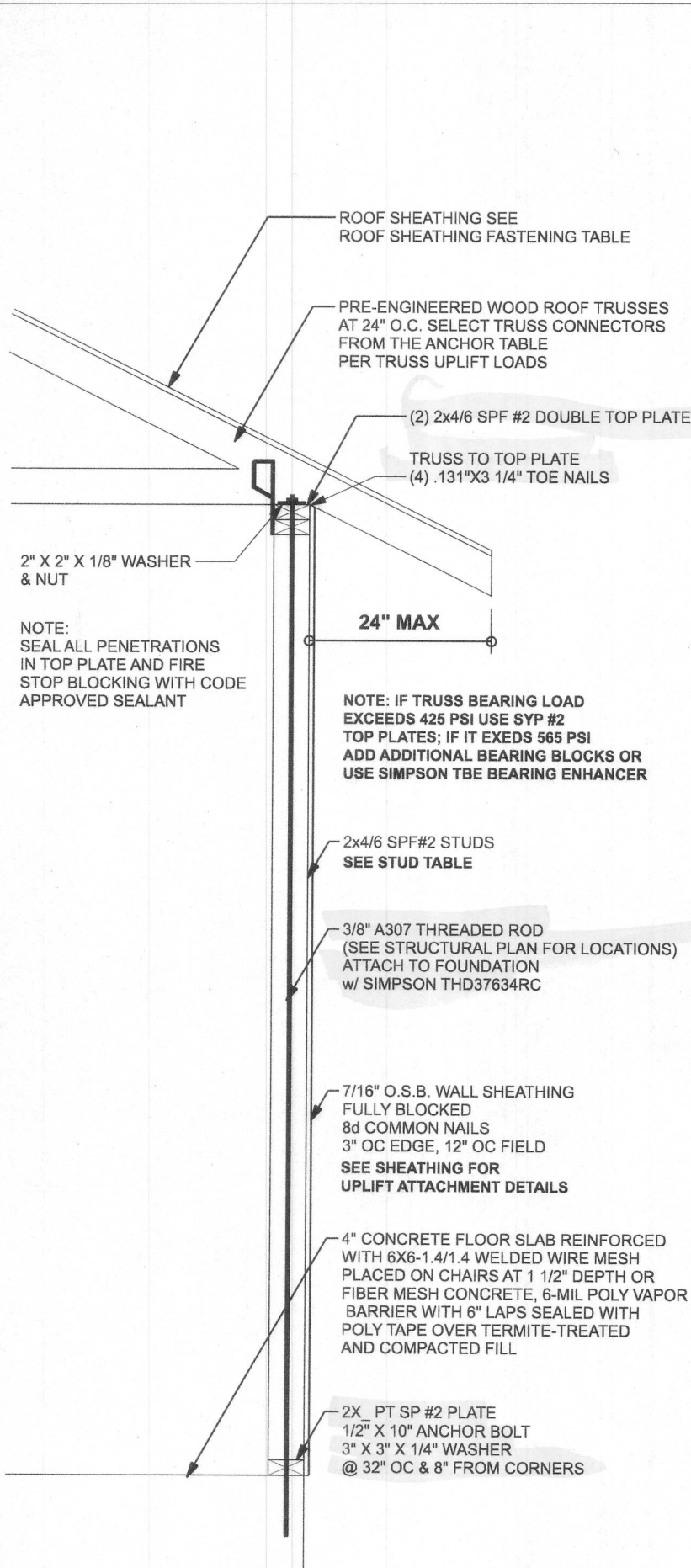


JOB NUMBER  
20220216

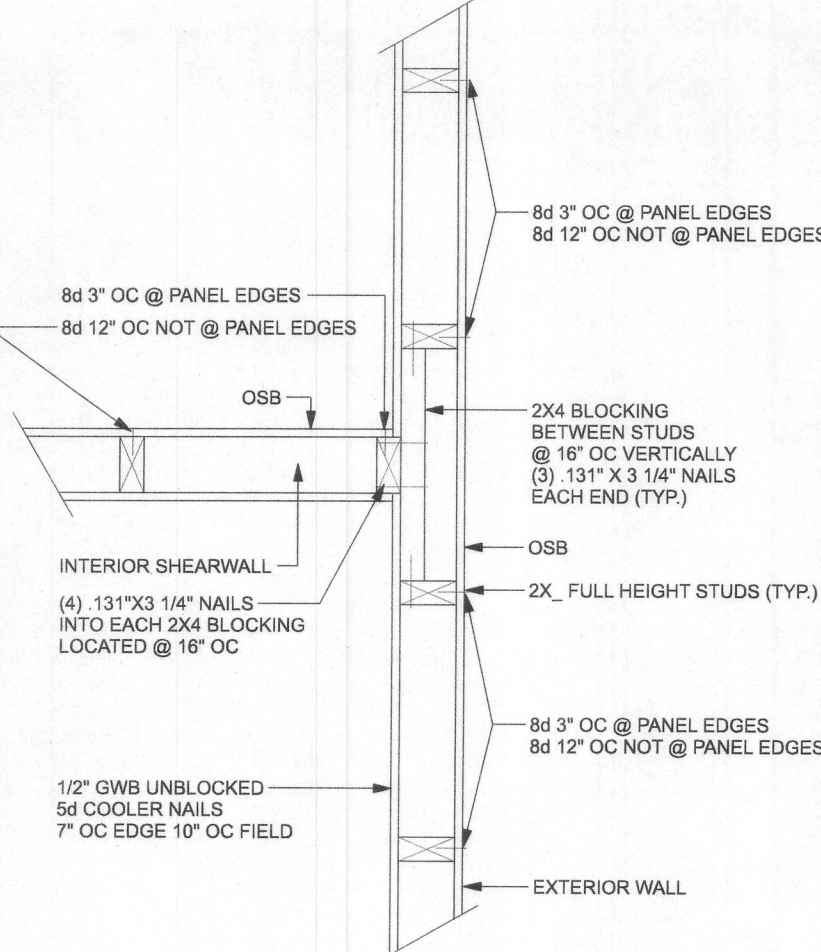
SHEET NUMBER  
A.4  
OF 4 SHEETS

*Wm C. Glin*

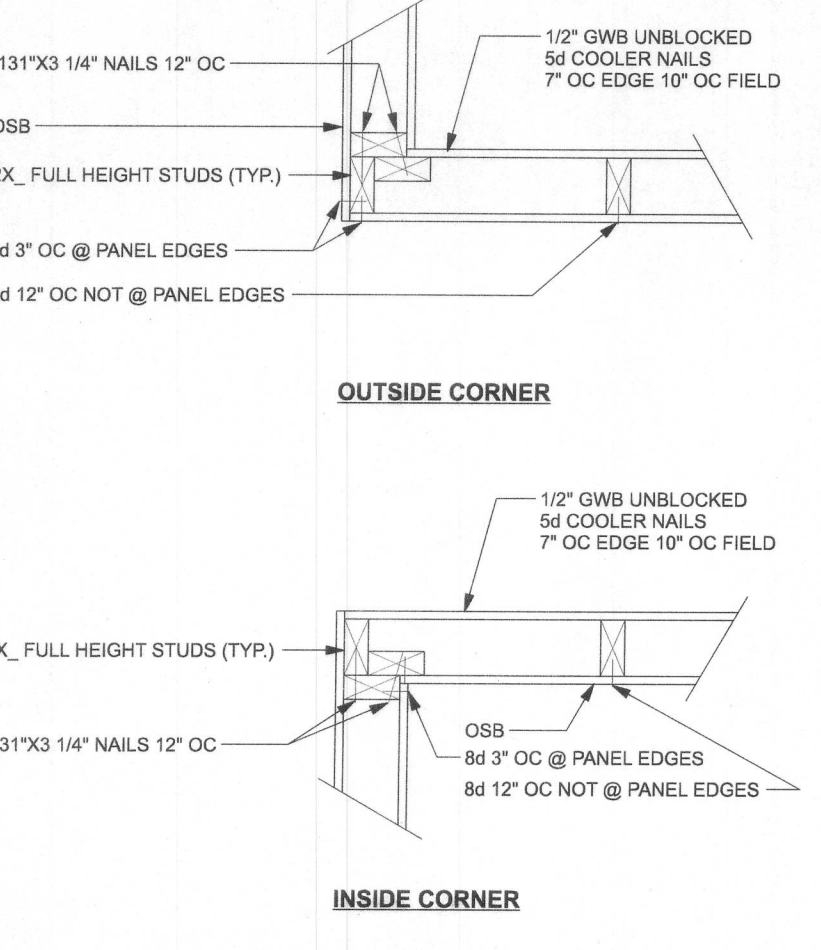




**ONE STORY WALL SECTION**  
SCALE: 3/4" = 1'-0"



**(TYP.) INTERSECTING WALL FRAMING**  
WOOD FRAME

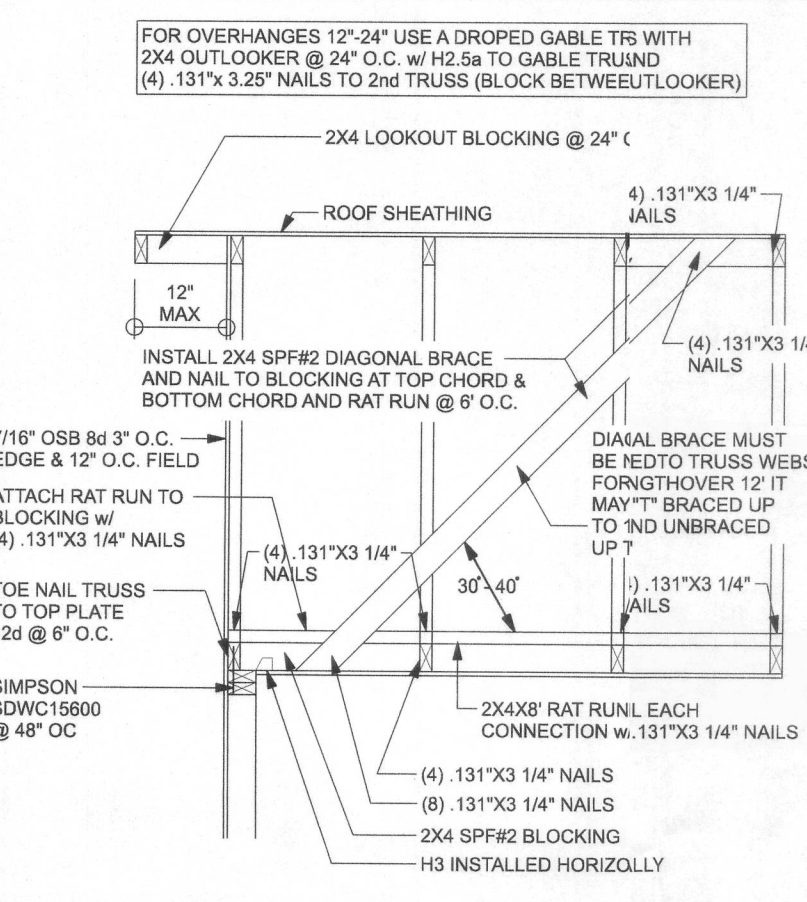


**(TYP.) CORNER FRAMING**  
WOOD FRAME

**ROOF SHEATHING FASTENING TABLE (RAFTER / TSS SG = 0.49)**

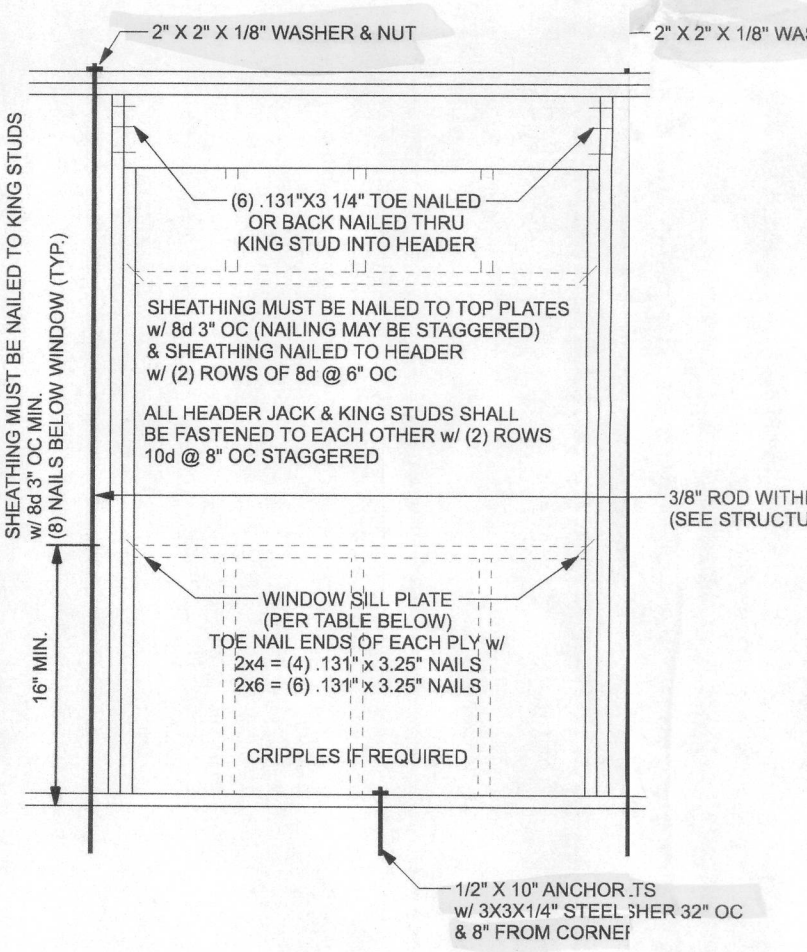
Wind Speed	Sheathing Thickness Plywood Or OSB	Required Nail	Nail spacing along panel edges	Nail spacing along intermediate supports in the panel field
120 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc	6" oc
120 mph Exp. C	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc	6" oc
120 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc	6" oc
130 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
130 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. B	7/16"	ASTM F1667 RSR-01 (2 3/8" x 0.131")	6" oc	6" oc
140 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
140 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. C	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc
150 mph Exp. D	19/32"	ASTM F1667 RSR-03 (2 1/2" x 0.131") or ASTM F1667 RSR-04 (3" x 0.120")	6" oc	6" oc

Note: For sheathing located a minimum of 4 feet from the perimeter edge of a roof, including 4 feet on each side of ridges and hips, nail spacing is permitted to be 6 inches on center along panel edges and 8 inches on center along intermediate supports in the panel field. Notes table specifies the code minimum thickness of roof sheathing. The thickness of the sheathing need to be increased based in the type of roofing material being used. See manufacturer Floridalect approval.

