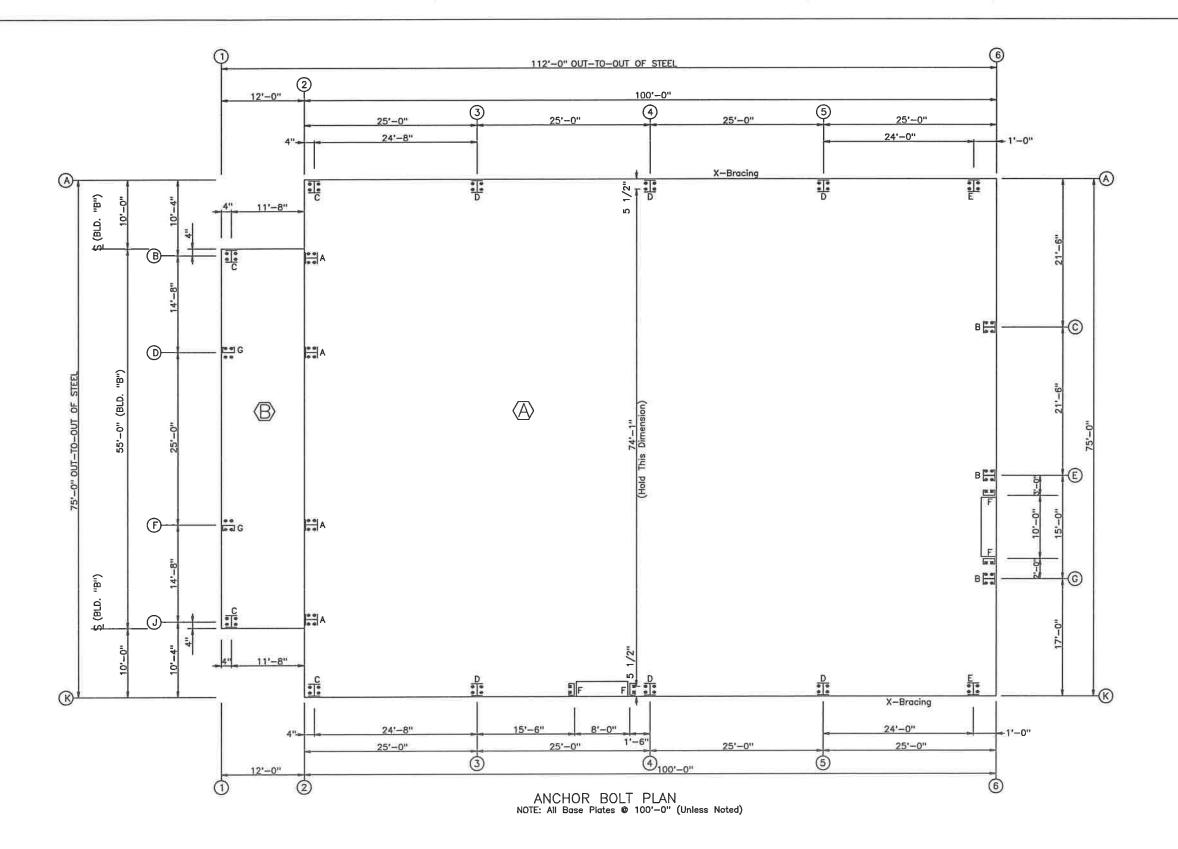
BUILDING PROFILE 10) SECONDARY MEMBERS AND FLANGE BRACE CONNECTIONS SHALL ALWAYS BE SNUG TIGHT, UNO. 11) ANCHOR BOLTS 3/4" IN DIAMETER THRU 1 1/4" IN DIAMETER CONFORM TO A.S.T.M. F1554 GR. 36. ANCHOR BOLTS 1/2" IN DIAMETER CONFORM TO A.S.T.M. A-307. D) UNLESS NOTED OTHERWISE ON FRAMING COLOR CHART: ALL STEEL MEMBERS EXCEPT BOLTS, FASTENERS, CABLE Width (ft) = 75Eave Height (ft) ACCREDITED Metal Building Systems LASTEEL AND RODS SHALL RECEIVE ONE COAT OF STANDARD RED OXIDE SHOP PRIMER Length (ft) = 100Roof Slope (Rise/12) = 3.0:12E) SHOP AND FIELD INSPECTIONS AND ASSOCIATED FEES ARE THE RESPONSIBILITY OF THE CONTRACTOR, UNLESS STIPULATED OTHERWISE IN THE CONTRACT. APPROVAL NOTES Width (ft) = 55Eave Height (ft) Roof Slope (Rise/12) = 3.0:12Length (ft) = 12THE FOLLOWING CONDITIONS APPLY IN THE EVENT THAT THESE DRAWINGS ARE USED AS APPROVAL DRAWINGS: METAL BUILDINGS A) IT IS IMPERATIVE THAT ANY CHANGES TO THESE DRAWINGS: BUILDING DESIGNED & MANUFACTURED BUILDING LOADS 1) BE MADE IN CONTRASTING INK. BY AN IAS ACCREDITED FACILITY. 2) HAVE ALL INSTANCES OF CHANGE CLEARLY INDICATED. THIS IS TO CERTIFY THAT THIS STRUCTURE IS DESIGNED UTILIZING THE LOADS INDICATED AND APPLIED AS REQUIRED BY FBC 23 / 8TH EDITION 3) BE LEGIBLE AND UNAMBIGUOUS. FLORIDA PRODUCT APPROVAL NUMBER THIS CERTIFICATION IS LIMITED TO THE STRUCTURAL DESIGN OF THE FRAMING AND COVERING B) DATED SIGNATURE IS REQUIRED ON ALL PAGES. PARTS MANUFACTURED BY THE BUILDING MANUFACTURER AND AS SPECIFIED IN THE CONTRACT. ACCESSORY ITEMS SUCH AS DOORS, WINDOWS, LOUVERS, TRANSLUCENT PARELS, VENTILATORS ARE PBR ROOF PANEL 36875.1 DRAWING INDEX C) MANUFACTURER RESERVES THE RIGHT TO RESUBMIT DRAWINGS WITH EXTENSIVE OR COMPLEX CHANGES REQUIRED PBR WALL PANEL 36876.1 NOT INCLUDED. ALSO EXCLUDED ARE OTHER PARTS OF THE PROJECT NOT PROVIDED BY THE TO AVOID MISEABRICATION. THIS MAY IMPACT THE DELIVERY SCHEDULE 3070 WALKDOORS +50/-50 PSF 22211.4 RFV. PAGE BUILDING MANUFACTURER SUCH AS FOUNDATIONS, MASONRY WALLS, MECHANICAL FOLIPMENT AND DESCRIPTION D) APPROVAL OF THESE DRAWINGS INDICATES CONCLUSIVELY THAT THE MANUFACTURER HAS CORRECTLY THE ERECTION AND INSPECTION OF THE BUILDING. THE BUILDING SHOULD BE ERECTED ON A INTERPRETED THE CONTRACT REQUIREMENTS, AND FURTHER CONSTITUTES AGREEMENT THAT THE BUILDING AS COVER PAGE PROPERLY DESIGNED FOUNDATION IN ACCORDANCE WITH THE BUILDING MANUFACTURER'S DESIGN IT IS THE RESPONSIBILITY OF THE CUSTOMER TO PROVIDE MANUAL, THE ATTACHED DRAWINGS, AND GOOD ERECTION PRACTICES. THE END USER AND/OR DRAWN, OR AS DRAWN WITH INDICATED CHANGES REPRESENTS THE TOTAL OF THE MATERIALS TO BE SUPPLIED ALL DOCUMENTATION REQUIRED FOR ANY ACCESSORIES NO PROVIDED BY MBM TO THEIR LOCAL PERMITTING OFFICE. ANCHOR BOLT LAYOUT ENGINEER OF RECORD IS TO CONFIRM THAT THESE LOADS COMPLY WITH REQUIREMENTS OF THE 1.1 LOCAL BUILDING DEPT. ANCHOR BOLT DETAILS ALL ACCESSORIES MUST COMPLY AND MEET ALL DESIGN E) ANY CHANGES NOTED ON THE DRAWINGS NOT IN CONFORMANCE WITH THE TERMS AND REQUIREMENTS OF THE REQUIREMENTS PER LOCAL CODES. CONTRACT BETWEEN MANUFACTURER AND ITS CUSTOMER ARE NOT BINDING ON MANUFACTURER UNLESS SUBSEQUENTLY SPECIFICALLY ACKNOWLEDGED AND AGREED TO IN WRITING BY CHANGE ORDER OR SEPARATE 1.2 A&B ANCHOR BOLT REACTIONS OCCUPANCY/RISK CATEGORY <u>II - Normal</u> <u>Is 1.0000</u> <u>le 1.00</u> ROOF FRAMING LAYOUT DOCUMENTATION, MANUFACTURER RECOGNIZES THAT RUBBER STAMPS ARE ROUTINELY USED FOR INDICATING ULTIMATE 120 MPH NOMINAL 92.95 MPH WIND EXPOSURE B WIND LOAD ALL VEHICULAR FRAMED OPENINGS SUPPLIED ON THIS PROJECT APPROVAL, DISAPPROVAL, REJECTION, OR MERE REVIEW OF THE DRAWINGS SUBMITTED. HOWEVER, MANUFACTURER HAVE BEEN DESIGNED TO SUPPORT WIND LOADS NORMAL TO A 2.1-2.4 <u>-0.18 / 0.18</u> RIGID FRAME CROSS SECTION CLOSURE TYPE Enclosed INTERNAL WIND COEF. DOES NOT ACCEPT CHANGES OR ADDITIONS TO CONTRACTUAL TERMS AND CONDITIONS THAT MAY APPEAR WITH USE OF A STAMP OR SIMILAR INDICATION OF APPROVAL, DISAPPROVAL, ETC. SUCH LANGUAGE APPLIED TO DOOR SYSTEM, BASED ON THE STANDARD BUILDING CODE CRITERIA 3-3.2 ENDWALL FRAMING LAYOUT GROUND SNOW LOAD 4.00 PSF ROOF SNOW LOAD 3.13 PSF Ce 1.0000 Ct 1.2000 THE VEHICULAR FRAMED OPENING HAS NOT BEEN DESIGNED FOR MANUFACTURER'S DRAWINGS BY THE CUSTOMER, ARCHITECT, ENGINEER, OR ANY OTHER PARTY WILL BE ANY ADDITIONAL MOMENT OR CATENARY FORCE FROM THE DOOR 4-4.3 SIDEWALL FRAMING LAYOUT PER CODE CONSIDERED AS UNACCEPTABLE ALTERATIONS TO THESE DRAWING NOTES, AND WILL NOT ALTER THE SNOW BANKING LOADS SYSTEM. ANY CHANGES TO THE INFORMATION SHOWN HERE WOULD CONTRACTUAL RIGHTS AND OBLIGATIONS EXISTING BETWEEN MANUFACTURER AND ITS CUSTOMER. REQUIRE AN ENGINEERING INVESTIGATION AND POSSIBLE BUILDING 5-5.6 0.50 PSF FRAMING DETAILS COLLATERAL DEAD LOAD SAFETY COMMITMENT REINFORCEMENT. 20.00 PSF (REDUCIBLE Yes) ROOF LIVE LOAD ROOF PANELS & TRIM DEAD LOAD 2.00 PSF (FOR ROOF PANELS AND PURLINS) 6.1 ROOF PANEL DETAILS A) THE BUILDING MANUFACTURER HAS A COMMITMENT TO MANUFACTURE QUALITY BUILDING COMPONENTS THAT CAN BE FRAMING COLORS SAFELY ERECTED, HOWEVER, THE SAFETY COMMITMENT AND JOB SITE PRACTICES OF THE ERECTOR ARE BEYOND THE CONTROL OF THE BUILDING MANUFACTURER. SEISMIC SIDEWALL PANEL DETAILS Ss 0.1300 S1_0.0580 Sds 0.1133 Sd1 0.0800 SPECTRAL RESPONSE IT IS STRONGLY RECOMMENDED THAT SAFE WORKING CONDITIONS AND ACCIDENT PREVENTION PRACTICES BE THE TOP ENDWALL PANEL DETAILS Flange brace: RO PRIORITY OF ANY JOB SITE SITE CLASS DESIGN RISK CATEGORY B Св 0.0378 SPECIAL DETAILS C) LOCAL, STATE AND FEDERAL SAFETY AND HEALTH STANDARDS SHOULD ALWAYS BE FOLLOWED TO HELP INSURE RO RESPONSE MODIFICATION FACTOR, R 3.000* FRAMES 3.000* BRACING MAKE CERTAIN ALL EMPLOYEES KNOW THE SAFEST AND MOST PRODUCTIVE WAY OF ERECTING A BUILDING. D) BASIC SEISMIC FORCE RESISTING SYSTEM (LATERAL DIRECTIONS) = ORDINARY STEEL MOMENT FRAMES EMERGENCY PROCEDURES SHOULD BE KNOWN TO ALL EMPLOYEES. E) DAILY MEETINGS HIGHLIGHTING SAFETY PROCEDURES ARE ALSO RECOMMENDED. THE USE OF HARD HATS. RUBBER BASIC SEISMIC FORCE RESISTING SYSTEM (ENDWALLS) = ORDINARY STEEL MOMENT FRAMES SOLE SHOES FOR ROOF WORK, PROPER EQUIPMENT FOR HANDLING MATERIAL, AND SAFETY NETS WHERE APPLICABLE, BASIC SEISMIC FORCE RESISTING SYSTEM (LONGITUDINAL DIRECTIONS) = ORDINARY STEEL CONC. BRACED FRAMES RO RO RO RO RO RO D. SECTION: ARE RECOMMENDED. ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE PROCEDURE ERECTOR / CONTRACTOR RESPONSIBILITIES BUILDING CODE SPECIFICATIONS REQUIRE CONSIDERATION OF SNOW SURCHARGES 32025 STEEL SYSTEM NOT SPECIFICALLY DETAILED FOR FOR ANY LOWER ROOF OF A STRUCTURE LOCATED WITHIN 20ft. OF A HIGHER STRUCTURE. INFORMATION SUPPLIED TO THE METAL BUILDING SUPPLIER DOES NOT INDICATE PRESENCE OF A SHADOWING STRUCTURE WITHIN THIS 20ft. C∏, SERVICEABILITY CRITERIA SEISMIC RESISTANCE A) IT IS THE RESPONSIBILITY OF THE FRECTOR/CONTRACTOR TO INSURE THAT ALL PROJECT PLANS AND SPECIFICATIONS W SECTION: RO RO RO RO RO RO COMPLY WITH THE APPLICABLE REQUIREMENTS OF ANY GOVERNING BUILDING AUTHORITIES. THE SUPPLYING OF SEALED ENGINEERING DATA AND DRAWINGS FOR THE METAL BUILDING SYSTEM DOES NOT IMPLY OR CONSTITUTE AN AGREEMENT ENVELOPE, AND AS SUCH, SNOW SURCHARGES HAVE NOT BEEN CONSIDERED IN THE DESIGN OF THE BUILDING(S) SHOWN ON THESE PLANS. MINIMUM DESIGN DEFLECTIONS WHEN GALVANIZED PROVIDED: ALL FINISHED Endwall Column THAT THE BUILDING MANUFACTURER OR ITS DESIGN ENGINEER IS ACTING AS THE ENGINEER OF RECORD OR DESIGN PRIMARY BUILT-UP AND HOT ROLL MEMBERS = 180 Roof Panel (Live) \exists THIS PROJECT IS DESIGNED AS AN ENGLOSED BUILDING ACCESSORIES. ARE HOT DIPPED GALVANIZED. ALL SECONDAR PROFESSIONAL FOR A CONSTRUCTION PROJECT. = 180 (DOORS, WINDOWS, ETC.) BY OTHERS MUST BE DESIGNED AS "COMPONENTS AND CLADDING" IN ACCORDANCE TO SPECIFIC WIND PROVISIONS OF Endwall Rafter (Live) Roof Panel (Wind) = 60 B) THE CONTRACTOR MUST SECURE ALL REQUIRED APPROVALS AND PERMITS FROM THE APPROPRIATE AGENCY AS COLD FORMED MEMBERS ARE PRE-GALVANIZED Endwall Rafter (Wind) = 180 Rigid Frame (Horz) = 60 OIT, REFERENCED BUILDING CODE. C) APPROVAL OF THE MANUFACTURER'S DRAWINGS AND CALCULATIONS INDICATE THAT THE BUILDING MANUFACTURER REGULERALLY INTERPRETED AND APPLIED THE REQUIREMENTS OF THE CONTRACT DRAWINGS AND SPECIFICATIONS. Wall Girt = 90 Rigid Frame (Vert) = 180 FOR OCCUPANCY (RISK) CATEGORY I OR II, IBC PROVISIONS INDICATE THAT JOBSITE: Roof Purlin (Live) = 150 Rigid Frame (Seismic) = 50 (SECT. 4.4.1 AISC CODE OF STANDARD PRACTICES, LATEST ED.) SINGLE-STORY BUILDINGS SHALL HAVE "NO DRIFT LIMIT" PROVIDED THAT Roof Purlin (Wind) = 150 D) WHERE DISCREPANCES EXIST BETWEEN THE MANUFACTURER'S STRUCTURAL STEEL PLANS AND THE PLANS FOR TRAFI LAKE INTERIOR WALLS, PARTITIONS, CEILINGS AND EXTERIOR WALL SYSTEMS HAVE OTHER TRADES, THE STRUCTURAL STEEL PLANS SHALL GOVERN. (SECT. 3.3 AISC CODE OF STANDARD PRACTICE LATEST ED.) E) DESIGN CONSIDERATIONS OF ANY MATERIALS IN THE STRUCTURE WHICH ARE NOT FURNISHED BY THE BUILDING MANUFACTURER ARE THE RESPONSIBILITY OF THE CONTRACTORS AND ENGINEERS OTHER THAN THE BUILDING MANUFACT— Wall Panel = 60 BEEN DESIGNED TO ACCOMMODATE THE SEISMIC STORY DRIFTS. INTERIOR WALLS, PARTITIONS, CEILINGS OR EXTERIOR SYSTEMS NOT PROVIDED BY MBM GENERAL NOTES SHALL BE DESIGNED AND DETAILED BY OTHERS TO ACCOMODATE THE SEISMIC URER'S ENGINEERS UNLESS SPECIFICALLY INDICATED. STORY DRIFTS. F) THE ERECTOR/CONTRACTOR IS RESPONSIBLE FOR ALL ERECTION OF STEEL AND ASSOCIATED WORK IN COMPLIANCE WITH THE BUILDING MANUFACTURER'S "FOR CONSTRUCTION" DRAWINGS. A) THE STRUCTURE UNDER THIS CONTRACT HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS 0.5 PSF COLL ONLY ALLOW LIGHTING TO HANG FROM ROOF SYSTEMS TO 43547 STIPULATED IN THE CONTRACT AND SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR C) PRODUCTS SHIPPED TO ERECTOR/CONTRACTOR OR HIS CUSTOMER SHALL BE INSPECTED BY ERECTOR/CONTRACTOR SUSPENSION OF ANY LOAD INDUCING SYSTEM IS EXPLICITLY PROHIBITED, UNLESS A CORRESPONDING REDUCTION IN CERTIFIED LIVE/SNOW LOADS REMOVAL OF ANY COMPONENT PARTS, OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE MMEDIATELY UPON ARRIVAL CLAMS FOR SHORTAGES OR DEFECTIVE MATERIAL BE INSPECTED BY EXECUTOR/CONTRACTOR OR HIS COSTOMER SHALL BE INSPECTED BY EXECUTOR/CONTRACTOR OR HIS COSTOMER SHALL BE INSPECTED BY EXECUTIVE OF THE BANDWEYER, IF A DEFECT IS OF SUCH A MATI IDEF THAT REASONABLE VISUAL INSPECTION WOULD FAIL TO DISCLOSE IT, THE SHIPMENT, HOWEVER, IF A DEFECT IS OF SUCH A MATI IDEF THAT REASONABLE VISUAL INSPECTION WOULD FAIL TO DISCLOSE IT, THE THE CLAIM MUST BE MADE WITHIN FIVE (5) DAY AFTER THE ERECTOR/CONTRACTOR LEARNS OF THE DEFECT. THE MANUFACTURER WILL NOT BE LIABLE FOR MAY DEFECT UP ST. DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CIVIL OR STRUCTURAL ENGINEER CAN BE PERMITTED BY CODE. THE BUILDING MANUFACTURER WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED. B) THIS METAL BUILDING IS DESIGNED WITH THE BUILDING MANUFACTURER'S STANDARD PRACTICES WHICH ARE BASED 76th ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES, CLAM IS MADE WITHIN ONE (1) YEAR AFTER DATE OF THE ORIGINAL SHIPMENT BY THE MANUFACTURER TO CONTRACTOR OR HIS CUSTOMER. THE MANUFACTURER WILL BE GIVEN A REASONABLE OPPORTUNITY TO INSPECT DEFECTIVE MATERIALS 1. AMERICAN INSTITUTE OF STEEL CONSTRUCTION: "AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS—ALLOWABLE STRESS DESIGN" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE. UPON RECEIPT OF CLAIM BY CONTRACTOR IF A DEFECT IS OF SUCH NATURE THAT IT CAN BE REMEDIED BY A FIELD OPERATION AT THE JOB SITE WITHOUT SW THE NECESSITY OF RETURNING THE MATERIAL TO THE MANUFACTURER, THEN UPON WRITTEN AUTHORIZATION OF THE MANUFACTURER THE CONTRACTOR MAY REPAIR OR CAUSE THE MATERIAL TO BE REPAIRED AND THE MANUFACTURER WILL REIMBURSE THE CONTRACTOR FOR THE COST OF THE REPAIR IN ACCORDANCE WITH THE WRITTEN AUTHORIZATION. 32054 AMERICAN IRON AND STEEL INSTITUTE: "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUMEMBERS" AS ADOPTED BY THE BUILDING CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE. "SPECIFICATION FOR THE DESIGN OF COLD FORMED STEEL STRUCTURAL 13237 3. AMERICAN WELDING SOCIETY: "STRUCTURAL WELDING CODE" AWS D1.1. AS ADOPTED BY THE BUIDLING CODE THE CORRECTION OF MINOR MISFITS BY THE USE OF DRIFT PINS TO DRAW THE COMPONENTS IN TO LINE, MODERATE AMOUNTS OF REAMING, CHIPPING AND CUTTING, AND THE REPLACEMENT OF MINOR SHORTAGES OF MATERIAL ARE A NO REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE. STAIL STAIL STAIL 4. METAL BUILDING MANUFACTURER'S ASSOCIATION: "LOW RISE BUILDING SYSTEMS MANUAL" AS ADOPTED BY THE BUILDING PART OF ERECTION AND ARE NOT SUBJECT TO CLAIM. CODE REFERENCED IN "BUILDING LOADS" SECTION "A" ABOVE. H) ALL BRACING AS SHOWN AND PROVIDED BY THE MANUFACTURER FOR THIS BUILDING IS REQUIRED AND SHALL BE C) 1) MATERIAL PROPERTIES OF STEEL PLATE USED IN THE FABRICATION OF PRIMARY RIGID FRAMES, AND OTHER PRIMARY STRUCTURAL EXCLUSIVE OF COLD-FORMED SECTIONS, CONFORM TO ASTM-A529 OR A572 . FLANGES AND INSTALLED BY THE ERECTOR AS A PERMANENT PART OF THE STRUCTURE. 1) TEMPORARY SUPPORTS, SUCH AS TEMPORARY GUYS, BRACES, FALSE WORK, CRIBBING OR OTHER ELEMENTS REQUIRED. LaSTEEL 537 BUTLER, This item has been digitally signed and sealed by Richard FOR THE ERECTION OPERATION WILL BE DETERMINED AND FURNISHED AND INSTALLED BY THE ERECTOR. THESE TEMPORARY SUPPORTS WILL SECURE THE STEEL FRAMING, OR ANY PARTLY ASSEMBLED STEEL FRAMING, AGAINST WEB MATERIAL CONFORMS TO ASTM-A529 OR A572 GRADE 55 WITH A MINIMUM YIELD POINT OF 55,000 psi. 2) MATERIAL PROPERTIES OF HSS ROUND SECTIONS CONFORM TO ASTM-A500, GRADE B OR C WITH A MINIMUM YIFLD T Smith on the date adjacent to the seal. Printed copies LOADS COMPARABLE IN INTENSITY TO THOSE FOR WHICH THE STRUCTURE WAS DESIGNED, RESULTING FROM WIND, BOX 3) MATERIAL PROPERTIES OF HSS RECT. OR SQUARE SECTIONS CONFORM TO ASTM-A500, GRADE B OR C WITH A MINIMUM SEISMIC FORCES AND ERECTION OPERATIONS. BUT NOT THE LOADS RESULTING FROM THE PERFORMANCE OF WORK BY of this document are not considered signed and sealed OR THE ACTS OF OTHERS, NOR SUCH UNPREDICTABLE LOADS AS THOSE DUE TO TORNADO, EXPLOSION OR COLLISION. NOINO 4) MATERIAL PROPERTIES OF HOT ROLLED CHANNEL AND ANGLE MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992 (SECT. 7.10.3 AISC CODE OF STANDARD PRACTICE, LATEST ED.) and the signature must be verified on any electronic FROM: LAKE WITHMINIMUM YIELD POINT OF 50,000 PSI. HOT ROLLED W-SHAPED MEMBERS CONFORM TO THE REQUIREMENTS OF ASTM-A992WITH MINIMUM YIELD POINT OF 50,000 PSI. J) METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL AND WORKMANSHIP OF FOUNDATION, ANCHOR BOLT P.0. PLANS PREPARED BY MBM ARE INTENDED TO SHOW ONLY LOCATION, DIAMETER AND PROJECTION OF THE ANCHOR RODS REQUIRED copies. 5) MATERIAL PROPERTIES OF COLD FORMED LIGHT GAGE STEEL MEMBERS CONFORM TO EITHER ASTM A653-06 GR 55 OR COLORS: TO ATTACH THE METAL BUILDING SYSTEM TO FOUNDATION, IT IS RESPONSIBILITY OF THE END CUSTOMER TO ENSURE THAT ADEQUATE A1011-04 HSLAS GRADE 55 WITH YIELD OF 55,000 psi. 6) MATERIAL PROPERTIES OF ROOF/WALL SHEETING, BASE METAL CONFORM TO ASTM-A792 GRADES 80 CLASS 1, 2 OR 3 PROVISIONS ARE MADE FOR SPECIFYING ROD EMBEDMENT, BEARING VALUES, TIE RODS AND OTHER ASSOCIATED ITEMS EMBEDDED IN THE JOB NO CONCRETE FOUNDATION. AS WELL AS FOUNDATION DESIGN FOR THE LOADS IMPOSED BY MB SYSTEM, OTHER IMPOSED LOAD, AND THE ROOF GALVALUME DRAWING STATUS WITH A MINIMUM YIELD STRENGTH OF 80,000 PSI. COATING OF BASE MATERIAL IS 55% ALUMINUM-ZINC ALLOY 8728 IN ACCORDANCE WITH AZ55 SPECIFICATIONS. BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE (MBMA 06 SECTIONS 3.2.2 AND A3) WALLS: FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL, AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE. COLOR 7) CABLE UTILIZED FOR BRACING CONFORMS TO ASTM A475. CABLE BRACING IS TO BE INSTALLED TO A TAUT K) METAL BUILDING MANUFACTURER DOES NOT PROVIDE ANY FIELD SUPERVISION FOR THE ERECTION, NOR DOES DATE GARLE: COLOR 8/14/24 MBM PERFORM ANY INSPECTIONS DURING OR AFTER ERECTION. B) ROD UTILIZED FOR BRACING MEMBERS CONFORM TO ASTM-A36 WITH MINIMUM YIELD POINT OF 36,000 PSI EAVE: COLOR 9) IT IS THE RESPONSIBILITY OF ERECTOR TO ENSURE PROPER BOLT TIGHTNESS IN ACCORDANCE WITH APPLICABLE PS NONE "RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING A-325 OR A-490 BOLTS" ALL A-325 BOLTS IN PRIMARY В CORNER COLOR Α FRAMING MUST BE "SNUG-TIGHT", EXCEPT AS FOLLOWS: "FULLY-PRETENSION" A-325 BOLTS IF: FOR PERMIT: THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL IN THAT, AS A MINIMUM, PIECE MARKINGS ARE NOT IDENTIFIED. ONLY DRAWINGS ISSUED FOR CONSTRUCTION CAN BE CONSIDERED AS COMPLETE. RAMED OPENINGS: COLOR COVER PAGE) BUILDING LOCATED IN A HIGH SEISMIC AREA. FOR IBC-BASED CODE, "HIGH SEISMIC AREA" IS DEFINED AS "SEISMIC COMPONENTS & CLADDING (unfactored) COMPONENTS & CLADDING (unfactored) असामा ध COLOR DESIGN CATEGORY" OF "D". "E" OR "F". UMBER : b) BUILDING SUPPORTS A CRANE SYSTEM WITH A CAPACITY GREATER THAN 5.00 TONS. 22.308 psf -24.198 psf Wall Field Values = 19.177 psf / -19,177 psf COLOR c) BUILDING SUPPORTS MACHINERY THAT CREATES VIBRATION, IMPACT OR STRESS - REVERSALS ON THE CONNECTIONS. Wall Edge Values = 22.308 Wall Edge Values = 19,177 psf / -35.157 psf PAGE FOR CONSTRUCTION: THESE DRAWINGS ARE FINAL AND ISSUED FOR FIELD USE FOR BUILDING ERECTION psf / -29.870 psf COLOR d) ANY CONNECTION DESIGNATED IN THESE DRAWINGS AS "A-325 - SC".



