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Alpine, an ITW Company 155 Harlem Ave North Building, 4th Floor Glenview, IL 60025 Phone: (800)755-6001 www.alpineitw.com

COA #0 278 Florida Certificate of Product Approval #FL1999 08/24/2022

Site Information:	Page 1:
Customer: W. B. Howland Company, Inc.	Job Number: 22-8147
Job Description: Edenfield	
Address: 3rd of sep	

Job Engineering Criteria:	
Design Code: FBC 7th Ed. 2020 Res.	IntelliVIEW Version: 21.02.00 through 21.02.01
	JRef #: 1XIc2150005
Wind Standard: ASCE 7-16 Wind Speed (mph): 130	Design Loading (psf): 40.00
Building Type: Closed	

This package contains general notes pages, 50 truss drawing(s) and 9 detail(s).

ltem	Drawing Number	Truss	Item	Drawing Number	Truss
1	235.22.1108.24190	A01	2	235.22.1108.34100	A02
3	235.22.1109.08450	A03	4	235.22.1109.13427	A04
5	235.22.1109.25000	A05	6	235.22.1109.29380	A06
7	235.22.1109.31657	A07	8	235.22.1109.34557	A08
9	235.22.1109.56620	A09	10	235.22.1110.05803	A10
11	235.22.1110.12403	A11	12	235.22.1110.20757	A12
13	235.22.1110.46783	B01	14	235.22.1111.12333	B02
15	235.22.1111.21293	B03	16	235.22.1111.25943	B04
17	235.22.1112.17080	B05	18	235.22.1112.18903	C01
19	235.22.1112.22037	C02	20	235.22.1112.29977	C03
21	235.22.1112.34110	C04	22	235.22.1112.37173	C05
23	235.22.1112.40643	C06	24	235.22.1112.43340	C07
25	235.22.1112.44820	C08	26	235.22.1112.50373	C09
27	235.22.1112.55807	C10	28	235.22.1112.57270	C11
29	235.22.1113.02720	C12	30	235.22.1113.09017	C13
31	235.22.1113.12750	C14	32	235.22.1113.16473	C15
33	235.22.1113.19153	C19	34	235.22.1113.21843	C20
35	235.22.1113.24357	C21	36	235.22.1113.47807	C22
37	235.22.1113.49600	PB01	38	235.22.1113.51867	PB02
39	235.22.1113.54017	PB03	40	235.22.1113.55540	PB04
41	235.22.1113.57120	PB05	42	235.22.1113.58840	PB06
43	235.22.1114.00550	PB07	44	235.22.1114.01863	PB08
45	235.22.1114.03333	PB09	46	235.22.1114.05583	V01
47	235.22.1114.06913	V02	48	235.22.1114.10070	V03
49	235.22.1114.21037	V04	50	235.22.1114.24400	V05



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Site Information:	Page 2:
Customer: W. B. Howland Company, Inc.	Job Number: 22-8147
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Item	Drawing Number	Truss	Item	Drawing Number	Truss
51	A14030ENC160118		52	BRCLBSUB0119	
53	GBLLETIN0118		54	A14015ENC160118	
55	CNNAILSP1014		56	DEFLCAMB1014	
57	PB160160118		58	VAL180160118	
59	VALTN160118				

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer. The Truss Design Engineer. The Truss Design Engineer on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for of all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for of all load cases.

Max Web CSI= Maximum bending and axial Combined Stress Index for Webs for of all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc). -R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment. W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

- 1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
- 2. ICC: International Code Council; <u>www.iccsafe.org</u>.
- 3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; <u>www.alpineitw.com</u>.
- 4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
- 5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.



Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of structural panels or rigid ceiling use purlins to brace all flat TC @ 24" oc, all BC @ 24" oc.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.



WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.







SEQN: 112258	GABL	Ply: 1		Job Nur	nber: 22-8147			C	ust: R 21	15 JRef:1)	(lc2150005	5 T16
FROM:		Qty: 1	1	Edentiel Truss L	a abel: A03			K	D /	235.22.110 DF	9.08450 08/23/202	22
			3*8	1:	3'4"8 <u>21'9"10</u> 3'1" 1 8'5"2 1 9	27"4 31'7"3 + 36'8"8 + 40 "10 8'11"15 + 5'1"5 + 33	1*15 44' 5'7 3'10*1					
					 5'4*8	al la 2° al la 115°7.						
					1 010 1 02.2	(TYP) <u>10"4</u>						
			8'	1*12		xx6 X4 X4 X4 X4 X4 X4 X4 X6 F X6 K6 X6 X6 X6 X6 X6 X6 						
		T						T				
			8	-	\$5X5 C		\$3X4					
		- 10'	- 1		(a) (a)	AL (a) AZ	3X4 QR 3X4	10'3"3-				
			#2.5X6	//			SC SC	, ,				
		⊥ <u>∓</u> 2						ΞΞ	ቀ ^{10′} ቀ ^{9′}			
		-	AG	4	AF [™] AE AD ■3X4 ■3X4 ■3X6 ■3	AC AB AA Z XY 3X4 3X4 14X5 3X4 = 3X4		(G1) —	•			
						≡5X6						
			8'	1"12			7					
		H	1'6"	8' 8'	5'4"8 13'4"8	$\frac{182^{\circ}11}{31'7'3} \xrightarrow{13''1}_{34'10''4} \frac{1'1'12}{37'}$	7' 44'					
						1 35 ¹ 10*4						
							(NNL) ┝─── 4' ───┥					
Loading Critoria (pol)	Wind	Critoria			Snow Critoria (De Di in DSE)	Dofl/CSI Critoria	A Maxim	um Res	octions	(lbs) or *	-PIF	
TCLL: 20.00	Wind S	Std: AS	CE 7-16		Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#		Gravity	ictions	(153), OI	Ion-Grav	rity
TCDL: 10.00	Speed	l: 130 m	iph sed		Pf: NA Ce: NA	VERT(LL): 0.039 H 999 240	Loc R+	/ R-	/ Rh	/ Rw	/U	/ RL
BCLL: 0.00 BCDL: 10.00	Risk C	ategory:	ll		Lu: NA Cs: NA Snow Duration: NA	HORZ(LL): 0.081 H 999 180	AG 523	/- /-	/- /-	/279 /872	/9 /-	/200 /-
Des Ld: 40.00	EXP: E Mean	3 Kzt: N Heiaht: 1	NA 5.00 ft			HORZ(TL): 0.031 Z	AA 180	/-	<i>i</i> -	/103	/0	<i>I</i> -
NCBCLL: 10.00	TCDL:	5.0 psf			Building Code: FBC 7th Ed. 2020 Res	Creep Factor: 2.0 Max TC CSI: 0 797	X* 150	/- /-	/- /-	/94	/- /-	/- /-
Load Duration: 1.25	MWFF	: 2.0 pst RS Paralle	el Dist: 0 to) h/2	TPI Std: 2014	Max BC CSI: 0.754	Wind rea	ctions b Nid = 3	ased o	n MWFRS n Reg = 1	5 (Truss)
Spacing: 24.0 "	C&C E	Dist a: 4.4	40 ft vall: pot in 6	S 50 ft	Rep Fac: Yes	Max Web CSI: 0.468	AF Brg \	Vid = 3	.5 Mi	n Req = 1	.5 (Truss)
	LUC. II	GCpi: (0.18	5.50 II	Plate Type(s):		AA Brg\ AA Brg\	/Vid = 3. Vid = 24	.5 Mi 4.0 Mi	n Req = 1. n Req = -	.5 (Truss	i)
	Wind [Duration:	1.60		WAVE	VIEW Ver: 21.02.01.1216.15	X Brg \ Bearings	Nid = 8-	4.0 Mi	n Req = - A&Xare		
Top chord: 2x4 SP #2	: T3.T4	2x4 SP M	M-31:		See DWGS A14030ENC16	60118 & GBLLETIN0118 for	a rigid su	irface.	, , , , , , , , , , , , , , , , , , , ,	, a x aic		
Bot chord: 2x4 SP #2; Webs: 2x4 SP #3:					gable wind bracing and oth	ner requirements.	Members Maximur	not list n Top (ed have Chord F	e forces les Forces Pe	រs than 3 r Ply (lb៖	575# 5)
Stack Chord: SC1 2x4	4 SP #2	;			Stacked top chord must NC area (NNL). Dropped top cl	OT be notched or cut in hord braced at 24" oc	Chords	Tens.Co	omp.	Chords	Tens.	Ćomp.
Ri Siub Wedge. 2x4 c	55 #3,				intervals. Attach stacked to top chord in notchable area	p chord (SC) to dropped a using 3x4 tie-plates 24"	C-D	0	- 835	E-F	30 25	- 453 - 457
(a) Continuous lateral	restrain	nt equally	spaced on	n	oc. Center plate on stacked	d/dropped chord interface,	D-L	U	- 302	1 - K	25	-407
member.			opacca c.	•	chord in notchable area usi	ing 3x6.	Maximur Chords	n Bot C	hord F	orces Per	Ply (lbs	s) Comp
Plating Notes								610	0 0		611	000000. 0
All plates are 2X4 exc	ept as r	noted.					AD-AC	610	Ő	AA- Z	615	Ő
Loading			-		at the	MAN REPORT FEELEN	AC-AB	910	0			
chord must not be cut	" max ra	ake overh hed.	nang. Top		G	AS FLEAM	Maximur	n Web	Forces	Per Ply (I	bs)	~
Purlins					O.S.	CENS		i ens.Co	omp.	VVebs	Tens.	Comp.
In lieu of structural pa	nels use	e purlins t	to brace all	l flat	No. A Start M	IN THE REAL PROPERTY IN	D -AC	686 493	0	AU-AW AV-AZ	477	- 789 0
TC @ 24° 0C.						0 86848	E -AL AI -AP	0	- 711 - 775	AW-AY AY- 7	0	- 855 - 774
Wind loads based on	MWFR	S with ad	ditional C8	ю.			AP-AU	Ő	- 811	AZ- O	447	0
member design.							AB-AU AU-AV	472 482	0	0 - X	0	- 476
Left end vertical not en	xposed	to wind p	oressure.	(D.O.C.	a s	TATE OF			-		<i></i>	
+ Member to be laters	n boun (ally brac	able and	u nip roor ty vrizontal wir	/pes.	21	CONDANS	Gables	n Gable Tens.Co	erce The second	Gables	(Ibs) Tens. /	Comp.
loads. bracing syste	m to be	desiged	and furnish	hed	C.C.	CORTE STORE	AG- B	93	- 488	D -AE	0	- 389
by ouners.					COA #0 278	ONAL EN MAN	AF- C	0 -	1244			
					Florida Certifi	cate of Product Approval #FL19	99					
	WA	RNING	* READ A	ND FO	LLOW ALL NOTES ON THIS D	RAWING!						
IMPORTA	ANT	FURNISI in fabrica	H THIS DR ating, hand	AWINC	G TO ALL CONTRACTORS INC	LUDING THE INSTALLERS Refer to and follow the latest edition	of BCSI (E	Buildina				
Component Safety Info bracing per BCSI. Unle	ormation	n, by TPI ed otherw	and SBCA vise, top ch) för sa ord sha	fety practices prior to performing Ill have properly attached structu	these functions. Installers shall p iral sheathing and bottom chord sh	orovide tem all have a p	porary				
auacheo rigio ceiling. I	Location	IS SHOWN	or permar	nent lat	eral restraint of webs shall have	pracing installed per BUSI sections		в10, or to				

as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information. Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org







Glenview, IL 60025





SEQN: 99606 FROM:	COMN	Ply: 1 Qty: 3	Job Nui Edenfiel Truss L	nber: 22-8147 d abel: A06		Cust: R 215 JRef: 1XIc2150005 T33 DrwNo: 235.22.1109.29380 KD / DF 08/23/2022
		4'2"15	8'3"8	13'4"8 = 19'5"1	15 <u> </u> 25'3"13	
	1	4'2"15	4'0"9	5'1" ' 6'1"7	' 5'9 " 15 '	6'9"11 ' 4'10"8 '
90 	B B S M III2X4	8 2 8 2 8 2 8 2 8 2 8 2 8 2 8 2 8 2 8 2	#25X6	=5X6 E (a)	F G G (a) (a) (a) (a) (b) (c) (c) (c) (c) (c) (c) (c) (c) (c) (c	$ = 5 \times 5 $
	k					
- ^{1'0}	⁶ "-	4' +	4'3"8 8'3"8	5'1" - - 5'9"15	5'9"15 - - 25'0"5	5'9"15
			030	1346 1927	230 5	
Loading Criteria (psf)	Wind Wind S	Criteria Std: ASCE 7	7-16	Snow Criteria (Pg,Pf in PSF Pg: NA Ct: NA CAT: N	Defl/CSI Criteria PP Deflection in loc I /defl	A Maximum Reactions (ibs) /# Gravity Non-Gravity
TCDL: 10.00	Speed	I: 130 mph		Pf: NA Ce: NA	VERT(LL): 0.029 F 999	240 Loc R+ /R- /Rh /Rw /U /RL
BCLL: 0.00	Enclos Risk C	sure: Closed		Lu: NA Cs: NA	VERT(CL): 0.054 F 999	180 S 409 /- /- /209 /- /175
BCDL: 10.00	EXP: E	B Kzt: NA		Show Duration: NA	HORZ(LL): 0.010 L -	- Q 1462 /- /- /838 /17 /- - L 1628 /- /- /673 /21 /-
NCBCLL: 10.00	Mean	Height: 15.00	ft	Building Code:	Creep Factor: 2.0	J 162 /-37 /- /111 /- /-
Soffit: 0.00	BCDL:	: 5.0 psi : 5.0 psf		FBC 7th Ed. 2020 Res.	Max TC CSI: 0.643	Wind reactions based on MWFRS
Load Duration: 1.25	MWFF	RS Parallel Di	st: h to 2h	TPI Std: 2014	Max BC CSI: 0.397	Q Brg Wid = 3.5 Min Req = 1.7 (Truss)
Spacing: 24.0 "	C&C E	Dist a: 3.70 ft rom endwall: r	not in 9 00 ft	FT/RT:20(0)/10(0)		L Brg Wid = 3.5 Min Req = 1.9
	200.11	GCpi: 0.18	101 III 3.00 II	Plate Type(s):		Bearings S. Q. & L are a rigid surface.
	Wind I	Duration: 1.60)	WAVE	VIEW Ver: 21.02.00.1005.17	Members not listed have forces less than 375#
Lumber						Maximum Top Chord Forces Per Ply (lbs)
Top chord: 2x4 SP #2 Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;	,					D-E 157 -717 F-G 172 -535 E-F 206 -728
Bracing (a) Continuous lateral	restrain	nt equally spa	ced on			Maximum Bot Chord Forces Per Ply (Ibs) Chords Tens.Comp. Chords Tens. Comp.
						O-N 525 -85 M-L 516 -88
Plating Notes		a stad				N - M 729 - 131
Hangers / Ties	eptasr	loted.				Maximum Web Forces Per Ply (lbs) Webs Tens Comp Webs Tens Comp
(J) Hanger Support Ro	equired,	, by others			AND AND ADDRESS FEELAND	B-S 29 - 376 M-G 551 - 4
Loading				and the second second	CIAS FLEA	Q-P 211 - 1241 G-L 283 - 1197
Truss passed check for chord live load in area clearance.	or 20 ps is with 4	f additional b 12"-high x 24"	ottom -wide	69	CENS	P - D 233 - 1205 L - H 141 - 455 D - O 750 - 108
Purlins In lieu of structural pa brace all flat TC @ 24	nels or i " oc. all	rigid ceiling us	se purlins to		No. 66648	
Wind	.,	0			✓★	· 🚆
Wind loads based on member design.	MWFR	S with additio	nal C&C	RI	STATE OF	
End verticals not expo	osed to v	wind pressure).	0	COPIDP.	Ý.
Wind loading based o	n both g	gable and hip	roof types.		Ch. Start Gill	
					WONAL EN MAN	
				Florida Ce	ertificate of Product Approval #	FL1999
				08/24/	/2022	
**MOODT	**WA			LLOW ALL NOTES ON THIS		
Trusses require extrem	ne care	in fabricating	, handling, sh	ipping, installing and bracing	. Refer to and follow the latest e	dition of BCSI (Building
bracing per BCSI. Unle attached rigid ceiling. I as applicable. Apply	ormation ess note ocation plates to	ed otherwise, as shown for p beach face of	top chord shapermanent lat truss and po	lefy practices prior to perform Il have properly attached stri eral restraint of webs shall ha sition as shown above and o	ning mese functions. Installers s uctural sheathing and bottom cho ave bracing installed per BCSI se in the Joint Details, unless noted	ran provide temporary rd shall have a property ctions B3, B7, or B10, otherwise. Refer to

as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, "unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information. Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org





Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.