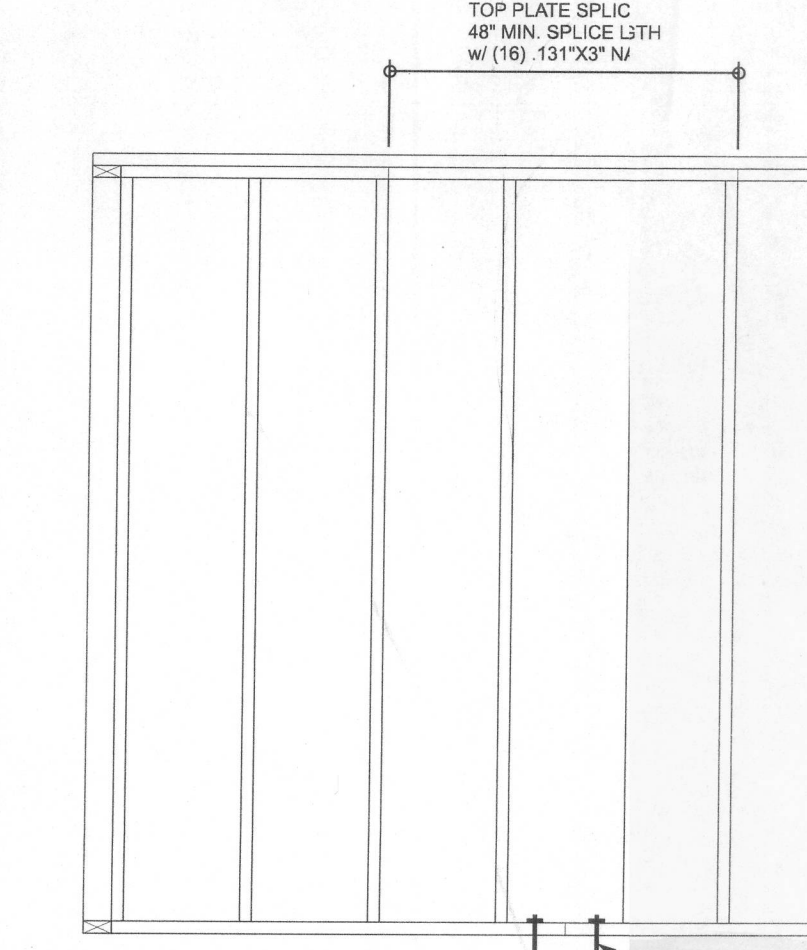


**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME

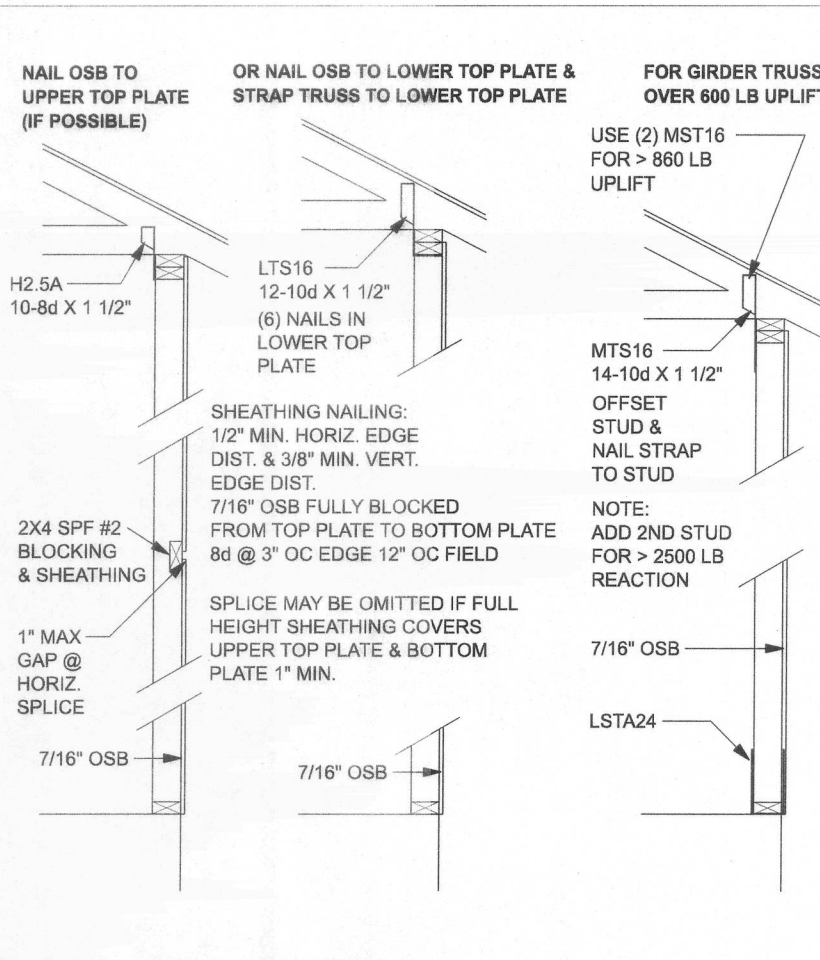
**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME



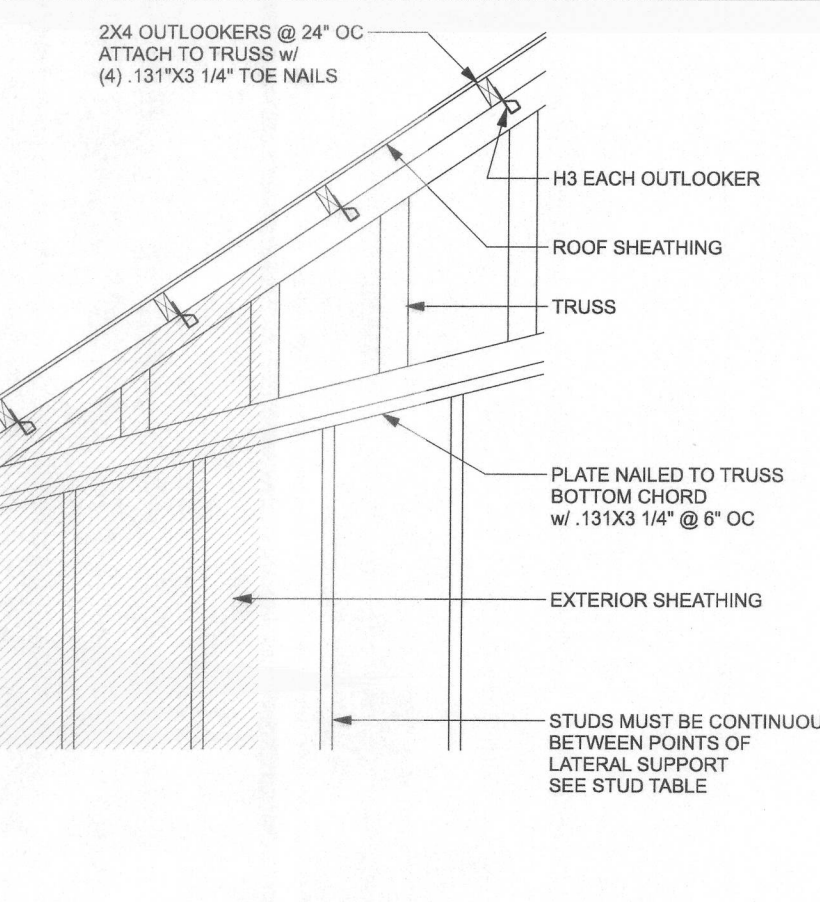
**(TYP.) INTERSECTING WALL FRAMING**  
WOOD FRAME



**(TYP.) CORNER FRAMING**  
WOOD FRAME

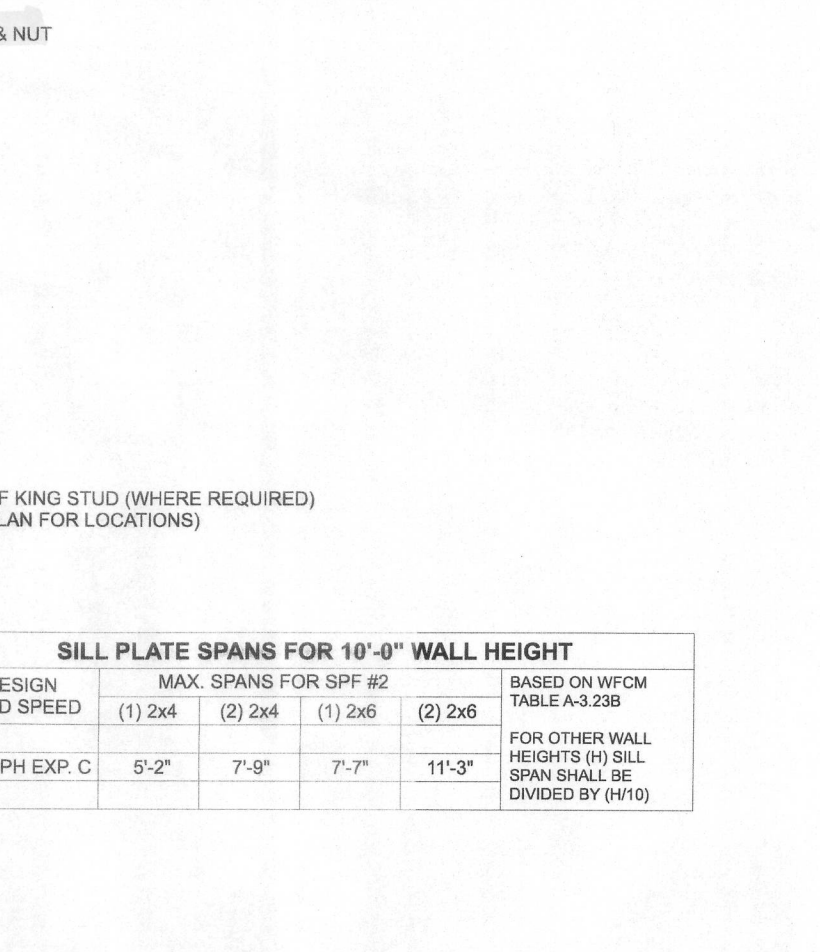


**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME

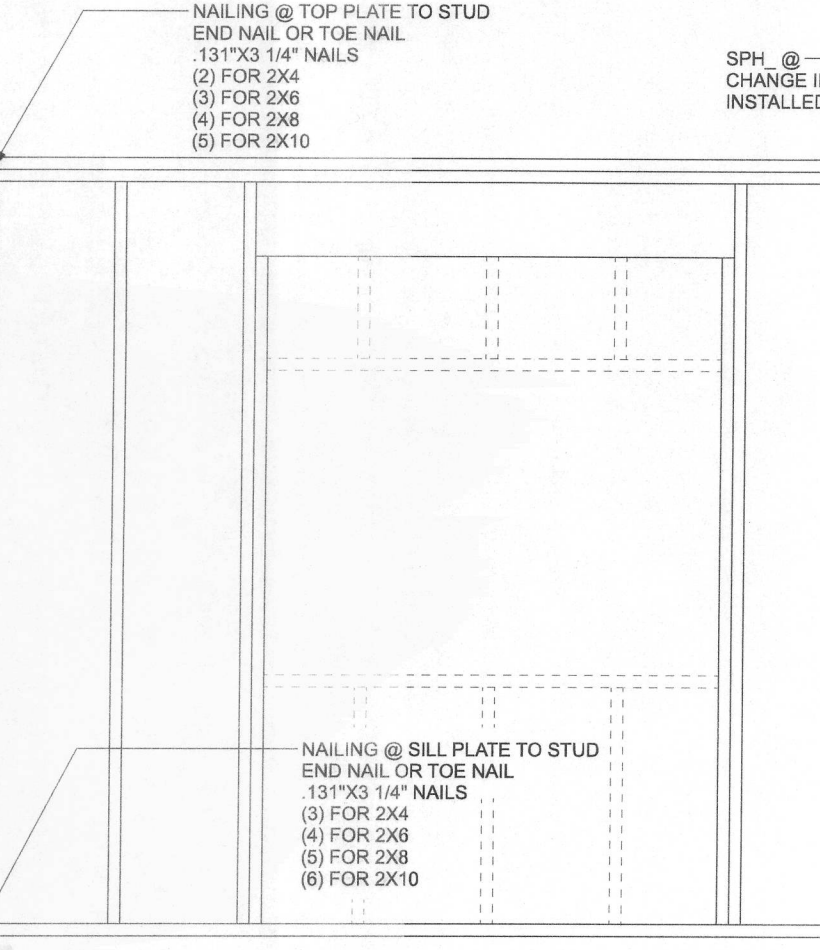


**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME

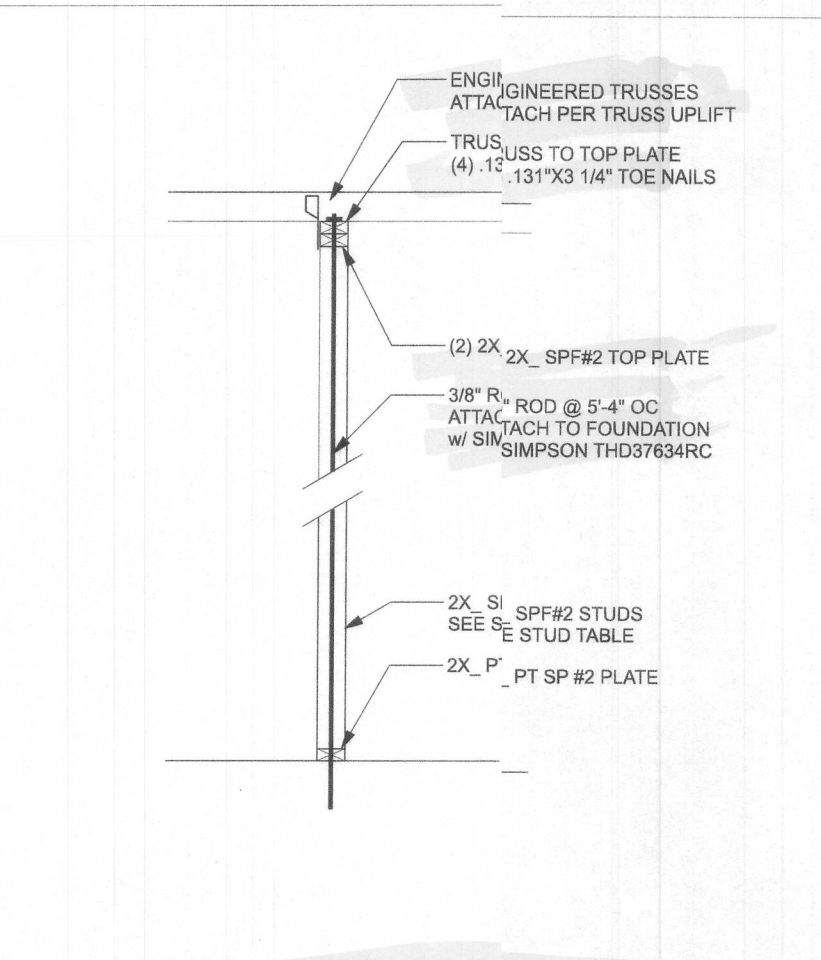
**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME



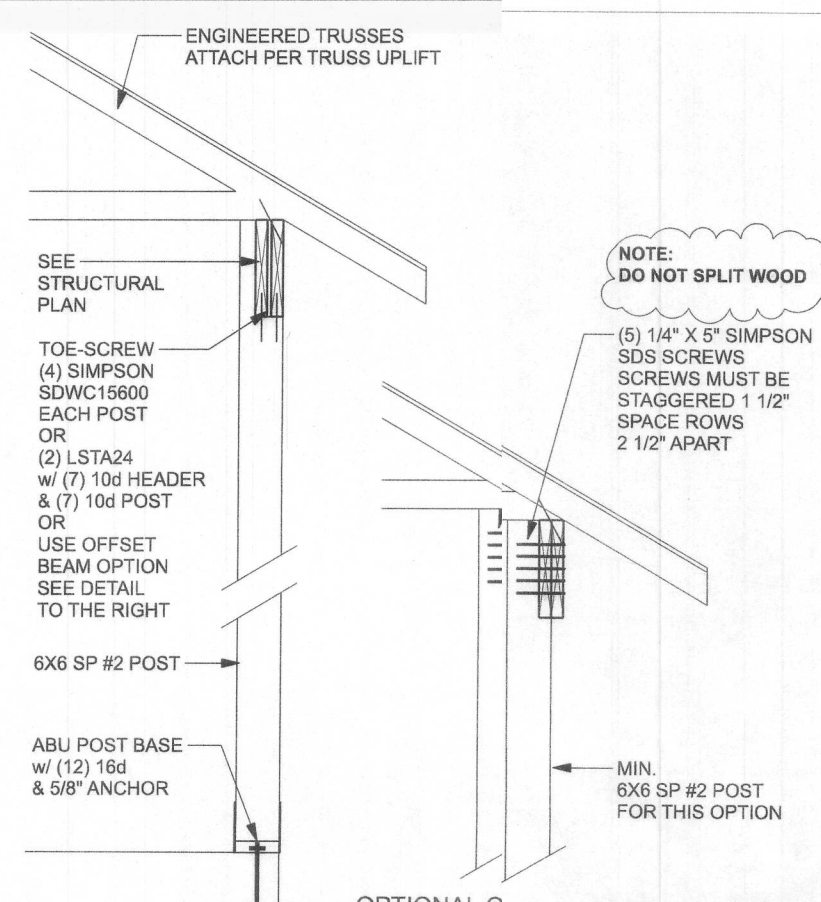
**(TYP.) INTERSECTING WALL FRAMING**  
WOOD FRAME



**(TYP.) CORNER FRAMING**  
WOOD FRAME

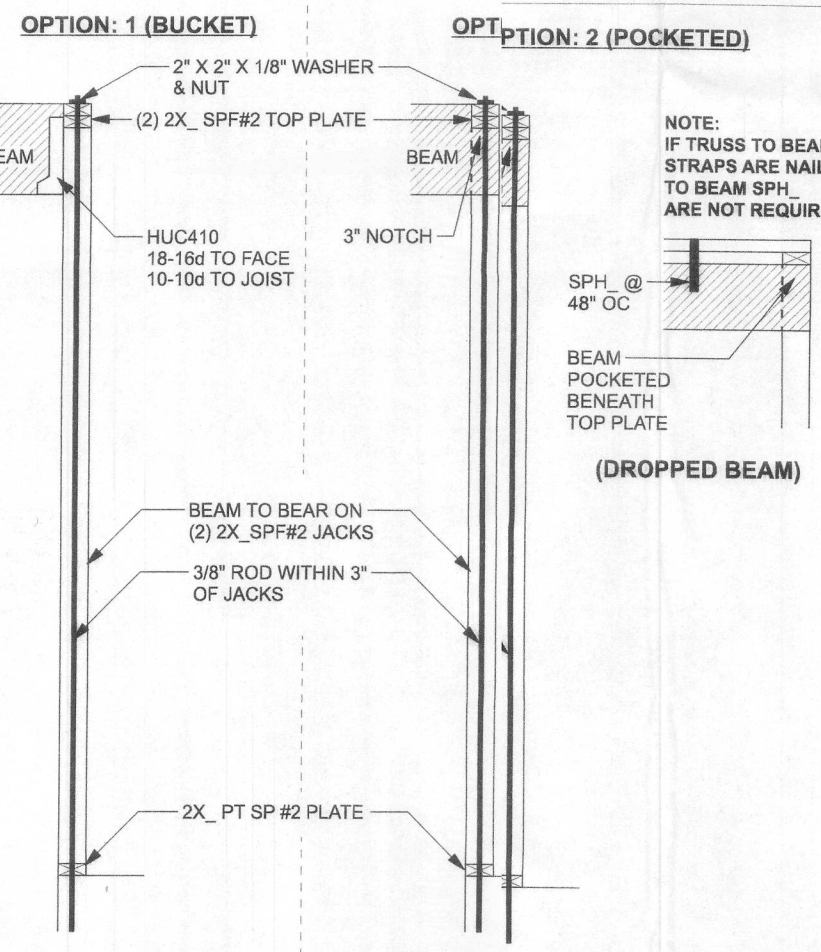


**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME

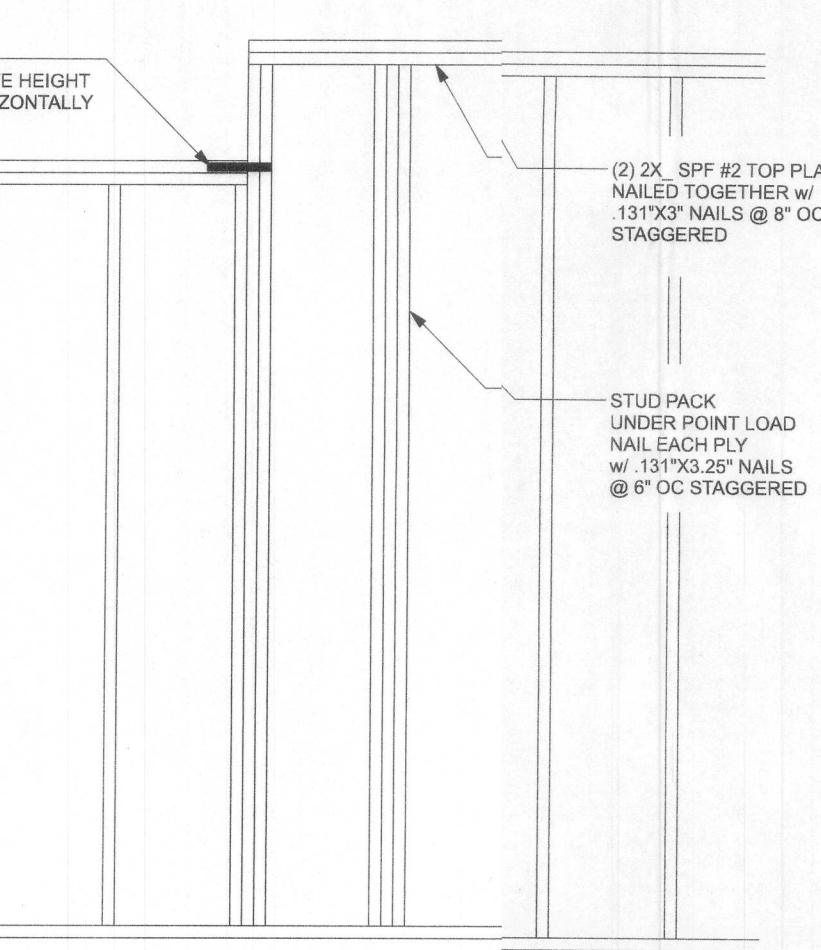


**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME

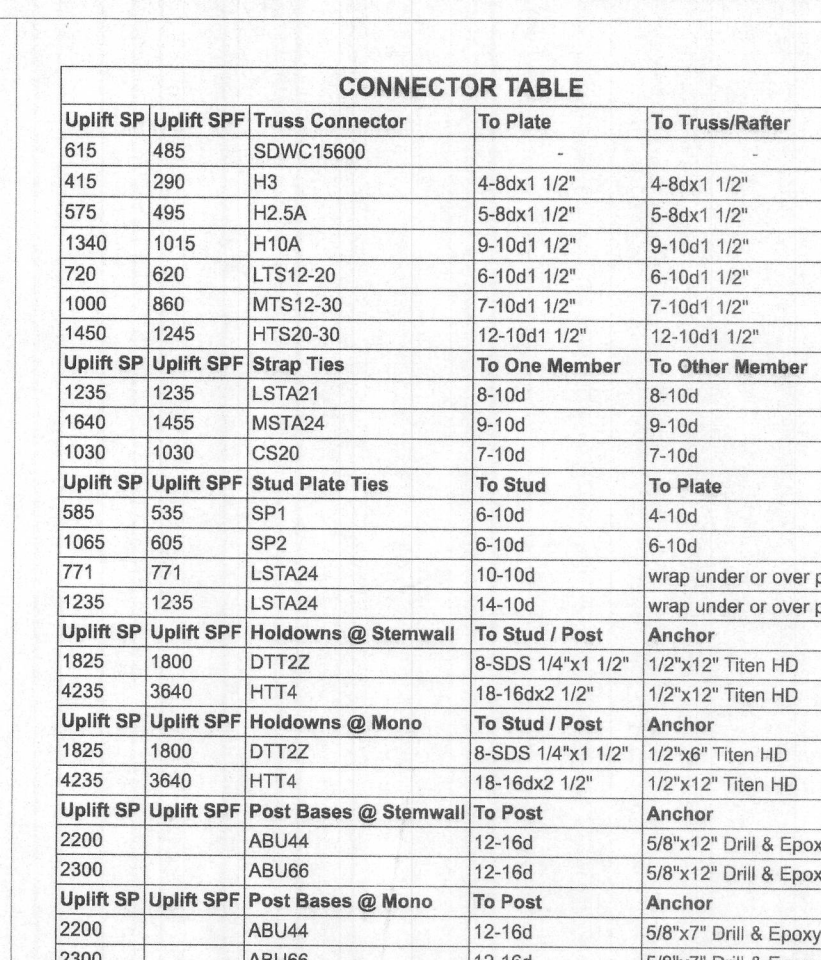
**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME



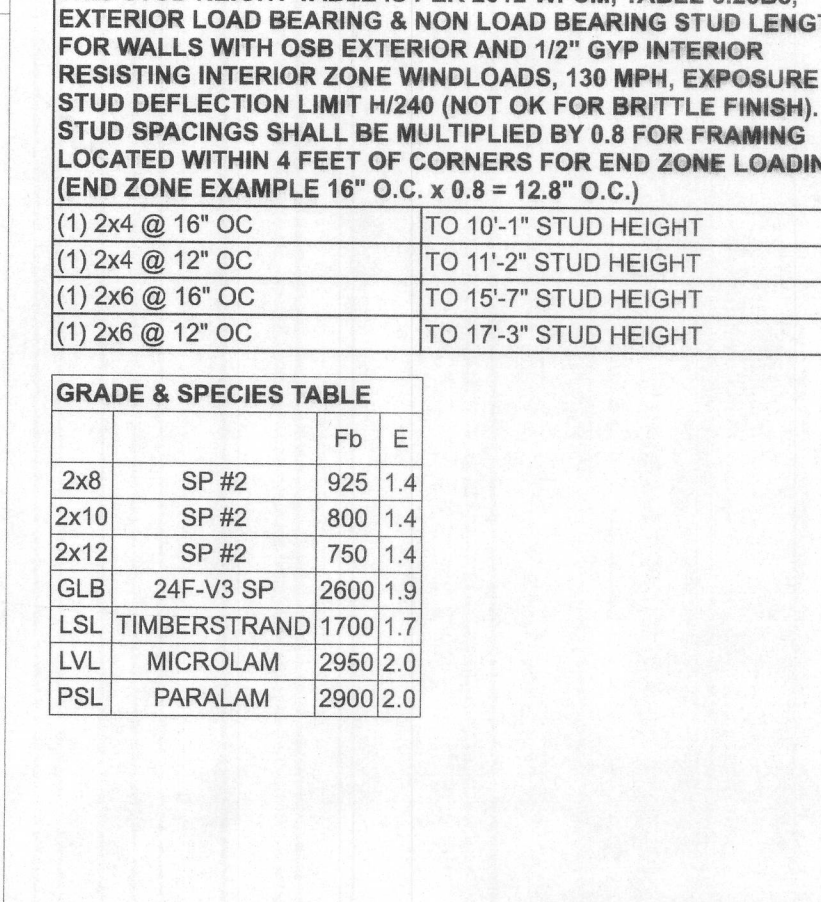
**(TYP.) INTERSECTING WALL FRAMING**  
WOOD FRAME