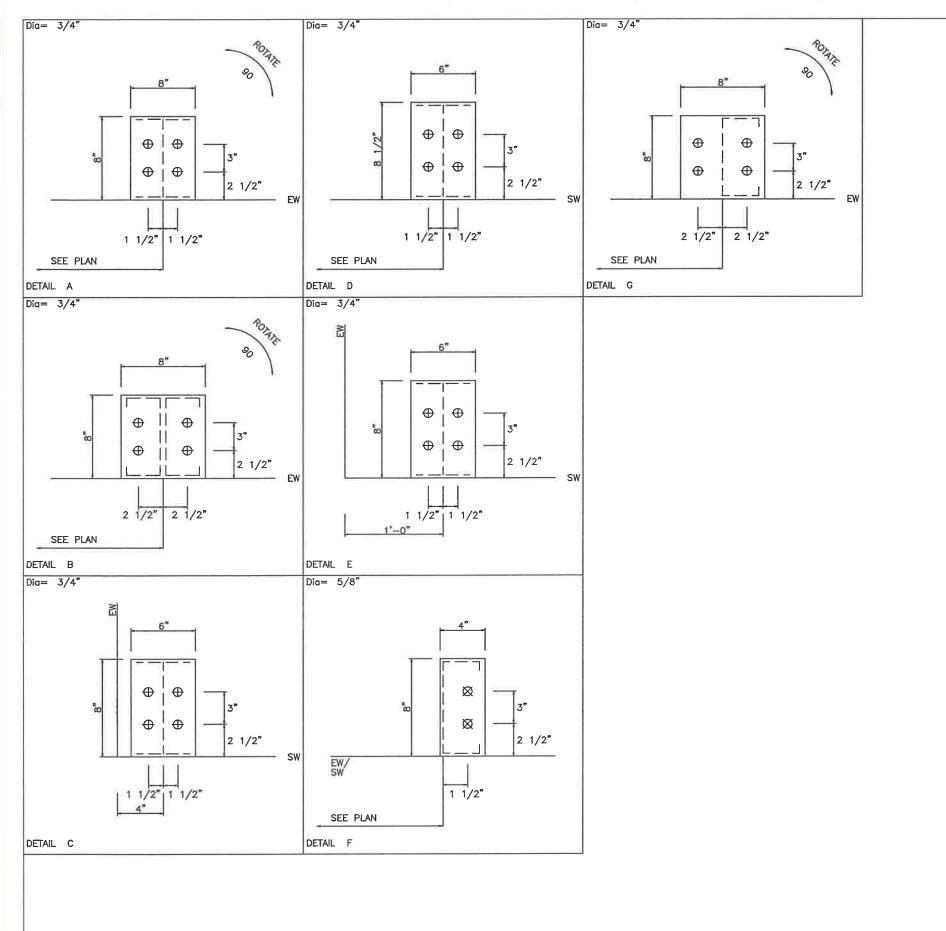
NOTE: ALL FIELD LOCATED FRAMED OPENING LOCATIONS SHALL BE AT THE DISCRETION OF THE ERECTOR/CUSTOMER. IT IS RECOMMENDED THAT THESE ANCHORS BE LOCATED AT TIME OF ERECTION.