SEQN: 112261	GABL	Ply: 1	Job Nun	nber: 22-8147			Cust: R 215 JRef:	1XIc2150005	5 T7
FROM: Page 1 of 2		Qty: 1	Edentield	a bel: A09			DrwNo: 235.22.11 KD / DF	09.56620	22
		3"8	5'2"15	13'4"8 19'6" 20'9"10	25'5"12 31'7"3 36'8"8 44	0'1"15 44'			
		3*8	4'11"7	8'1"9 ⁻¹⁻ 6'1"8 ⁻¹⁻ 1'3 ^L 10	4'8"2 ^{-1"} 6'1"7 ^{-1"} 5'1"5 ^{-1"} 3'	5"7 "1" 3'10"1 "1			
		 -	13'6"4	ا ا	- <u>2' -1'5'12</u> (TYP)				
				=5X6	3X4				
		Ŧ					Ŧ		
					ARA ARA				
		8				[●] 3X4			
		ie I	SX6 C			RS 3X4	10'3"3		
		#3X4			AUTO				
		TA B				BF	-+ ¹⁰		
			AJ AI ≣3X4	■5X6 ■3X4	AF =5x6 ≥2x4(**) AD AC AB 3x4 #2.5x6 #4x5		└ <u>╁-</u> ╋°		
			III3X6		334 2.5X6 ≡3X8				
		5'1	12 	28'8"8	112 2999 1102				
		A 101		2000		- 000 - X			
		H ² *H*	5'3"8	13'4"8	25'4"	44' +			
					1' 34'11"2				
						(NNL)			
						10 10 11			
Loading Criteria (psf)	Wind	Criteria	_	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum F	Reactions (Ibs), or	*=PLF	
TCLL: 20.00	Wind Speed	Std: ASCE 7-1	5	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc R+ /R	.y - /Rh /R¹	won-Gravi	/ity /RL
BCLL: 0.00	Enclos	sure: Closed		Lu: NA Cs: NA	VERT(LL): 0.047 H 999 240	AK 255 /-	/_ /18	7 /54	/197
BCDL: 10.00	Risk C	Category: II		Snow Duration: NA	HORZ(LL): 0.020 AB	AJ 1605 /-	/- /10	56 /-	/-
Des Ld: 40.00	Mean	B KZT: NA Height: 15.00 ft			-HORZ(TL): 0.042 AB	AC 480 /-	/- /25	9 /20	/-
NCBCLL: 10.00	TCDL:	: 5.0 psf		Building Code:	Creep Factor: 2.0	Z - /-9	/- /54 4 /- /17	/- /49	/- /-
Somit: 0.00	BCDL:	: 5.0 psf	h to 2h	TPI Std: 2014	Max FC CSI: 0.744 Max BC CSI: 0.575	Y* 173 /-	/- /99	/-	/-
Spacing: 24.0 "	C&C E	Dist a: 4.40 ft	11 10 211	Rep Fac: Yes	Max Web CSI: 0.461	BF 160 /- Wind reaction	/- /10 s based on MWFR	0 /1/ S	/-
	Loc. fr	om endwall: not	in 13.00 ft	FT/RT:20(0)/10(0)		AK Brg Wid =	= 3.5 Min Req =	1.5 (Truss)	5)
	Wind [GCpi: 0.18		Plate Type(s):	VIEW Ver: 21.02.01.1216.15	AJ Brg Wid =	3.5 Min Req =	1.9 (Truss)	5) .)
Lumber	WINGL	Duration. 1.00		Additional Notes	VIEW Vel. 21.02.01.1210.15	AC Brg Wid =	= 32.5 Min Req =	1.5 (11uss) -	9
Top chord: 2x4 SP #2	2:			See DWGS A14015ENC16	60118 & GBLLETIN0118 for	Z Brg Wid =	= 3.5 Min Req =	1.5 (Truss)	;)
Bot chord: 2x4 SP #2	;			gable wind bracing and oth	her requirements.	Y Brg Wid = BF Brg Wid =	= 80.5 Min Req = = 3.5 Min Req =	- 1.5 (Truss	;)
Stack Chord: SC1 2x4	4 SP #2	;		Stacked top chord must NC area (NNL) Dropped top cl	OT be notched or cut in hord braced at 24" oc	Bearings AK,	AJ, AC, AC, Z, Y, 8	ž	,
Rt Stub Wedge: 2x4 S	SP #3;			intervals. Attach stacked to	p chord (SC) to dropped	BF are a rigid	surrace.	oee than 3	75#
Bracing				oc. Center plate on stacked	d/dropped chord interface,	Maximum To	p Chord Forces P	er Ply (lbs	s)
(a) Continuous lateral	restrain	nt equally spaced	lon	plate length perpendicular	to chord length. Splice top	Chords Tens	.Comp. Chords	Tens.	Comp.
member.				. Momber to be leterally br	ing oxo.	C-D 259	9 - 1211 I - L	236	- 778
Plating Notes				loads. bracing system to	be desiged and furnished	D-E 300 F-F 310	3-1082 L-O 7-1076 O-P	192 111	- 458 - 384
All plates are 2X4 exc	ept as r	noted.		by others.		F-I 230	6 - 779		
scaled plate plot detail	ils for sp	positioning. Rela		MARTIN	A C CI MINING			D 1 (11)	
requirements.				A STATE	LAD FLEM	Chords Tens	Comp. Chords Porces Po	Tens.	5) Comp.
Loading					ICENS:	AH-AG 88	-56 AG-AE	1088	- 104
Gable end supports 8	" max ra	ake overhang. To	ор			AII-AO 00.	-30 AC-AL	1000	- 104
chora must not be cut	or note	nea.				Maximum We	b Forces Per Ply	(lbs)	
Purlins					NO.06648	Webs Tens	.Comp. Webs	Tens. (Comp.
In lieu of structural pa	nels use	e purlins to brace	e all flat			AJ-AI 202	2 - 1534 AR-L	774	- 129
Wind					STATE OF	C-AH 95	3 - 1400 AS-AX 3 - 57 AD-AX	689	-499 0
Wind loads based ar		S with additional	C&C	181	STATE OF 181	F-AN 11	3-390 AX-AY	635	0
member design.	WINNLERS		Jac	101	ALOPIOP ST	AE-AP 619 AE-AD 71) -75 AX-AZ 1 -86 AC-A7	141 119	- 435 - 403
Left end vertical not e	xposed	to wind pressure).	1. C.A.	Give and a give	AP-AQ 65	3 - 85 AY-BB	558	0
Wind loading based of	on both g	gable and hip roo	of types.		YONAL EN	AP-AS 14	3 - 461 BB- P	583	0
				Florida Certi	ificate of Product Approval #FL	1999	- 120 F - I	0	- 007
				08/24/20	022	Maximum Ga	ble Forces Per Pl	y (Ibs)	
	WA	RNING REAL	O AND FOI	LLOW ALL NOTES ON THIS D	RAWING!				
	ANT**	FURNISH THIS	DRAWING	TO ALL CONTRACTORS INC	LUDING THE INSTALLERS	of BCSI (Building	a		
Component Safety Info	ormation	n, by TPI and SE	CA) for sal	fety practices prior to performing	these functions. Installers shall p	rovide temporal	Ϋ́Υ Ϋ́		
attached rigid ceiling. I	Location	is shown for per	nanent late	eral restraint of webs shall have	bracing installed per BCSI sections	B3, B7, or B10 wise Potor to	, ,		

as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information. Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org



155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

SEQN: 112261 G	ABL	Ply: 1	Job Number:	22-8147							Cust: R 2	15 JRef:1>	(lc2150005	T7 [.]
FROM: Page 2 of 2		Qty: 1	Edenfield Truss I abel:	A09							DrwNo:	235.22.110 DF	9.56620 08/23/202	2
			Thuếo Luben	100					Gables	Tens.0	Comp.	Gables	Tens. C	- Comp.
									AU-AD	75	- 550	AU- L	63	- 479
						THE REAL PROPERTY LABOR	WHITE MAN							
					a statistic	GLAS	FLFA	THE .						
					0	A CE	No	NAL						
					12	· YIUL	78.	· C · N						
						No R	6648							
							040							
						T		171						
					121	STATI	e of	19:3						
					1201	S-Los	SIDA	SUN						
						Sec	- nic							
					COA #0	278 UN	IL EI	A PAR .						
					Florida C 08/24	ertificate of 4/2022	Product Ap	pproval #FL1	999					
	**W^C						1							
	VVAN NT** F	URNISH THIS D	RAWING TO	ALL CON	FRACTORS I	NCLUDING	THE INSTA	LLERS	of BCSI /	Ruildin	a			
Component Safety Inforr bracing per BCSI. Unles	mation	, by TPI and SBC	CA) for safety r	e properiv	ior to perform attached stru	ning these fur	nctions. Ins	tallers shall p	rovide ten all have a	porary	y y			
attached rigid ceiling. Lo as applicable. Apply pla	ates to	s shown for perm each face of trus	anent lateral r	estraint of as shown	webs shall ha	ave bracing in the Joint De dditional inter-	stălled per l etails, unles	BCSI sections s noted other	B3, B7, c wise. Re	or B10, fer to	-			. I r
Alpine, a division of ITW	Buildi	ng Components (Group Inc. sha	ll not be re	es page for a sponsible for	any deviation	n from this d	Irawing, any fa	ailure to b	uild the	•	ΆL		
truss in conformance wit listing this drawing, indic	th ANS	il/IPI 1, or for ha	andling, shipp ressional engine the Building D	ing, install	ation and bra ponsibility sol	lely for the de	es. A seal o sign shown	on this drawin . The suitabilit	g or cover ty and use	page of this	;	155 Harl North Bi	em Ave ildina. 4th	Floor
For more information see	e these	e web sites: Alpin	e: alpineitw.co	om; TPI: tpi	inst.org; SBC	A: sbcacomp	onents.com	; ICC: iccsafe	.org; AWC	C: awc.	org	Glenviev	v, IL 60025	5













North Building, 4th Floor Glenview, IL 60025











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SEQN: 99371	COMN	Ply:	1	Job Nun	nber: 22-8147			Cust: R 215	5 JRef:1XI	:2150005	5 T18
FROM:		Qty:	1	Edenfield	j abol: C05			DrwNo: 2	35.22.1112	.37173 18/23/202	22
				HAXEGRE ^{3X4} HAXEGRE ^{3X4} HAXEGRE ^{3X4} HAXEGRE ^{3X4} HAXEGRE ^{3X4} HAXEGRE ^{3X4}		177'14 239' 59'14 60'2		9 ⁹	<u>, , , , , , , , , , , , , , , , , , , </u>		
			I	+ 1'1"14 1'1"14 + 1'2"2 + 1'2"4	4'1'4 46'8 8'8 65'4 + 10'11*12 11'8'4	60°12 5'11" 179" 238"	— -†- 1'6* - -				
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: <u>10.00</u> Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind C Wind S Speed Enclos Risk C EXP: E Mean I TCDL: BCDL: BCDL: C&C D	Criteria Std: A : 130 ure: Cl ategory 3 Kzt: Height: 5.0 ps 5.0 ps (S Para Dist a: 3 on on on	a SCE 7-16 mph losed y: II : NA 15.00 ft f f allel Dist: h/ 8.00 ft tworlt not in	/2 to h	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0)	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.040 O 999 24 VERT(CL): 0.085 O 999 18 HORZ(LL): 0.033 I - HORZ(TL): 0.070 I - Creep Factor: 2.0 Max TC CSI: 0.441 Max BC CSI: 0.371 Max Web CSI: 0.444	A Maximum F Gravii 0 Loc R+ / R 0 S 1015 /- 1 1120 /- Wind reaction S Brg Wid = I Brg Wid = Bearings S & Members not Maximum To	eactions (y - / Rh /- s based on : 3.5 Min are a rigid isted have p Chord F	(Ibs) No / Rw /559 /654 MWFRS Req = 1.5 Req = 1.5 Req = 1.5 surface. forces less porces Per	on-Gravi /U /15 /17 ; (Truss) ; (Truss) ; (Truss) s than 3: Ply (Ibs	ty / RL /222 /-) 75#
	LOC. IT	om end GCpi	i: 0.18	19.00 π	Plate Type(s):		Chords Tens	Comp.	Chords	Tens. C	Comp.
1	Wind E	Juratio	n: 1.60		WAVE	VIEW Ver: 21.02.00.1005.17		- 497 3 - 1681	D-E E-F	125 123	- 929 - 951
Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Plating Notes All plates are 2X4 exce	ept as n	ioted.					C - D 67 Maximum Bo <u>Chords Tens</u> P - O 1379 O - M 902	' - 1264 t Chord Fc . <u>Comp.</u> 5 - 152 2 - 38	F - G orces Per Chords M - K	66 P ly (Ibs) Tens. (878	- 1154) <u>Comp.</u> - 39
Purlins In lieu of structural par TC @ 24" oc.	nels use) purlin:	s to brace a	all flat			Maximum We Webs Tens	b Forces I .Comp.	Per Ply (lb Webs	s) Tens. (Comp.
Wind Wind loads based on I member design. End verticals not expo Wind loading based on Note: Laterally brace 2'0" O.C.Max. includin	MWFRS sed to w n both g bottom g a late	3 with a wind pr jable a chord a ral brad	additional C essure. nd hip roof above filler ce at chord	C&C types. at ends.	COA #0 278 Florida Certific	AS FLEM CENSE 0. 66648 TATE OF CORIDA GRIDA CORIDA	A - S 6 A - R 103 R - B (R - P 91 B - P 95 C - O 12	- 977 - 42 - 1658 2 - 181 - 46 2 - 503	D-K E-K K-J J-G G-I	88 694 784 771 84	- 404 - 67 0 0 - 1071
IMPORTA Trusses require extrem Component Safety Info bracing per BCSI. Unle attached rigid ceiling. L as applicable. Apply p drawings 160A-2 for st Alpine, a division of ITN truss in conformance w	**WAF NT I be care i rmation ss note ocation lates to andard V Buildi vith ANS	RURNI FURNI in fabri by TF d other s show each f plate p ing Cor SI/TPI	** READ SH THIS D cating, han PI and SBC rwise, top c rwise, top c rwise, top c rwise, top c rwise, top c for perm bositions. R mponents (1, or for ha	AND FOI PRAWING Idling, shi CA) for sat anent late s and pos efer to jol Group Inc andling, s	10000000000000000000000000000000000000	RAWING! LUDING THE INSTALLERS efer to and follow the latest editic these functions. Installers shall ral sheathing and bottom chord s bracing installed per BCSI sectio p Joint Details, unless noted oth ional information. y deviation from this drawing, any g of frusses. A seal on this drawing, any	n of BCSI (Buildi provide temporai hall have a prope ns B3, B7, or B10 erwise. Refer to failure to build th ing or cover page	ng y riy e	AL 155 Harle		

listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 99374	COMN	Ply: 1	Job Nu	mber: 22-8147			Cust: R 215 JRef: 12	XIc2150005 T31
FROM:		Qty: 1	Edenfiel	d abel: C06			DrwNo: 235.22.111	2.40643
			11033 E					00/20/2022
		k	2'10"13	70"2 11'10"	17'7"14 _1_ 23'8"			
		1	2'10"13	4'1"6 4'9"14	5'9"14 6'0"2	1		
		т		=4X4 E			т	
				10 3 X4				
					€5X5 F			
		112.5	112 ¥4				115-14	
		=4X4 A	₩4X5(SRS B C				ĺ	
		3.6	WL.			G T		
			4				4.8	
		⊥ ⊥ ⊥ R	PQ ≡7X6 ⊯2X4	2X4 ≡5X6 2X4	≡3X8		<u></u> Τ−Φ	
				₩2X4				
		F		23'8"				
		H	2'4" 2'4" +	4'4"10 ++ 4'3"2 + <mark>8"8</mark> 6'8"10 +0'11"12 +11'8"4	60°12 5'11" 17'9" 23'8"			
Loading Criteria (psf)	Wind	Criteria		Snow Criteria (Pa.Pf in PSF)	Defl/CSI Criteria	▲ Maximum R	leactions (lbs)	
TCLL: 20.00	Wind	Std: ASCE 7-16		Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Gravit	y ` 1	Non-Gravity
TCDL: 10.00	Speed	1: 130 mph sure: Closed		Pf: NA Ce: NA	VERT(LL): 0.035 L 999 240	$\frac{LOC}{D}$ 4045 /	- / KII / KW	/0 /RL
BCDL: 10.00	Risk C	Category: II		Snow Duration: NA	HORZ(LL): 0.024 I	I 1120 /-	/- /656	/13 /-
Des Ld: 40.00	Mean	B KZT: NA Height: 15.00 ft		Duilding Code:	HORZ(TL): 0.052 I	Wind reactions	s based on MWFRS	5 (Truss)
NCBCLL: 10.00 Soffit: 0.00	TCDL	: 5.0 psf		FBC 7th Ed. 2020 Res.	Max TC CSI: 0.441	I Brg Wid =	3.5 Min Req = 1	.5 (Truss)
Load Duration: 1.25	MWFF	RS Parallel Dist: h	/2 to h	TPI Std: 2014	Max BC CSI: 0.336	Bearings R & I Members not I	are a rigid surface.	ss than 375#
Spacing: 24.0 "	C&C I	Dist a: 3.00 ft rom endwall: not ir	n 9 00 ft	Rep Fac: Yes FT/RT:20(0)/10(0)	Max Web CSI: 0.482	Maximum To	Chord Forces Pe	r Ply (lbs)
	200.11	GCpi: 0.18	10.00 1	Plate Type(s):			Comp. Choras	Tens. Comp.
Lumbor	Wind	Duration: 1.60		WAVE	VIEW Ver: 21.02.00.1005.17	B-C 29	9-875 D-E 9-889 E-F	127 - 921 124 - 951
Top chord: 2x4 SP #2	2;					C - D 72	2 - 1226 F - G	66 - 1154
Bot chord: 2x4 SP #2 Webs: 2x4 SP #3	;					Maximum Bo	t Chord Forces Pe	r Ply (lbs)
Purline						Chords Tens	Comp. Chords	Tens. Comp.
In lieu of structural pa	anels us	e purlins to brace	all flat			P-O 1128 O-M 883	3 -73 M-K 3 -23	859 - 26
TC @ 24" oc.		•				·		
Wind						Maximum We	b Forces Per Ply (Ibs) Tens Comp
Wind loads based on member design.	MWFR	S with additional (C&C			A-R 60	0.982 F.K	701 - 71
End verticals not expe	osed to	wind pressure.				A - P 1266	6 - 41 K - J	785 0
Wind loading based of	on both g	gable and hip roof	types.		ANTEN CONCERN PERFORMAN	P-C 54 D-K 86	I-800 J-G 3-398 G-I	771 0 84 - 1071
Note: Laterally brace 2'0" O.C.Max. includi	bottom	chord above filler eral brace at chord	at ends.	C.	AS FLEAD			
				0	CENS			
					IN THE REAL			
					0 86848			
				0/				
				**				
				ai s	TATE OF			
				81	A another the			
				182	UKIYA			
					ONAL ENGINE			
				Florida Certif	icate of Product Approval #FL1	999		
	++14/ -			08/24/202				
IMPORT	ANT	KNING** READ		G TO ALL CONTRACTORS INC	LUDING THE INSTALLERS			
I russes require extrem Component Safety Inf	ne care ormatio	In tabricating, har n, by TPI and SBC	ndling, sh CA) for sa	pping, installing and bracing. F	these functions. Installers shall p	ot BCSI (Buildir rovide temporar	ng Y	
attached rigid ceiling.	Location	ns shown for perm peach face of trus	anent lat	eral restraint of webs shall have sition as shown above and on th	bracing installed per BCSI sections provint Details, unless noted other	B3, B7, or B10 wise. Refer to	, ,	