**(TYP.) CORNER FRAMING**  
WOOD FRAME

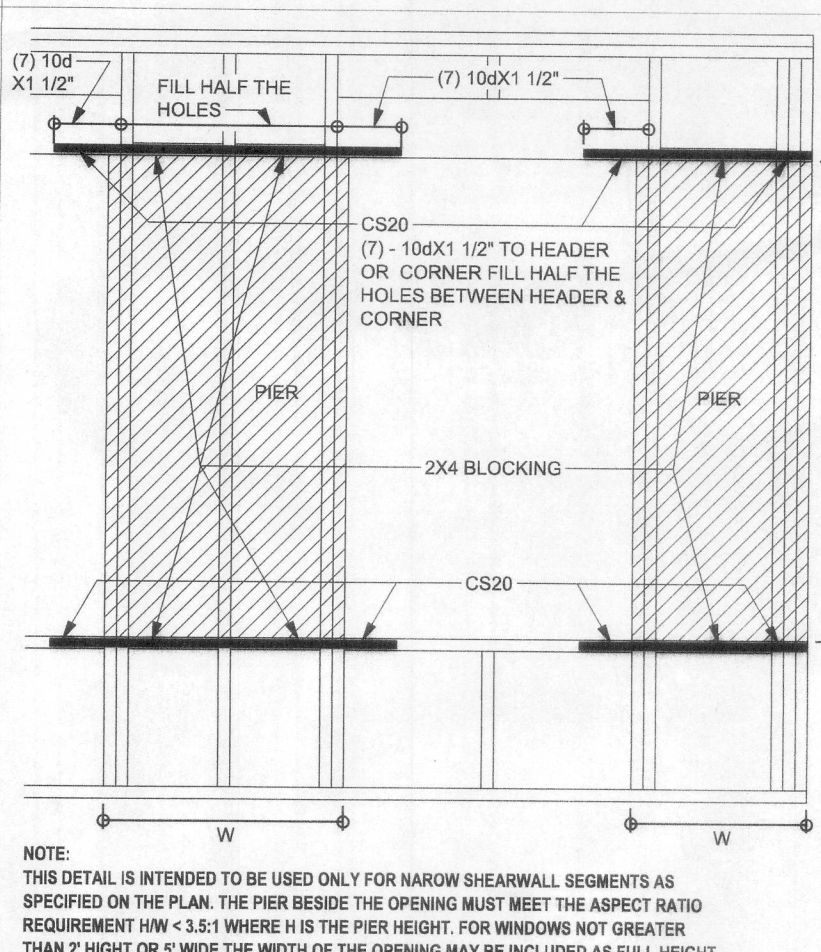


**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME

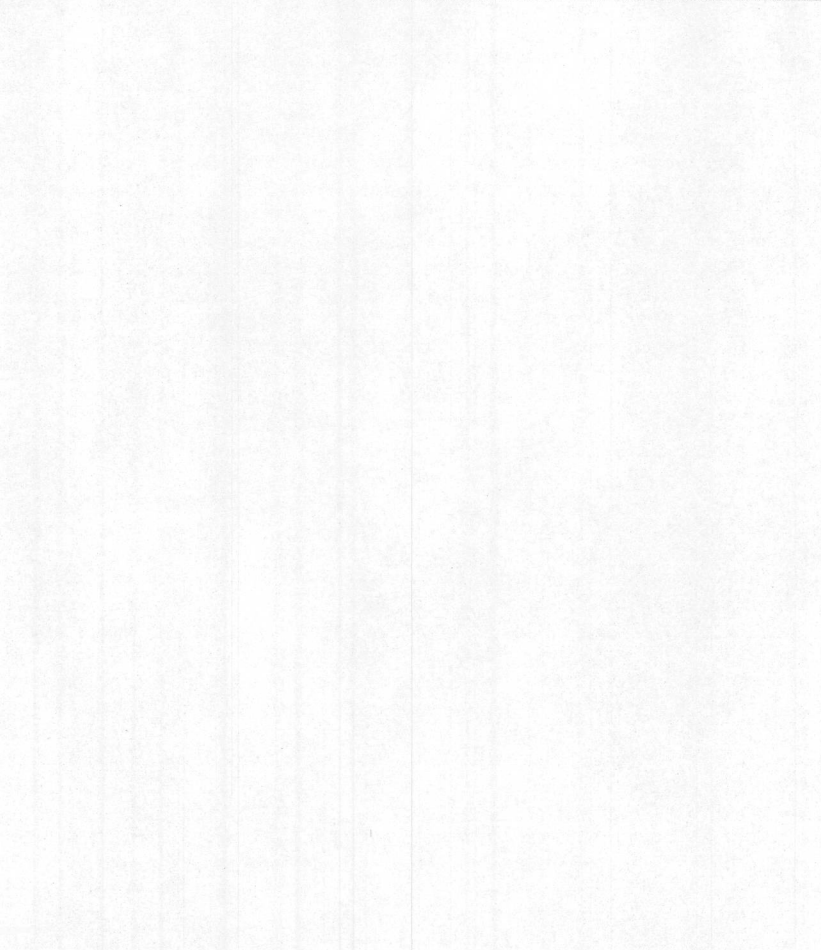


**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME

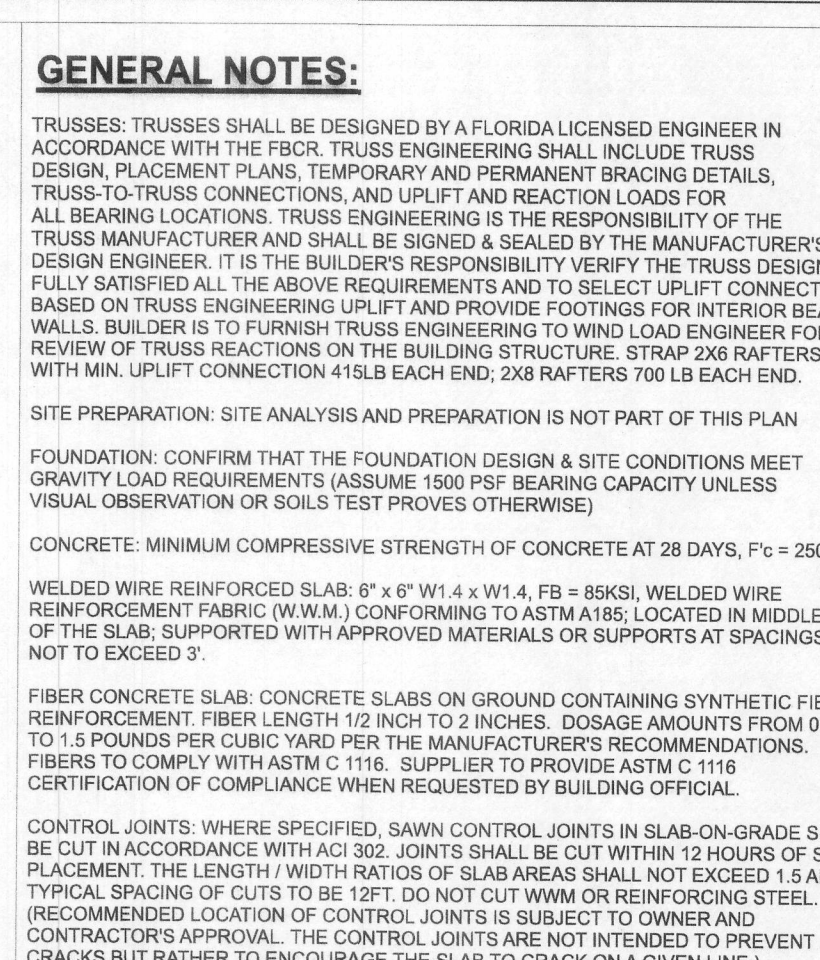
**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME



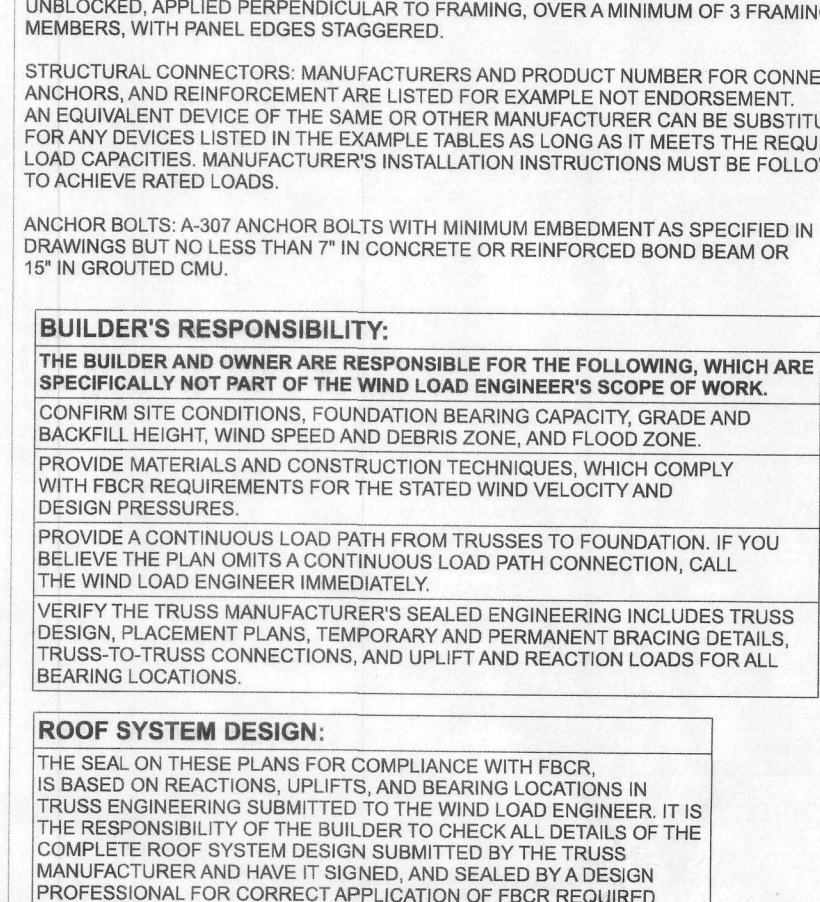
**(TYP.) INTERSECTING WALL FRAMING**  
WOOD FRAME