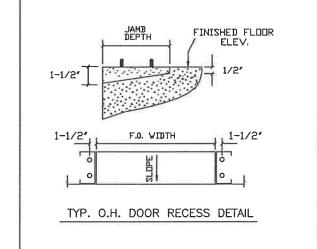
FIELD LOCATE:

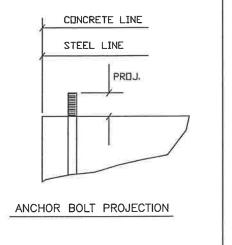
(5) 3070 WALKDOORS

(7) 3'-0" x 5'-0" FRAMED OPENING

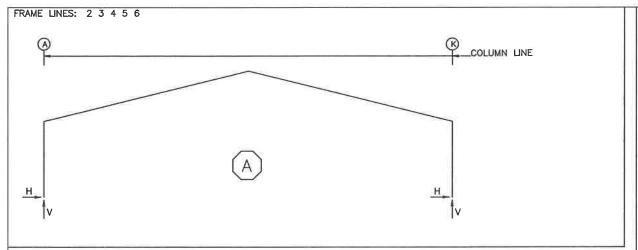
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RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	_Bolt Dia	Base. Width	_Plate (i Length	n) Thick	Grout (in)
2 2	A K		6.000 6.000		0.500 0.500	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm	Col		_Bolt	Base	_Plate (i	in)	Grout
Line	Line		Dia	Width	Length	Thick	(in)
3* 3*	A K	4 4		6.000 6.000		0.500 0.500	0.0

3* Frame lines: 3 4 5

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm	Col		_Bolt	Base.	_Plate (i	in)	Grout
Line	Line		Dia	Width	Length	Thick	(in)
6	A K	4		6.000 6.000	8.000 8.000	0.375 0.375	0.0

END	WALI	L COL	UMN:	BASIC	COLUMN	REACTIO								
Frm Line 2 2 2 2 2 6 6	Col Line B D F J G E C	Dead Vert 0.2 0.6 0.6 0.2 0.1 0.2	Collat Vert 0.0 0.1 0.1 0.0 0.0 0.0	Horz 0,0 -0.1 -0.1 0.0 0.0 0.0	Ve Vert 0.6 2.8 2.8 0.6 0.0 0.0	Snow Vert 0.1 0.4 0.4 0.1 0.0 0.0	Snow Drift Vert 0.1 0.6 0.6 0.1 0.0 0.0	Wind_L Horz 0.0 0.1 0.1 0.0 0.0 0.0	Vert -0.3 -1.7 -1.5 -0.2 0.0 0.0	Wind_I Horz 0.0 0.1 0.1 0.0 0.0 0.0	Right1 Vert -0.2 -1.5 -1.7 -0.3 0.0 0.0	Wind Left2 Vert 0.1 -0.9 -0.8 0.1 0.0 0.0	Wind Right2 Vert 0.1 -0.8 -0.9 0.1 0.0 0.0	
Frm Line 2 2 2 2 6 6	Col Line BDF JGEC	Wind Press Horz -0.7 -0.1 -0.7 -2.5 -3.4 -3.5	Wind Suct Horz 0.8 0.1 0.1 0.8 2.7 3.8 3.9	Wind_I Horz 0.0 0.1 0.1 0.0 0.0 0.0	ong1 Vert -0.8 -2.7 -2.7 -0.8 0.0 0.0	Wind Long2 Vert -0.1 0.1 -0.1 0.0 0.0	Seis Left Vert 0.0 0.0 0.0 0.0 0.0	Seis Right Vert 0.0 0.0 0.0 0.0 0.0 0.0	Seis Long Vert 0.0 0.0 0.0 0.0 0.0					

BUIL	DINC	BRA	CING	RE	ACTIO	ONS		
	ıll — Line		Wi	nd —) smic- Vert	Panel_Shear (lb/ft) Wind Seis	Note
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Frame Line 2 2	Column Line A K	Wind_ Horz -11.2 5.1	Vert		Right2- Vert -7.5 -13.8	Horz	-12.0	Wind. Horz -7.0 5.8	Vert	Horz -0.2	Vert -0.1	Seismic Horz 0.2 0.2	Vert	
Frame Line 2 2	Line	-MIN_SN Horz 2.3 -2.3	Vert	F1UNB_ Horz 1.4 -1.4	Vert	F1UNB_ Horz 1.4 -1.4	Vert							
Frame Line 3* 3*	Line	[Horz 2.1 -2.1	Vert	Collo Horz 0.4 -0.4	Vert	Horz 9.1 -9.1	Live Vert 11.2 13.3	Horz 2.4 -2.4	Snow Vert 2.9 3.3	Wind Horz 14.7 6.9	_Left1- Vert 16.8 14.0	-7.1		
Frame Line 3* 3*	Column Line A K	Wind_ Horz -11.2 3.4	Vert −9.5	Horz -3.7	Right2- Vert -5.8 -12.0	Wind Horz -7.6 9.2	-15.8	Horz -9.2	Vert	Horz -0.2	Vert −0.1	Horz		
Frame Line 3* 3*	Line	-MIN_SN Horz 3.0 -3.0	Vert	Horz	SL_L- Vert 2.7 1.8	F2UNB_ Horz 1.9 -1.9	SL_R- Vert 1.6 3.0							
Frame Line 6 6	Line	[Horz 1.2 -1.2	Vert	Horz	Vert	Horz 4.5 -4.5	Live Vert 5.8 6.9	Horz	Vert	Horz	Vert	Horz -5.4	Vert	
Frame Line 6 6	Line A	Wind_ Horz -7.4 3.2		-Wind_ Horz -3.4 7.0		Wind, Horz -3.7 4.6	_Long1- Vert -8.2 -6.9	Horz	Vert	Horz -0.1		Seismic Horz 0.1 0.1		
Frame Line 6 6	Line	-MIN_SN Horz 1.5 -1.5	Vert	Horz	Vert	Horz	Vert							
3*	Frame lin	es:	3 4 5											

END	.TS & E	ASE PLATES					
Frm	Col Line		_Bolt Dia	Base. Width	_Plate (i Length	n) Thick	Grout (in)
2	В	4	0.750	8.000	8.000	0.250	0.0
2	D	4	0.750	8.000	8.000	0.250	0.0
2	F	4	0.750	8.000	8.000	0.250	0.0
2	J	4	0.750	8.000	8.000	0.250	0.0
6	G	4	0.750	8.000	8.000	0.250	0.0
6	E	4	0.750	8.000	8.000	0.250	0.0
6	С	4	0.750	8.000	8.000	0.250	0.0

ANCHOR BOLT SUMMARY

	Qty	Locate	Dia (in)	Туре	Projection (in)
₩	8	Jamb	5/8"	A307	1.50
	28	Endwall	3/4"	GR36	1.50
	40	Frame	3/4"	GR36	2.50

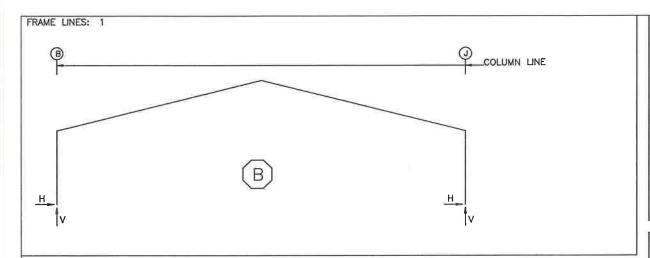
GENERAL NOTES

- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
- 2. ALL REACTIONS ARE UNFACTORED.
- 3 ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
- ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- 5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

NOTES FOR REACTIONS
Building reactions are based on the following building data: Width Length Eave Height Roof Slope Roof Dead Load Wall Dead Load Left Endwall Front Sidewall Roof Live Load

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

ISS	ISSUE						
UNION LaSTEEL							
CUSTOMER: TRAFICANTE							
JOB NO: 8728		DATE:	/14/	24			
LAKE CITY, FL	32025	- to - cost					
DRAWING HAME: ANCHOR BOLT F							
PAGE 1.2 A	DRAWN BY: PS	SPW	SCA	NONE			



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

	Col Line		_Bolt Dia	Base. Width	_Plate (i Length	n) Thick	Grout (in)
1	В	4 4		6.000 6.000	8.000 8.000	0.375 0.375	0.0 0.0

 ENDWALL
 COLUMN:
 ANCHOR BOLTS & BASE PLATES

 Frm Col Line
 Anc._Bolt Qty Dia
 Base_Plate (in) Width Length Thick (in)
 Grout (in)

 1
 D
 4
 0.750
 8.000
 8.000
 0.250
 0.0

 1
 F
 4
 0.750
 8.000
 8.000
 0.250
 0.0

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Туре	Projection (in)
⊕ 8	Endwall	3/4"	GR36	1.50
⊕ 8	Frame	3/4"	GR36	2.50

GENERAL NOTES

- FOUNDATION DESIGN AND CONSTRUCTION ARE NOT THE RESPONSIBILITY OF METAL BUILDING MANUFACTURER.
- 2. ALL REACTIONS ARE UNFACTORED.
- 3 ULTIMATE WIND LOADS ARE USED TO DERIVE THE WIND REACTION.
- 4. ANCHOR BOLTS SHALL BE ACCURATELY SET TO A TOLLERANCE OF +/- 1/8" IN BOTH ELEVATION AND LOCATION.
- 5. COLUMN BASE PLATES ARE DESIGNED NOT TO EXCEED A BEARING PRESSURE OF 1050 POUNDS PER SQUARE INCH.

BUILDING BRACING REACTIONS

		Shear 7ft) Seis	Note
L_EW 1 F_SW J R_EW 2 Bracing Not Used B_SW B	440 0 440	10 0 10	(h)
(h)Rigid frame at endwall			

Reactions for seismic represent shear force, Eh Reaction values shown are unfactored

RIGIE) FRAN	/E: £	BASIC COL	LUMN REA	CTIONS ((k)								
Frame Line 1	Column Line B J	Horz 0.5 -0.5	-Dead Vert 0.8 0.8	Colla Horz 0.1 0.1	teral— Vert 0.1 0.1	Horz 2.1 -2.1	-Live Vert 3.0 3.0	Horz 0.4 -0.4	Snow Vert 0.5 0.5	Snow Horz 0.1 -0.1	/_Drift- Vert 0.1 0.1	Wind Horz -2.2 0.9	l_Left1- Vert -2.4 -2.1	
Frame Line 1	Column Line B J	-Wind_ Horz -0.9 2.2	Right1- Vert -2.1 -2.4	Wind Horz -1.7 0.4	Left2- Vert -1.1 -0.7		Right2- Vert -0.7 -1.1	Wind Horz -2.4 2.4	_Long1- Vert -4.3 -4.3	Wind Horz 0.4 -0.4	_Long2 Vert 0.1 0.1	-Seism Horz 0.0 -0.1	ic_Left Vert 0.0 0.0	
Frame Line 1	Column Line B J	Seismic Horz 0.0 0.1	Right Vert 0.0 0.0	Wind Horz -1.7 0.3	_Left3- Vert -1.3 -1.6	-Wind_ Horz -0.3 1.7	Right3- Vert -1.6 -1.3	Wind Horz -1.1 -0.3	_Left4- Vert 0.1 -0.3	-Wind_l Horz 0.3 1.1	Right4- Vert -0.3 0.1	-MIN_S Horz 0.5 -0.5	NOW Vert 0.7 0.7	
Frame Line 1	Column Line B J	F1UNB_ Horz 0.3 0.3	SL_L- Vert 0.5 0.3	F1UNB_ Horz 0.3 -0.3	SL_R- Vert 0.3 0.5									

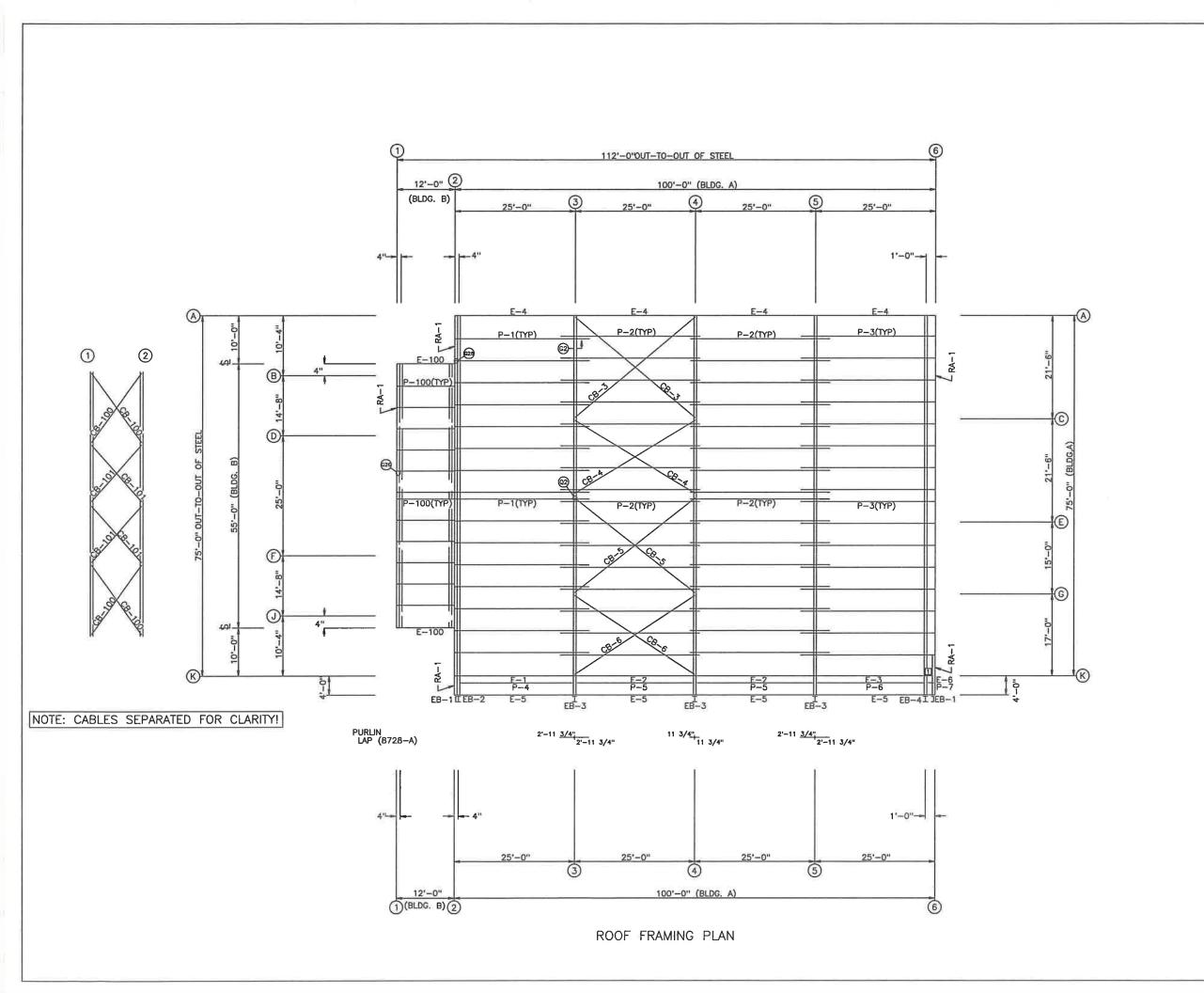
ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Col Dead Press Suct Long
Line Line Vert Horz Horz Vert
1 D 0.0 -2.0 2.1 0.0
1 F 0.0 -2.0 2.1 0.0

NOTES FOR REACTIONS
Building reactions are based on the following building data:
Width $(ft) = 55.0$
Length (ft) = 12.0 Eave Height (ft) = 10.0 / 10.0 Roof Slope (rise/12) = 3.00 / 3.00
Roof Dead Load (psf) = 2.0 Wall Dead Load
Left Endwall (psf) = 2.0 Right Endwall (psf) = 2.0
Front Sidewall (psf) = 2.0 Back Sidewall (psf) = 2.0
Roof Live Load (psf) = 20.0 Frame Live Load (psf) = 17.4
Collateral Load (psf) = 0.5 Snow Load (psf) = 3.1
Minimum Snow (psf) = 4.0 Wind Speed (mph) = 120.0
Wind Code = FBC 23 (8TH EDITION) Exposure = B
Closure = Enclosed Internal Wind Coeff = -0.18, +0.18
Risk Category = II — Normal Importance — Wind = 1.00
Importance - Seismic = 1.00 Seismic Design Category = B
Seismic Coeff (Sms) = 0.17

NOTE: THE FRAMING AT BOTH ENDWALLS IS NOT DESIGNED TO ACCOMMODATE FUTURE ADDITIONS. REACTIONS CORRESPONDING TO THESE FRAME LINES REFLECT LOADINGS FOR ACTUAL TRIBUTARY AREA AND ARE NOT INTENDED TO INCLUDE ANY FUTURE MODIFICATIONS UNLESS NOTED OTHERWISE.