SEQN: 99377	SPEC	Ply: 1	Job	Number: 22-8147			Cust: R 215 JRef: 1X	lic2150005 T3
FROM:		Qty: 1	Eden	s Label: C07			DrwNo: 235.22.1112 KD / DF	2.43340 08/23/2022
		<u> </u>	46*		177*14123'8"			
			4'6"	* * 7'4*	5'9"14 *1* 6'0"2			
		↓	II 2X4 A II 2X4 II 2X4	5X10(SRS) B B B B C (a) (a) (a) (a) (b) (c) (c) (c) (c) (c) (c) (c) (c) (c) (c	(a) +5X5 (a) + =3X4		, 1 −− Φ ₀ ,	
			4'4"4	75*12	6'0°12 5'9°4			
			4'4"4	11'10"	171012 23%			
Loading Criteria (psf) TCLL: 20.00	Wind Wind S	Criteria Std: AS	SCE 7-16	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	▲ Maximum I Gravi	reactions (Ibs) ity N	lon-Gravity
TCDL: 10.00	Speed	: 130 m	nph load	Pf: NA Ce: NA	VERT(LL): 0.025 I 999 240	Loc R+ /F	<u>≀- / Rh / Rw</u>	/U /RL
BCLL: 0.00 BCDL: 10.00	Risk C	ategory:		Lu: NA Cs: NA Snow Duration: NA	VERT(CL): 0.054 I 999 180 HORZ(LL): 0.013 A	K 1015 /-	/- /564 /- /659	/33 /205 /8 /-
Des Ld: 40.00	EXP: E	B Kzt:	NA		HORZ(TL): 0.027 A	Wind reaction	is based on MWFRS	10 1
NCBCLL: 10.00	TCDL:	5.0 psf	15.00 1	Building Code:	Creep Factor: 2.0	K Brg Wid	=- Min Req = - = 3.5 Min Req = 1	5 (Truss)
Soffit: 0.00	BCDL:	5.0 psf	lol Dict: h/2 to h	TPI Std: 2014	Max BC CSI: 0.535 Max BC CSI: 0.496	Bearing G is a	a rigid surface.	0 (11000)
Spacing: 24.0 "	C&C E	ist a: 3.0	00 ft	Rep Fac: Yes	Max Web CSI: 0.880	Members not	listed have forces les Chord Forces Per	is than 375# (Ibs)
	Loc. fr	om endv	vall: not in 9.00	ft FT/RT:20(0)/10(0)		Chords Tens	S.Comp. Chords	Tens. Comp.
Lumber	Wind [Ouration:	: 1.60	WAVE	VIEW Ver: 21.02.00.1005.17	B-C 12 C-D 14	2 - 890 D - E 2 - 874	67 - 1145
Top chord: 2x4 SP #2 Bot chord: 2x4 SP #2 Wobs: 2x4 SP #2	; T2 2x4	SP M-3	31;			Maximum Bo Chords Tens	ot Chord Forces Per s.Comp. Chords	Ply (lbs) Tens. Comp.
Breeing						K-J 79	2 0 I-H	790 0
(a) Continuous lateral member.	restrain	t equally	y spaced on			J-I 78 Maximum W	8 - 1 eb Forces Per Ply (II	bs)
Hangers / Ties						Webs Tens	S.Comp. Webs	Tens. Comp.
(J) Hanger Support R	equired,	by othe	rs			K-B 9 C-I 55	9-1154 H-E 3-42 E-G	758 0 86 - 1070
In lieu of structural pa	nels use	purlins	to brace all flat		- A R MA AR HOUSE			
TC @ 24" oc.		-		-UPINT	AS SI AND INTERNAL			
Wind				in the second seco	LAS FLEMING			
wind loads based on member design.	MWFR	s with ac	dditional C&C	09.7	CENS			
End verticals not expo	osed to v	vind pre	ssure.					
Wind loading based o	n both g	able and	d hip roof types		No. 66648 👔 📳			
					∀ ★ ★			
					STATE OF			
				31				
					CORID			
				3 S S S	MALAL END			
				COA #0 278 Florida Certi	ficate of Product Approval #FI 1	999		
				08/24/20)22			
**IMPORT	**WAI	RNING*	* READ AND H THIS DRAW	FOLLOW ALL NOTES ON THIS I	DRAWING! CLUDING THE INSTALLERS			
Trusses require extrem Component Safety Info	ne care	in fabric	ating, handling, I and SBCA) for	shipping, installing and bracing. safety practices prior to performin	Refer to and follow the latest edition of these functions. Installers shall r	of BCSI (Buildi	ing Iry	
pracing per BCSI. Unle attached rigid ceiling. I	ess note	a otherv s showr	vise, top chórd s n for permanent	snall have properly attached struct lateral restraint of webs shall have	tural sheathing and bottom chord sh e bracing installed per BCSI section	all have a prope s B3, B7, or B10	sriy),	A .
drawings 160A-Z for s	tandard	plate po	sitions. Refer to	b job's General Notes page for add	ditional information.		" AĹ	PÌNE

SEQN: 99380 FROM:	SPEC	Ply: 1 Otv: 1	Job Nu Edenfie	imber: 22-8147 eld			Cust: R 21 DrwNo:	5 JRef:1X	ic2150005 T40 2.44820
		Gillion i	Truss L	Label: C08			KD /	DF (08/23/2022
		ļ	6'1"3 6'1"3	- - <u>11'10*</u> - 5'8'13 ■44	<u>+ 177°14 + 23'8'</u> 5'9°14 + 60°2 ≿5	+			
			(a) 1 51177		(a) + + + + + + + + + + + + + + + + + + +				
Loading Criteria (psf)	Wind	Critoria	-	Snow Criteria (Pa Pf in PS		▲ Maxim	um Reactions	(lbs)	
TCLL: 20.00	Wind S	Std: ASCE 7	7-16	Pg: NA Ct: NA CAT:	NA PP Deflection in loc L/defl L/#	G	Gravity	(on-Gravity
TCDL: 10.00	Speed Enclos	: 130 mph sure: Closed		Pf: NA Ce: NA	A VERT(LL): 0.027 I 999 240 VERT(CL): 0.058 I 999 180	LOC R+	/ K- / KII	/ KW	/U / KL
BCDL: 10.00	Risk C	ategory: II		Snow Duration: NA	HORZ(LL): 0.013 A	G 1120	- -	/662	/- /- /- /-
Des Ld: 40.00	Mean	Height: 15.00	ft	Building Code:		Wind read	ctions based o Vid = - Mi	n Req = -	
Soffit: 0.00	TCDL: BCDL:	5.0 psf 5.0 psf		FBC 7th Ed. 2020 Res.	Max TC CSI: 0.762	G Brg V	Vid = 3.5 Mi	n Req = 1.	5 (Truss)
Load Duration: 1.25	MWFR	RS Parallel Di	st: h to 2h	TPI Std: 2014 Rep Fac: Yes	Max BC CSI: 0.482 Max Web CSI: 0.414	Members	not listed have	orces les	s than 375#
Spacing. 24.0	Loc. fr	om endwall: r	not in 9.00 ft	FT/RT:20(0)/10(0)		Maximun Chords	n Top Chord I Tens.Comp.	Chords	Ply (lbs) Tens. Comp.
	Wind E	GCpi: 0.18 Duration: 1.60)	Plate Type(s):	VIEW Ver: 21.02.00.1005.17	В-С	139 - 845	D - E	70 - 1149
Lumber					I	⊐ C - D	148 - 872		
Top chord: 2x4 SP #2 Bot chord: 2x4 SP #2 Webs: 2x4 SP #3;	;					Maximun Chords	n Bot Chord F Tens.Comp.	orces Per Chords	Ply (lbs) Tens. Comp.
Bracing						K - J	765 0 762 0	I - H	794 0
(a) Continuous lateral member.	restrain	t equally spa	ced on			Maximun	n Web Forces	Per Ply (It	os)
Hangers / Ties		h				Webs	Tens.Comp.	Webs	Tens. Comp.
(J) Hanger Support R	equirea,	by others				к-в С-І	133 - 1082 568 - 76	H-E E-G	765 0 88 - 1071
In lieu of structural pa	nels use	e purlins to bra	ace all flat						
TC @ 24" oc.					antimental land and in the state of the stat				
Wind				and the second se	GLAD FLEM				
member design.	MWFR	S with additio	nalC&C	69	CENSA				
End verticals not expo	osed to v	wind pressure).						
Wind loading based o	n both g	gable and hip	roof types.		No. 66648				
				*	✓★ ★				
					STATE OF				
				121	AL- ALLY				
				12	CORIV				
				COA #0	STONAL ENGLAND				
				Florida C	ertificate of Product Approval #FL	1999			
	WAI	RNING RF		U8/24 DLLOW ALL NOTES ON TH	IS DRAWING!				
IMPORT	ANT	FURNISH TH	IIS DRAWIN	G TO ALL CONTRACTORS	INCLUDING THE INSTALLERS g. Refer to and follow the latest edition	n of BCSI (R	uildina		
Component Safety Info bracing per BCSI. Unle	ormation	n, by TPI and d otherwise,	SBCA) for sa top chord sha	afety practices prior to perfor all have properly attached st	ming these functions. Installers shall ructural sheathing and bottom chord s	provide tem hall have a p	porary roperly		
as applicable. Apply drawings 160A-Z for s	plates to tandard	plate position	truss and po is. Refer to ic	ob's General Notes page for	on the Joint Details, unless noted othe additional information.	erwise. Refe	er to		
last interaction		· •	(an ann a dao da taon tao an tha ann a'	4-11			

SEQN: 99382	SPEC	Ply: 1		Job Nu	nber: 22-8	147							Cust: R 2	15 JRef:1X	lc2150005	т30 ́
FROM:		Qty: 1		Truss L	a bel: C09								Drwino: KD /	235.22.1112 DF	2.50373 08/23/202	2
			<u> </u>	7'8" 7'8	6	-+	11'10" 4'1°10 = + ≡4X4	17'7*14 5'9*14	- +	23'8" 6'0"2	+					
		+	= 3X5 Å (a) (a) K E2.5X6		#5 T1			(a)	€5X5 0 H =3X4				т т т т т т т т			
			-	7'4"1	4	+ 1	11'10"	17'10"12		23'8"		4				
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00	Wind S Wind S Speed Enclos Risk C	Criteria Std: AS : 130 n sure: Clo ategory 3 Kzt [.]	SCE 7-16 nph osed : II NA		Snow Cr Pg: NA Pf: NA Lu: NA Snow Du	iteria (Po Ct: NA Cs: NA ration: N/	g,Pf in PSF) CAT: NA Ce: NA	Defl/CSI CI PP Deflection VERT(LL): VERT(CL): HORZ(LL):	riteria on in loc L/d 0.033 B 9 0.070 B 9 0.011 A	efl L/# 199 240 199 180 	▲ Maxim <u>Loc R+</u> K 101: G 112:	num Re Gravity - /R- 5 /- 0 /-	eactions / Rh /- /-	(lbs) / Rw /565 /667	on-Gravit /U /4 /-	ty / RL /197 /-
Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Mean I TCDL: BCDL: MWFR C&C D	Height: 5.0 psf 5.0 psf S Paral Dist a: 3.	15.00 ft Ilel Dist: h 00 ft	to 2h	Building (FBC 7th TPI Std: Rep Fac:	Code: Ed. 2020 2014 Yes	Res.	HORZ(TL): Creep Fact Max TC CS Max BC CS Max Web C	0.023 A or: 2.0 SI: 0.475 SI: 0.496 CSI: 0.634		K Brg G Brg Bearing Member	actions Wid = - Wid = 3 G is a r rs not lis	based o Mi 3.5 Mi igid surf ited have	n MWFRS n Req = - n Req = 1. ace. e forces les	5 (Truss) is than 37	75#
	Loc. fro Wind D	om end GCpi: Duration	wall: not ir 0.18 :: 1.60	n 9.00 ft	FT/RT:20 Plate Typ WAVF	/(0)/10(0) /e(s):		VIEW Ver: :	21.02.00.100	5.17	Chords A - B	Tens.C	- 728	Chords C - D	Tens. 0	Comp. - 871
Lumber	1										B - C	182	- 840	D - E	76	- 1153
Top chord: 2x4 SP #2 Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;	; T1 2x4	ISPM∹	31;								Maximu Chords	m Bot Tens.C	Chord F Comp.	chords	Ply (lbs) Tens. C	Comp.
Bracing (a) Continuous lateral	restrain	t equall	y spaced (on							J-I Maximu	750 Im Web	0 Forces	I - H	799 hs)	0
member. Hangers / Ties											Webs	Tens.C	Comp.	Webs	Tens. C	Comp.
(J) Hanger Support Re	equired,	by othe	ers								A-J J-B	1032 205	- 185 - 588	H-E E-G	773 91	- 1073
In lieu of structural par TC @ 24" oc.	nels use	e purlins	to brace	all flat			att	NALEN COMPANY	11/10/1/100		В-1	143	- 385			
Wind Wind loads based on member design	MWFR	S with a	dditional (C&C			0016	CEN	EMING							
End verticals not expo Wind loading based o	osed to v n both g	wind pre gable an	essure. Id hip roof	types.				No. 666	48	Support of the local division of the local d						
						C F	COA #0 278 lorida Certi 08/24/20		OF DA. INF ENGINE duct Approv	val #FL1	999					
**IMPORTA Trusses require extrem Component Safety Info bracing per BCSI. Unle attached rigid ceiling. I as applicable. Apply p drawings 160A-Z for st	**WAI	RNING* FURNIS in fabric by TP of other s shown o each fa plate po	* READ SH THIS D ating, han I and SBC wise, top c n for perm ace of trus ositions. R	AND FO PRAWING dling, sh A) for sa chord sha anent lat is and po efer to jo	LOW AL pping, ins fety practic ll have pro eral restra sition as s b's Genera	L NOTES CONTRA talling and ces prior f perly atta int of web hown abc al Notes p	S ON THIS D ACTORS INC d bracing. F to performing ached structions shall have by e and on the bage for addi	RAWING! LUDING TH Refer to and for these function iral sheathing bracing instance bracing instance bracing information tional information	E INSTALLEI follow the late ions. Installe g and bottom alled per BCS ils, unless no ation.	RS st edition rs shall p chord sha l sections ted other	of BCSI (rovide ter all have a s B3, B7, o wise. Re	Building nporary properly or B10, efer to	y	Â		۷Ę