**(TYP.) CORNER FRAMING**  
WOOD FRAME

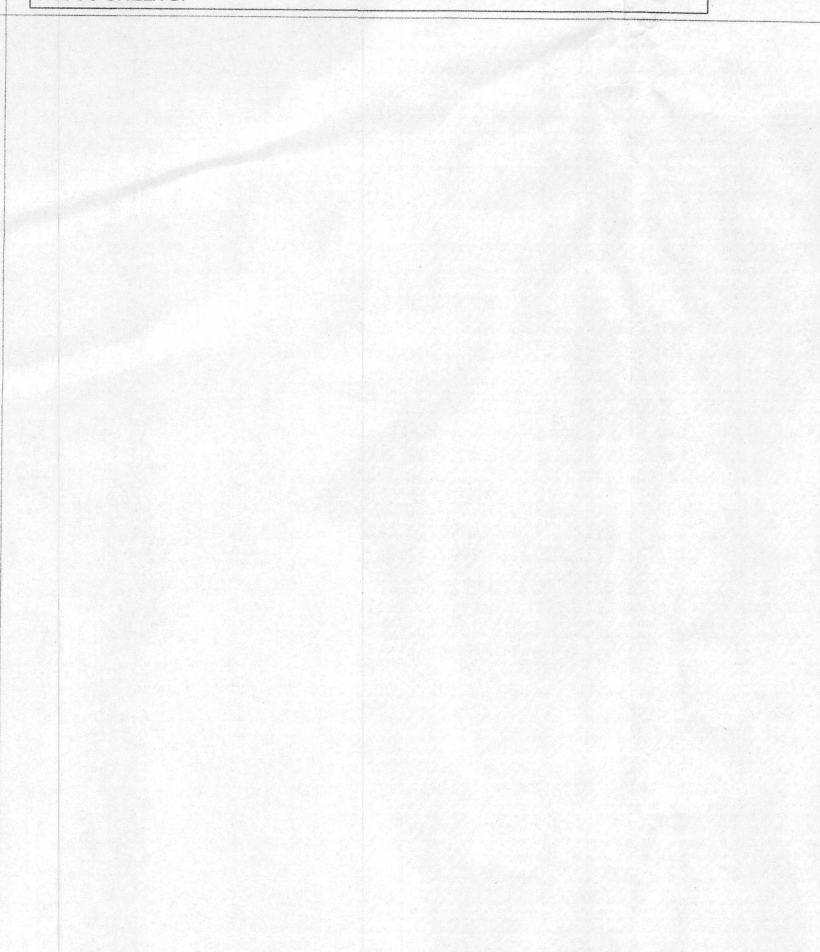


**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME

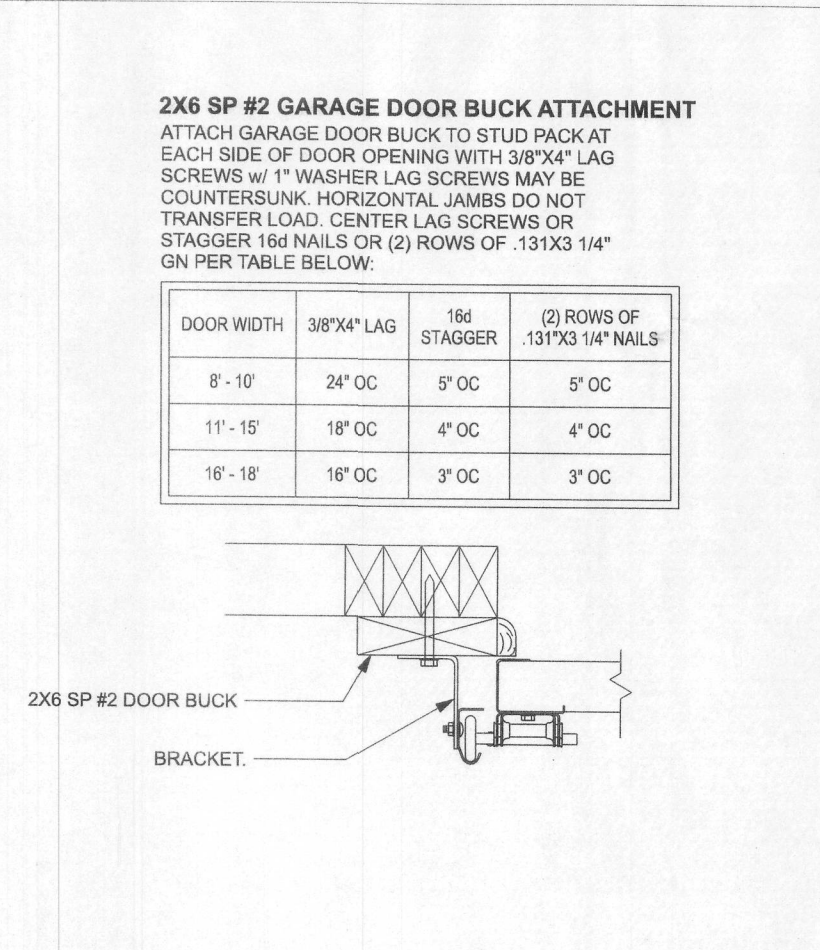


**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME

**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME



**(TYP.) INTERSECTING WALL FRAMING**  
WOOD FRAME

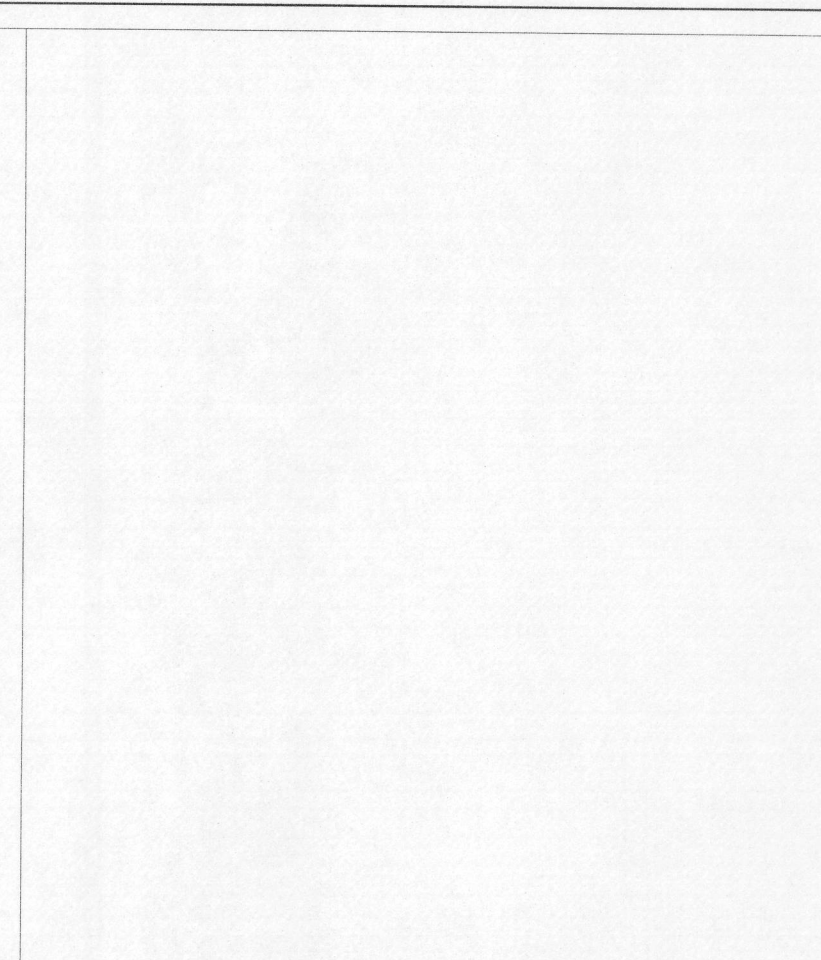


**(TYP.) CORNER FRAMING**  
WOOD FRAME

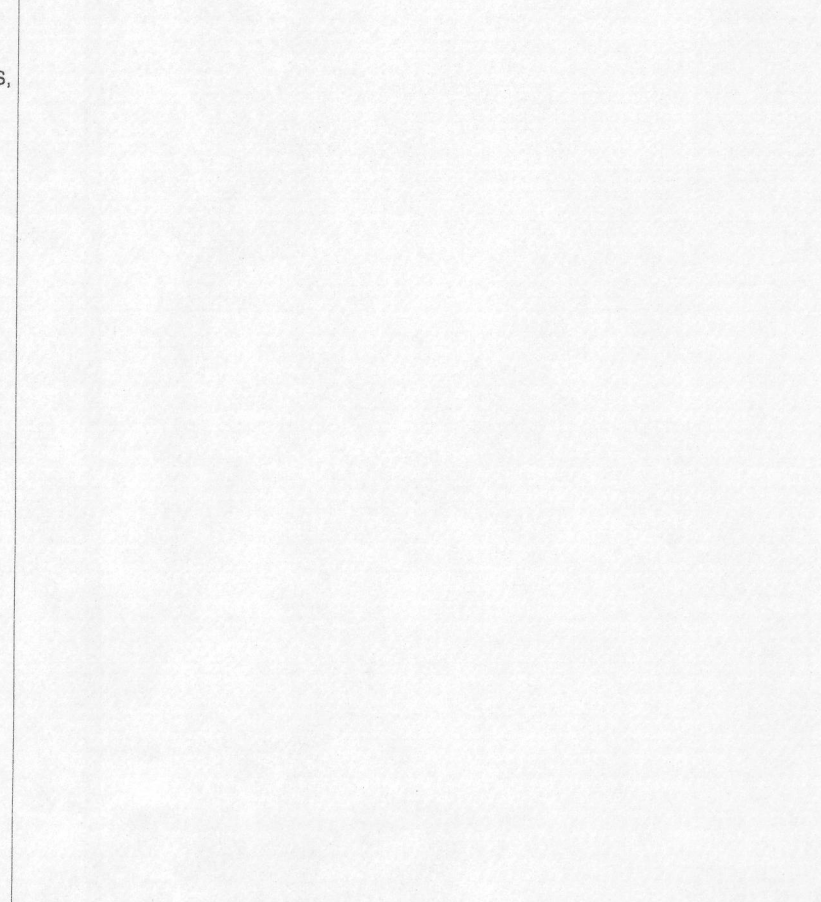
**(TYP.) CORNER FRAMING**  
WOOD FRAME

**(TYP.) CORNER FRAMING**  
WOOD FRAME

**(TYP.) CORNER FRAMING**  
WOOD FRAME

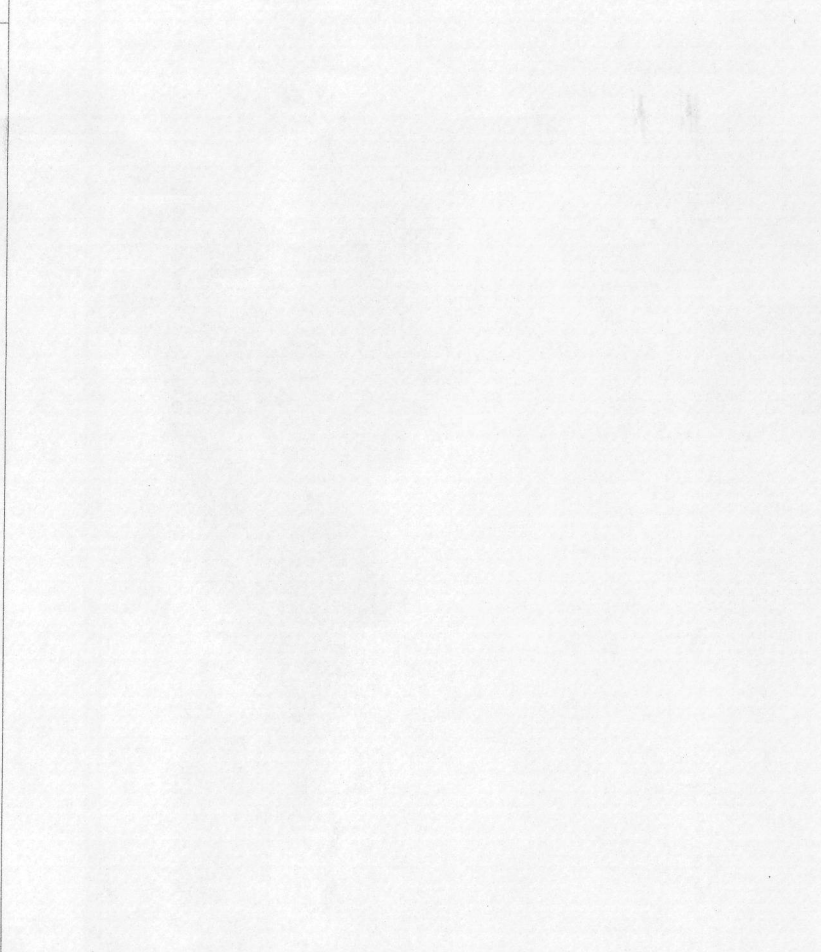


**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME

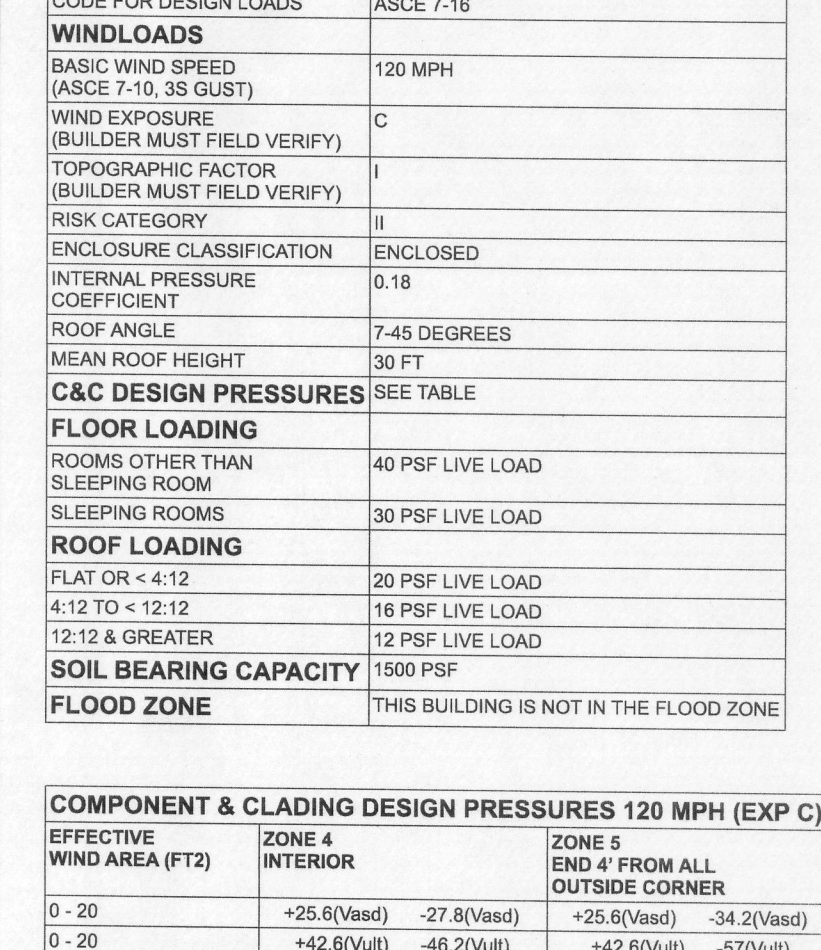


**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME

**(TYP.) GABLE BRACING DETAIL**  
WOOD FRAME



**(TYP.) INTERSECTING WALL FRAMING**  
WOOD FRAME



**(TYP.) CORNER FRAMING**  
WOOD FRAME

**(TYP.) CORNER FRAMING**  
WOOD FRAME

**(TYP.) CORNER FRAMING**  
WOOD FRAME

**(TYP.) CORNER FRAMING**  
WOOD FRAME

**CONNECTOR TABLE**

Uplift SP	Uplift SPF	Truss Connector	To Plate	To Truss/Rafter
415	485	SDWC15600	-	-
415	290	H3	4-8d x 1 1/2"	4-8d x 1 1/2"
575	495	H2-SA	5-8d x 1 1/2"	5-8d x 1 1/2"
1240	1015	H10A	9-10d x 1 1/2"	9-10d x 1 1/2"
720	620	MTS12-20	6-10d x 1 1/2"	6-10d x 1 1/2"
1000	860	MTS12-30	7-10d x 1 1/2"	7-10d x 1 1/2"
1450	1245	HTS20-30	12-10d x 1 1/2"	12-10d x 1 1/2"
1235	1235	LSTA24	8-10d	8-10d
1640	1455	MSTA24	9-10d	9-10d
1030	1030	CS20	7-10d	7-10d
Uplift SP	Uplift SPF	Stud Plate Ties	To Stud	To Plate
585	535	SP1	6-10d	4-10d
1065	605	SP2	6-10d	6-10d
771	771	LSTA24	10-10d	wrap under or over plate
1235	1235	LSTA24	14-10d	wrap under or over plate
Uplift SP	Uplift SPF	Holdowns @ Stenwall	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16d x 1 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Holdowns @ Mono	To Stud / Post	Anchor
1825	1800	DTT22	8-SDS 1/4"x1 1/2"	1/2"x12" Titen HD
4235	3640	HTT4	18-16d x 1 1/2"	1/2"x12" Titen HD
Uplift SP	Uplift SPF	Post Bases @ Stenwall	To Stud	Anchor
2200	2200	ABU44	12-16d	5/8"x12" Drill & Epoxy
2300	2300	ABU66	12-16d	5/8"x12" Drill & Epoxy
Uplift SP	Uplift SPF	Post Bases @ Mono	To Stud	Anchor
2200	2200	ABU44	12-16d	5/8"x12" Drill & Epoxy
2300	2300	ABU66	12-16d	5/8"x12" Drill & Epoxy

**EXTERIOR WALL STUD TABLE FOR SPF #2 STUDS:**

THIS STUD HEIGHT TABLE IS PER 2012 WFCM, TABLE 3.20B5, EXTERIOR LOAD BEARING & NON LOAD BEARING STUD LENGTHS FOR WALLS WITH OSB EXTERIOR AND 1/2" GYP INTERIOR RESISTING INTERIOR ZONE WINDLOADS, 130 MPH, EXPOSURE C, STUD DEFLECTION LIMIT H/240 (NOT OK FOR BRITTLE FINISH). STUD SPACINGS SHALL BE MULTIPLIED BY 0.8 FOR FRAMING LOCATED WITHIN 4 FEET OF CORNERS FOR END ZONE LOADING. (END ZONE EXAMPLE 16" O.C. x 0.8 = 12.8" O.C.)

(1) 2x4 @ 16" OC	TO 10'-1" STUD HEIGHT
(1) 2x4 @ 12" OC	TO 11'-2" STUD HEIGHT
(1) 2x6 @ 16" OC	TO 15'-7" STUD HEIGHT
(1) 2x6 @ 12" OC	TO 17'-3" STUD HEIGHT

**GRADE & SPECIES TABLE**

	SP #2	Fb	E
2x8	SP #2	925	1.4
2x10	SP #2	800	1.4
2x12	SP #2	750	1.4
GLB	24F-V3 SP	2600	1.9
LSL	TIMBERSTRAND	1700	1.7
LVL	MICROLAM	2950	2.0
PSL	PARALAM	2900	2.0

**ROOF SYSTEM DESIGN:**

THE SEAL ON THESE PLANS FOR COMPLIANCE WITH FBRC, IS BASED ON REACTIONS, UPLIFTS, AND BEARING LOCATIONS IN TRUSS ENGINEERING SUBMITTED TO THE WIND LOAD ENGINEER. IT IS THE RESPONSIBILITY OF THE BUILDER TO CHECK ALL DETAILS OF THE COMPLETE ROOF SYSTEM DESIGN SUBMITTED BY THE TRUSS MANUFACTURER AND HAVE IT SIGNED, AND SEALED BY A DESIGN PROFESSIONAL FOR CORRECT APPLICATION OF FBRC REQUIRED LOADS AND ANY SPECIAL LOADS. THE BUILDER IS RESPONSIBLE TO REVIEW EACH INDIVIDUAL TRUSS MEMBER AND THE TRUSS ROOF BRACING. THE BUILDER SHOULD USE CARE CHECKING THE ROOF DESIGN BECAUSE THE WIND LOAD ENGINEER IS SPECIFICALLY NOT RESPONSIBLE FOR THE TRUSS LAYOUT WHICH WAS CREATED BY THE TRUSS MANUFACTURER AND THE TRUSS DESIGNER. A SO DENIES RESPONSIBILITY FOR THE LAYOUT PER NOTES ON THEIR SEALED TRUSS SHEETS.