ISS	DET	СНК	DATE	
UNION LaSTEEL				
CUSTOMER: TRAFICANTE				
JOB NO: 8728		DATE:	/14/	24
LAKE CITY, FL 3	32025	11711		
DRAWING NAME: ANCHOR BOLT F	REACTIONS			
DRAWING NO: PAGE 1.2 B	DRAWN BY: PS	SPW SPW	sc.	NONE

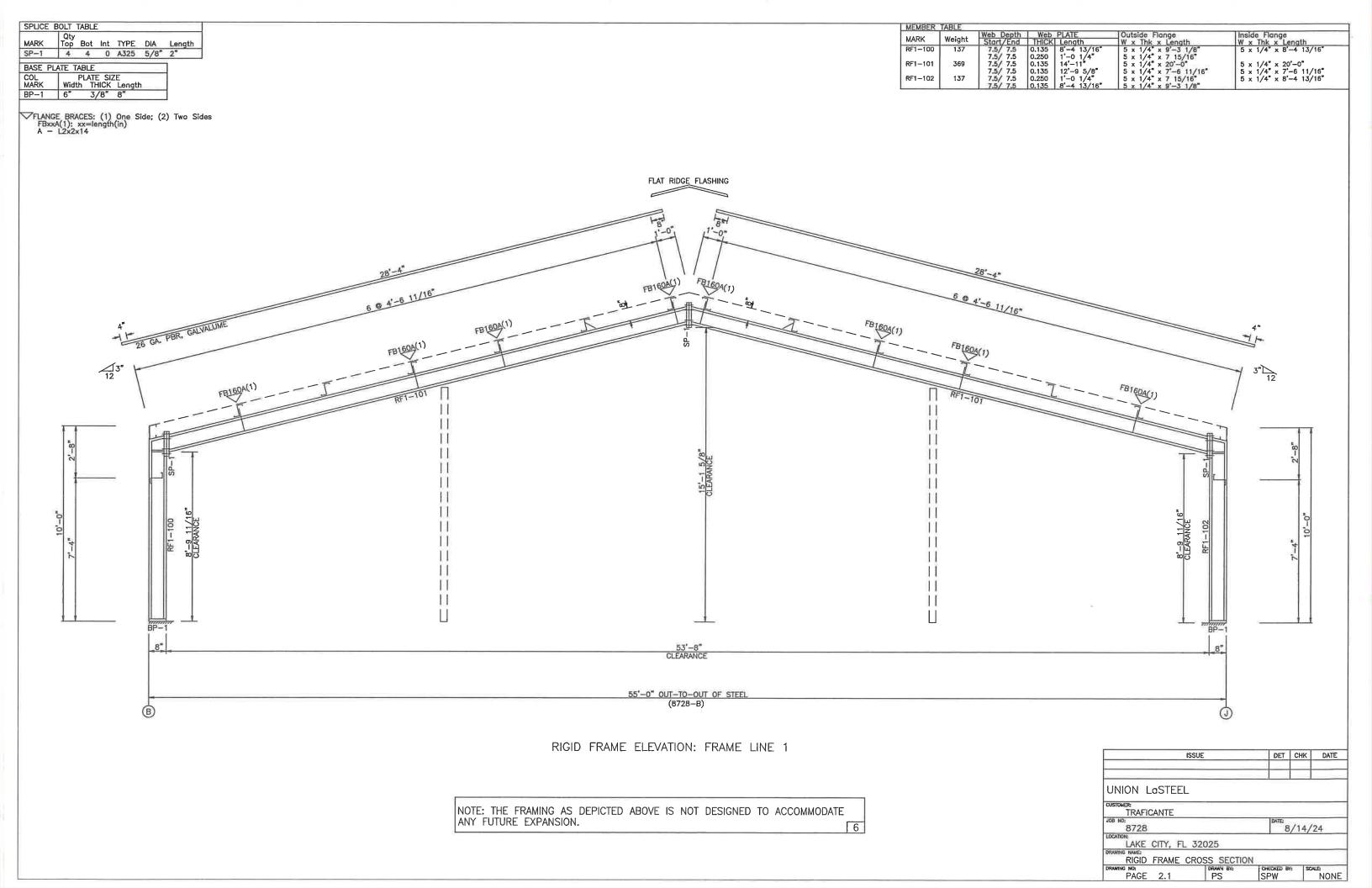


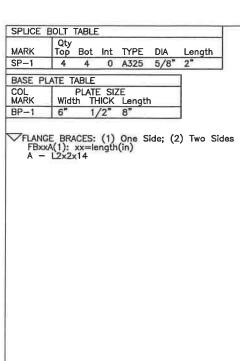
EXTENSION/CANOR	PY BOL	TS		
MARK	QUAN	TYPE	DIA	LENGTH
EB-2	4	A325	5/8"	2"
EB-3	4	A325	5′/8"	2"
EB-4	4	A325	5/8"	2"

T		T AJZ	0 0/0 2
	MEMBER ROOF P		
	MARK	PART	LENGTH
	8728-A		
	EB-1	10X2CH16	9'-3 15/16"
I)	EB-2 EB-3	W10X12 W10X12	5'-7 1/2" 5'-7 1/2"
П	EB-4	W10X12	5'-4 9/16"
	P-1	10x25Z14	27'-11 1/2"
	P-2	10x25Z16	28'-11 1/2"
	P-3	10x25Z16	27'-11 1/2"
N	P-4 P-5	10x25Z16 10x25Z16	23'-11 1/2" 24'-3 1/2"
N	P-6	10x25Z16	23'-3 1/2"
	P-7	10x25Z16	7 1/2"
	E-1	10LE14@3	23'-11 1 <i>/</i> 2"
	E-2	10LE14@3	24'-3 1/2" 23'-3 1/2"
	E-3	10LE14@3	23'-3 1/2"
	E−4 E−5	10LE14@3 10x35C14	24'-11 1/2" 24'-11 1/2"
	E-6	10LE14@3	7 1/2"
П	CB-3	1/4 CBL	33'-1"
П	CB-4	1/4 CBL	30'-1"
	CB-5	1/4 CBL	32'-9"
	CB-6	1/4 CBL	30'-4"
	8728-B P-100		11'-11 1/2"
	E-100		11'-11 1/2"
		1/4 CBL	18'-6"
	CB-101		17'-1"

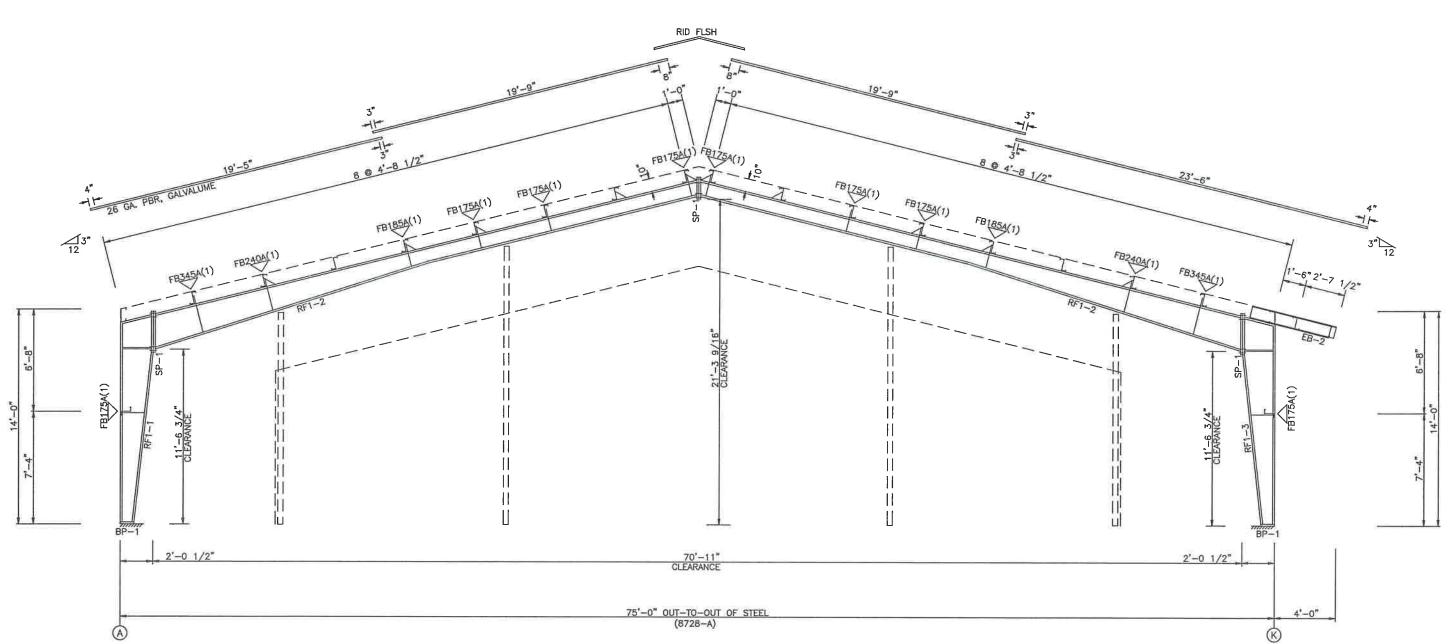
CONNECTION PLATES
ROOF PLAN
DID MARK/PART
1 FBX1

ISSUE	DET	CHK	DATE	
UNION LaSTEEL				
308 NO: 8728		DATE:	/14/	24
LAKE CITY, FL 32	025			
ROOF FRAMING LA				
DRAWING NO: PACE 2	DRAWN BY:	CHECKED B	r: SC/	NUN re:



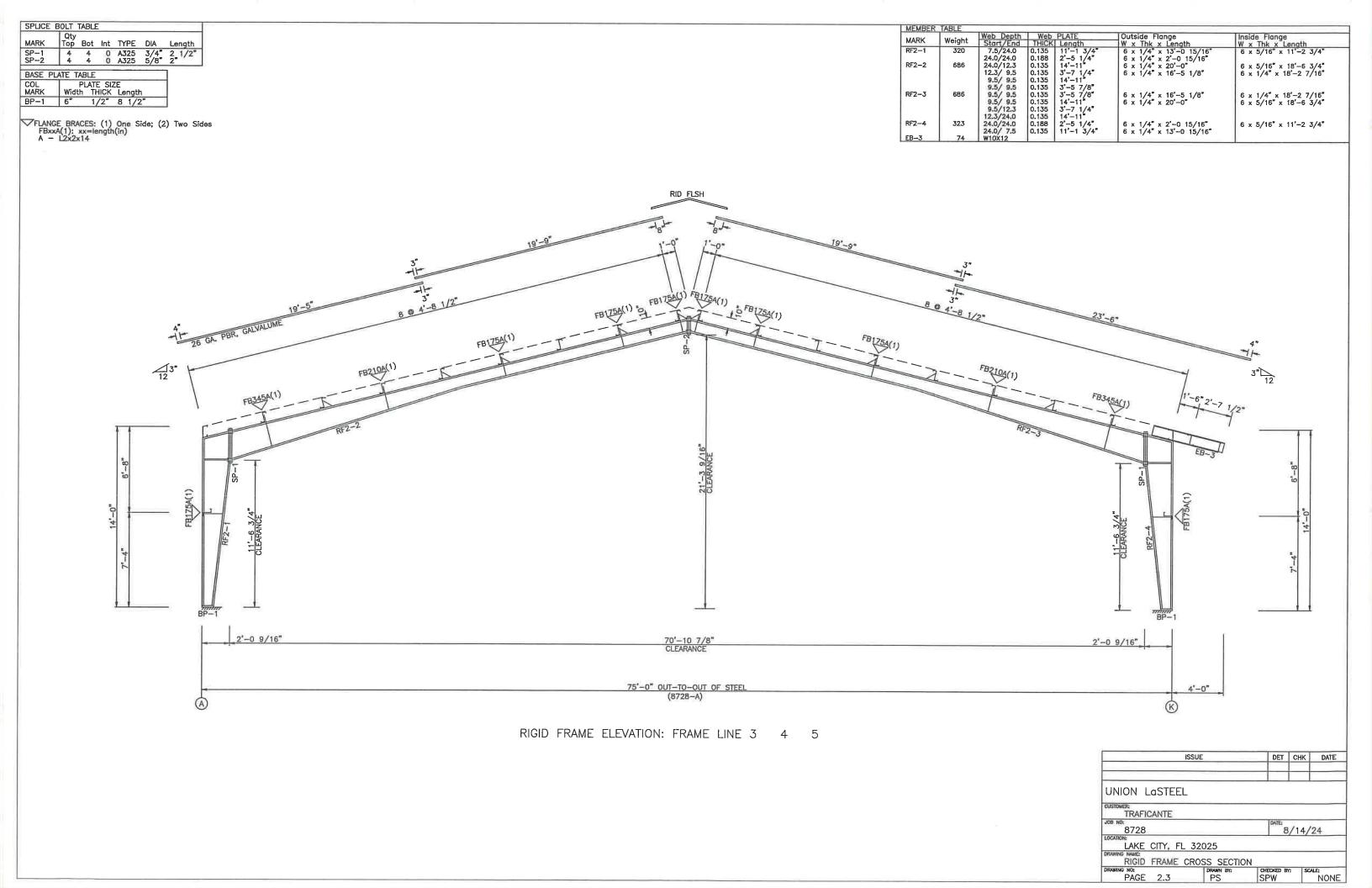


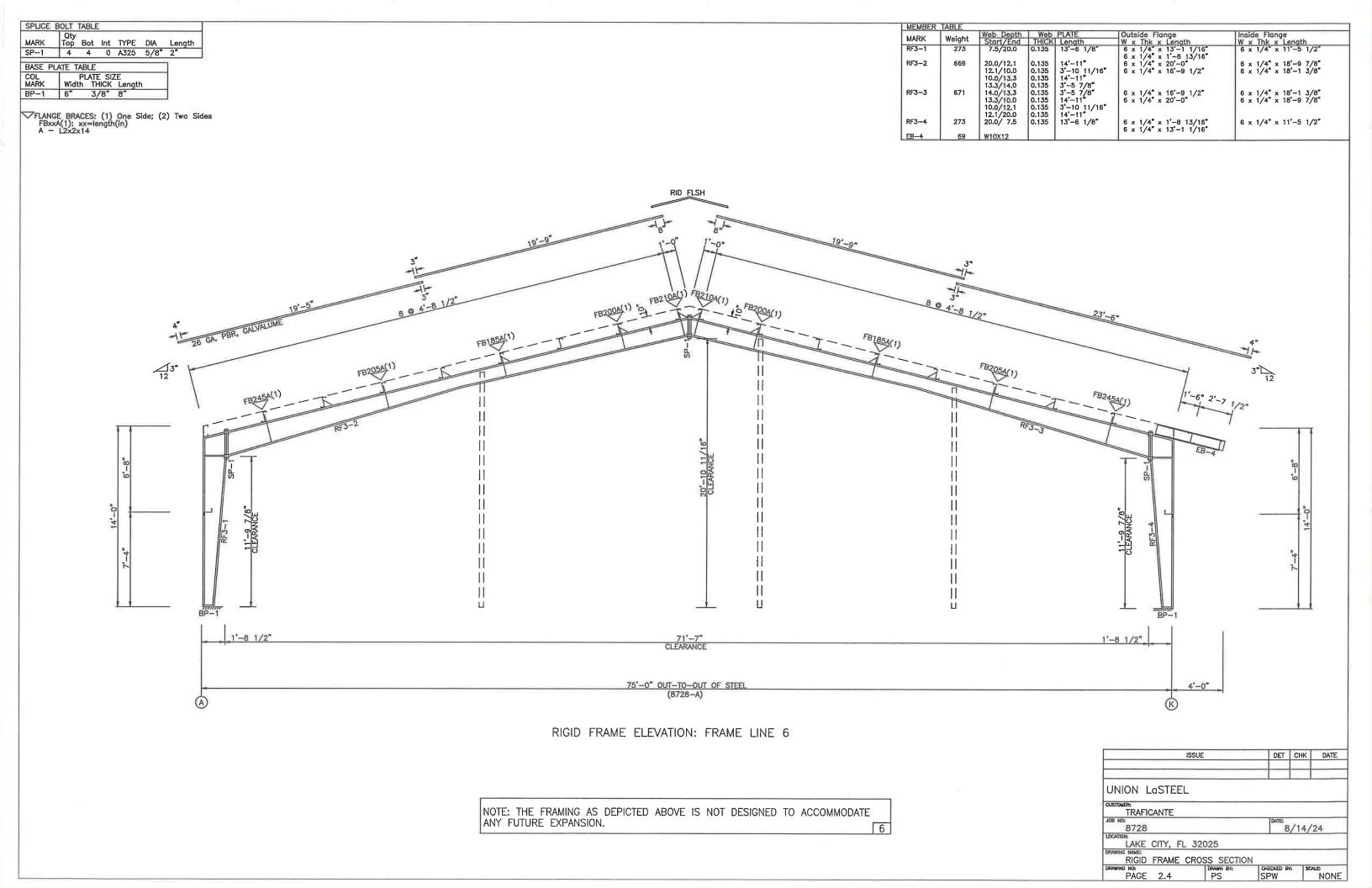
	1	Web Depth	Web	PLATE	Outside Flange	Inside Flange
MARK	Weight	Start/End	THICK	Length	W x Thk x Length	W x Thk x Length
RF1-1	285	7.5/24.0	0.135	13'-7"	6 x 1/4" x 13'-0 15/16" 6 x 1/4" x 2'-0 15/16"	6 x 1/4" x 11"-2 11/16"
RF1-2	659	24.0/12.3 12.3/ 9.5 9.5/ 9.5 9.5/ 9.5	0.135 0.135 0.135 0.135	14'-11" 3'-7 9/16" 14'-11" 3'-5 7/8"	6 x 1/4" x 20'-0" 6 x 1/4" x 16'-5 3/8"	6 x 1/4" x 18'-7" 6 x 1/4" x 18'-2 7/16"
RF1-3	285	24.0/ 7.5	0.135	13'-7"	6 x 1/4" x 2'-0 15/16" 6 x 1/4" x 13'-0 15/16"	6 x 1/4" x 11'-2 11/16"
ED-2	l 60	W10V12	1		1 '	