SEQN: 99385	SPEC	Ply: 1		Job Nu	mber: 22-8	3147					Cust: R 21	5 JRef:1XI	2150005	T27 [·]
FROM:		Qty: 1		Edentiel Truss L	d abel: C10						DrwNo: KD / I	235.22.1112 DF 0	.55807 18/23/2021	2
			E3X4 A (a)		abel: C10 93'10 93'10 T1 T1	11110 [°] 26°6 10 12 = 4X4 10 12 = 4X4 10 12 = 4X4 10 12 = 4X4 10 12 = 100 10 12 = 100 10 12 = 100 10 10 12 = 100 10 10 10 10 10 10 10 10 10 10 10 10 10 1	177714 59714	238*				DF 0	8/23/2022	2
			Ŀ			00108			-1					
			A		9'0"2	238	6'0"12	5'9"4						
			ŀ		9'0"2	+ <u>23 (4</u> +	17'10"12	23'8"						
Loading Criteria (psf)	Wind	Criteria			Snow Cr	iteria (Pg,Pf in PSF)	Defl/CSI Criteria		▲ Maxim	um Re	actions	(lbs)		
TCLL: 20.00	Speed	sta: As 1: 130 m	3CE 7-16		Pg: NA	Ct: NA CAT: NA Ce [.] NA	PP Deflection in lo	C L/defl L/# B 999 240	Loc R+	/R-	/ Rh	/ Rw	/U /	y / RL
BCLL: 0.00	Enclos	ure: Clo	sed		Lu: NA	Cs: NA	VERT(CL): 0.078	B 999 180	K 1015	5 /-	/-	/564	/28 /	/193
BCDL: 10.00	EXP: E	ategory: 3 Kzt:	: II NA		Snow Du	ration: NA	HORZ(LL): 0.011	A	G 1120) /-	/-	/673	-	/-
Des Ld: 40.00	Mean I	Height: 1	15.00 ft		Building (Code:	HORZ(TL): 0.024 /	A	K Brg	Wid = -	based of Mi	n Mwrks n Reg = -		
Soffit: 0.00	TCDL:	5.0 psf			FBC 7th	Ed. 2020 Res.	Max TC CSI: 0.8	77	G Brg	Wid = 3	3.5 Mi	n Req = 1.5	i (Truss)	
Load Duration: 1.25	MWFR	tS Paral	lel Dist: h í	to 2h	TPI Std:	2014	Max BC CSI: 0.7	79	Members	s not lis	igid surra ted have	ace. e forces less	s than 37	'5#
Spacing: 24.0 "	C&C D)ist a: 3.0	00 ft vall: pot in	0.00.#	FT/RT-20	Yes)(0)/10(0)	Max Web CSI: 0.6	63	Maximu	m Top	Chord F	orces Per	Ply (lbs)	
	LUC. IN	GCpi:	0.18	9.00 II	Plate Typ	be(s):			Chords	Tens.C	comp.	Chords	Tens. C	comp.
	Wind D	Juration	: 1.60		WAVE		VIEW Ver: 21.02.00).1005.17	A-B B-C	158 243	- 695 - 858	C - D D - F	163 84	- 870 - 1154
Lumber Top chord: 2x4 SP #2 Bot chord: 2x4 SP #2; Webs: 2x4 SP #3:	; T1 2x4	I SP M-3	31;						Maximu Chords	m Bot (Tens.C	Chord F	orces Per l	Ply (Ibs) Tens. C	Comp.
Bracing									J - I	711	0	I - H	800	0
(a) Continuous lateral member.	restrain	t equally	/ spaced o	n					Maximu Webs	m Web Tens.C	Forces	Per Ply (lb Webs	s) Tens. C	Comp.
Hangers / Ties									A - K	318	- 941	C - I	801	- 250
(J) Hanger Support Re	equired,	by othe	rs						A-J	971	- 219	H-E	775	0
Purlins									J-Б В-І	240	- 497 - 545	E-0	97 -	- 10/3
In lieu of structural par	nels use) purlins	to brace a	all flat			ANTH REALFILLEN							
10 @ 24 00.						and C.	AS FLEA	The						
Wind Wind loads based on member design.	MWFR	S with ac	dditional C	&C		0940	CENSE	NG						
End verticals not expo	osed to v	wind pre	ssure.				~							
Wind loading based o	n both g	jable an	d hip roof	types.		0 / N	lo. 66648	**** 1						
						*	$\checkmark \star$							
							TATE OF							
						1901 ·	CORID	NY .						
						B. C.	IGALAL ENG	A TRACE						
						COA #0 278	UNAL	Mar						
						Florida Certif 08/24/202	icate of Product Ap 22	pproval #FL1	999					
	**WAI	RNING*	* RFAD				AWING							
IMPORTA	ANT	FURNIS	H THIS D	RAWING	G TO ALL	CONTRACTORS INC	LUDING THE INSTA		of PCOL /	- سالمان ر				
Component Safety Info	ormation	1, by TPI	and SBC	A) for sa	fety practi	ces prior to performing	these functions. In:	stallers shall p	rovide tem	porary) /		•	
attached rigid ceiling. L	ocation	s shown	1 for perma	anent lat	eral restra	int of webs shall have hown above and on the	bracing installed per	BCSI sections	B3, B7, c	r B10, fer to	,			_ e
drawings 160A-Z for st	tandard	plate po	sitions. Re	efer to jo	b's Genera	al Notes page for addit	ional information.			5. 10		A	PIr	JE

SEQN: 99388	SPEC	Ply: 1	Job Nu	mber: 22-8	3147				Cust: R 2	15 JRef: 1Xlc	2150005 T6
FROM:		Qty: 1	Truss L	a abel: C11					KD /	235.22.1112. DF 0≀	3/270 3/23/2022
		 	5'5"6		10'10"13 11'10" 5'5"6 1 11'4	17'7*14 5'9*14	23'8"				
			550		≤5X10(SRS)	5514	002				
					$10 \qquad = 4X4 \\ D \qquad = 0$						
		T ≡3X4 A T □		112X4 B		\sim			Ţ		
						\mathcal{M}					
			$\langle \rangle$								
						\$5% F	5				
		(a)		-(a)	(a)	×			5"14		
					(a)				÷		
			//			(a) s					
				$\parallel \parallel / /$				III3X4			
								A C	T .		
		⊥⊥ K ∥2.5×6		=4X8	=6X8	=3	l IX4		⊤ Ť-⊕ _a		
		L			20101						
		r	5'5"6		6'4"10	60"12	5'9"4	1			
			5'5"6		11'10"	17'10*12	23'8"				
Loading Criteria (psf)	Wind	Criteria		Snow Cr	iteria (Pg,Pf in PSF)	Defl/CSI Criteri	a	▲ Maxim	um Reactions	; (lbs)	n-Gravity
TCLL: 20.00	Speed	Std: ASCE 7-10 1: 130 mph)	Pg: NA Pf: NA	Ct: NA CAT: NA Ce: NA	VERT(LL): 0.0	1 loc L/defl L/# 132 C 999 240	Loc R+	/R- /Rh	ino Rw	/U /RL
BCLL: 0.00	Enclos	sure: Closed		Lu: NA	Cs: NA	VERT(CL): 0.0	68 C 999 180	L 1015	5 /- /-	/563	/50 /189
BCDL: 10.00	EXP: E	B Kzt: NA		Snow Du	ration: NA	HORZ(LL): 0.0	11 A	H 1120 Wind rea) /- /- actions based c	/679 m MWFRS	/- /-
NCBCLL: 10.00	Mean	Height: 15.00 ft		Building	Code:	Creep Factor: 2	.0	L Brg	Wid = - M	in Req = -	<i>—</i> 、
Soffit: 0.00	BCDL	: 5.0 psf		FBC 7th	Ed. 2020 Res.	Max TC CSI: Max BC CSI:	0.436	H Brg Bearing	Wid = 3.5 M Hisarigid surl	ace.	(Truss)
Spacing: 24.0 "	MWFF C&C E	RS Parallel Dist: Dist a: 3.00 ft	h to 2h	Rep Fac:	Yes	Max Web CSI:	0.717	Member	s not listed hav	e forces less	than 375#
	Loc. fr	rom endwall: not	in 9.00 ft	FT/RT:20	0(0)/10(0)			Chords	Tens.Comp.	Chords	Tens. Comp.
	Wind I	Duration: 1.60		WAVE	be(s):	VIEW Ver: 21.0	2.00.1005.17	А-В	137 - 448	D - E	170 - 874
Lumber						•		B-C C-D	137 - 449 198 - 691	E-F	92 - 1150
Top chord: 2x4 SP #2 Bot chord: 2x4 SP #2	;										
Webs: 2x4 SP #3;	,							Maximu Chords	m Bot Chord I Tens.Comp.	Forces Per F Chords	'ly (lbs) Tens. Comp.
Bracing								K - J	595 0	 J - I	796 0
(a) Continuous lateral member.	restrair	nt equally spaced	lon					M			->
Hangers / Ties								Webs	m web Forces Tens.Comp.	Webs	5) Tens. Comp.
(J) Hanger Support R	equired,	, by others						A - L	337 - 972	D - J	627 - 167
Purlins								А-К В-К	946 - 288 267 - 430	I-F F-H	768 0 104 - 1071
In lieu of structural pa	nels use	e purlins to brace	e all flat			ALTER REPORT AND	Bran.				
10 @ 24 00.					and the second second	GLAS FLE	A a man				
Wind loads based on	MWFR	S with additional	C&C			A CENS	NA				
member design.	•.					YICK					
End verticals not expo	osed to v In both (wind pressure. nable and hip roo	of types			No 86848					
	in bour y	gable and mp lot	, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,								
						T					
					2	STATE O	Fiq				
					30,1	ALOBIO	A and the second				
						A STATISTICS	JGL				
					COA #0 2	NAL E	and the second second				
					Florida Ce 08/24	rtificate of Produce 2022	ct Approval #FL	1999			
	**\\$! * '										
IMPORT	ANT	FURNISH THIS		G TO ALL	CONTRACTORS II		STALLERS	at DOOL "	Duildir -		
Component Safety Info	ormation	n, by TPI and SB	CA) for sa	ipping, ins ifety practi all have pro	ces prior to perform	ng these functions.	Installers shall p	or BCSI (I provide terr all have a	ouliuing iporary properly		
attached rigid ceiling. I as applicable. Apply	Location	s shown for per	nanent la	eral restra	int of webs shall ha	e bracing installed the Joint Details, ι	per BCSI sections inless noted other	s B3, B7, c wise. Re	r B10, fer to		
arawings 160A-Z for s	tandard	plate positions.	kerer to jo	o's Genera	al Notes page for ac	anional information					

SEQN: 101440	COMN	Ply: 1	Job Nu	mber: 22-8147			Cust: R 215 JRef:	IXIc2150005 T13
FROM:		Qty: 1	Edenfiel	d ahel: C12			DrwNo: 235.22.11	13.02720
FROM:		Qty: 1	Edenfiel Truss L	d abel: C12	$x_{0} = \frac{158^{2}}{107^{2}} + \frac{193^{2}14}{2108} + \frac{238^{2}}{2914} + \frac{1410}{2914}$ $x_{0} = \frac{14107}{107^{2}} + \frac{193^{2}}{928} + \frac{20107}{1074}$ $x_{0} = \frac{14107}{1074} + \frac{110}{1074} + \frac{110}{107$		DrwNo: 235.22.11 KD / DF	13.02720 08/23/2022
			k		7117 32211 + 2275 1710° + 210°11 + 238° + ⊢18° -	I		
Loading Criteria (psf)	Wind (Criteria		Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum R	eactions (lbs)	Non Orreit
TCLL: 20.00	Wind Speed	Std: ASCE 7-16		Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#	Loc R+ /R-	y - /Rh /R\	w /U /RL
BCLL: 0.00	Enclos	sure: Closed		Lu: NA Cs: NA	VERT(CL): 0.488 K 582 180	N 1944 /-	/- /58	5 /53 /197
BCDL: 10.00	Risk C	ategory: II		Snow Duration: NA	HORZ(LL): -0.174 F	T 2163 /-	/- /73	8 /- /-
Des Ld: 40.00	Mean	Height: 15.00 ft		Building Codo:	-HORZ(TL): 0.392 E	Wind reactions	based on MWFR 3.5 Min Reg =	S 1.6 (Truss)
NCBCLL: 10.00	TCDL:	5.0 psf		FBC 7th Ed. 2020 Res.	Max TC CSI: 0.810	T Brg Wid =	3.5 Min Req =	1.8 (Truss)
Load Duration: 1.25	MWFR	: 5.0 psr RS Parallel Dist: h	to 2h	TPI Std: 2014	Max BC CSI: 0.536	Bearings N & T	Fare a rigid surfac	e.
Spacing: 24.0 "	C&C D	Dist a: 3.00 ft		Rep Fac: Yes	Max Web CSI: 0.768	Maximum Top	Chord Forces P	er Ply (lbs)
	Loc. fr	om endwall: not ir	n 9.00 ft	FT/RT:20(0)/10(0)		Chords Tens.	Comp. Chords	Tens. Comp.
	Wind D	Duration: 1.60		WAVE	VIEW Ver: 21.02.00.1005.17	S-F 857	-2848 E-R	1345 - 2460
Lumber				Blocking		A-B 131 C-D 154	-1129 F-G	34 - 2213 77 - 2266
Top chord: 2x4 SP #2	; T2 2x6	6 SP 2400f-2.0E;		Blocking reinforcement req	uired to	D-E 163	- 744	
Bot chord: 2x10 SP 24	400f-2.0	E; B3 2x4 SP #2;		Bearing 2 located at 23.4	(blocking >= 5.50" if used)			
Webs: 2x4 SP #3; W1	I,W2 2x F∙ W10	4 SP M-31; W3,W 2x4 SP #2'	V4,W5,			Chords Tens.	Comp. Chords	Tens. Comp.
Rt Slider: 2x6 SP 240	0f-2.0E;	block length = 1	.500'			M_I 1286		1581 0
Bracing						L-K 1286	0 J-H	1629 0
(a) Continuous lateral	restrain	t equally spaced	on					
member.						Maximum Wel	b Forces Per Ply	(Ibs) Tens Comp
Loading						A NI 252	2396 O D	01 377
Attic room loading from	n 6-1-8	to 17-6-8: Live Lo	ad: 40		CONTRACTOR DE LA CONTRACT	A-N 303 A-M 2504	-2386 Q-D	91 - 377 0 - 670
10 PSF. Dead Load: 10 F	SF Cei	lling: 10 PSF, Kne	ewalls:	ALMATE.	NS ELE	M-O 326	-420 E-S	1514 - 13
Burline				6	LEMIN	B-Q 3 P-R 0	6-844 K-S)-673 K-F	542 - 15 101 - 467
In lieu of structural par	nels use	e ourlins to brace	all flat		ICENSA VOI			101 101
TC @ 24" oc.								
Collar-tie braced with oc. or rigid ceiling.	continuo	ous lateral bracing	g at 24"		No. 66648			
Wind								
Wind loads based on	MWFR	S with additional C	C&C		PATE OF 1			
l eft end vertical not e	hosod	to wind pressure		131	STATE OF 10-1			
Wind loading based o	n both r	able and hin roof	types	1.01	ALOBIO A. SUL			
Deflect	8	,	,,	1 Ca	A A A A A A A A A A A A A A A A A A A			
	11.022		dotail		YONAL EN			
DEFLCAMB1014 for	camber	recommendation	S.	COA #0 278 Florida Certi	ficate of Product Approval #FL1	999		
Provide for adequate	drainage	e of roof.		08/24/20)22			
	WAI	RNING READ	AND FO	LLOW ALL NOTES ON THIS D	RAWING!			
**IMPORTA		FURNISH THIS D	RAWIN	G TO ALL CONTRACTORS INC	LUDING THE INSTALLERS	of BCSI (Buildin	na	
Component Safety Info	ormation ess note	n, by TPI and SBC	A) for sa	all have properly attached structure	these functions. Installers shall p	all have a proper	v 1v	
attached rigid ceiling. Las applicable. Apply r	ocation	s shown for perm	anent lat	eral restraint of webs shall have sition as shown above and on th	bracing installed per BCSI sections the Joint Details. unless noted other	s B3, B7, or B10, wise. Refer to		
drawings 160A-Z for st	andard	plate positions. R	efer to jo	b's General Notes page for addi	tional information.		A	LPINE