**OPENING FORCE TRANSFER**  
WOOD FRAME

**2X6 SP #2 GARAGE DOOR BUCK ATTACHMENT**  
ATTACH GARAGE DOOR BUCK TO STUD PACK AT EACH SIDE OF DOOR OPENING WITH 3/8"x4" LAG SCREWS. WOOD WASHER LAG SCREWS MAY BE COUNTERSUNK. HORIZONTAL JAMBS DO NOT TRANSFER LOAD. CENTER LAG SCREWS OR STAGGER 16d NAILS OR (2) ROWS OF 131X3 1/4" ON PER TABLE BELOW:

DOOR WIDTH	3/8"x4" LAG	16d STAGGER	(2) ROWS OF 131X3 1/4" NAILS
8'-10"	24" OC	5' OC	5' OC
11'-15"	18" OC	4' OC	4' OC
16'-18"	16" OC	3' OC	3' OC

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

2X6 SP #2 DOOR BUCK  
BRACKET

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**(TYP.) GARAGE DOOR BUCK INSTALLATION**  
WOOD FRAME

**GENERAL NOTES:**

TRUSSES: TRUSSES SHALL BE DESIGNED BY A FLORIDA LICENSED ENGINEER IN ACCORDANCE WITH THE FBRC. TRUSS ENGINEERING SHALL INCLUDE TRUSS DESIGN, PLACEMENT PLANS, TEMPORARY AND PERMANENT BRACING DETAILS, TRUSS-TO-TRUSS CONNECTIONS, AND UPLIFT AND REACTION LOADS FOR ALL BEARING LOCATIONS. TRUSS ENGINEERING IS THE RESPONSIBILITY OF THE TRUSS MANUFACTURER AND SHALL BE SIGNED & SEALED BY THE MANUFACTURER'S DESIGN ENGINEER. IT IS THE BUILDER'S RESPONSIBILITY TO VERIFY THE TRUSS DESIGNER'S FULLY SATISFIED ALL THE ABOVE REQUIREMENTS AND TO SELECT UPLIFT CONNECTIONS BASED ON TRUSS ENGINEERING UPLIFT AND PROVIDE FOOTINGS FOR INTERIOR BEARING WALLS. BUILDER IS TO FURNISH TRUSS ENGINEERING TO WIND LOAD ENGINEER FOR REVIEW OF TRUSS REACTIONS ON THE BUILDING STRUCTURE. STRAP 2X6 RAFTERS WITH MIN. UPLIFT CONNECTION 41LB EACH END; 2X8 RAFTERS 700 LB EACH END.

SITE PREPARATION: SITE ANALYSIS AND PREPARATION IS NOT PART OF THIS PLAN.

FOUNDATION: CONFIRM THAT THE FOUNDATION DESIGN & SITE CONDITIONS MEET GRAVITY LOAD REQUIREMENTS (ASSUME 1500 PSF BEARING CAPACITY UNLESS VISUAL OBSERVATION OR SOILS TEST PROVES OTHERWISE).

CONCRETE: MINIMUM COMPRESSIVE STRENGTH OF CONCRETE AT 28 DAYS, P<sub>c</sub> = 2500 PSI.

WELDED WIRE REINFORCED SLAB: 6" x 6" W1.4 x W1.4, FB = 85KSI, WELDED WIRE REINFORCEMENT FABRIC (W.W.M.) CONFORMING TO ASTM A185, LOCATED IN MIDDLE OF THE SLAB, SUPPORTED WITH APPROVED MATERIALS OR SUPPORTS AT SPACING NOT TO EXCEED 3'.

FIBER CONCRETE SLAB: CONCRETE SLABS ON GROUND CONTAINING SYNTHETIC FIBER REINFORCEMENT, FIBER LENGTH 1/2 INCH TO 2 INCHES. DOSAGE AMOUNTS FROM 0.75 TO 1.5 POUNDS PER CUBIC YARD PER THE MANUFACTURER'S RECOMMENDATIONS. CERTIFICATION OF COMPLIANCE WHEN REQUESTED BY BUILDING OFFICIAL.

CONTROL JOINTS: WHERE SPECIFIED, SAWN CONTROL JOINTS IN SLAB-ON-GRADE SHALL BE CUT IN ACCORDANCE WITH ACI 302. JOINTS SHALL BE CUT WITHIN 12 HOURS OF SLAB CASTING. THE LENGTH / WIDTH RATIOS OF SLAB AREAS SHALL NOT EXCEED 1.5 AND TYPICAL SPACING OF CUTS TO BE 1FT. DO NOT CUT WMM OR REINFORCING STEEL (RECOMMENDED LOCATION OF CONTROL JOINTS IS SUBJECT TO OWNER AND CONTRACTOR'S APPROVAL. THE CONTROL JOINTS ARE NOT INTENDED TO PREVENT CRACKS BUT RATHER TO ENCOURAGE THE SLAB TO CRACK ON A GIVEN LINE.)

REBAR: ASTM A615, GRADE 40, DEFORMED BARS, F<sub>y</sub> = 40 KSI. ALL LAP SPLICES 40" DB (25" FOR #5 BARS). UNCL. REINFORCEMENT SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318-08, U.N.C.

ROOF SHEATHING: ALL ROOFS ARE HORIZONTAL. DIAPHRAGMS, SHEATHING, UNBLOCKED, APPLIED PERPENDICULAR TO FRAMING, OVER A MINIMUM OF 3 FRAMING MEMBERS, WITH PANEL EDGES STAGGERED.

STRUCTURAL CONNECTORS: MANUFACTURERS AND PRODUCT NUMBER FOR CONNECTORS, ANCHORS, AND REINFORCEMENT ARE LISTED FOR EXAMPLE NOT ENDORSEMENT. AN EQUIVALENT DEVICE OF THE SAME OR OTHER MANUFACTURER CAN BE SUBSTITUTED FOR ANY DEVICES LISTED IN THE EXAMPLE TABLES AS LONG AS IT MEETS THE REQUIRED LOAD CAPACITIES. MANUFACTURER'S INSTALLATION INSTRUCTIONS MUST BE FOLLOWED TO ACHIEVE RATED LOADS.

ANCHOR BOLTS: A-307 ANCHOR BOLTS WITH MINIMUM EMBEDMENT AS SPECIFIED IN DRAWINGS BUT NO LESS THAN 7" IN CONCRETE OR REINFORCED BOND BEAM OR 12" IN GROUTED CMU.

**BUILDER'S RESPONSIBILITY:**

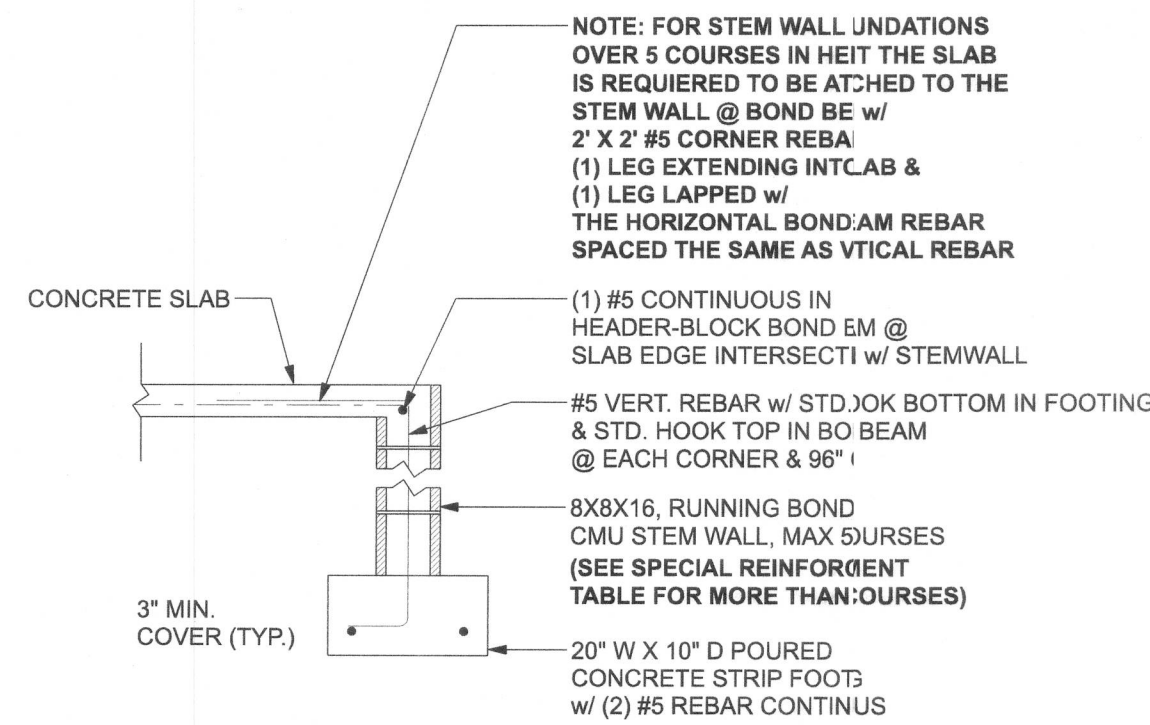
THE BUILDER AND OWNER ARE RESPONSIBLE FOR THE FOLLOWING, WHICH ARE SPECIFICALLY NOT PART OF THE WIND LOAD ENGINEER'S SCOPE OF WORK, INCLUDING BUT NOT LIMITED TO:

CONFIRM SITE CONDITIONS, FOUNDATION BEARING CAPACITY, GRADE AND BACKFILL HEIGHT, WIND SPEED AND DEBRIS ZONE, AND FLOOD ZONE.

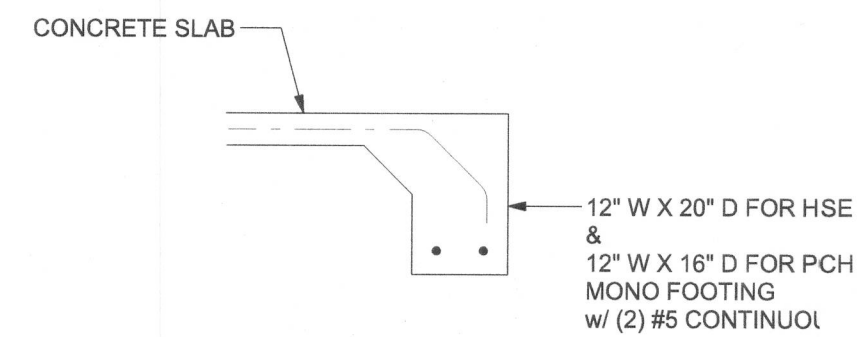
PROVIDE MATERIALS AND CONSTRUCTION TECHNIQUES, WHICH COMPLY WITH FBRC REQUIREMENTS FOR THE STATED WIND VELOCITY AND DESIGN PRESSURES.

</

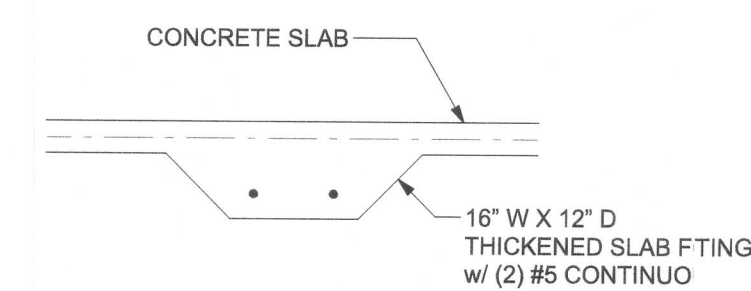




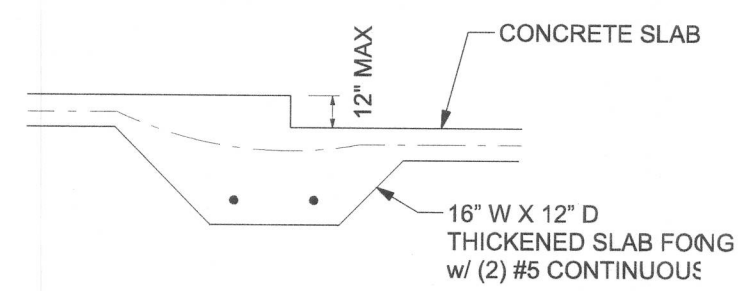
**F1 S-2** OPTIONAL STEM WALL FOOTING  
SCALE: 1/2" = 1'-0"



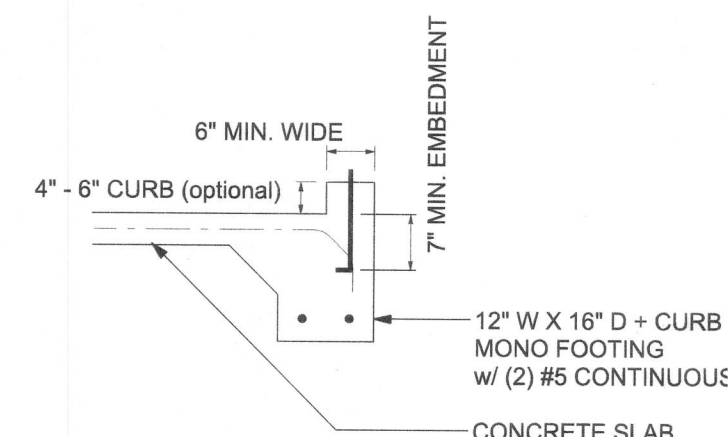
**F1 S-2** MONOLITHIC FOOTING  
SCALE: 1/2" = 1'-0"