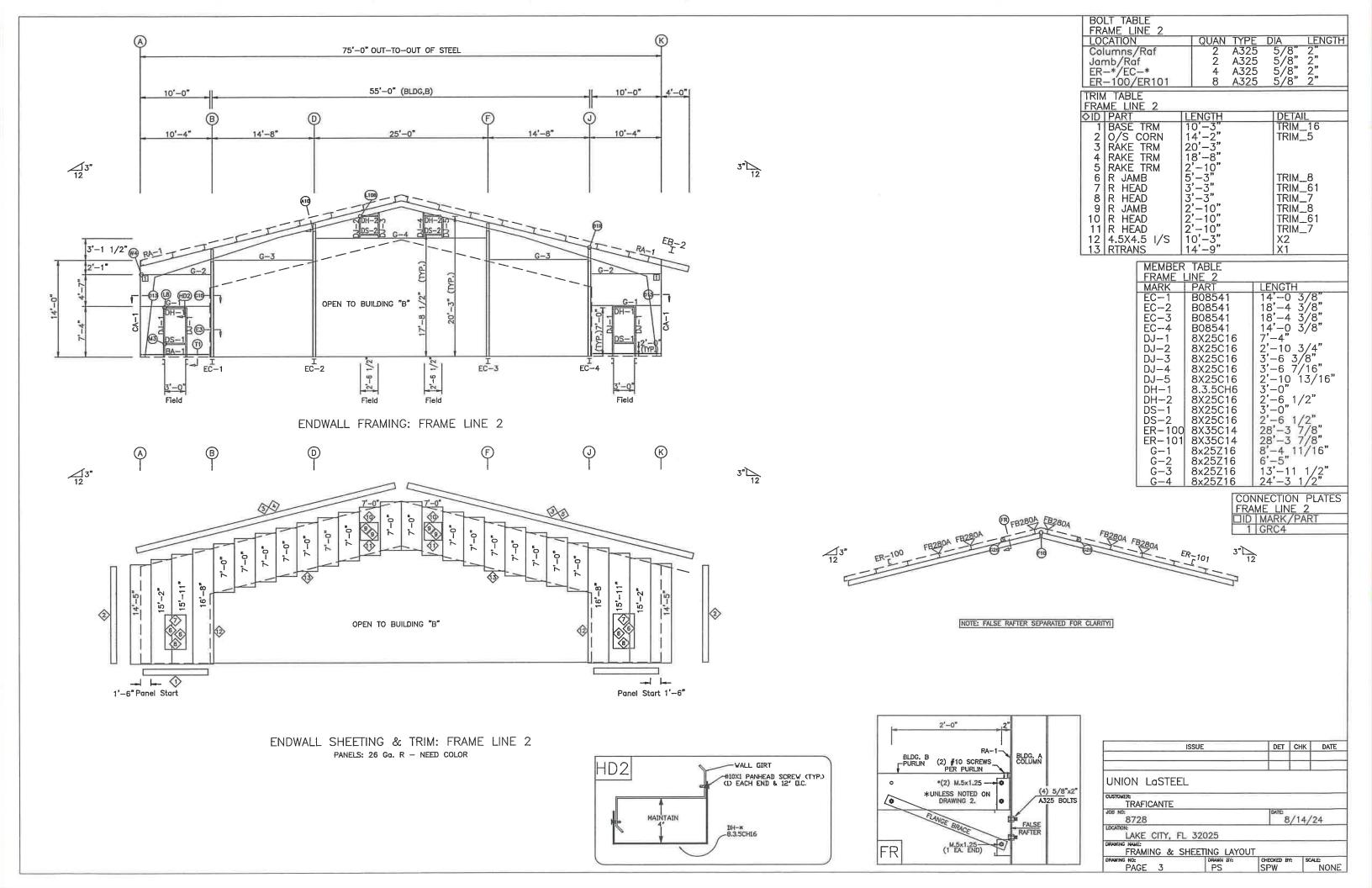


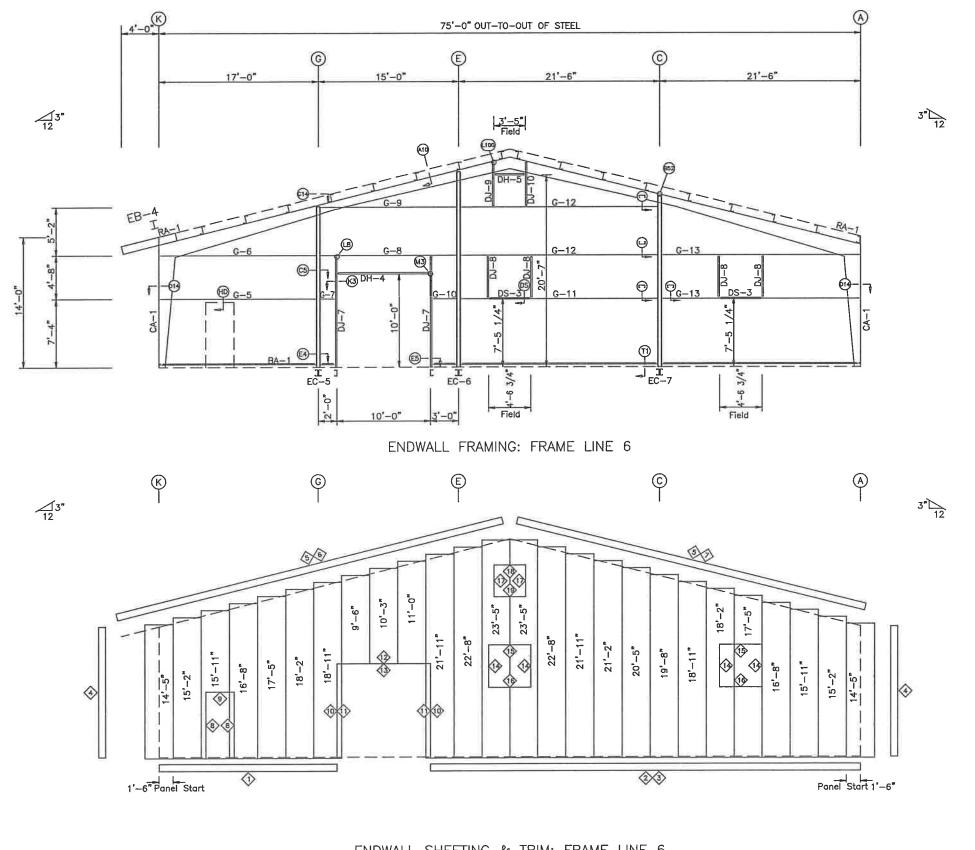
RIGID FRAME ELEVATION: FRAME LINE 2

ISSU	JE	DET	СНК	DATE
				_
JNION LaSTEEL				
TRAFICANTE				
8728		DATE:	/14/:	24
LAKE CITY, FL 3	2025			
RIGID FRAME CRO	OSS SECTIO	N		
PAGE 2.2	PS	SPW BY	: SCA	NONE









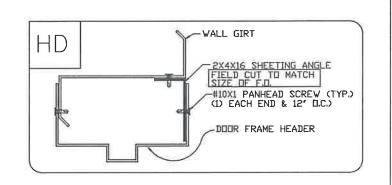


NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

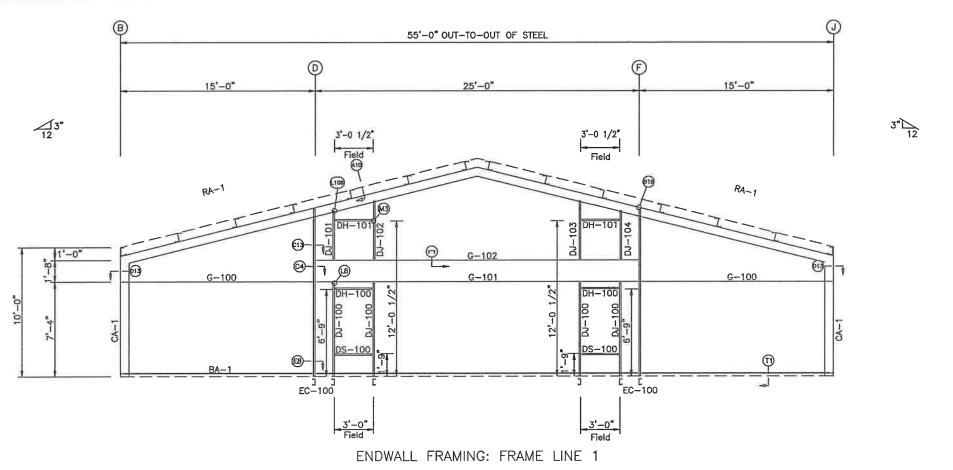
(1) EACH END & 12' D.C. -#IOXI PANHEAD SCREW (TYP.)

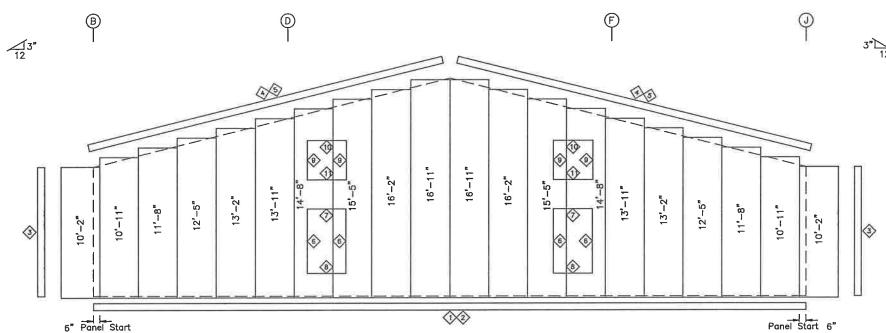
BOLT TABLE FRAME LINE 6		
LOCATION	QUAN TYPE I	DIA LENGTH
Columns/Raf	2 A325	5/8" 2"
Jamb	2 A325 2 A325	DIA LENGTH 5/8" 2" 5/8" 2"
TRIM TABLE		
FRAME LINE 6	TNOTH	TDETAIL
	ENGTH C. 7,7	DETAIL
	9'-3" 0'-3"	TRIM_16
2 BASE TRM 2 3 BASE TRM 6 4 0/S CORN 1 5 RAKE TRM 2 6 RAKE TRM 2	0'-3" '-3" 4'-2 <u>"</u>	TRIM_16
3 BASE TRM 6	_3 4' 0"	TRIM_16
4 0/S CORN 1 5 RAKE TRM 2	4 -Z	TRIM_5
DIRAKE IRM Z	7 0'-3" '-10" 8'-8" '-3"	
6 RAKE TRM 2	-10 c' c"	
7 RAKE TRM 1	5 -8	TD114 D
8 R JAMB 7	, - 3 "	TRIM_8
9 R HEAD 3	'–3"	TRIM_61
10 CT8 1		TRIM_11
11 R JAMB 1	0:-3:	TRIM_8
1 12 I C I 8 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	() -()	TRIM_10
13 R HEAD 1	0'-3" '-10"	TRIM_61
14 R JAMB 4	<u>-10"</u>	TRIM_8
15 R HEAD 4	'-10"	TRIM_61
16 R HEAD 4	'-10"	TRIM_7
17 R JAMB 3	'-8" '-8"	TRIM_8
18 R HEAD 3	'-8"	TRIM_61
19 R HEAD 3	'-8"	TRIM_7

MEMBER FRAME	TABLE LINE 6	
MARK	PART	LENGTH
EC-5 EC-6 EC-7 DJ-7 DJ-8 DJ-9 DJ-10 DH-4 DH-5 DS-3 G-5 G-7 G-8 G-9 G-10 G-11 G-12 G-13	8X7DC16 8X7DC12 8X8DC14 8X35C12 8X25C16 8X25C16 8X25C16 8X25C16 8X25C16 8X25C16 8x25Z16 8x25Z16 8x25Z16 8x25Z16 8x25Z16 8x25Z16 8x25Z16 8x25Z16 8x25Z16 8x25Z16 8x25Z16 8x25Z16 8x25Z16	17'-3 1/8" 21'-0 1/8" 18'-4 5/8" 12'-0" 4'-8" 4'-9 1/2" 10'-0" 3'-5" 4'-6 3/4" 16'-7 1/2" 16'-7 1/2" 14'-3 1/2" 14'-3 1/2" 2'-3 1/2" 20'-9 1/2" 20'-9 1/2" 21'-1 1/2"



IS	SUE	DET	CHK	DATE
UNION LaSTEEL				
CUSTOMER: TRAFICANTE				
ов NO: 8728		DATE:	/14/	/24
LAKE CITY, FL	32025			
FRAMING & SH	EETING LAYO	JT		
DRAWING NO: PAGE 3.1	DRAWN BY: PS	SPW	S	NONE





ENDWALL SHEETING & TRIM: FRAME LINE 1 PANELS: 26 Ga. R - NEED COLOR

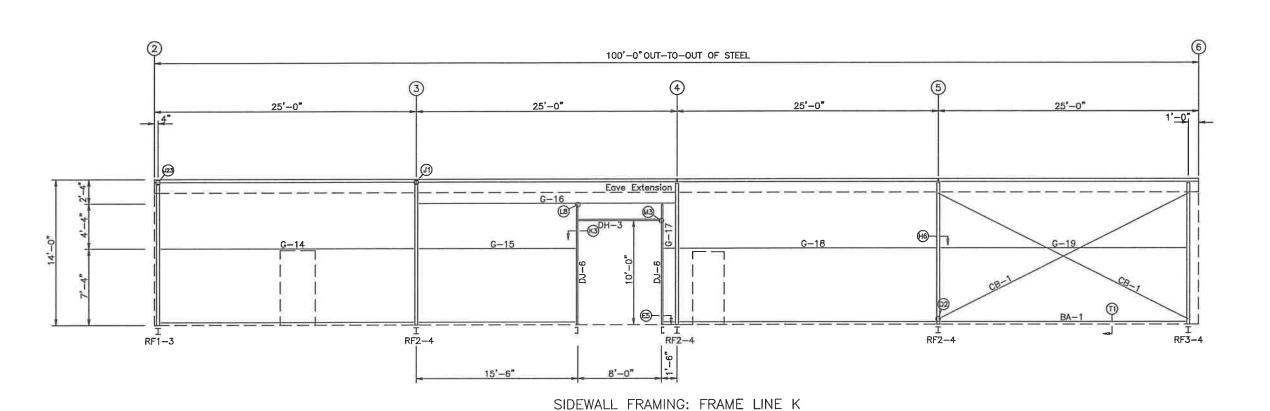
6" Panel Start

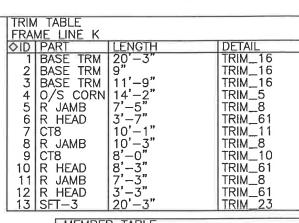
NOTE: THE FRAMING AS DEPICTED ABOVE IS NOT DESIGNED TO ACCOMMODATE ANY FUTURE EXPANSION.