SEQN: 101436	COMN	Ply: 1	Job	Numbe	er: 22-8147			Cust: R 215	5 JRef:1)	(lc2150005 T43
FROM:		Qty: 1	Tru	nneid ss Labe	el: C13			KD / D	35.22.111 F	08/23/2022
			⊢	6'1*8 6'1*8	+ 113'3 113'3 511'11' 51'1' 51'1' 51'1' 51'1' 51'1' 51'1'	H158° H 19277 H978° H 2971 H 2971 H 2971 H 1877 H978° H 206°14 238° H1477 3172 H				
				(a) 51111 51111	= 336 dex = 336 dex	6 + + 3X4 + + 5X4 + + + 5X4 + + + 5X4 + + + + + + + + + + + + + + + + + + +				
Loading Criteria (psf)	Wind C	riteria		Si	now Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum	Reactions (lbs)	
TCLL: 20.00 TCDL: 10.00	Wind S Speed:	td: AS 130 n	SCE 7-16 nph	Pg	g:NA Ct:NA CAT:NA	PP Deflection in loc L/defl L/# VERT(LL): 0.182 K 999 240	Loc R+ /F	τy ₹- /Rh	۲ Rw/	/U / RL
BCLL: 0.00	Enclosu Risk Ca	ure: Clo	sed	Li	I: NA Cs: NA	VERT(CL): 0.358 K 793 180	N 1939 /-	/-	/583	/53 /195
BCDL: 10.00	EXP: B	Kzt:	NA	Sr	now Duration: NA	HORZ(LL): -0.133 F HORZ(TL): 0.263 F	T 2155 /- Wind reaction	/- Is based on	/735 MWFRS	/- /-
NCBCLL: 10.00	Mean H	leight: 5.0 psf	15.00 ft	Bu	uilding Code:	Creep Factor: 2.0	N Brg Wid	= 3.5 Min = 3.5 Min	Req = 1 Req = 1	.6 (Truss) 8 (Truss)
Soffit: 0.00	BCDL:	5.0 psf S Paral	llel Dist: h to 2h	FE	3C 7th Ed. 2020 Res. PI Std: 2014	Max TC CSI: 0.622 Max BC CSI: 0.387	Bearings N &	T are a rigi	d surface	
Spacing: 24.0 "	C&C Di	sta: 3.	00 ft	Re	ep Fac: Yes	Max Web CSI: 0.686	Members not Maximum To	listed have p Chord Fo	forces les orces Pe	s than 375# r Ply (lbs)
	Loc. fro	m end GCpi:	wall: not in 9.00 0.18	PI	ate Type(s):		Chords Tens	s.Comp.	Chords	Tens. Comp.
	Wind D	uration	: 1.60	w	AVE	VIEW Ver: 21.02.00.1005.17	S-F 107 A-B 8	6-3224 1-800	E-R F-G	531 - 1733 37 - 2210
Lumber Top chord: 2x4 SP #2 Bot chord: 2x10 SP #2 Webs: 2x4 SP #3; W4 W10 2x6 SP 2400f-2. Rt Slider: 2x6 SP 240	; T2 2x6 400f-2.0E I,W11 2x 0E; 0F-2.0E;	SP 240 E; B3 2 4 SP N block le	00f-2.0E; x4 SP #2; //-31; W5,W6, ength = 1.500'		Blocking Blocking reinforcement req prevent buckling of membe Bearing 2 located at 23.4	uired to ers over the bearings: ' (blocking >= 5.50" if used)	B - C 43 C - D 40 Maximum Bo Chords Tens	6 - 48 2 - 65 o t Chord Fc 5.Comp.	G - H orces Per Chords	72 - 2262 • Ply (Ibs) Tens. Comp.
Bracing	,		5				N - M 142	4 0	K - J	1603 0
(a) Continuous lateral member.	restraint	equall	y spaced on				M-L 140 L-K 140	4 0 4 0	J - H	1631 0
Loading		47.0		~			Maximum Webs Tens	eb Forces I	Per Ply (I Webs	bs) Tens Comp
PSF. Dead Load: 10 F	PSF Ceili	ing: 10	PSF, Kneewal	0 s:			A - N 17	5 - 901	P - Q	455 0
Durding					- MARTIN		A-O 108 N-O 25	8 - 110 0 - 1933	P-R R-F	96 - 1362 96 - 1357
In lieu of structural pa	nels use	purlins	to brace all fla	t	GIG	AS FLEMIA	O-M 153	6 0 0 767	E-S	1591 - 34
TC @ 24" oc. Collar-tie braced with oc. or rigid ceiling.	continuo	us late	ral bracing at 2	4"		CENSE	B-Q 6	4 - 1264	K - F	85 - 386
Wind						lo. 66648				
Wind loads based on	MWFRS	with a	dditional C&C							
Left end vertical not e	xposed to	o wind	pressure.							
Wind loading based o	n both ga	able an	d hip roof type	6.	3	ALL OF A				
Deflection						LORID				
Max JT VERT DEFL: DEFLCAMB1014 for	LL: 0.18' camber i	' DL: 0. recomn	20". See detail nendations.		25.02	MAINI ENG				
Provide for adequate	drainage	of roof			COA #0 278 Florida Certifi	cate of Product Approval #FL19	999			
	**\A/ A 🗅	NINO+		FOLL	08/24/202					
IMPORTA	NT F Ne care ii	URNIS	H THIS DRAV	/ING T	O ALL CONTRACTORS INC	LUDING THE INSTALLERS Refer to and follow the latest edition	of BCSI (Build	ing		
Component Safety Info bracing per BCSI. Unle	ormation,	by TP	I and SBCA) fc wise, top chord	r safet shall h	y practices prior to performing ave properly attached structu	these functions. Installers shall p iral sheathing and bottom chord sh	provide tempora all have a prope	ry erly		
as applicable. Apply drawings 160A-Z for si	blates to andard p	each fa plate po	ace of truss and ositions. Refer	i positi o job's	on as shown above and on th General Notes page for addit	tional information.	wise. Refer to	<i>,</i>	Á	PINF

155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

SEQN: 442384	COMN	Ply: 1	Job Nur	mber: 22-8147			Cust: R 215 JRef: 12	XIc2150005 T4
FROM:		Qty: 1	Edenfiel	d ahel: C14			DrwNo: 235.22.111	3.12750 08/23/2022
		тТÍ		11112 + 11100 + 154 11112 + 5104 + 163 = 10040 = 305 # 20 B = 305 # 20 B	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	T	<u></u>	
		F	(a) (a) (a) (b) (c) (c) (c) (c) (c) (c) (c) (c) (c) (c	W W B B S C S S S S S S S S S S S S S S S S			¢°	
					F	1'6" -		
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00	Wind S Wind S Speed Enclos Risk C	Criteria Std: ASCE 7-16 I: 130 mph sure: Closed category: II		Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.213 J 999 240 VERT(CL): 0.416 J 682 180 HORZ(L): -0.162 E	▲ Maximum R Gravity Loc R+ / R- M 1922 /- R 2143 /-	eactions (lbs) y N - / Rh / Rw /- /575	Von-Gravity / U / RL / /52 /187
Des Ld: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	EXP: E Mean I TCDL: BCDL: MWFR C&C D Loc. fr	3 Kzt: NA Height: 15.00 ft 5.0 psf S Parallel Dist: h Dist a: 3.00 ft om endwall: not in	to 2h n 9.00 ft	Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: No FT/RT:20(0)/10(0) Plate Turg(s):	HORZ(LL): -0.162 E HORZ(TL): 0.319 E Creep Factor: 2.0 Max TC CSI: 0.637 Max BC CSI: 0.430 Max Web CSI: 0.681	Wind reactions M Brg Wid = R Brg Wid = Bearings M & I Members not li Maximum Top Chords Tens.	based on MWFRS based on MWFRS 3.5 Min Req = 1 3.5 Min Req = 1 3.5 Min Req = 1 R are a rigid surface isted have forces le p Chord Forces Pe Comp. Chords	/- /- ; .6 (Truss) .8 (Truss) 3. ss than 375# r Ply (lbs) Tens. Comp.
	Wind D	Duration: 1.60		WAVE	VIEW Ver: 21.02.01.1214.12	Q-E 1150) - 3179 D - P	1248 - 2353
Lumber Top chord: 2x4 SP #2 Bot chord: 2x10 SP 22 Webs: 2x4 SP #3; W4 W8 2x6 SP 2400f-2.01	; T2 2x6 100f-2.0 I,W9 2x E;	6 SP 2400f-2.0E;)E; B3 2x4 SP #2; 44 SP M-31; W5,V	V6,	Additional Notes The overall height of this tru 11-2-5.	uss excluding overhang is	A-B 92 B-C 94 C-D 143 Maximum Bot	2 -986 E-F -989 F-G -601	52 - 2192 75 - 2245 r Ply (lbs)
Rt Slider: 2x6 SP 240	0f-2.0E;	; block length = 1	.500'			Chords Tens.	Comp. Chords	Tens. Comp.
Bracing (a) Continuous lateral member.	restrain	nt equally spaced	on			M - L 1307 L - K 1307 K - J 1307	0 J-1 7 0 I-G 7 0	1583 0 1618 0
Loading						Maximum We	b Forces Per Ply (lbs)
Attic room loading fror PSF. Dead Load: 10 F 10 PSF	n 6-1-8 PSF Cei	to 17-6-8: Live Lo iling: 10 PSF, Kne	oad: 40 eewalls:		111211 142442411 14274844	A - M 177 A - N 1297	Comp. Webs ' - 1005 O - P ' - 121 P - D	45 - 839 45 - 838
Purlins In lieu of structural par TC @ 24" oc.	nels use	e purlins to brace	all flat	OUG	AS FLEMIN	M - N 252 B - N 281 N - L 1599	2 - 1784 D - Q - 407 J - Q) 0 J - E	1776 - 39 721 0 86 - 495
Collar-tie braced with oc. or rigid ceiling.	continuo	ous lateral bracine	g at 24"	· · · · ·	A BEBAR	0-0 409	0	
Wind Wind loads based on member design.	MWFR	S with additional (C&C	*	* *			
Left end vertical not ex Wind loading based o	kposed n both g	to wind pressure. gable and hip roof	types.	8 8	TATE OF			
Blocking Blocking reinforcemer prevent buckling of me Bearing 2 located at	nt requir embers 23.4' (t	red to over the bearings blocking >= 5.50'	s: ' if used)	COA #0 278 Florida Certifu	CORID ENGINE	99		
	**\&/ * *			08/24/202				
IMPORTA Trusses require extrem Component Safety Info bracing per BCSI. Unle attached rigid ceiling. L as applicable. Apply p drawings 160A-Z for st	NT	FURNISH THIS C FURNISH THIS C in fabricating, har by TPI and SBC d otherwise, top (is shown for perm b each face of trus plate positions. R	AND FO DRAWING dling, shi CA) for sa chord sha lanent lat ss and po lefer to jo	5 TO ALL NOTES ON THIS DI 5 TO ALL CONTRACTORS INC ipping, installing and bracing. R fety practices prior to performing il have properly attached structu eral restraint of webs shall have sition as shown above and on th b's General Notes page for addit	LUDING THE INSTALLERS LUDING THE INSTALLERS tefer to and follow the latest edition these functions. Installers shall p pral sheathing and bottom chord sha bracing installed per BCSI sections e Joint Details, unless noted other tional information.	of BCSI (Buildin rovide temporan all have a proper s B3, B7, or B10, wise. Refer to	ig Ay	

155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

Filt Date List Cross Date List Cross <thdate cross<="" list="" th=""> <thdate cro<="" list="" th=""><th>SEQN: 442377</th><th>COMN</th><th>P</th><th>ly:</th><th>3</th><th>Job N</th><th>umber: 22-8147</th><th></th><th></th><th></th><th>Cust: R 2</th><th>15 JRef: 1</th><th>XIc215000</th><th>)5 T8</th></thdate></thdate>	SEQN: 442377	COMN	P	ly:	3	Job N	umber: 22-8147				Cust: R 2	15 JRef: 1	XIc215000)5 T8
1 Complete Trustee Required 1	FROM:		Q	ty:	1	Edenfi	eld Label: C15				DrwNo:	235.22.111 DF	3.16473	122
$ \frac{1}{1} \int_{\mathbb{R}^{2}} \int_{\mathbb{R}$				-<	3	Comple	te Trusses Required							
<image/>					•	Compic								
							10'3*13	23'8"						
$\frac{1}{10^{10} \text{ m}^{10} \text{ m}^{$							5'10'4	1343						
$ \frac{1}{\sqrt{1 + 1 + 1}} \int_{1}^{1} \frac{1}{\sqrt{1 + 1}} \int_$								=5X6 =2X4						
Image: stand st					Ţ		*3¥10	F 33X10	T	-				
						Ŧ	\$3X4	B3 3X4 B3 3X4						
							43X4 0 10	10 13X4 13X4 113X6						
$ \frac{1}{10000000000000000000000000000000000$					1'2"5			H H SX4	4					
					ĪĪ	94" -	6X10 /	58 TX10	‡ 					
$ \frac{1}{12} + \frac{1}{12}$					45"11 -		#3X4 C T1 B	-115'						
Image: the transmission of transmissin sach of transmission of transmission of						Ŧ			×™					
$\frac{1}{10000000000000000000000000000000000$					Ŧ	1 <u>1</u> "	S Q P O 3X16(E3) #2X4 #4X12 =SS0712	N M 14X12 112X4 1133	K16(E3)	-Ф _а				
$\frac{1}{12} + \frac{1}{12} $							=4x6(E3)	=4Xt	s(E3)					
Londing Chiefrain (and Wind Criteria (Dar III) and Dar Delated (Dar Delated (D							▲ + 27'5 - 275 +	18'5'6						
Landing Criteria (pp) Mind Criteria TGUL: 20.00 TGUL: 20.00 CGUL: 10.00 Relac Category: II EXP: B, Kr: NA Mass Height: 15.00 TGUL: 50.pd EXP: B, Kr: NA Mass Height: 15.00 Mass Height: 15.00 TGUL: 50.pd EXP: B, Kr: NA Mass Height: 15.00 TGUL: 50.pd EXP: B, Kr: NA Mass Height: 15.00 Mass Height: 15.00 TGUL: 50.pd EXP: B, Kr: NA Mass Height: 15.00 TGUL: 50.pd EXP: B, Kr: NA Mass Height: 15.00 Mass Height: 15.00 TGUL: 50.pd EXP: B, Kr: NA Mass Height: 15.00 Mass Height: 15.00 TGUL: 50.pd EXP: B, Kr: NA Mass Height: 15.00 Mass Height: 15.00 TGUL: 50.pd EXP: B, Kr: NA Mass Height: 15.00 TGUL: 50.pd EXP: A 245 P. 24007-20.E: TJ 24 SP Add: EXP: B, Kr: MA Mass C CSI: 0.43 Mass Height: 15.00 TGUL: 24.50 FGUL: 24.5							270	21011 230	 ≠ 1'6" ≠					
TCLL 20.00 Wind Std: ASCE 7-16 Pp (FA C: NA PD petiestion is to Ludel Lute Core NA VERTICL: 0.02 Pi (FA Pi (FA Pi (FA Pi (FA Pi (F	Loading Criteria (psf)	Wind	Crit	teri	a		Snow Criteria (Pg Pf in PSF)	Defl/CSI Criteria	▲ Max	imum F	eactions	s (lbs)		
TCDL: 10.00 EVEN PB Kethouse of the product of the	TCLL: 20.00	Wind	Std:	: /	ASCE 7-1	6	Pg: NA Ct: NA CAT: NA	PP Deflection in loc L/defl L/#		Gravit	y / Pł) /Pu	Non-Grav	vity / PI
BCDL: 10.00 EXP: B. KZ: MX: NA Mean Height 15:00 H CDL:: 50 pd EXP: B. KX: NA Mean Height 15:00 H CDL:: 50 pd EXP: SX: 2014 Mean Mean EXP: SX: 20	TCDL: 10.00 BCLL: 0.00	Enclos	i: 1 sure	130 e: C	mpn losed		Pf: NA Ce: NA Lu: NA Cs: NA	VERT(LL): 0.114 P 999 240 VERT(CL): 0.232 P 999 180	S 56	<u>46</u> /-	- /N	/191	0 /24	/757
Des Ld: 40.00 NORDELL: 0.00 Land Duration: 1.50 Sorie: 0.00 Land Duration: 1.60 WhYER 5 analel Dist to 10e Co. Form end wath: Any GCp: 0.15 op ef Loc. from end wath: Any Mark EG SE: 0.333 HEW Ver: 21.02.01.1214.12 Hew SES Heloking Apply additional nalling, op er the following bearings: apply blocking residure at 23.4' (blocking >= 3.50'' flued) Bearing 2 located at 23.4'' (blocking >= 3.	BCDL: 10.00	Risk C	ate B	gor Kzt	y: II ∵NA		Snow Duration: NA	HORZ(LL): -0.082 H	T 59	74 /-	, <i>'</i>	/217	1 /58	/-
Conternation Conte Content of the second	Des Ld: 40.00	Mean	Hei	ight	: 15.00 ft		Building Code:	HORZ(TL): 0.169 H	S Br	eaction: g Wid =	s based o 3.5 M	in Req = 1	; .6 (Trus:	s)
Land Duration: 1.25 Spacing: 7.9. * Low MPKRS Parallel Die to to hz CaC Die at: 3.00 ft Lac Die ta: 3.00 ft Die ta: 1.00 ft Die	Soffit: 0.00	BCDL	: 5.0 : 5.0) ps 0 ps	st sf		FBC 7th Ed. 2020 Res.	Max TC CSI: 0.444	T Br Bearin	g Wid =	:3.5 M Farearia	in Req = 1	.6 (Trus:	s)
Color Data as upper loss Los Trans and wait any Grip 0.13 Trans and wait any Grip 0.11	Load Duration: 1.25	MWFF	RS F	Para	allel Dist:	0 to h/2	TPI Std: 2014 Rep Fac: No	Max BC CSI: 0.330 Max Web CSI: 0.393	Membe	ers not	isted hav	e forces le	ss than (375#
GCp: 0.18 Pilet Type(s): WARE, 1858 VIEW Ver: 21.02.01.1214.12 C - D 2003 -257 B - C 139 -2023 Lumber Blocking Allowing bearings Write Ver: 21.02.01.1214.12 C - D 2003 -257 B - C 139 -2023 Top Jord: 25.8 P #20. Blocking Apply additional nating over the following bearings Apply additional nating, apply blocking reinforcement to practice and additional nating, apply blocking reinforcement to bearings: Bearing 1 located at 0.0 (blocking >= 3.50 if used) A.B 128 -288 -0.N 1338 0.0 -185 -185 -185 -185 -185 -185 -185 -185 -185 -28 -207 J - K 151 -285 -207 J - K 153 -185 -185 -28 -207 J - K 153 -58 -105 -26 -207 -26 -27 -26 -27 -27 -26 -27 -26 -27	opaoing. 75.0	Loc. fr	om	ene	dwall: Any	,	FT/RT:20(0)/10(0)		Chords	um To Tens	Chord	Forces Pe Chords	r Ply (lb Tens.	s) Comp.
Lumber Co-R 947 -1500 Top chord: 2x6 SP 2400F-2.0E; T1 2x4 SP M-31; Biocking Apply additional nalling over the following bearings H-1 1513 -3564 1-1 1513 -3654 1-1 1513 -3654 1-1 1513 -3654 1-1 1513 -3654 1-1 1513 -3654 1-1 1513 -3654 1-1 1513 -3654 1-1 1513 -3654 1-1 1513 -3654 1-1 1513 -3654 1-1 1513 -3654 1-1 1513 -3654 1-1 1513 -3654 1-1 1514 -2035 53 -58 0-N 1365 -0 N 1365 -20 1530 -58 0-N 13656 0 -0 1530 -365 0-N 13656 0 -0 1530 -365 0-N 1365 0 -1 1466 -9 2 -20 1530 -36 -N N 1466 -9 2 -7 1719 -N 1450 0 N 1496 -9 2		Wind I	G Dura	SCp atio	i: 0.18 n: 1.60		Plate Type(s):	VIEW Ver: 21.02.01.1214.12	C - D	2903	3 - 257	B - C	139	- 2023
Top chord: 26 SP 2400F2.0E; 11 24 SP M-31; To 24 SP M2; Bot chord: 2x10 SP 2400F2.0E; B3 24 SP H2; Webs: 2x4 SP P2; Webs: 2x4 SP P2; W3/WS 24 SP H2; Li Sider: 2x6 SP 2400F2.0E; B3 24 SP H2; Webs: 2x4 SP P2; W3/WS 24 SP H2; Li Sider: 2x6 SP 2400F2.0E; block length = 1.500' Nailoote	Lumber						Blocking		⊐G-H H-I	382 ⁻ 1513	- 310 3 - 3654	G - R I - J	947 138	- 1850 - 1989
Bid chord: 2x10 SP 24004:2.0E: B3 2x4 SP #2: Webs: 2x4 SP 24004:2.0E: block length = 1.500' Rilder: 2x6 SP 24004:2.0E: block length = 1.500' Rider: 2x6 SP 24004:2.0E: b	Top chord: 2x6 SP 24	100f-2.0	Е; Т	[1 2	x4 SP M-	31;	Apply additional nailing ove with fasteners at 4" oc both	er the following bearings	A - B	126	6 - 2071	J-K	154	- 2035
Li Silder: 2x6 SP 24001:2:0E; block length = 1.500 Rt Silder: 2x6 SP 24001:2:0E; block length = 1.500 Rt Silder: 2x6 SP 24001:2:0E; block length = 1.500 Rainote Nainote Nainote Nainote Nainote Nainote Nainote Shedule: 0.131'x3', min. nalis Top Chord: 1 Row @ 12:00' o.c. Webs : 1:Row @ 12:00	Bot chord: 2x10 SP 2 Webs: 2x4 SP #3: W	400f-2.0)E; I	B3	2x4 SP #2	2;	parallel to grain. In lieu of a	dditional nailing,	Maxim	um Bo	t Chord I	Forces Pe	r Ply (lb	s)
Namince A-Q 1530 -58 0-N 1336 0 Nainote Bearing 2 located at 23.4 (blocking >= 3.50° if used) A-Q 1530 -58 0-N 1336 0 Nainote Additional Notes Additional Notes P-O 1336 0 N-K 1499 -12 Nainote Additional Notes The overall height of this truss excluding overhang is P-O 1336 0 N-K 1499 -12 Nainote The overall height of this truss excluding overhang is 11-2-5. Maximum Web Forces Per Ply (bs) Vetos Tens. Comp. Vind Dading from 6-1-8 to 17-6-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewals: 11-2-5. STATE OF OF 1486 N-H 1450 0 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewals: To accord additional C&C STATE OF No. 866448 N-H 1450 0 P 1686 N-H 1495 0 N-H 1495 0 P 16864 N-H 1495 0 N-H 1495 0 D Co 13 N-H 1495 0 D D<	Lt Slider: 2x6 SP 240 Rt Slider: 2x6 SP 240	0f-2.0E;	blo	ock	length = '	1.500'	prevent buckling of membe	rs over the bearings: (blocking >= 3.50" if used)	Chords	Tens	.Comp.	Chords	Tens.	Comp.
Additional Notes Tail Schedule:0.131'x3', min. nails Top Chord: 1 Row @ 3.25' o.c. Bot Chord: 1 Row @ 4 o.c. Repeat nailing as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting. Loading Atticroom backing from 6-1-8 to 17-6-8: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF Purins In leu of structural panels use purlins to brace TC @ 24' oc. Coller-tie braced with continuous lateral bracing at 24' oc. Wind loading based on both gable and hip roof types. Wind loading based on both gable positions. Wind loading based on both gable and hip roof types gable type for the dealer of the set of the gabl	Nailnoto	01 2.02	, 010	001	iongui –	1.000	Bearing 2 located at 23.4	$(blocking \ge 3.50" \text{ if used})$	A - Q Q - P	1530 1536) -58 3 -53	O - N N - M	1336 1496	0 -9
Top Chord: 1 Row @ 3.25° o.c. Bot Chord: 1 Row @ 4.20° o.c. Webs : 1 Row @ 4 o.c. Repeat naling as each layer is applied. Use equal spacing between rows and stagger nails in each row to avoid splitting. Leading Attic room bading from 6-1-8 to 17-6-8: Live Load: 40 PSF. Dead Load: 10 PSF Celling: 10 PSF, Kneewalls: 10 PSF Purins In leu of structural panels use purins to brace TC @ 24° oc. Collar-tie braced with continuous lateral bracing at 24° oc. Wind Mind loading based on both gable and hip roof types. Wind loading based on both gable based on the type of the dotterwise. Refer to and follow the latest difticulture the type of the latest difticulture the latest difticul the	Nail Schedule:0.131"	x3", min	. na	ails			Additional Notes		P - 0	1336	5 0	M - K	1489	- 12
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oc. Wind Wind loads based on MWFRS with additional C&C STATE OF wind loading based on both gable and hip roof types. COA #0.278 **WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING! Florida Certificate of Product Approval #FL1999 **WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING! Florida Certificate of Product Approval #FL1999 **WARNING** READ AND FOLLOW ALL CONTRACTORS INCLUDING THE INSTALLERS Florida Certificate of Product Approval #FL1999 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building bracing per BCSI) Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid Ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installing and bottom chord shall have a properly attached rigid ceiling. Locations Shown for permanent lateral restraint of webs shall have bracing installing and bottom chord shall have a properly attached rigid ceiling. Locations Shown for peremenent and position as shown above and on	24" oc. Collar-tie braced with	continu	ous	lat	eral bracii	no:at:24"		T.						
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Wind loading based on both gable and hip roof types. COA #0.278 Florida Certificate of Product Approval #FL1999 08/24/2022 **WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI: Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information. Alpine, a division of ITW Building Components Group Inc. shall not be responsibile for any deviation from this drawing, any failure to build the fursts in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page for awings for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. Every met information are under brow the brief of the Building Designer per ANSI/TPI 1 Sec.2.	Wind loads based on member design.	MWFR	Sw	/ith	additional	C&C	The second second	Source SNG1						
Florida Certificate of Product Approval #FL1999	Wind loading based of	on both g	gabl	le a	ind hip roo	of types.	COA #0 278	UNAL EL TRADUTA						
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	drawing for any struct	ure is the	acce e re	epta	ance of pro	otessiona If the Bui	al engineering responsibility solely Iding Designer per ANSI/TPI 1 Sec	tor the design shown. The suitabil	ity and u	se of thi	S	North B	uilding, 4t	h Floor 25

listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 442360 FROM:	COMN Ply: 3 Qty: 1	Job Number: 22-8147 Edenfield			Cust: R 215 JRef: 1XIc2150005 DrwNo: 235.22.1113.19153
		Truss Label: C19			SSB / DF 08/23/2022
	<u> </u>	Complete Trusses Required			
		10/3*13 10/3*13	+ 238* 13'4"3	-1	
		5'10'4			
		511*12	5X6		
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				-4	
		▲ <u>27'5</u>	18'5'6 J. 27'5	۔	
		27*5	210'11 23'8'		
Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	▲ Maximum R	eactions (lbs)
TCLL: 20.00	Wind Std: ASCE 7-16	Pg: NA Ct: NA CAT: NA	A PP Deflection in loc L/defl L/#	Gravity	/ Non-Gravity /Rh /Rw /U /F
ICDL: 10.00 BCLL: 0.00	Enclosure: Closed	PT: NA Ce: NA	VERT(LL): 0.114 P 999 240		/ /1010 /24 /7
BCDL: 10.00	Risk Category: II	Snow Duration: NA	HORZ(LL): -0.082 H	T 5974 /-	/- /1910 /24 //:
Des Ld: 40.00	EXP: B Kzt: NA		HORZ(TL): 0.169 H	Wind reactions	based on MWFRS
NCBCLL: 0.00	TCDL: 5.0 psf	Building Code:	Creep Factor: 2.0	S Brg Wid =	3.5 Min Req = 1.6 (Truss)
Soffit: 0.00	BCDL: 5.0 psf	FBC 7th Ed. 2020 Res.	Max TC CSI: 0.444	Bearings S & T	are a rigid surface.
Load Duration: 1.25 Spacing: 79.0 "	MWFRS Parallel Dist: 0	Rep Fac: Yes	Max Web CSI: 0.393	Members not li	sted have forces less than 375#
opaoing. 75.0	Loc. from endwall: Any	FT/RT:20(0)/10(0)		Maximum Top	Comp Chords Tens Cor
	GCpi: 0.18	Plate Type(s):			
	Wind Duration: 1.60	WAVE, 18SS	VIEW Ver: 21.02.01.1214.12	G-H 3821	-257 B-C 139 -2 -310 G-R 947 -1
Lumber	004 0 0E. T4 0.4 CD M 0	Blocking		H - I 1513	-3654 I-J 138 -1
T6 2x4 SP #2;	001-2.0E; 11 2x4 5P M-3	with fasteners at 4" oc b	oth perpendicular and	A - B 126	-2071 J-K 154-2
Bot chord: 2x10 SP 24 Webs: 2x4 SP #3: W2	400f-2.0E; B3 2x4 SP #2;	; parallel to grain. In lieu o	of additional nailing,	Maximum Bot	Chord Forces Per Plv (lbs)
Lt Slider: 2x6 SP 240	0f-2.0E; block length = 1.	.500' prevent buckling of men	bers over the bearings:	Chords Tens.	Comp. Chords Tens. Cor
Rt Slider: 2x6 SP 240	0f-2.0E; block length = 1	.500' Bearing 1 located at 0 Bearing 2 located at 2).0' (blocking >= 3.50" if used) 3.4' (blocking >= 3.50" if used)	A - Q 1530	-58 O-N 1336
Nailnote				Q-P 1536	-53 N-M 1496
Nail Schedule:0.131">	k3", min. nails	Additional Notes		P-O 1336	0 M - K 1489
Top Chord: 1 Row @ Bot Chord: 1 Row @	2 3.25" o.c. 12.00" o.c.	The overall height of this 11-2-5.	s truss excluding overhang is	Maximum Wel	b Forces Per Ply (lbs)
Webs :1 Row @	4" o.c.			Webs Tens.	Comp. Webs Tens. Cor
spacing between rows	s and stagger nails in eac	juai ch row	- and third have -	Q-C 13	-718 R-G 287-1
to avoid splitting.				D-P 1468	0 N-H 1450
Loading		and the second se	GLAD FLEM	E-K 20/	-1/19 1-1/1 24 -
Attic room loading fro	m 6-1-8 to 17-6-8: Live Lo	oad: 40	ICENS:		
PSF. Dead Load: 10 I 10 PSF	PSF Ceiling: 10 PSF, Kne	eewalls:	VIO CENTON		
Purlins			No. 06648		
TC/BC @24" oc.	neis use punins to brace				
Collar-tie braced with	continuous lateral bracing	g at 24"	CTATE OF		
OC.			STATE OF 10-1		
Wind		181	FLOOD A HUN		
Wind loads based on	MWFRS with additional C	C&C 🔨	- ORIE CIT		
member design.	n hath apple and his reaf	the second se	JONAL ENGINE		
wind loading based d	on both gable and hip roof	Florida Cox	tificate of Product Approval #EL 1	000	
		Fionda Cer 08/24/2	2022	777	
	WARNING READ	AND FOLLOW ALL NOTES ON THIS	DRAWING!		
	ANT** FURNISH THIS D	DRAWING TO ALL CONTRACTORS I	NCLUDING THE INSTALLERS	of BCSI (Buildin	a
Component Safety Info	prmation, by TPI and SBC	CA) for safety practices prior to perform	ing these functions. Installers shall p	rovide temporary	
attached rigid ceiling. I	Locations shown for perm	nanent lateral restraint of webs shall ha	ve bracing installed per BCSI sections	s B3, B7, or B10,	"
drawings 160A-Z for s	tandard plate positions. R	Refer to job's General Notes page for a	dditional information.		
Alpine, a division of IT	W Building Components (with ANSI/TPL 1 or for ha	Group Inc. shall not be responsible for and ling, shipping, installation and bra	any deviation from this drawing, any f cing of trusses. A seal on this drawir	ailure to build the	ANITW

155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

SEQN: 101427	COMN	Ply:	1	Job Nu	mber: 22-8147			Cust: R 21	5 JRef:1X	lc215000)5 T26
FROM:		Qty:	1	Edentiel Truss L	ld .abel: C20			KD / I	235.22.111: DF	3.21843 08/23/20)22
			I					I			
				┣	10'3"13 10'3"13	238" 13'4"3	- -+				
					5'10"4 (TYP)						
				⊢		14X6 12X4					
			T		4 <u>4</u> ×6	4x6	Ţ				
			T		#3X4	R 43X4					
					#3X4 #3X4	B3 6 •3X4					
					12 #3X4 #3X4	12,5x6 3X4					
			17.2		10	H H H	11'5'14				
				# 3	6X10	w5 /					
			4.21		B W3	-115	*3X4				
			T I	, A			K ™				
			1 1 1	S T				–⊕ _a			
				■4X6(E	3) #2249 #4472 = 330712	1194A12 112A4	■4X6(E3)				
				k	21715	- 23'8"					
				 -	27'5	21'0"11 +	238"				
							₩ 16" *				
Loading Criteria (psf)	Wind	Criteria			Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria		n Reactions	(lbs)	lon-Grav	vitv
TCDL: 20.00	Speed	l: 130	mph		Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA	VERT(LL): 0.104 P 999 240	Loc R+	/R- /Rh	/ Rw	/U	/RL
BCLL: 0.00	Enclos Bick C	sure: Cl	losed		Lu: NA Cs: NA	VERT(CL): 0.211 P 999 180	S 1715	/- /-	/580	/7	/230
BCDL: 10.00	EXP: I	B Kzt	u: NA		Snow Duration: NA	HORZ(LL): -0.075 H	T 1815 Wind reacti	/- /- ons based o	/659 n MWFRS	/18	/-
NCBCLL: 10.00	Mean	Height:	: 15.00 ft		Building Code:	Creep Factor: 2.0	S Brg Wi	d = 3.5 Mi	n Req = 1.	5 (Truss	s)
Soffit: 0.00	BCDL	: 5.0 ps : 5.0 ps	sf		FBC 7th Ed. 2020 Res.	Max TC CSI: 0.395	T Brg Wi Bearings S	d = 3.5 Mi & Tare a rio	n Req = 1. id surface.	5 (Truss	5)
Load Duration: 1.25 Spacing: 24.0 "	MWFF	RS Para	allel Dist: 0 to	o h/2	Rep Fac: Yes	Max Web CSI: 0.300	Members n	ot listed have	e forces les	s than 3	375#
opaoing. 24.0	Loc. fr	om end	dwall: not in 4	4.50 ft	FT/RT:20(0)/10(0)		Chords Te	Fop Chord F ens.Comp.	Chords	Tens.	s) Comp.
	Wind	GCpi Duratio	i: 0.18 m: 1.60		Plate Type(s):	VIEW Ver: 21.02.00.1005.17	C-D 2	646 - 131	B-C	45	- 1844
Lumber		Bulato			WAVE, 1855		- G-H 3	482 - 150	G-R	764	- 1686
Top chord: 2x6 SP 24	100f-2.0	E; T1 2	x4 SP M-31;	;			A-B	32 - 1888	J-K	69	- 1854
Bot chord: 2x10 SP 2	400f-2.0)E; B3 2	2x4 SP #2;				Movimum	Pot Chard E	oroop Dor		~)
Lt Slider: 2x4 SP #3; W3	3,W52x 0f-2.0E;	4 SP # block l	⊭2; length = 1.50	00'			Chords Te	ins.Comp.	Chords	Tens.	Somp.
Rt Slider: 2x6 SP 240	0f-2.0E	; block	length = 1.5	600'			A - Q 1	395 - 53	0 - N	1218	0
Loading							Q-P 1 P-O 1	400 - 48 218 0	N - M M - K	1363 1357	0
Attic room loading fro PSF. Dead Load: 10	m 6-1-8 PSF Ce	to 17-6 ilina: 10	6-8: Live Loa 0 PSF. Knee	d: 40 walls:			101	210 0		1007	Ŭ
10 PSF		5	,				Maximum V	Web Forces	Per Ply (II	bs) Tens	Comp
Purlins							0-0	0 - 655	R-G	167	- 1566
Collar-tie braced with oc. or rigid ceiling.	continu	ous late	eral bracing a	at 24"	WITE		D-P 1	338 0	N - H	1321	0
Wind					16	LAD FLEM	E - R	167 - 1566	I - M	22	- 675
Wind loads based on	MWFR	S with a	additional C8	3C		CENS					
member design.											
Wind loading based o	on both g	gable a	ind hip roof ty	ypes.		No. 66648					
Blocking Blocking spinforcome											
prevent buckling of m	embers	over th	ne bearings:			I i					
Bearing 1 located at Bearing 2 located at	0.0' (b 23.4' (l	blocking	g >= 3.50" if q >= 3.50" if	used) f used)	21	STATE OF					
Ū			•		201	ALOPIOP STA					
					1. C.C.	GIL					
					COA #0.278	ONAL EN MAL					
					Florida Certi	ficate of Product Approval #FL	1999				
					08/24/20						
IMPORT	**WA ANT	rning Furni	** READ A	ND FC	OLLOW ALL NOTES ON THIS D G TO ALL CONTRACTORS INC	RAWING! CLUDING THE INSTALLERS					
Trusses require extrer Component Safety Inf	ne care ormation	in fabri n, by Tl	icating, hand PI and SBCA	lling, sh \) for sa	hipping, installing and bracing. F	Refer to and follow the latest edition these functions. Installers shall	n of BCSI (Bui provide tempo	lding rary			
attached rigid ceiling.	ess note	to othe	vn for permai	nent lat	an nave properly attached structu teral restraint of webs shall have sition as shown above and on the	bracing installed per BCSI section	nall nave a pro	peny 510, to			
drawings 160A-Z for s	tandard	plate p	ositions. Re	fer to jo	bb's General Notes page for add	itional information.	failur- 1- 1		AL	_PI	NE
Appine, a division of IT	vv Build	ing Co	mponents G	roup In	c. șnaii not pe responsible for an	ly deviation from this drawing, any	railure to build	ıtne		-	AN ITW COMPANY

Truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing are page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 101429	COMN	Ply:	1	Job Nu	mber: 22-8147				Cus	st: R 215	JRef: 1XIc2	2150005	5 T19
		QUY.	1	Truss L	abel: C21				KD	/ DF	08	/23/202	22
			+		103'13 103'13 510'4 (1YP) 511'12 511'12 10 511'12 10 511'12 10 511'12 10 511'12 10 510'4 (1YP) 10 510'4 (1YP) 10 510'4 (1YP) 10 510'4 (1YP) 10 510'4 (1YP) 10 510'4 10 10 10 10 10 10 10 10 10 10	238' 134'3	5						
				+ 1'6" - +	2775 2775	18'5"6 21'0"11		2775 2378" - - 116" -					
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind G Wind S Speed Enclos Risk C EXP: E Mean I TCDL: BCDL: MWFR C&C E	Criteria Std: A : 130 i ure: Cl ategory 3 Kzt: Height: 5.0 psi 5.0 psi 2.5 Para 0ist a: 3	RSCE 7-16 mph osed /: II : NA 15.00 ft f f f llel Dist: 0 .00 ft	to h/2	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes	Defl/CSI Criteria PP Deflection in loc VERT(LL): 0.103 Q VERT(CL): 0.209 Q HORZ(LL): -0.076 I HORZ(TL): 0.155 I Creep Factor: 2.0 Max TC CSI: 0.393 Max BC CSI: 0.306 Max Web CSI: 0.356	L/defl L/# 999 240 999 180 	▲ Maxim Loc R+ T 1818 U 1812 Wind rea T Brg U Brg Bearings Members Maximu	A Constant Strain Strai	tions (lb / Rh /- /- sed on M Min R Min R e a rigid s I have fo ord Fore	Non / Rw / /659 / /659 / IWFRS eq = 1.5 (eq = 1.5 (surface. orces less ces Per P		ity /RL /246 /-)) 75#
	Loc. fro Wind E	om end GCpi Duratior	lwall: Any : 0.18 n: 1.60		FT/RT:20(0)/10(0) Plate Type(s): WAVE_18SS	VIEW Ver: 21.02.00.1	005.17	Chords D - E	Tens.Con 2634 - 2	np. C 231 C	hords T	123	Comp. - 1830
Lumber Top chord: 2x6 SP 24 Bot chord: 2x10 SP 24 Webs: 2x4 SP #3; W3 Lt Slider: 2x6 SP 2400 Rt Slider: 2x6 SP 2400 Loading Attic room loading fror PSF. Dead Load: 10 F	00f-2.0E 400f-2.0 3,W5 2x 0f-2.0E; 0f-2.0E; n 6-1-8 PSF Cei	E; T1,T E; B3 2 4 SP # block l block l to 17-6 ling: 10	6 2x4 SP M 2x4 SP #2; ength = 1. ength = 1. 6-8: Live Lc PSF. Kne	/I-31; 500' 500' ad: 40 ewalls:				Maximur B - C Maximur Chords B - R R - Q Q - P	3481 - 2 1377 - 3 138 - 18 m Bot Ch Tens.Con 1369 - 1375 - 1213	281 F 323 J 372 K ord Ford np. C 53 F 649 C 0 N	- S - K (- L :es Per P (- O) - O) - N N - L	866 126 137 Iy (Ibs) Γens. (1213 1370 1364	- 1683 - 1824 - 1865) <u>Comp.</u> 0 - 7 - 10
10 PSF Purlins		0						Maximu Webs	m Web Fo Tens.Con	orces Pe	⊧r Ply (lbs Vebs 7) Fens. (Comp.
Collar-tie braced with oc. or rigid ceiling. Wind Wind loads based on member design. Wind loading based o	continue MWFRS	ous late S with a able a	eral bracing additional C nd hip roof	at 24" C&C types.	5946	AS FLEMIN		R - D E - Q F - S	3 - 6 1319 259 - 15	351 S 0 C 560 J	; - H) - I - N	259 1318 24	- 1560 0 - 652
Blocking Blocking reinforcemer prevent buckling of me Bearing 1 located at Bearing 2 located at	nt requir embers 0.0' (b 23.4' (b	ed to over th locking locking	e bearings >= 3.50" y >= 3.50"	: if used) if used)	COA #0 278 Florida Certifi 08/24/202	tATE OF CORIDA CORIDA CORIDA CALENGING CALENGIN CALENGING CALENGIN CALENGIN CALENGIN CALENGIN CALENGIN CAL	oval #FL19	99					
**IMPORTA Trusses require extrem Component Satety Info Pracing per BCS1. Unle attached rigid ceiling. I as applicable. Apply drawings 160A-Z for st Alpine, a division of ITT truss in conformance w	**WAI	CNING FURNIS In fabrid by TF d other s show each f plate p ing Cor SI/TPI	** READ SH THIS D cating, han PI and SBC mise, top c in for perm ace of trus ositions. R mponents (1. or for ha	AND FO RAWING dling, sh A) for sa chord sha anent lat s and po efer to jo Group Ing andling.	NLOW ALL NOTES ON THIS D G TO ALL CONTRACTORS INC ipping, installing and bracing. If fety practices prior to performing all have properly attached structi teral restraint of webs shall have sition as shown above and on th b's General Notes page for addi c. shall not be responsible for an shipping, installation and bracir	KAWING! CLUDING THE INSTAL Refer to and follow the I g these functions. Inst Iral sheathing and botto bracing installed per B e Joint Details, unless tional information. y deviation from this dr g of trusses. A seal of	LERS atest edition allers shall p om chord sha CSI sections noted other awing, any fi n this drawin	of BCSI (I rovide tem all have a B3, B7, o wise. Ref ailure to bu g or cover	Building porary properly ir B10, fer to uild the page	-			

listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 99628 FROM:	GABL	Ply: 1 Qty: 1	Job Number: 22-8147 Edenfield Truss Label: PB01	Cust: R 215 JRef:1XIc2150005 T36 DrwNo: 235.22.1113.49600 KD / DF 08/23/2022
		<u>1</u> 23°1 −−−−	$\begin{array}{c} 8^{*15} \\ 8^{*15} \\ 8^{*15} \\ \hline 29^{*14} \\ \hline 8^{*15} \\ \hline \\ 14^{*3} \\ \hline \\ 8 \\ \hline \\ 12 \\ \hline \\ (TYP) \\ \hline \\ 14^{*3} \\ \hline \\ 18^{*15} \\ \hline 18^{*15} \\ \hline \\ 18^{*15} \\ \hline 18^{*$	1*12
			$\begin{vmatrix} 8^{*}15_{1} & 5'7^{*}12 \\ & 8^{*}15^{1} & 6'4^{*}11 \end{vmatrix} = \begin{vmatrix} 8^{*}15_{1} \\ 7'1^{*}10 \end{vmatrix}$	
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Dos Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 " Lumber Top chord: 2x4 SP #2 Bot chord: 2x4 SP #3; Plating Notes All plates are 2X4 exc	Wind C Wind S Speed Enclos Risk C EXP: E Mean TCDL: BCDL: BCDL: MWFF C&C E Loc. fn Wind I	Criteria Std: ASCE 7-16 I: 130 mph sure: Closed ategory: II 3 Kzt: NA Height: 20.21 ft 5.0 psf 2.0 psf 3S Parallel Dist: 0 Dist a: 3.00 ft om endwall: Any GCpi: 0.18 Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Def//CSI Criteria A Max Pg: NA Ct: NA CAT: NA PD Deflection in loc L/defl L/# VERT(LL): 0.000 D 999 240 Loc F Lu: NA Cs: NA VERT(LL): 0.000 D 999 180 A 18 Snow Duration: NA HORZ(LL): 0.000 F HORZ(TL): 0.000 F G 18 Building Code: Creep Factor: 2.0 Max TC CSI: 0.051 B B FBC 7th Ed. 2020 Res. Max BC CSI: 0.016 B B B TPI Std: 2014 Max Web CSI: 0.021 Bearin Bearin FT/RT:20(0)/10(0) Plate Type(s): VIEW Ver: 21.02.00.1005.17 VIEW Ver: 21.02.00.1005.17	kimum Reactions (lbs), or *=PLF Gravity Non-Gravity R+ /R- /Rh /Rw /U / RL 8 /- /- /35 /20 /43 4 /- /- /54 /9 /- 8 /- /- /15 /3 /- reactions based on MWFRS irg Wid = 5.9 Min Req = 1.5 (Truss) irg Wid = 67.8 Min Req = - irg Wid = 5.9 Min Req = 1.5 (Truss) ings A, B, & G are a rigid surface. ings A, B, & G are a rigid surface. bers not listed have forces less than 375# ings A, B, A G are a rigid surface. ings A, B, A G are a rigid surface.
(**) 2 plate(s) require scaled plate plot detai requirements.	special ils for sp	positioning. Refer pecial positioning	to	
Gable end supports 8 chord must not be cut	" max ra	ake overhang. Toj hed.		
Purlins In lieu of rigid ceiling u oc.	use purli	ins to brace BC @	24" OUGLAS FLEMING	
Wind Wind loads based on member design. Wind loading based of Additional Notes See DWGS A14030E gable wind bracing an Refer to DWG PB160	MWFR: n both <u>c</u> NC160 ² nd other 160118	S with additional (gable and hip roof 118 & GBLLETING r requirements. for piggyback de	CaC types. D118 for tails. COA #0 278 Florida Certificate of Product Approval #FL1999 08/24/2022	

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building component Satety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of rigid ceiling use purlins to brace BC @ 24" 00

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements. Refer to DWG PB160160118 for piggyback details.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Purlins

In lieu of rigid ceiling use purlins to brace BC @ 24" OC.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements. Refer to DWG PB160160118 for piggyback details.

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Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements. Refer to DWG PB160160118 for piggyback details.

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

North Building, 4th Floor Glenview, IL 60025

SEQN: 99508 FROM:	GABL	Ply: 1 Qty: 1	Job Number: 22-8147 Edenfield Truss Label: PB07		Cust: R 215 JRef:1XIc2150005 T61 DrwNo: 235.22.1114.00550 KD / DF 08/23/2022
		[]+	$\begin{array}{c} 12 \\ 10 \\ 12 \\ 10 \\ 12 \\ 10 \\ 12 \\ 10 \\ 12 \\ 10 \\ 10$	62*15 29*11 	28/1*12
			57"6 -7"9 -57"6 -57"6 -62"15		
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00	Wind Speed Speed Enclos Risk C EXP: E Mean TCDL: BCDL:	Criteria Std: ASCE 7-16 : 130 mph sure: Closed :ategory: II 3 Kzt: Height: 17.31 ft 5.0 psf : :5.0 psf	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res.	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.000 D 999 240 VERT(CL): 0.000 D 999 180 HORZ(LL): 0.000 F - - HORZ(TL): 0.000 E - - Creep Factor: 2.0 Max TC CSI: 0.054	▲ Maximum Reactions (Ibs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 16 /- /- /48 /34 /56 B* 75 /- /- /58 /10 /- G 16 /- /- /13 /2 /- Wind reactions based on MWFRS A Brg Wid = 5.2 Min Req = 1.5 (Truss) B Brg Wid = 67.4 Min Req = -
Load Duration: 1.25 Spacing: 24.0 "	MWFF C&C E Loc. fr	RS Parallel Dist: 0 to Dist a: 3.00 ft om endwall: Any GCpi: 0.18 Duration: 1.60	o h/2 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Max BC CSI: 0.017 Max Web CSI: 0.023 VIEW Ver: 21.02.00.1005.17	G Brg Wid = 5.2 Min Req = 1.5 (Truss) Bearings A, B, & G are a rigid surface. Members not listed have forces less than 375#
Lumber Top chord: 2x4 SP #2	2;				

Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements. Refer to DWG PB160160118 for piggyback details.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

SEQN: 99512 FROM:	SPEC	Ply: 1 Qty: 3	Job Nur Edenfiel	nber: 22-8147 d						Cust: R 215 DrwNo: 2	JRef:1XIc21500 35.22.1114.01863	05 T60 [°] }
			Truss L	abel: PB08						<u>KD / D</u>	F 08/23/2	022
			- 7"9 - 7"9 -	3'10"12 3'3"2		7'1"14 3'3"2		7'9"7 7"9				
					≡4X4 C			H M	£.22 28'1"12			
			↓ ^{7"9} ↓ • 7"9 • <mark>7"9</mark>	3'3"2 3'10"12	- 6'6"4	<u>3'3"2</u> 7'1"14		7"9 7"9 7'9"7				
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind S Speed Enclos Risk C EXP: E Mean TCDL: BCDL: BCDL: MWFF C&C E Loc. fr	Criteria Std: ASCE 7-16 : 130 mph sure: Closed ategory: II 3 Kzt: NA Height: 18.32 ft 5.0 psf 5.0 psf S Parallel Dist: 0 Dist a: 3.00 ft om endwall: Any GCDi: 0.18	to h/2	Snow Criteria (Pg,Pf Pg: NA Ct: NA C Pf: NA C: NA C Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Ret TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s):	in PSF) AT: NA e: NA 3.	Defl/CSI Crit PP Deflection VERT(LL): VERT(CL): HORZ(LL): HORZ(TL): Creep Facton Max TC CSI: Max BC CSI: Max Web CS	eria 0.000 B 0.001 D 0.001 D 0.001 D 	/defi L/# 999 24 999 18 -	▲ Maximum Re Gravity 0 Loc R+ / R- 0 A - /-57 - E - /-57 Wind reactions A Brg Wid = B Brg Wid = E Brg Wid = Bearings A, B, Members not list	Actions (/ Rh /- /- based on 5.2 Min 5.2 Min 5.2 Min & E are a sted have	Ibs), or *=PLF Non-Gra / Rw / U /67 /91 /69 /23 /35 /48 MWFRS Req = 1.5 (Trus Req = - Req = 1.5 (Trus rigid surface. forces less than	vvity / RL /65 /- /- ss) ss) 375#
Lumber Top chord: 2x4 SP #2 Bot chord: 2x4 SP #2;	Wind [Duration: 1.60		WAVE		VIEW Ver: 2	1.02.00.10	005.17				
VVebS: 2X4 SP #3; Plating Notes All plates are 2X4(A1) Loading Gable end supports 8' chord must not be cut	except ' max ra or notc	as noted. ake overhang. Top hed.	D									
Wind Wind loads based on member design.	MWFR	S with additional C	C&C		ي. الأنام	101214 (SPACESSES 152)	Mar.					

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A14030ENC160118 & GBLLETIN0118 for gable wind bracing and other requirements. Refer to DWG PB160160118 for piggyback details.