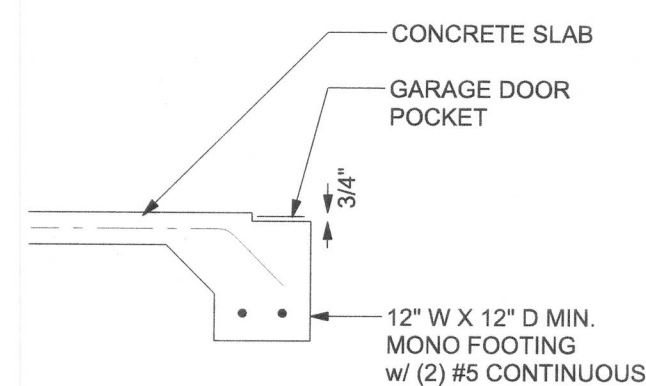
**F2 S-2** INTERIOR BEARING FOOTING  
SCALE: 1/2" = 1'-0"



**F3 S-2** INTERIOR BEARING STEP FOOTIN  
SCALE: 1/2" = 1'-0"



**F4 S-2** MONOLITHIC CURB FOOTING  
SCALE: 1/2" = 1'-0"



**F5 S-2** GARAGE DOOR POCKET FOOTIN  
SCALE: 1/2" = 1'-0"

TALL STEM WALL TABLE:

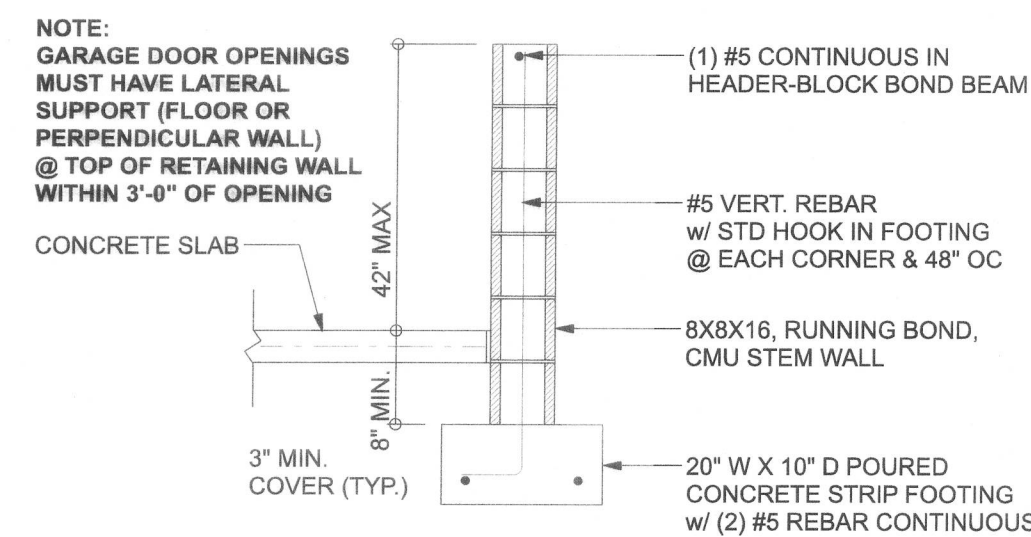
The table assumes 40 ksi for #5 rebar and 60 ksi for #7 & #8 rebar with 6" hook in the footing and bent 24" into the reinforced slab at the top. The vertical steel is to be placed toward the tension side of the CMU wall (away from the soil pressure, within 2" of the exterior side of the wall). If the wall is over 8' high, add Duowall ladder reinforcement at 18" OC vertically or a horizontal bond beam with 1#5 continuous at mid height. For higher parts of the wall 12" CMU may be used with reinforcement as shown in the table below.

STEMWALL HEIGHT (FEET)	UNBALANCED BACKFILL HEIGHT	VERTICAL REINFORCEMENT FOR 8" CMU STEMWALL (INCHES O.C.)			VERTICAL REINFORCEMENT FOR 12" CMU STEMWALL (INCHES O.C.)		
		#5	#7	#8	#5	#7	#8
3.3	3.0	96	96	96	96	96	96
4.0	3.7	96	96	96	96	96	96
4.7	4.3	88	96	96	96	96	96
5.3	5.0	56	96	96	96	96	96
6.0	5.7	40	80	96	80	96	96
6.7	6.3	32	56	80	56	96	96
7.3	7.0	24	40	56	40	80	96
8.0	7.7	16	32	48	32	64	80
8.7	8.3	8	24	32	24	48	64
9.3	9.0	8	16	24	16	40	48

MASONRY NOTE: MASONRY CONSTRUCTION AND MATERIALS FOR THIS PROJECT SHALL CONFORM TO ALL REQUIREMENTS OF "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TMS 602). THE CONTRACTOR AND MASON MUST IMMEDIATELY, BEFORE PROCEEDING, NOTIFY THE ENGINEER OF ANY CONFLICTS BETWEEN ACI 530.1-02 AND THESE DESIGN DRAWINGS. ANY EXCEPTIONS TO ACI 530.1-02 MUST BE APPROVED BY THE ENGINEER IN WRITING.

ACI 530.1-02 Section	Specific Requirements
1.4A Compressive strength	8" block bearing walls Fm = 1500 psi
2.1 Mortar	ASTM C 270, Type N, UNO
2.2 Grout	ASTM C 476, admixtures require approval
2.3 CMU standard	ASTM C 90-02, Normal weight, Hollow, medium surface finish, 8"x8"x16" running bond and 12"x12" or 16"x16" column block
2.3 Clay brick standard	ASTM C 216-02, Grade SW, Type FBS, 5.5"x2.75"x11.5"
2.4 Reinforcing bars, #3 - #11	ASTM 615, Grade 40, Fy = 40 ksi, Lap splices min 40 bar dia. (28" for #5)
2.4F Coating for corrosion protection	Anchors, sheet metal ties completely embedded in mortar or grout, ASTM A555, Class G90, 0.50 oz/ft <sup>2</sup> or 3MSS
2.4F Coating for corrosion protection	Joint reinforcement in walls exposed to moisture or wire ties, anchors, sheet metal ties not completely embedded in mortar or grout, ASTM A153, Class B2, 1.50 oz/ft <sup>2</sup> or 3MSS
3.3.E.2 Pipes, conduits, and accessories	Any not shown on the project drawings require engineering approval.
3.3.E.7 Movement joints	Contractor assumes responsibility for type and location of movement joints if not detailed on project drawings.

BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 12" BELOW UNDISTURBED SOIL OR ENGINEERED FILL



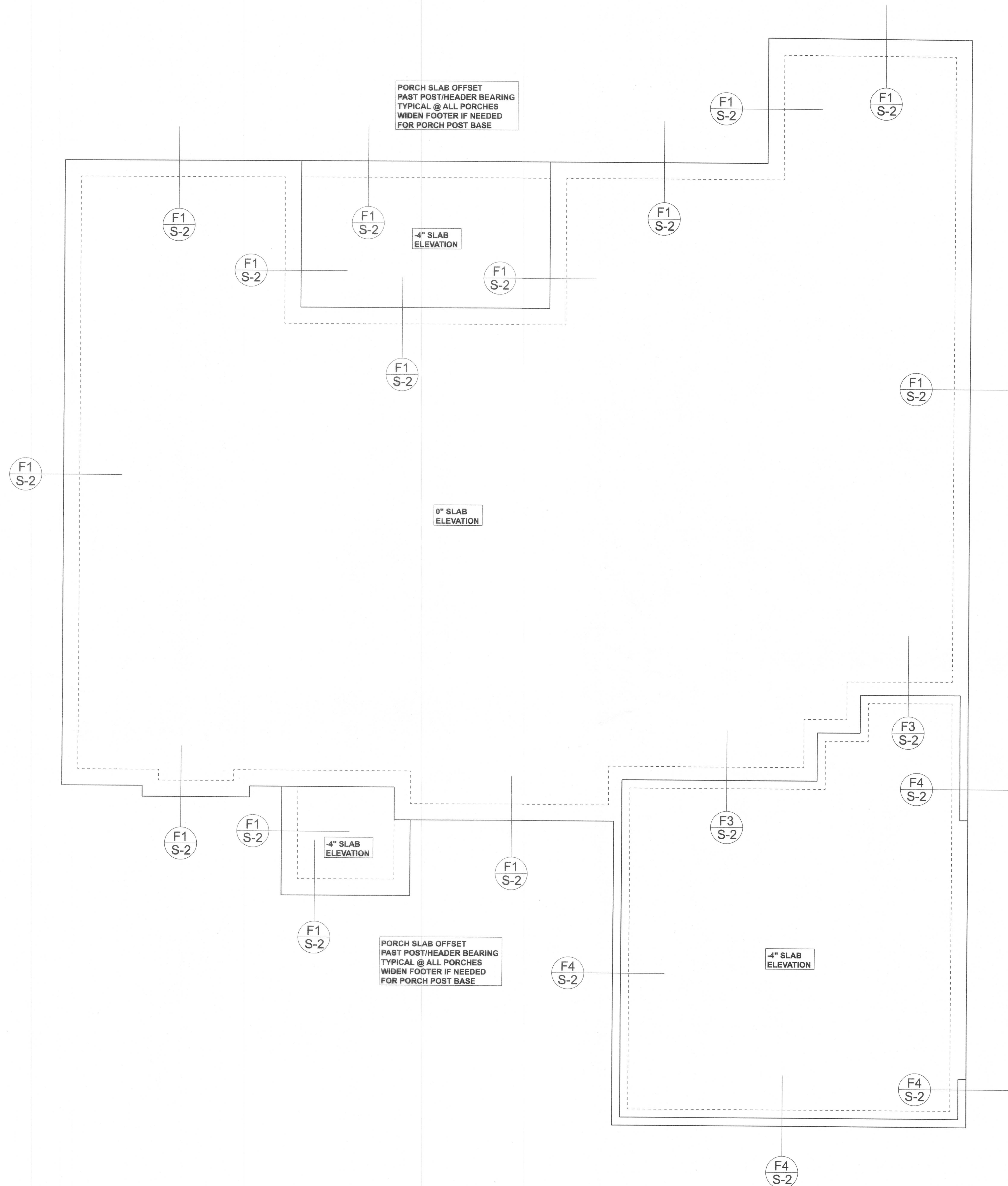
**F4 S-2** OPTIONAL STEM WALL CURB FOOTING  
SCALE: 1/2" = 1'-0"

## FOUNDATION PLAN

SCALE: 1/4" = 1'-0"

### FOUNDATION NOTES

- FN - 1 DIMENSIONS ON FOUNDATION & STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL PLANS FOR ACTUAL DIMENSIONS, RECESSES IN SLAB, STEP DOWNS, ETC. DISOSWAY DESIGN GROUP OR MARK DISOSWAY, P.E. IS NOT RESPONSIBLE FOR DIMENSION ERRORS ON THIS PLAN.
- FN - 2 CONTRACTOR SHALL VERIFY NEED FOR INTERIOR BEARING IN ALL AREAS BY REVIEWING THE ROOF TRUSS PLAN (BY THE SUPPLIER) BEFORE FINALIZING FOUNDATION PLAN.
- FN - 3 THE SLAB SHALL BE: 4" CONCRETE SLAB REINFORCED w/ 18X5-1 AT 1'-4" WELDED WIRE MESH PLACED ON CHAIRS @ 1'-0" DEPTH OR FIBER MESH CONCRETE, 6-MIL POLY VAPOR BARRIER w/ 8" LAPS SEALED w/ POLY TAPE OVER TERMITES-TREATED & COMPACTED FILL.



Gibraltar Contracting, LLC

1880 Model - Lot 50 Emerald Cove

PROJECT ADDRESS:  
Lot 50 Emerald Cove  
Columbia County FL

DIMENSIONS:  
Stated dimensions supersede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS:  
Mark Disosway, P.E. hereby expressly reserves its common law copyrights and property right in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building at specified location.

MARK DISOSWAY P.E. 53915



Mark Disosway P.E.  
16 SW Midtown Place  
Suite 103  
Lafayette, Florida 32025  
386.754.5419  
disoswaydesign@gmail.com

JOB NUMBER:

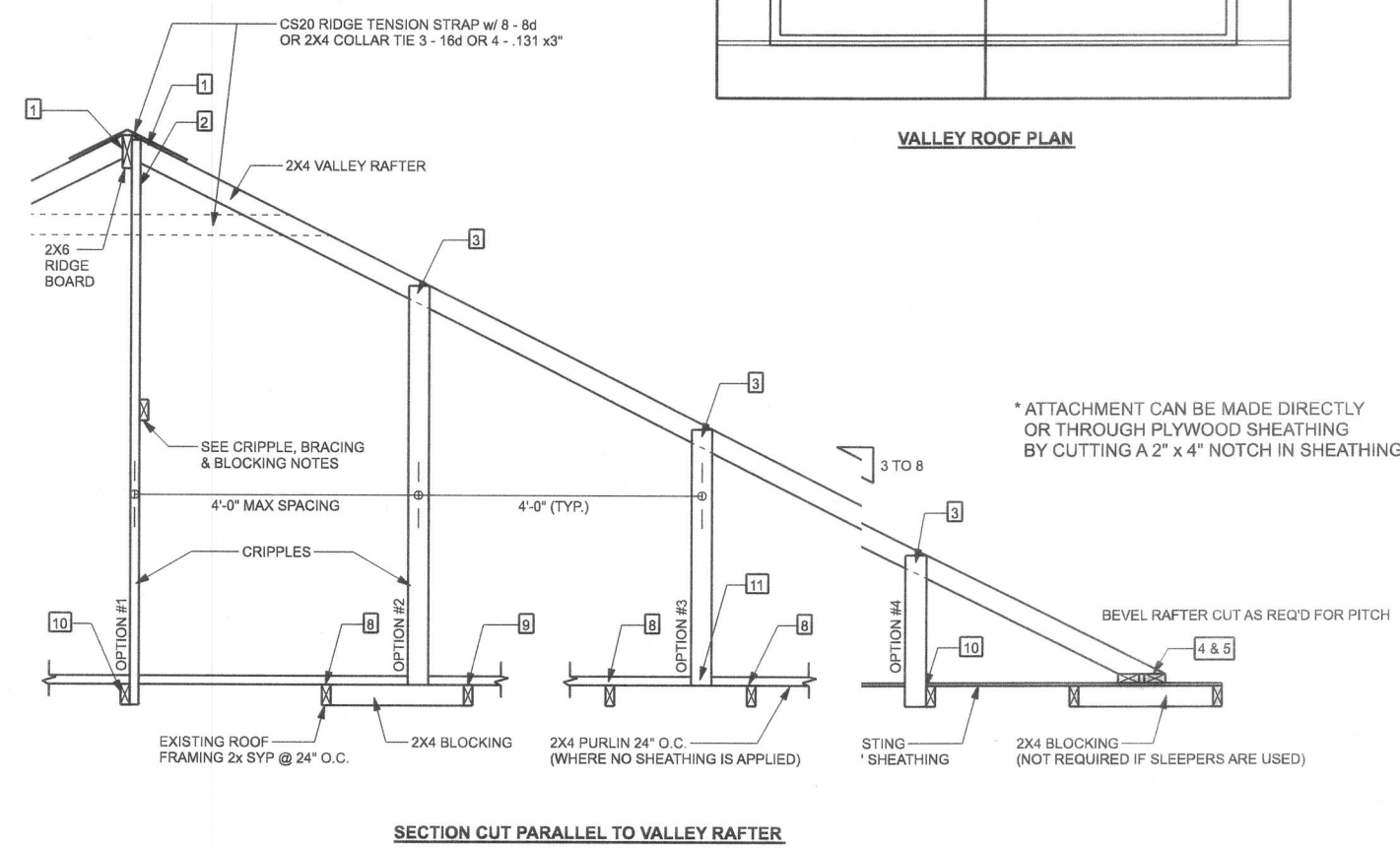
220531

**S-2**

OF 3 SHEETS



LUMBER SIZE & GRADE MINIMUM REQUIREMENTS	
ROOF BOARD	2X6 SYP #2
RAFTER SPANS 20'-0" OR LESS	2X4 SYP #2
PURLIN / LATERAL BRACING	2X4 SPF #2
SLEEPERS	2X (WIDTH OF RAFTER BEAT CUT) SPF #3 OR 2X (WIDTH OF RAFTER BEAT CUT) 2X4 SYP #2
CRIPPLES & BLOCKING	2X4 SPF #2 OR BETTER
TRUSS BELOW	SEE TRUSS DESIGN - SOUTHERN PINE MATERIAL