BOLT TABLE FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	2	A325	5/8"	2"
Jamb	2	A325	5/8"	2
TRIM TABLE				
FRAME LINE 1	ENICTH		DETAI	1/
	<u>ENGIH</u> 0'-3"		TRIM	
	5'-3"		TRIM	
2 BASE TRM 1 3 0/S CORN 1	0'-2"		TRIM_	
4 RAKE TRM 2	0'-3"			
5 RAKE TRM 8	'-5"			
6 R JAMB 5	<u>'-3"</u>		TRIM_	
7 R HEAD 3	'-3"		TRIM_	
8 R HEAD 3	',—3",		TRIM_	
9 R JAMB 3	,-4,,		TRIM_ TRIM	
10 R HEAD 3 11 R HEAD 3	'_4"		TRIM_	
			1111111	,

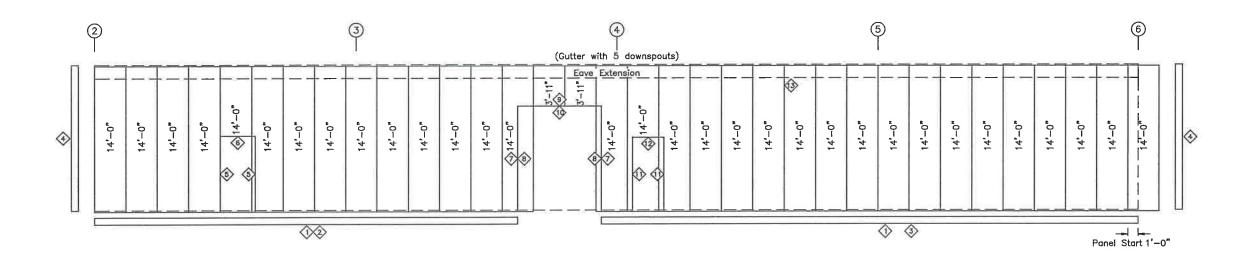
MARK PART	LENGTH
DH-101 8.3.5CH6 DS-100 8.3.5CH6 G-100 8x25Z16	12'-2 1/2" 7'-4" 3'-7" 4'-4 1/8" 4'-4" 3'-6 7/8" 3'-0 1/2" 3'-0" 13'-11 1/2" 24'-11 1/2"

IS:	SUE	DET	СНК	DATE
				_
UNION LaSTEEL				
CUSTOMER: TRAFICANTE				
VOB NO: 8728		DATE:	/14/	24
LAKE CITY, FL	32025			
FRAMING & SHI	EETING LAYO	UT		
PAGE 3.2	PS	SPW	r: SCA	NONE



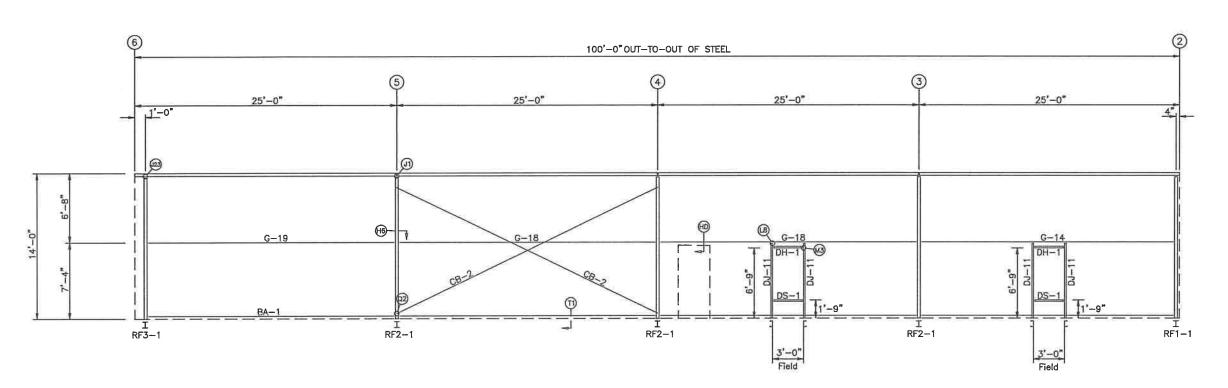


MEMBER FRAME	TABLE LINE K	
MARK	PART	LENGTH
DJ-6 DH-3 G-14 G-15	8X35C12 8X35C12 8x25Z12 8x25Z16	11'-8" 8'-0" 23'-11 1/2" 14'-9 1/2"
G-16 G-17 G-18 G-19 CB-1	8x25Z12 8x25Z16 8x25Z12 8x25Z12 5/16 CBL	24'-3 1/2" 9 1/2" 24'-3 1/2" 23'-3 1/2" 27'-7"



SIDEWALL SHEETING & TRIM: FRAME LINE K PANELS: 26 Ga. R - NEED COLOR

ISSUE	DET	СНК	DATE	
UNION LaSTEEL				
CUSTOMER: TRAFICANTE				
JOB NO: 8728		DATE 8	/14	/24
LAKE CITY, FL 320	025	-		,
DRAWING NAME: FRAMING & SHEET	ING LAYOUT			
DRAWING NO: PAGE 4	PS PS	SPW SPW	r: :	SCALE: NONE



SIDEWALL FRAMING: FRAME LINE A

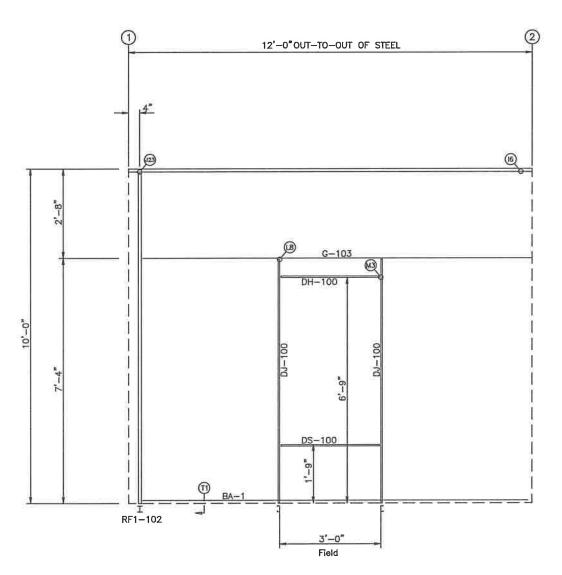
П	(a)	\							(5)						(Gut	ter witi	4 h 5 do	ownspou	ts)					ı——	3						r —		(2 	
*	14'-0"	14'-0"	14, -0"	14'-0"	14,-0"	14'-0"	14,-0"	14,-0"	14,-0"	14'-0"	14'-0"	14'-0"	14'-0"	14,-0"	14'-0"	14,-0"	14,-0"	14,-0"	\$\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	14'-0"	14'-0"	(a) (b) (c) (c) (c) (c) (c) (c) (c) (c) (c) (c	14, -0"	14,-0"	14'-0"	14,-0"	14'-0"	14,-0"	14'-0"	(a) (b) (14'-0"	6	14,-0"	14'-0"	14,-0"	*
Ш	<u> </u>	11'-0" Panel	Start						<u></u>			L											L				L				l				

SIDEWALL SHEETING & TRIM: FRAME LINE A PANELS: 26 Ga. R - NEED COLOR

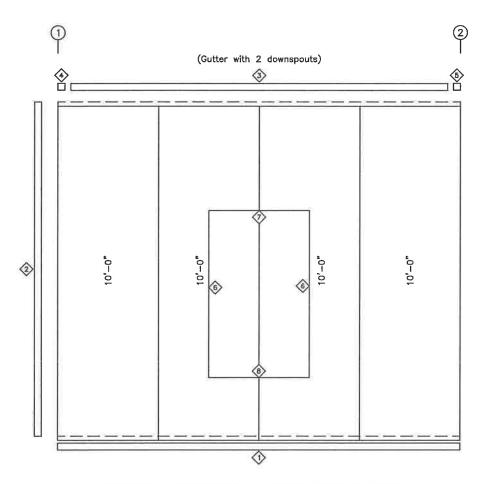
TRIM TABLE FRAME LINE A		
ØID I PART	LENGTH	DETAIL
1 BASE TRM	20'-3"	TRIM_16
2 0/S CORN 3 GUTTER	1 14' – 2"	TRIM_5
	20'-3"	TRIM_1
4 GUTEND L	1"	TRIM_2
5 GUTEND R	1	TRIM_2
6 R JAMB	5'-3"	TRIM_8
7 R HEAD	3,-3,	TRIM_61
8 R HEAD 9 R JAMB	7'_3"	TRIM_7 TRIM_8
9 K JAND	17 -3	TIVINI_O

MEMBER FRAMF	TABLE LINF A	
MARK	PART	LENGTH
DJ-11	8X25C16	7'-4"
DH-1	8X25C16	3',-0"
DS-1	8X25C16	3'-0"
G-14	8x25Z12	23'-11 1/2"
G-18	8x25Z12	24'-3 1/2"
G-19	8x25Z12	23'-3 1/2"
CB-2	1.5/16 CBL	28′-6″

IS	SUE	DET	CHK	DATE
UNION LaSTEEL				
OUSTOMER: TRAFICANTE				
NO: 8728		DATE:	/14/	/24
LAKE CITY, FL	32025			
FRAMING & SH	EETING LAYO	UT		
PAGE 4.1	PS	SPW	: 50	NONE



SIDEWALL FRAMING: FRAME LINE J

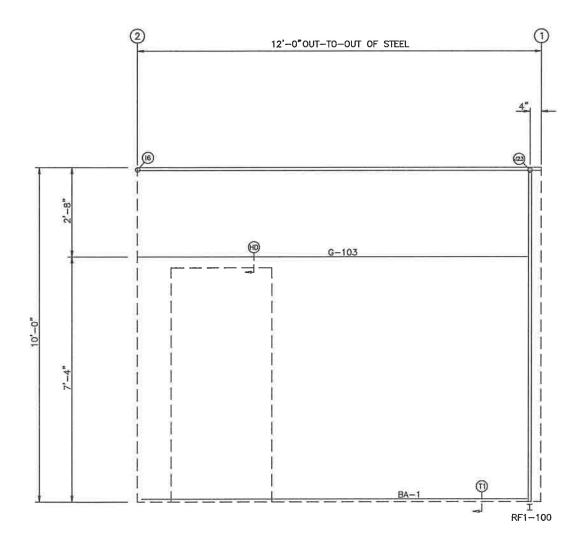


SIDEWALL SHEETING & TRIM: FRAME LINE J PANELS: 26 Ga. R - NEED COLOR

TRIM TABLE FRAME LINE J		
ØID I PART	LENGTH	DETAIL
1 BASE TRM 2 O/S CORN 3 GUTTER 4 GUTEND L 5 GUTEND R 6 R JAMB 7 R HEAD 8 R HEAD	12'-3" 10'-2" 12'-0" 1" 5'-3" 3'-3" 3'-3"	TRIM_16 TRIM_5 TRIM_1 TRIM_2 TRIM_2 TRIM_2 TRIM_8 TRIM_61 TRIM_7

MEMBER FRAME L	TABLE INE J	
MARK	PART	LENGTH
DJ-100	8X25C16	7'-4"
DH-10d	8X25C16	3'-0"
DS-100	8X25C16	3'-0"
G-103	8x25716	11'-3 1/2"

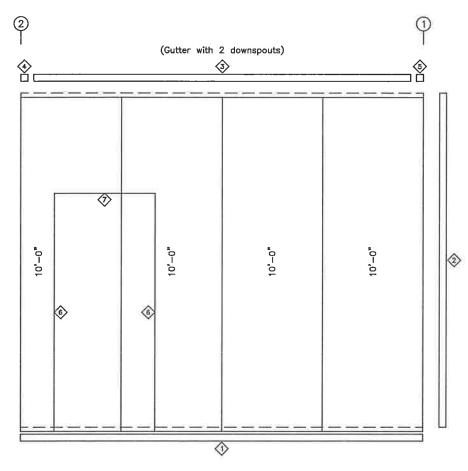
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SIDEWALL FRAMING: FRAME LINE B

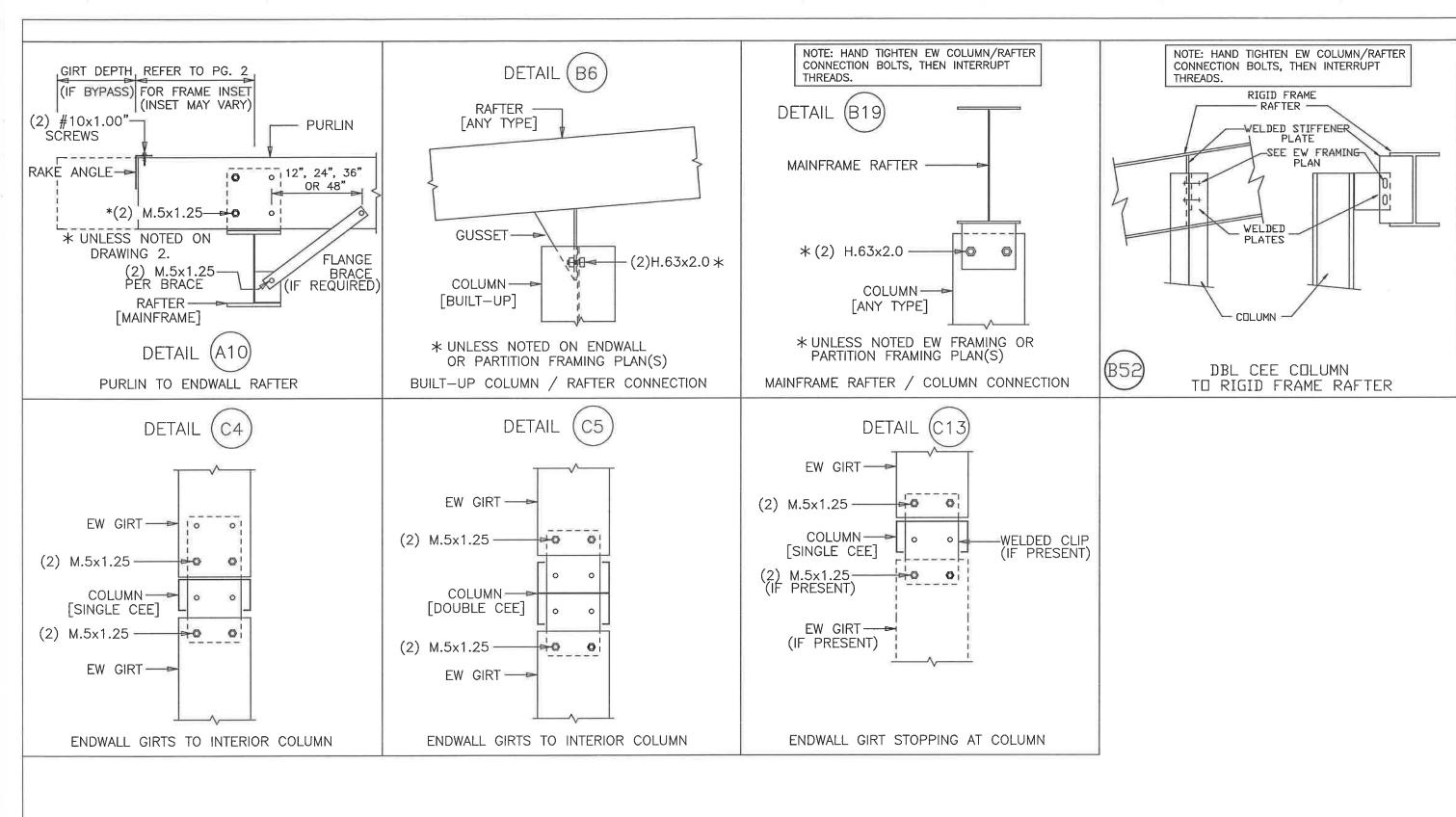
	TABLE ME LINE B		
OID	PART	LENGTH	DETAIL
1	BASE TRM	12'-3"	TRIM_16
2	O/S CORN	10'-2"	TRIM_5
3	GUTTER	12'-0"	TRIM_1
4	GUTEND L	1"	TRIM_2
5	GUTEND R	1."	TRIM_2
6	R JAMB	7'-3"	TRIM_8
7	R HEAD	3'-3"	TRIM_61