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

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SEQN: 99514	SPEC	Ply: 1	Job Nur	nber: 22-8147				Cust: R 215 JRef	:1XIc2150005	T62 .
FROM:		Qty: 1	Edenfiel	t				DrwNo: 235.22.1	114.03333	
			Truss L	abel: PB09				KD / DF	08/23/2022	2
			7"9	3'10"12	7'1"14 7'	9"7				
			' 7"9'	3'3"2	3'3"2 7	"9 '				
				≡4X4						
		T		C ∕∽		-	Г			
					<					
				12	$\langle \rangle$					
		- 7	10			c.	 ?			
		- 31					ი ი			
				в						
						F				
		1*12 -0*4	Ž			, ,	28,1*12			
			×			X	Ψ			
				 2X4						
			7"0			0				
			₽ , a₽		<u> </u>					
			7"0	0,0,0	01010 7	"0				
			+ 7 9 7 9	33"2	7'1"14	9"7				
							A Maximum D	antione (lbs) a	- * DI C	
TCLL: 20.00	Wind S	Std: ASCE 7-16		Snow Criteria (Pg,Pf in PSF)	PP Deflection in loc I /d	lefi i /#	Gravity	eactions (ibs), d	Non-Gravit	y
TCDL: 10.00	Speed	: 130 mph		Pf: NA Ce: NA	VERT(LL): 0.000 B 9	999 240	Loc R+ /R-	/ Rh / F	₩ /U /	/ RL
BCLL: 0.00	Enclos	ure: Closed		Lu: NA Cs: NA	VERT(CL): 0.001 D 9	999 180	A - /-57	/- /6	3 /91 /	/66
BCDL: 10.00	EXP: E	3 Kzt: NA		Snow Duration: NA	HORZ(LL): 0.001 D		B*97 /-	/- /6) /25 / 7 /48	/- /_
Des La: 40.00	Mean	Height: 19.47 ft		Building Code:	Creep Factor: 2.0		Wind reactions	based on MWF	, s	
Soffit: 0.00	BCDL:	5.0 pst 5.0 psf		FBC 7th Ed. 2020 Res.	Max TC CSI: 0.117		A Brg Wid =	5.2 Min Req =	1.5 (Truss)	
Load Duration: 1.25	MWFF	S Parallel Dist: 0	to h/2	TPI Std: 2014	Max BC CSI: 0.046		E Brg Wid =	5.2 Min Req =	- 1.5 (Truss)	
Spacing: 24.0 "	C&C E	/ist a: 3.00 ft	o 4 50 ft	Rep Fac: Yes	Max Web CSI: 0.018		Bearings A, B,	& E are a rigid su	urface.	
	L00. II	GCpi: 0.18	14.50 ft	Plate Type(s):			Members not li	sted have forces	less than 37	5#
	Wind [Juration: 1.60		WAVE	VIEW Ver: 21.02.00.100	5.17				
Lumber										
Top chord: 2x4 SP #2 Bot chord: 2x4 SP #2 Webs: 2x4 SP #3;	·,									
Plating Notes										
All plates are 2X4(A1)	except	as noted.								
Loading	•									
Gable end supports 8 chord must not be cut	" max ra	ike overhang. Top hed.	р							
VA/: m al										
Wind loads based on		e								
Wind loading based of	n both c	able and hip roof	types.	attl	BALLAN PROPERTY AND					
		,	77 30.	diffinite C	AS FLFA					
Additional Notes	NC160		0119 for	allo.	MIAM	2				
gable wind bracing a	nd other	requirements.			UENSE	illen a				
Refer to DWG PB160	160118	for piggyback det	tails.			No.				

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING! **IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.

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SEQN: 99428 FROM:	VAL	Ply: 1 Qty: 1	Job Nur Edenfiel Truss L	m ber: 22-8147 d abel: V01			Cust: R 215 JRef:1XIc2150005 T51 DrwNo: 235.22.1114.05583 KD / DF 08/23/2022
			==3X4(C	75° 75° 4° (TYP) 35° (TYP) 35° =	$\begin{array}{c} + & \frac{14^{\circ}10^{\circ}}{75^{\circ}} \\ - \\ - \\ 4 \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\$	D1)	
					14'10" 14'10"		
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 " Lumber Top chord: 2x4 SP #2 Bot chord: 2x4 SP #3; Wind Wind loads based on member design. Wind loading based o	Wind Speed Enclos Risk C EXP: E Mean TCDL: BCDL: MWFR C&C D Loc. fr Wind I ;	Criteria Std: ASCE 7-16 : 130 mph sure: Closed ategory: II 3 Kzt: NA Height: 20.34 ft 5.0 psf 5.0 psf SS Parallel Dist: 0 Dist a: 3.00 ft om endwall: not ir GCpi: 0.18 Duration: 1.60	to h/2 n 9.00 ft C&C types.	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.002 E 999 240 VERT(CL): 0.004 E 999 180 HORZ(LL): -0.002 B - - HORZ(TL): 0.002 E - - Creep Factor: 2.0 Max TC CSI: 0.268 Max BC CSI: 0.119 Max Web CSI: 0.138	▲ Maximum F Gravit Loc R+ / R E* 86 /- Wind reactions E Brg Wid = Bearing A is a Members not I	Reactions (Ibs), or *=PLF y Non-Gravity - /Rh /Rw /U /RL /- /46 /5 /8 s based on MWFRS - 178 Min Req = - rigid surface. isted have forces less than 375#
See DWGS VALTN16 valley details.	60118 a	nd VAL18016011	8 for	T SOULS	AS FLEMING		
IMPORTA Trusses require extrem	**WAI	RNING READ FURNISH THIS I in fa <u>bric</u> ating, har	AND FO DRAWING	COA #0 278 Florida Certi 08/24/20 LLOW ALL NOTES ON THIS D 3 TO ALL CONTRACTORS INC ipping, installing and bracing.	CUDING THE INSTALLERS Refer to and follow the latest edition	999 of BCSI (Buildin	ng

as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information. Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2. For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 99434 FROM:	VAL	Ply: 1 Qty: 1	Job Nur Edenfiel Truss L	mber: 22-8147 d abel: V04		Cust: R 215 JRef:1XIc2150005 T4 DrwNo: 235.22.1114.21037 KD / DF 08/23/2022
				<mark>- 2'7"6 -</mark> 2'7"6 -		
				=4x 10 =3X4(D1) A III III III III IIII	34 =3X4(D1) C C C C C	21'1"
				- 5'2"	"13 ──── ─ 2'7"6	
				2'7"6	<u></u> 5′2″13	
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 "	Wind Speed Enclos Risk C EXP: E Mean TCDL: BCDL: MWFF C&C E Loc. fr	Criteria Std: ASCE 7-16 I: 130 mph sure: Closed ategory: II 3 Kzt: NA Height: 22.34 ft 5.0 psf S: 5.0 psf S: Parallel Dist: h Dist a: 3.00 ft om endwall: not ir GCpi: 0.18 Duration: 1.60	/2 to h n 9.00 ft	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.002 C 999 240 VERT(CL): 0.003 C 999 180 HORZ(LL): -0.001 C - - HORZ(TL): 0.002 C - - Creep Factor: 2.0 Max TC CSI: 0.078 Max BC CSI: 0.067 Max Web CSI: 0.038	▲ Maximum Reactions (Ibs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 85 /- /- /44 /- /7 Wind reactions based on MWFRS C Brg Wid = 62.8 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#
Wind Wind loads based on member design. Wind loading based o	MWFR:	S with additional (gable and hip roof	C&C types.			
Additional Notes See DWGS VALTN16 valley details.	60118 a	nd VAL18016011	8 for			
	\\\\\\ A	RNING DEAD		COA #0 278 Florida Certific 08/24/200	AS FLEN CENS 0. 66648 TATE OF CORIDA Cate of Product Approval #FL19 2	199
IMPORT/ Trusses require extren Component Safety Infi pracing per BCSI. Unla attached rigid ceiling. as applicable. Apply drawings 160A-2 for s Alpine, a division of IT	ANT ne care prmatior ess note Location plates to tandard W. Build	FURNO ^{TT} READ FURNISH THIS C in fabricating, har i, by TPI and SBC d otherwise, top o s shown for perm o each face of trus plate positions. R ing Components (AND FO DRAWING Idling, sh CA) for sa chord sha chord sha	COW ALL NOTES ON THIS D 3 TO ALL CONTRACTORS INC jpping, installing and bracing. F fety practices prior to performing all have properly attached structu eral restraint of webs shall have sition as shown above and on th b's General Notes page for addi c, shall not be responsible for an	CAWING: ELUDING THE INSTALLERS Refer to and follow the latest edition these functions. Installers shall p iral sheathing and bottom chord shi bracing installed per BCSI sections the Joint Details, unless noted other tional information. y deviation from this drawing, any fi	n of BCSI (Building provide temporary all have a property s B3, B7, or B10, twise. Refer to failure to build the

Iruss in conformance with ANSI/TP1 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TP1 1 Sec.2. For more information see these web sites: Alpine: alpineitw.com; TP1: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

SEQN: 99436 FROM:	VAL	Ply: 1 Qty: 1	Job Nun Edenfield	nber: 22-8147 J			Cust: R 215 JRef: DrwNo: 235.22.11	1XIc2150005 Ta 114.24400 08/23/2022	53
		<u> </u>		<u> </u>	2'0"6 1'0"3			06/23/2022	
			<mark> →</mark> 10"7 ->	=3X4(D 12 10 	$ \begin{array}{c} 1) \\ 3X4 \\ \equiv 3X4(D1) \\ B \\ \hline C \\ 2'0"6 \\ \hline e \\ \hline e \\ 2'0"6 \\ \hline e \\ \hline e \\ e \\ e \\ \hline e \\ e \\ e \\ \hline e \\ e \\ e \\ e \\ \hline e \\ e \\ e \\ \hline e \\ e \\ e \\ \hline e \\ e \\ e \\ \hline e \\ e \\ \hline e \\ e \\ e \\ \hline e \\ e \\ e \\ \hline e \\ e \\$	5"			
					2'0"6 2'0"6				
Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 0.00 Load Duration: 1.25 Spacing: 24.0 " Lumber Top chord: 2x4 SP #2 Bot chord: 2x4 SP #2 Wind Wind loads based on member design. Wind loading based of Additional Notes See DWGS VALTN16 valley details.	Wind C Wind S Speed Enclos Risk C EXP: E Mean I TCDL: BCDL: MWFR C&C D Loc. fro Wind D Wind D	Criteria Std: ASCE 7-16 : 130 mph ure: Closed ategory: II 3 Kzt: NA Height: 23.00 ft 5.0 psf S Parallel Dist: If Vist a: 3.00 ft om endwall: not i GCpi: 0.18 Duration: 1.60	v/2 to h n 9.00 ft C&C f types. 8 for	Snow Criteria (Pg,Pf in PSF) Pg: NA Ct: NA CAT: NA Pf: NA Ce: NA Lu: NA Cs: NA Snow Duration: NA Building Code: FBC 7th Ed. 2020 Res. TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.000 C 999 240 VERT(CL): 0.001 C 999 180 HORZ(LL): -0.000 A HORZ(TL): 0.000 A Creep Factor: 2.0 Max TC CSI: 0.015 Max Web CSI: 0.026 Max Web CSI: 0.000 VIEW Ver: 21.02.00.1005.17	▲ Maximum R Gravit Loc R+ / R C* 84 /- Wind reactions C Brg Wid = Bearing A is a Members not I	eactions (Ibs), or y - / Rh / R /- /38 s based on MWFR 24.4 Min Req = rigid surface. isted have forces I	**=PLF Non-Gravity w /U /RL /- /5 S - ess than 375#	
				COA #0 278 Florida Certifi	AS FLEN CENSE 0. 66648 TATE OF CORIDA Gate of Product Approval #FL19	99			

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CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforecement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

Web Member	Specified CLR	Alternative Reir	iforecement
Size	Restraint	T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2×4	1-2×4
2x3 or 2x4	2 rows	2×6	2-2×4
2×6	1 row	2×4	1-2×6
2×6	2 rows	2×6	2-2×4(%)
2×8	1 row	2×6	1-2×8
2×8	2 rows	2×6	2-2×6(%)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

AN ITW COMPAN

155 Harlem Ave North Building, 4th Floor

Glenview II 60025

SPACING

Commentary: Deflection and Camber

Camber may be built into trusses to compensate for the vertical deflection that results from the application of loads. Providing camber has the following advantages:

- Helps to ensure level ceilings and floors after dead loads are applied.
- Facilitates drainage to avoid ponding on flat or low slope roofs.
- Compensates for different deflection characteristics between adjacent trusses.
- Improves appearance of garage door headers and other long spans that can appear to "sag."
- Avoids "dips" in roof ridgelines at the transition from the gable to adjacent clear span trusses.

In accordance with ANSI/TPI 1 the Building Designer, through the Construction Documents, shall provide the location, direction, and magnitude of all loads attributable to ponding that may occur due to the design of the ro drainage system. The Building Designer shall also specify any dead load, live load, and in-service creep deflection criteria for flat or low-slope roofs subject to ponding loads.

The amount of camber is dependent on the truss type, span, loading, application, etceteras.

More restrictive limits for allowable deflection and slenderness ratio (L/D) may be required to help control vibration.

The following tables are provided as guidelines for limiting deflection and estimating camber. Conditions or codes may exist that require exceeding these recommendations, or past experience may warrant using more stringent limitations.

For more information see this job's general notes page and these web sites: ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.lccsafe.org

L = Span of Truss (inches)

D = Depth of Truss at Deflection Point (inches)

Recommended Truss Deflection Limits

re applied.	<u>Truss Type</u>	<u>L/D</u>	<u>Deflection</u>	<u>Limits</u>		
ainage to avoid ponding on flat or			<u>Live Load</u>	<u>Total Load</u>		
fs.	Pitched Roof Trusses	24	L/240 (vertical)	L/180 (vertical)		
for different deflection cs between adjacent trusses.	Floor of Room-In-Attic Trusses	24	L/360 (vertical)	L/240 (vertical)		
earance of garage door headers ng spans that can appear to "sag."	Flat or Shallow Pitched Roof Trusses	24	L/360 (vertical)	L/240 (vertical)		
in roof ridgelines at the transition le to adjacent clear span trusses.	Residential Floor Trusse	es 24	L/360 (vertical)	L/240 (vertical)		
ANSI/TPI 1 the Building Designer,	Commercial Floor Trusse	es 20	L/480 (vertical)	L/240 (vertical)		
ruction Documents, shall provide the and magnitude of all loads attributable	Scissors Trusses	24	0.75" (horizontal)	1.25" (horizontal)		
he Building Designer shall also specify	<u>Truss Type</u> <u>Re</u>		<u>Recommended Camber</u>			
load, and in-service creep deflection or low-slope roofs subject to ponding	Pitched Trusses	1.00 $ imes$ Deflection from Actual Dead Load				
	Sloping Parallel Chord Trusses	1.5 x Vertical Deflection from Actual Dead Load				
cation, etceteras.	Floor Tructor (0.25 v Dofloction from Live Load)		valood) +			
nits for allowable deflection and	r toor ir usses	Actual	Dead Load			
(L/D) may be required to help	Flat Poof Tourson	(0.25 v	Deflection from Li			
	(1.5 x Design Dead Load Deflecti			eflection)		
s are provided as guidelines for and estimating camber. Conditions or	Note: The actual dead	load ma	y be considerably l	ess than		
nat require exceeding these r past experience may warrant using	SAUCENS	C				
tations.						
##VARNDNG### READ AND FOLLOV ALL NOTES ON THIS DRAVING! ##UMPORTANT## FURNISH THIS DRAVING TO ALL CONTRACTORS INCLUDING THE II	NO.00048			REF DEFLEC/CAMB		
Trusses require extreme care in fabricating, handling, shipping, installing and brac follow the latest edition of BCSI (Building Corponent Safety Information, by TPI and presting point to another the formation in the safety information by TPI and	ing. Reference and SBCA) fc safet			DATE 10/01/14		
Unless noted otherwise, top chord shall have properly attached structural sheathi shall have a properly attached rigid celling. Locations shown for permanent lateral shall have bonching hashalled new RSI sections 83 87 on 810 as analicable.	ng and bottom bord I restraint of was ates to got the state	181		DRWG DEFLCAMB1014		
of truss and position as shown above and on the Joint Details, unless noted other Refer to drawings 1604-2 for standard plate positions.	wise. CORIDA	AV A				
hip drawing, any falure to build the truss in conformance with ANSI/TPI 1, or for installation & bracing of trusses.	handling, shipping	Jan Martin				
in a sear on vins arawing or cover page using vins arawing, indicates acceptance of engineering responsibility solely for the design shown. The suitability and use of the for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.	his drawing 08/2/20278	CI-				

155 Harlem Ave North Building, 4th Floor Glenview, IL 60025

AN ITW COMPANY

Valley Detail - ASCE 7-16: 180 mph, 30' Mean Height, Partially Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better. Bot Chord 2x4 SP #2N or SPF #1/#2 or better. Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with: 535# connection or with (1) Simpson H2.5A or equivalent connector for ASCE 7-16 180 mph. 30' Mean Height, Part. Enc. Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00 Dr ASCE 7-16 160 mph. 30' Mean Height, Part. Enc. Building, Exp. D, Wind TC DL=5 psf, Kzt = 1.00

Bottom chord may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

All plates shown are Alpine Wave Plates.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with: properly attached, rated sheathing applied prior to valley truss installation.

Purlins at 24" o.c. or as otherwise specified on engineer's sealed design Dr

By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design.

- *** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.
- ++ Larger spans may be built as long as the vertical height does not exceed 14'-0''.

Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better. Bot Chord 2x4 SP #2N or SPF #1/#2 or better. Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with: (2) 16d box (0.135" x 3.5") nails toe-nailed for ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C, Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on supporting truss material at connection location: 170 mph for SP (G = 0.55, min.),155 mph for DF-L (G = 0.50, min.), or 120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses below valley trusses.

Bottom chord of valley trusses may be square or pitched cut as shown.

Valleys short enough to be cut as solid triangular members from a single 2x6, or larger as required, shall be permitted in lieu of fabricating from separate 2x4 members.

Unless specified otherwise on engineer's sealed design, for vertical valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of web, same species and grade or better, attached with 10d box (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous Lateral Restraint applied at mid-length of web is permitted with diagonal bracing as shown in DRWG BRCLBANC1014.

- Top chord of truss beneath valley set must be braced with: properly attached, rated sheathing applied prior to valley truss installation. Πr
 - Purlins at 24" o.c. or as otherwise specified on engineer's sealed design Πr
 - By valley trusses used in lieu of purlin spacing as specified on Engineer's sealed design
- *** Note that the purlin spacing for bracing the top chord of the truss beneath the valley is measured along the slope of the top chord.
- ++ Larger spans may be built as long as the vertical height does not exceed 14'-0''.

All plates shown are Alpine Wave Plates.