ROOF OVER FRAM & BRACING DETAIL  
SCALE 1/8"

#### VALLEY ROOF PLAN MEMBER LEGEND

—	TRUSS
—	TRUSS UNDER VALLEY FRAMING
—	VALLEY RAFTER OR RIDGE
—	CRIPPLE

#### CONNECTION REQUIREMENT NOTES

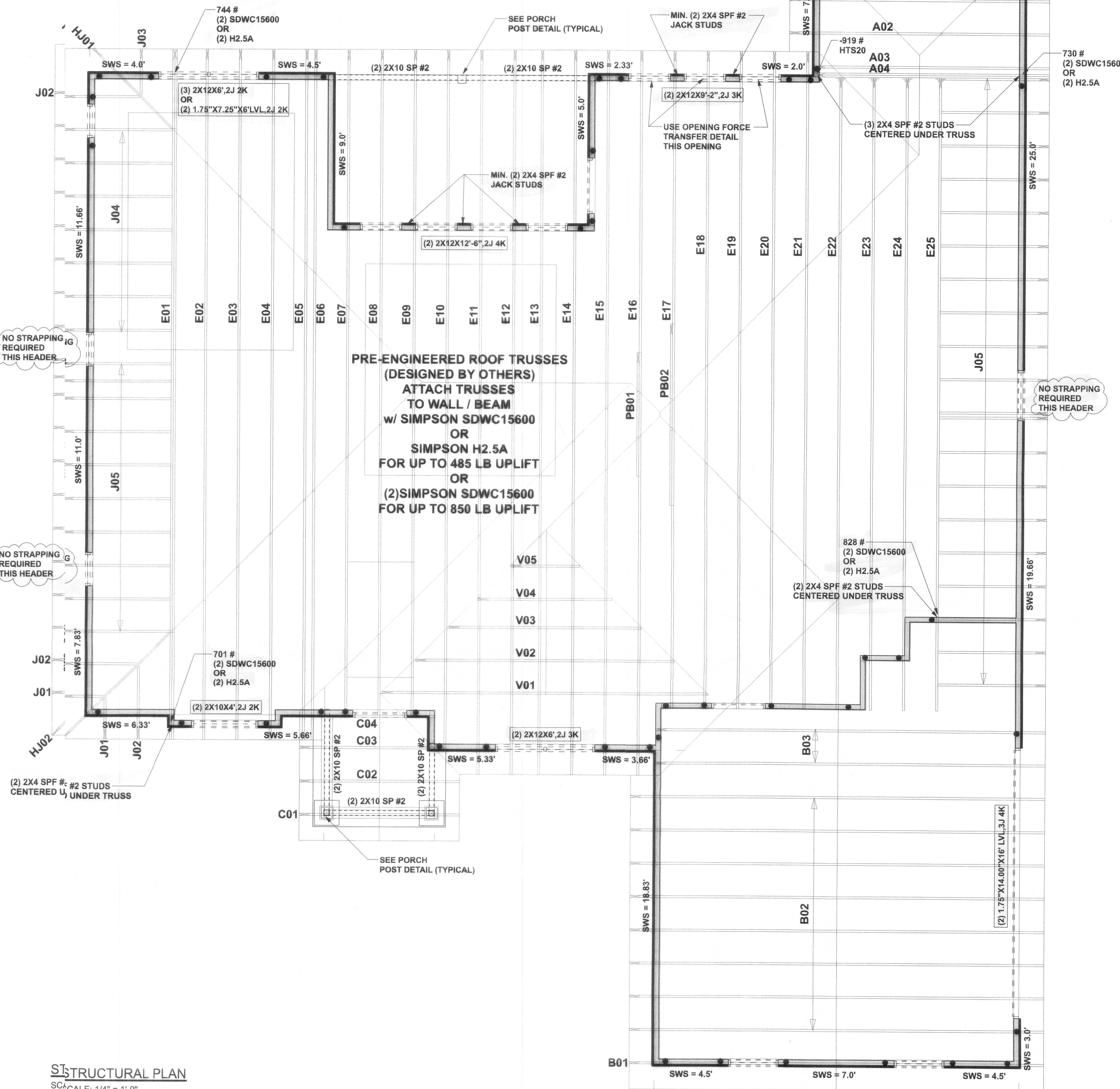
1. 2X4 RAFTERS TO RIDGE	3-16d OR 6-131 x 3" TOE NAILS
2. CRIPPLE TO RIDGE	3-16d OR 6-131 x 3" TOE NAILS
3. CRIPPLE TO RAFTER	3-16d OR 6-131 x 3" TOE NAILS
4. RAFTER TO SLEEPER OR BLOCKING	6-16d OR 12-131 x 3" TOE NAILS
5. SLEEPER TO TRUSS	4-16d OR 8-131 x 3" TOE NAILS EACH TRUSS
6. RIDGE BOARD TO RIDGE BLOCK	3-16d OR 6-131 x 3" TOE NAILS
7. RIDGE BOARD TO TRUSS	3-16d OR 6-131 x 3" TOE NAILS
8. PURLIN TO TRUSS (TYP)	3-16d OR 6-131 x 3" TOE NAILS
9. PURLIN TO TRUSS IF CRIPPLE IS ATTACHED TO PURLIN	4-16d OR 8-131 x 3" TOE NAILS
10. TRUSS TO BLOCKING	3-16d OR 6-131 x 3" TOE NAILS
11. CRIPPLE TO TRUSS	3-16d OR 6-131 x 3" TOE NAILS
11. CRIPPLE TO PURLIN	3-16d OR 6-131 x 3" TOE NAILS

#### GENERAL NOTES

MAXIMUM RAFTER SPAN: 10' 0" FOR 2X4 SYP #2 OR 2X4 SYP #2; 12' 0" FOR 2X6 SYP #2 OR 2X6 SYP #2. MAXIMUM ROOF AREA PER SUPPORT: 1800 IN SQUARE (2.5 S.F. PER SQUARE FOOT). (EXAMPLE: 4' 0" O.C. X 4' 0" SPAN = 1600 OR 2' 0" X 8' 0" SPAN = 1600). PURLIN REQUIRED: 2" O.C. IF EXISTING SHEATHING IS REMOVED. PURLIN SHOULD OVERLAP EXISTING ONE TRUSS SPACING MINIMUM. IN CASES THAT THIS IS IMPRACTICAL, OVERLAP SHEATHING MINIMUM 12" IF AND ONLY IF PURLIN THROUGH SHEATHING INTO PURLIN WITH A MINIMUM OF 8-32 COMMON WIRE NAILS. THIS DRAWING APPLIES TO VALLEYS WITH THE FOLLOWING CONDITIONS: SPAN DISTANCES BETWEEN KEELS 48" OR LESS. MAXIMUM VALLEY HEIGHT: 14' 0" OR LESS. MAXIMUM WIND SPEED: 130 MPH. MAXIMUM MEAN ROOF HEIGHT: 30 FEET. MAXIMUM TOTAL LOADING: 40 psf. SHEETS: 1/2" OR THICKER 1/2" WIND REQUIREMENTS. SCHEDULE: 40 LUMBER "C", 1-1/2, K&E 1-10. ENCLOSED BUILDING.

#### CRIPPLE, BRACING & BLOCKING NOTES

2X4 CONTINUOUS LATERAL BRACE (CLB) MIN. IS REQUIRED FOR CRIPPLES 8'-0" TO 10'-0" LONG. MAILED NAILS: 10d NAILS ON 2X4 10" OR SCAB BRACE NAIL TO FLAT EDGE OF CRIPPLE WITH 16 NAILS @ 4" O.C. 1" OR SCAB MUST BE 90% OF CRIPPLE LENGTH. CRIPPLES OVER 10'-0" LONG REQUIRE TWO CLB OR BOTH FACES W/ 1" OR SCAB. USE STRESS GRADO LUMBER & BOX OR COMMON WIRE NAILS. NAILING: (EDGE OF CRIPPLE CAN FACE RIDGE OR RAFTER. ALL LONG AS THE PROPER NUMBER OF NAILS ARE. INSTALLED INTO RIDGE BOARD. INSTALL BLOCKING UNDER RAFTER IF SLEEPERS ARE NOT USED. INSTALL BLOCKING UNDER CRIPPLES IF CRIPPLES HAVE BEEN THIN. LOWER TRUSS TOP CHORDS AND LATERAL BRACING IS NOT USED. APPLY ALL NAILING IN ACCORDANCE TO NDS-1997 SECTION 12. NAILS ARE COMMON WIRE NAILS UNLESS NOTED OTHERWISE.



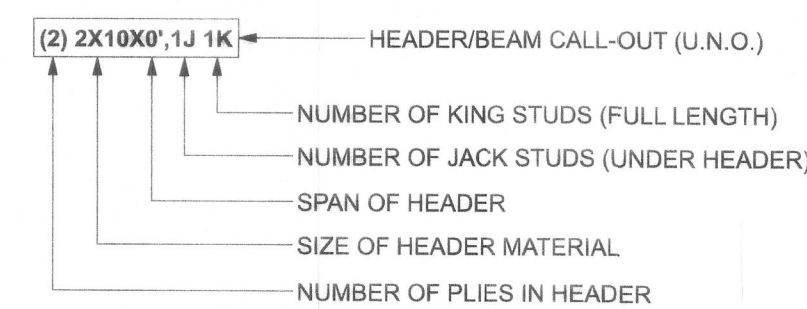
#### STRUCTURAL PLAN

SCALE: 1/4" = 1'-0"

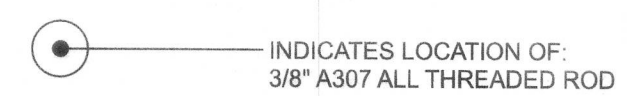
#### STRUCTURAL PLAN NOTES

- S<sub>N</sub>-1 ALL LOAD BEARING FRAME WALL & PORCH HEADERS SHALL BE A MINIMUM OF (2) 2X10 SP #2 (U.N.O.)
- S<sub>N</sub>-2 ALL LOAD BEARING FRAME WALL HEADERS SHALL HAVE (1) JACK STUD & (1) KING STUD EACH SIDE (U.N.O.)
- S<sub>N</sub>-3 USE ONE JACK STUD GIRDER SUPPORT PER 2500 LB LOAD
- S<sub>N</sub>-4 DIMENSIONS ON STRUCTURAL SHEETS ARE NOT EXACT. REFER TO ARCHITECTURAL FLOOR PLAN FOR ACTUAL DIMENSIONS
- S<sub>N</sub>-5 PERMANENT TRUSS BRACING IS TO BE INSTALLED AT LOCATIONS AS SHOWN ON THE SEALED TRUSS DRAWINGS. LATERAL BRACING IS TO BE RESTRAINED PER BCS11-03, BCS1-B1, BCS1-B2, & BCS1-B3. BCS1-B1, BCS1-B2, & BCS1-B3 ARE FURNISHED BY THE TRUSS SUPPLIER, WITH THE SEALED TRUSS PACKAGE

#### HEADER LEGEND



#### THREADED ROD LEGEND



ACTUAL vs REQUIRED SHEARWALL	
	TRANSVERSE
ACTUAL	35592 LBF
REQUIRED	13063 LBF
	LONGITUDINAL
ACTUAL	17841 LBF
REQUIRED	15147 LBF

CONNECTIONS, WALL, & HEADER DESIGN IS BASED ON REACTIONS & UPLIFTS FROM TRUSS ENGINEERING FURNISHED BY BUILDER: W.B. HOWLAND TRUSS CO. JOB #21-5391

Gibraltar Contracting, LLC

1880 Model - Lot 50 Emerald Cove

PROJECT ADDRESS:  
1880 Model - Lot 50 Emerald Cove  
Columbia County, FL

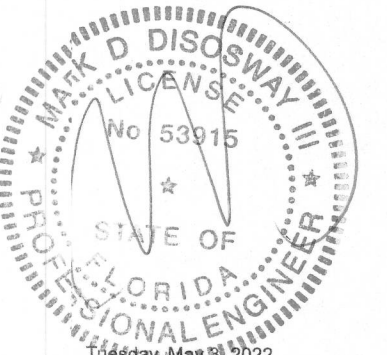
DIMENSIONS:  
Stated dimensions supercede scaled dimensions. Refer all questions to Mark Disosway, P.E. for resolution. Do not proceed without clarification.

COPYRIGHTS AND PROPERTY RIGHTS:  
Mark Disosway, P.E. hereby expressly reserves its common law copyright and property rights in these instruments of service. This document is not to be reproduced, altered or copied in any form or manner without first the express written permission and consent of Mark Disosway.

CERTIFICATION: I hereby certify that I have examined this plan, and that the applicable portions of the plan, relating to wind engineering comply with the 7th Edition Florida Building Code Residential (2020) to the best of my knowledge.

LIMITATION: This design is valid for one building, at specified location.

MARK DISOSWAY P.E. 53915



Mark Disosway P.E.  
16: SW Midtown Place  
Suite 103  
Largo, Florida 32025  
386.754.5419  
disoswaydesign@gmail.com

JOB NUMBER:  
220531

S-3  
OF 3 SHEETS