MEMBER TABLE
FRAME LINE B
MARK PART LENGTH
G-103 8x25Z16 11'-3 1/2"

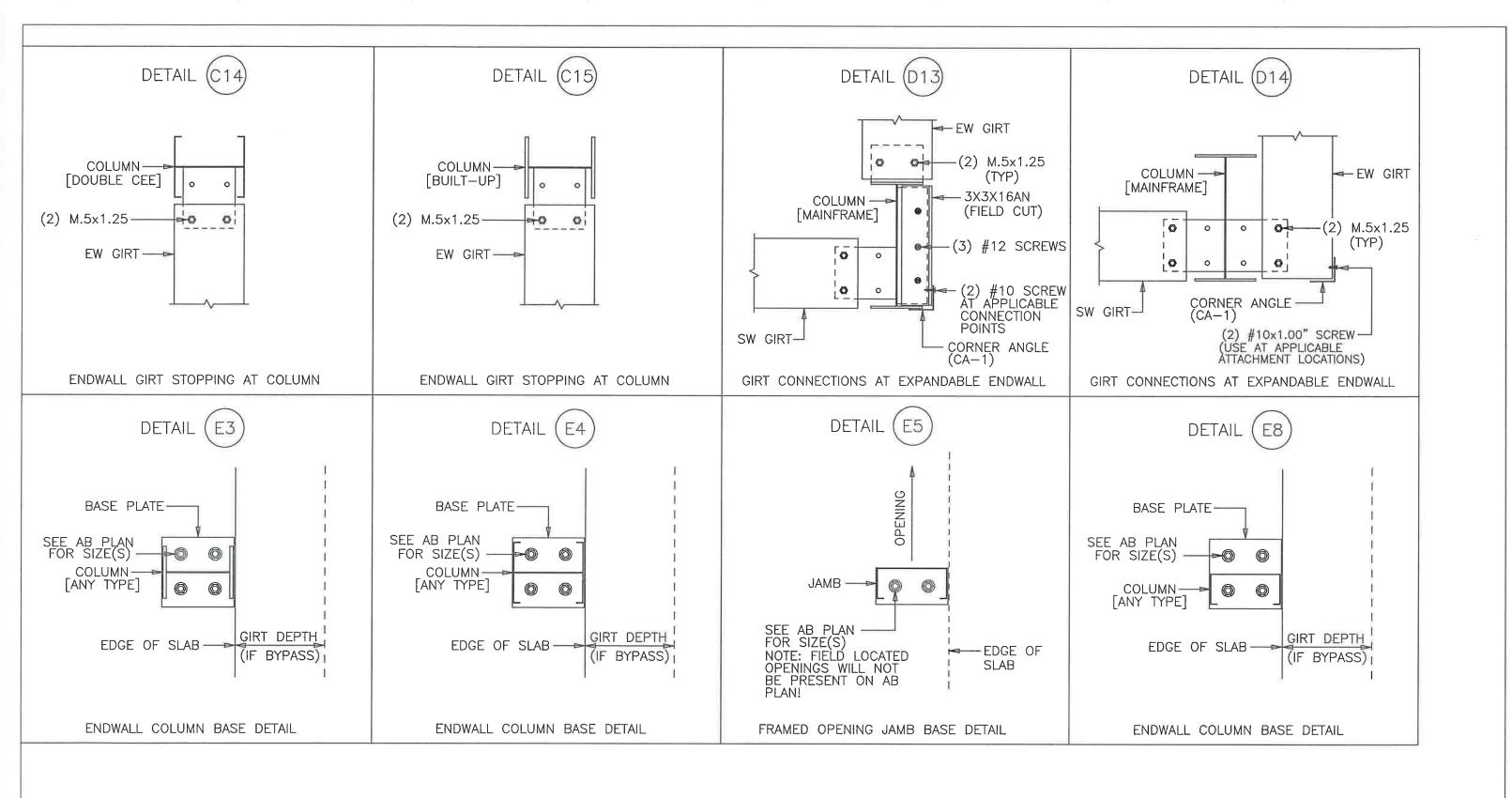


SIDEWALL SHEETING & TRIM: FRAME LINE B
PANELS: 26 Ga. R - NEED COLOR

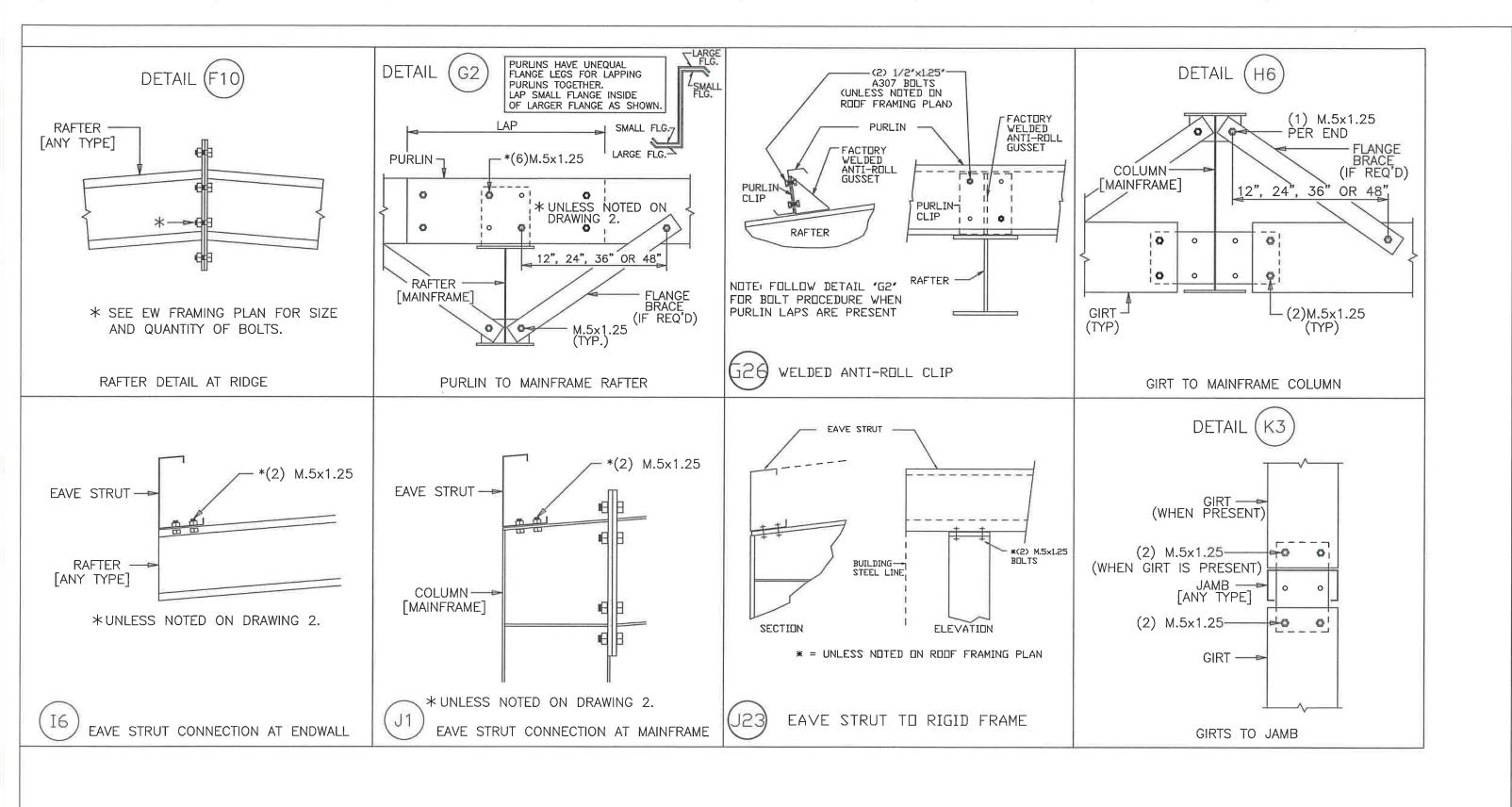
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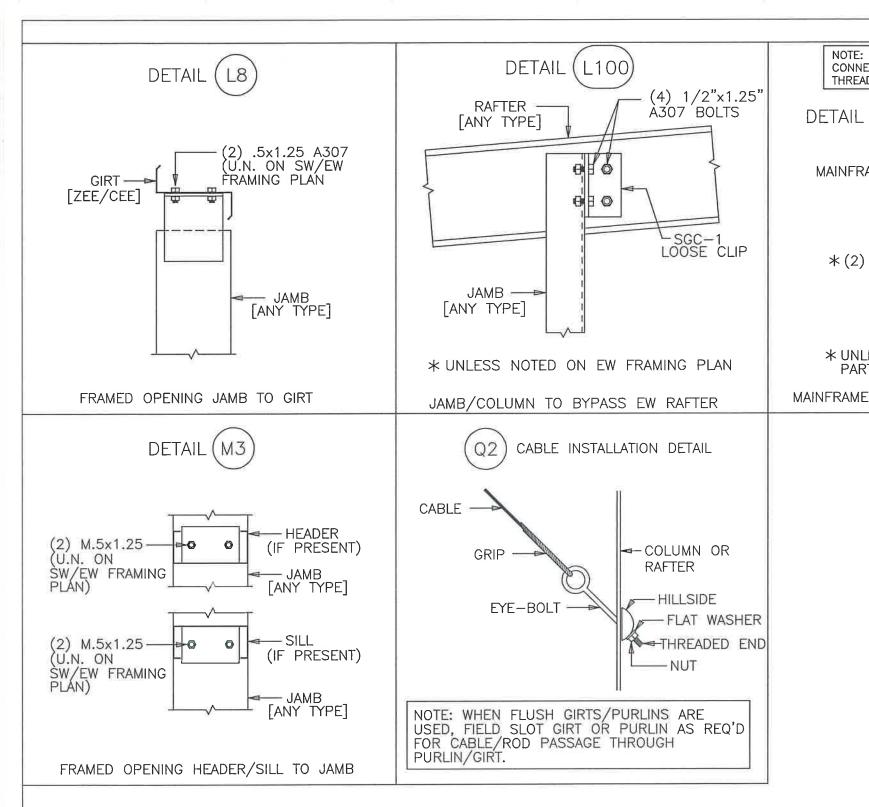
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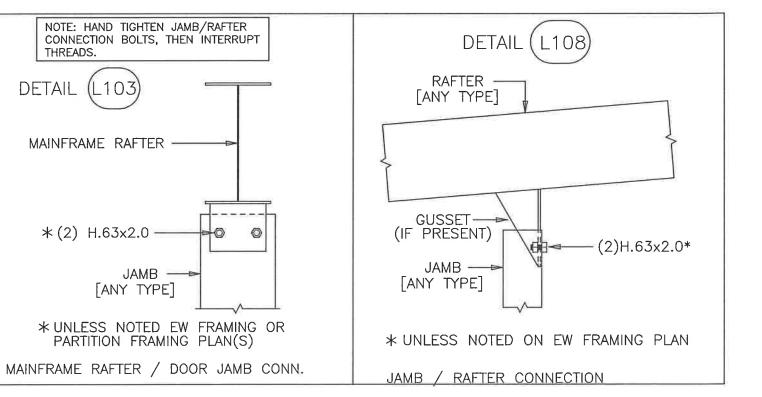


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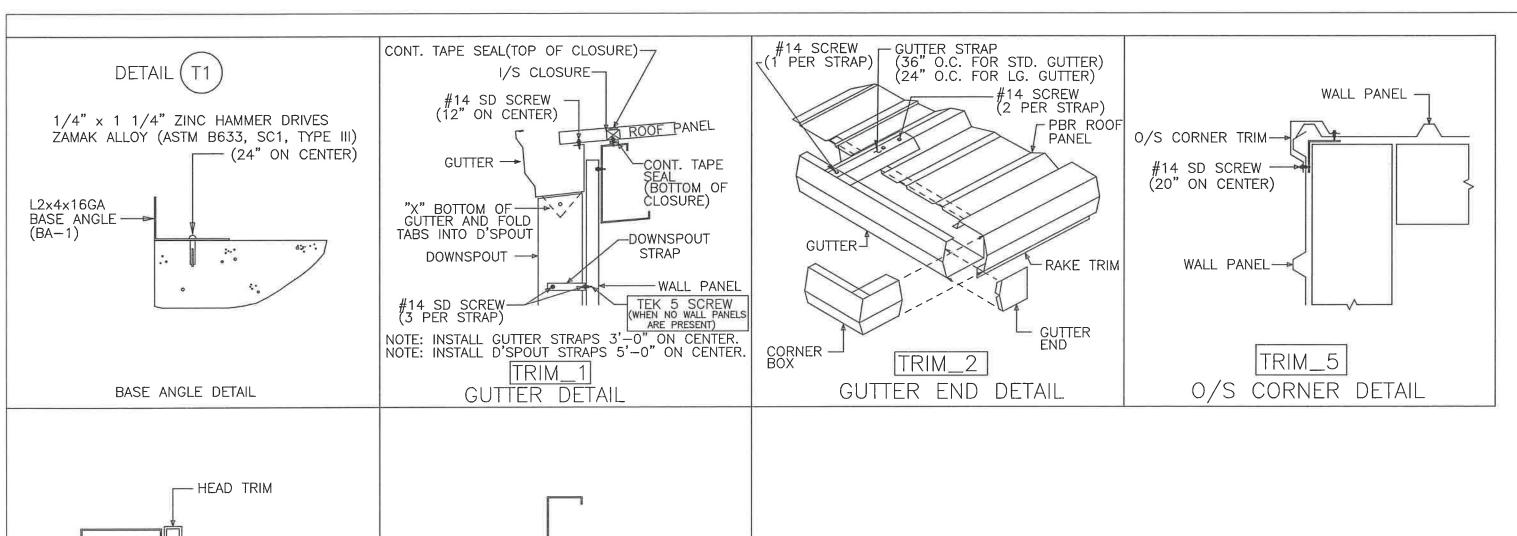


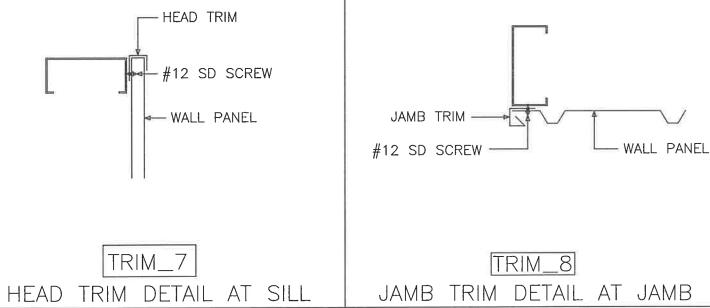
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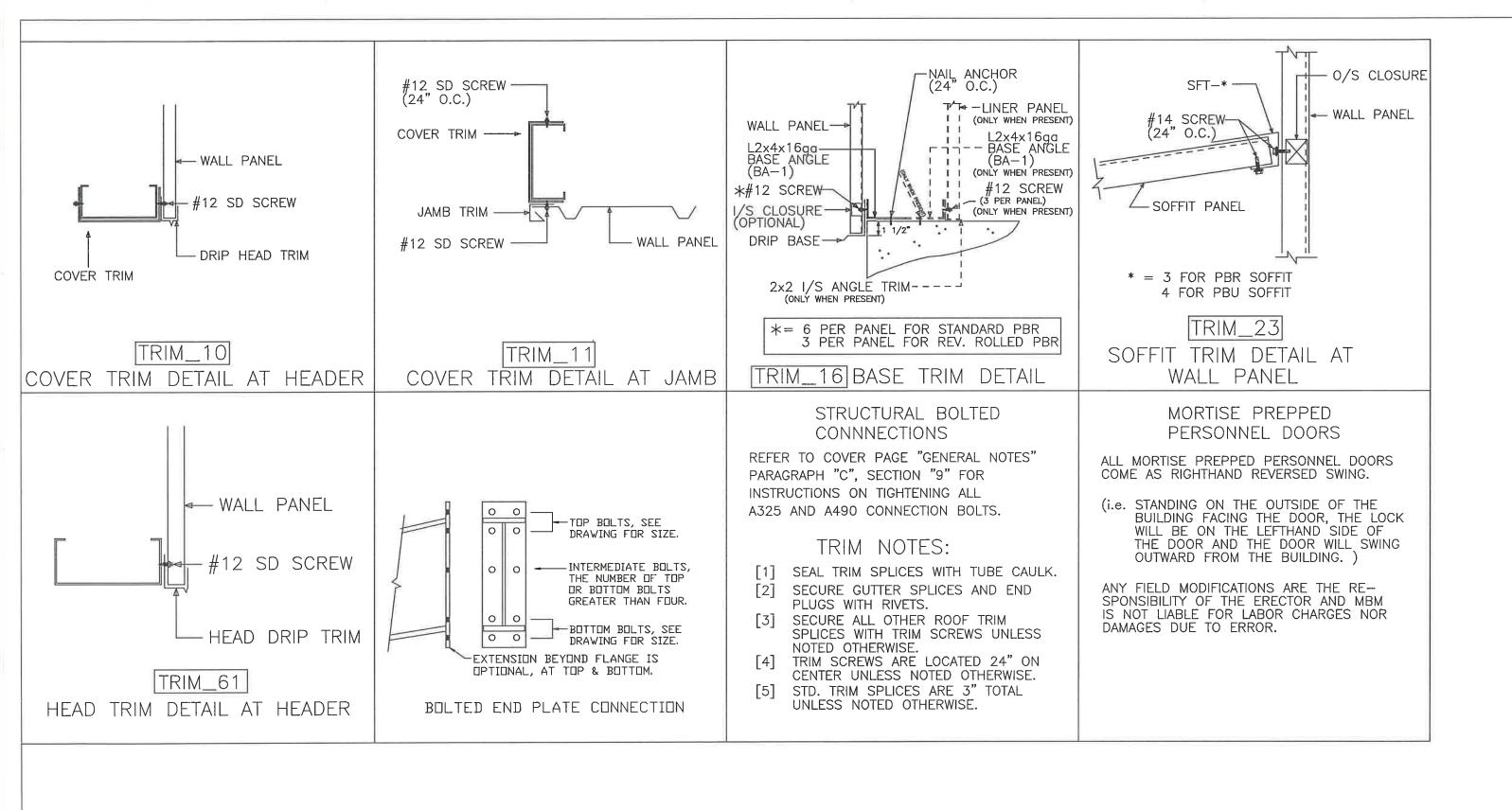


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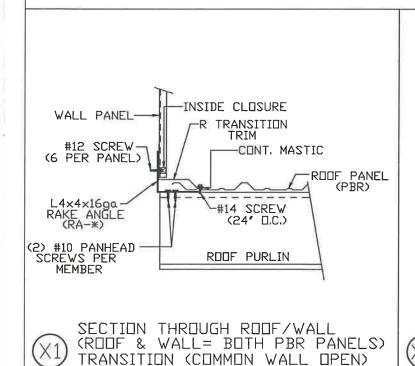


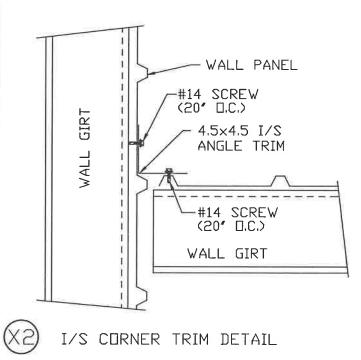


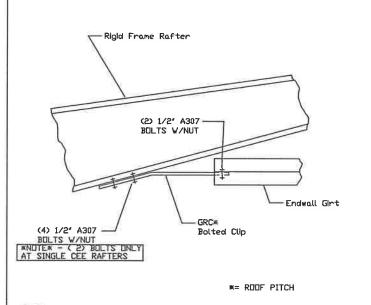
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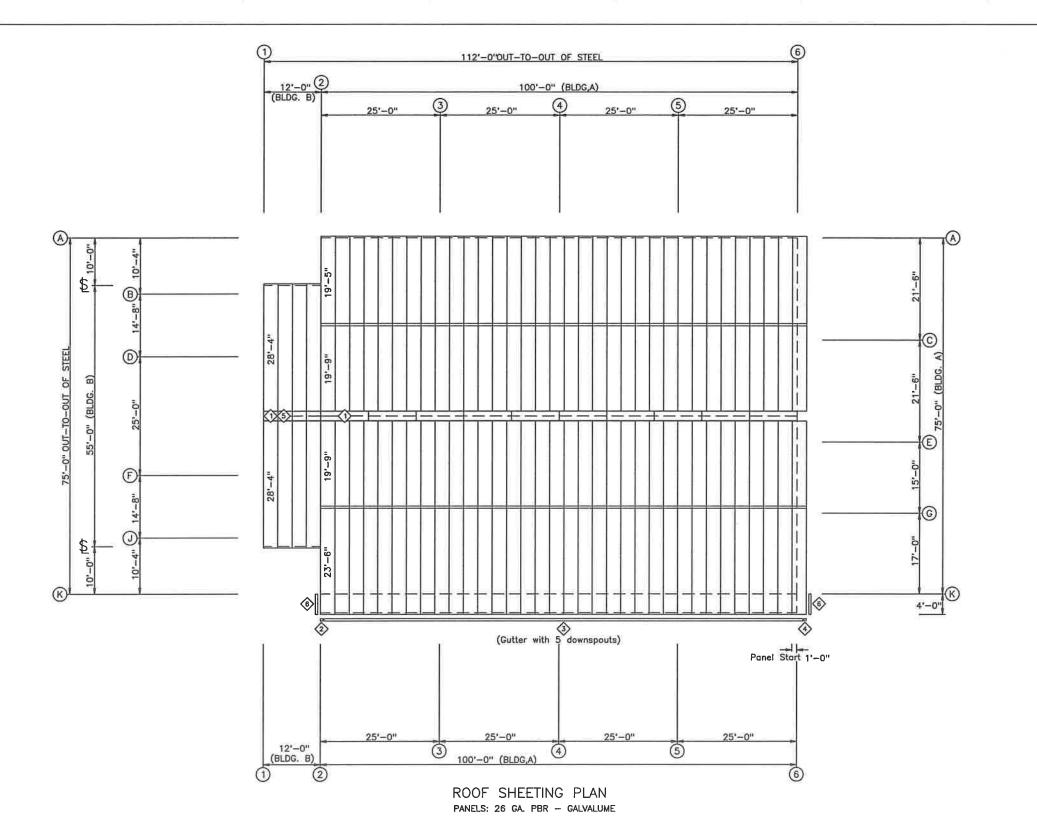
/4) SECTION OF ENDWALL GIRT TO RAFTER

GENERAL SKYLIGHT NOTES

- 1) LTP'S (LIGHT TRANSMITTING PANELS) SHALL NOT BE LOCATED END—TO—END AND SHALL NOT BE LOCATED SIDE—TO—SIDE. IT IS RECOMMENDED THAT A MINIMUM OF (3) METAL ROOF PANELS BE INSTALLED BETWEEN EACH LTP SIDELAP.
- 2) METAL ROOF PANELS ARE NOT TO BE INSTALLED USING A SIMPLE—SPAN CONDITION. IT IS RECOMMENDED THAT A MIN. OF (3) ROOF FRAMING MEMBERS, PREFERABLY (4), SUPPORT ALL METAL ROOF PANELS WHERE ROOF PANELS HAVE BEEN CUT (FACTORY OR FIELD) TO ALLOW FOR LTP INSTALLATION.
- 3) LTP'S SHALL NOT BE INSTALLED EAVE—TO—PEAK OR EAVE—TO—EAVE (ONE LTP PER SINGLE "RUN" OF SHEETING).
- 4) BUILDINGS WITH LESS THAN ~60'-0" OF ROOF PANELS IN A SINGLE "RUN" TYPICALLY ARE ONLY ALLOWED (1) LTP PER "RUN".
- 5) ANY INSTALLATIONS FOR LTP'S OUTSIDE OF THESE GUIDLINES AND THE DETAILS PROVIDED BY THE M.B.M. REMOVE SAID M.B.M. FROM ANY LIABILITIES OR FAULTS DESPITE CLAIMS AGAINST M.B.M.
- 6) FOR ALL LTP'S PROVIDED BY M.B.M., LIGHT STONE FASTENERS WILL BE PROVIDED FOR A CLOSE MATCH TO LTP COLOR. THIS INCLUDES WALL & ROOF LTP'S.
- 7) DO <u>NOT</u> STEP ON LTP'S ONCE THEY HAVE BEEN INSTALLED! STEPPING ON LPT'S AFTER INSTALLATION MAY RESULT IN INJURY OR DEATH!

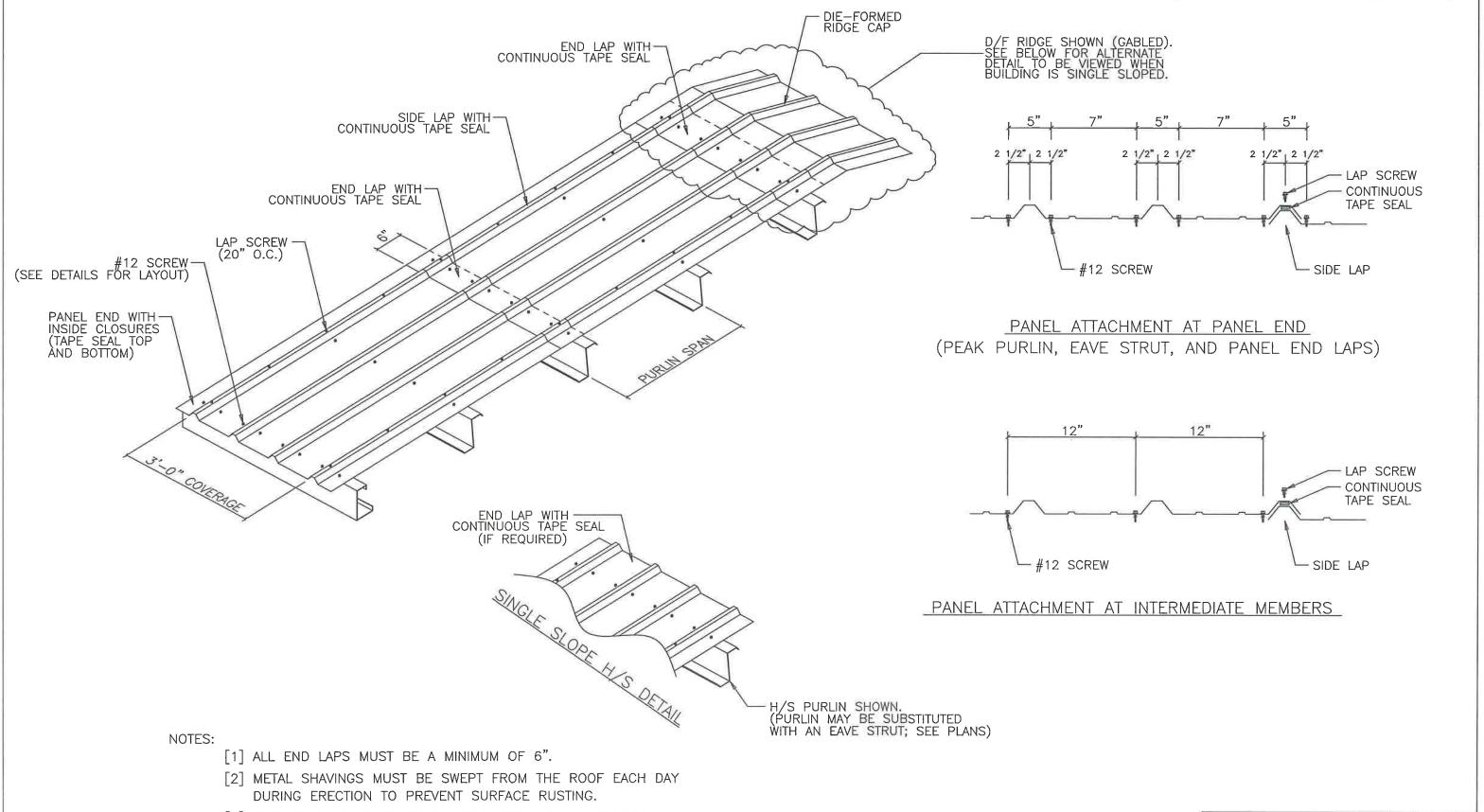
BUILT-UP MEMBER LEGEND					
BEAM TYPE	BEAM DEPTH	FLANGE WIDTH	FLANGE THK.	WEB THK.	
B	08	5	4		
BUILT-UP	08= 8" 10= 10" 12= 12" 14= 14" ETC.	5,6,8,10 OR 12 (INCHES)	MEASURED IN 16ths. (4= 1/4", 5= 5/16" ETC.)	1= 10ga 3= 3/16" ETC.	

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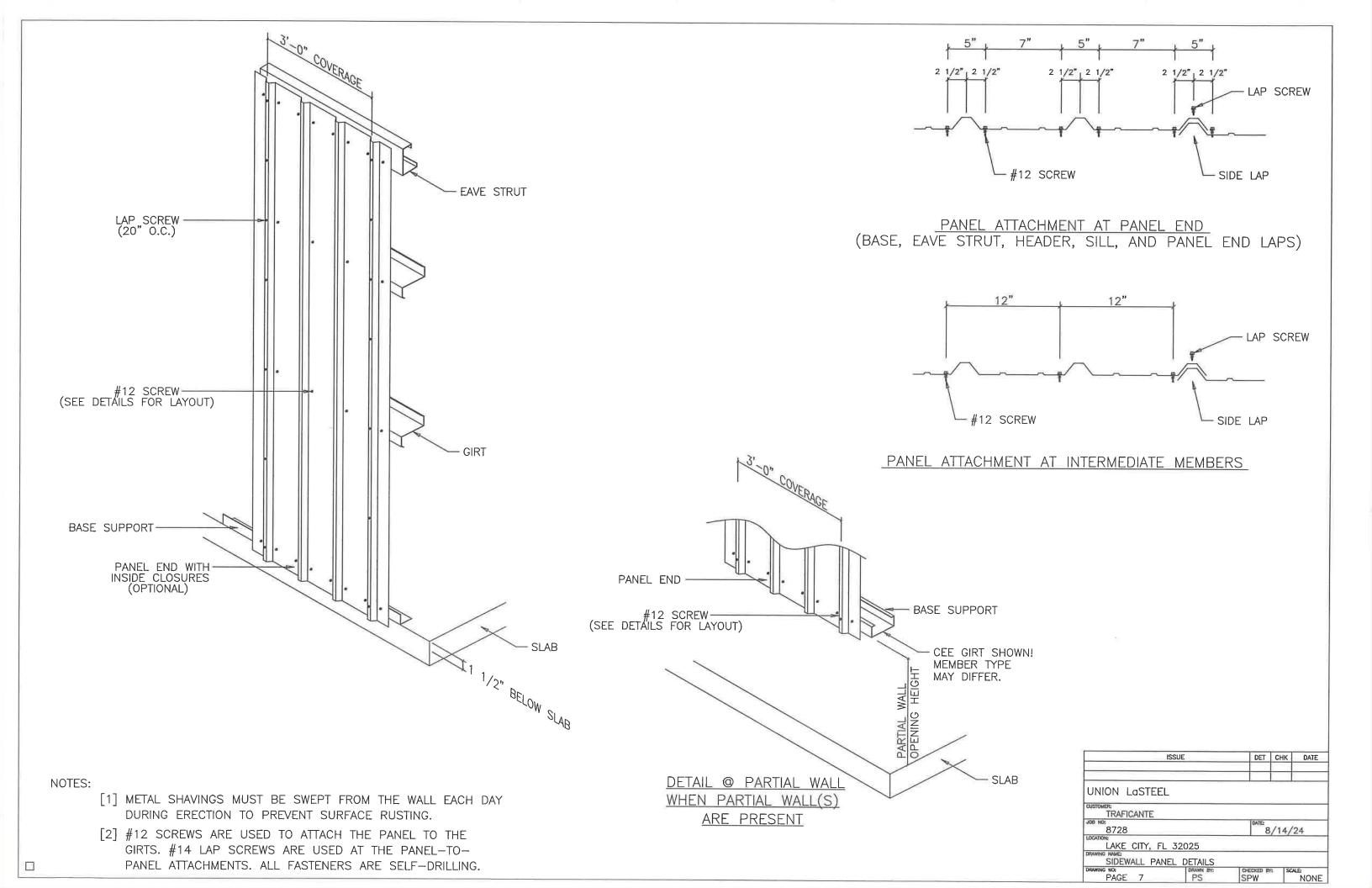
	TRIM TABLE ROOF PLAN						
1	♦ID	PART	LENGTH	DETAIL			
		RID FLSH	10'-3"				
	2	GUTEND L GUTTER	1"	TRIM_2			
	3	GUTTER	20'-3"	TRIM_1			
	4	GUTEND R	1"	TRIM_2			
	5	RID FLSH	2'-0"				
	6	RST10	4'-5"	TRIM_24			

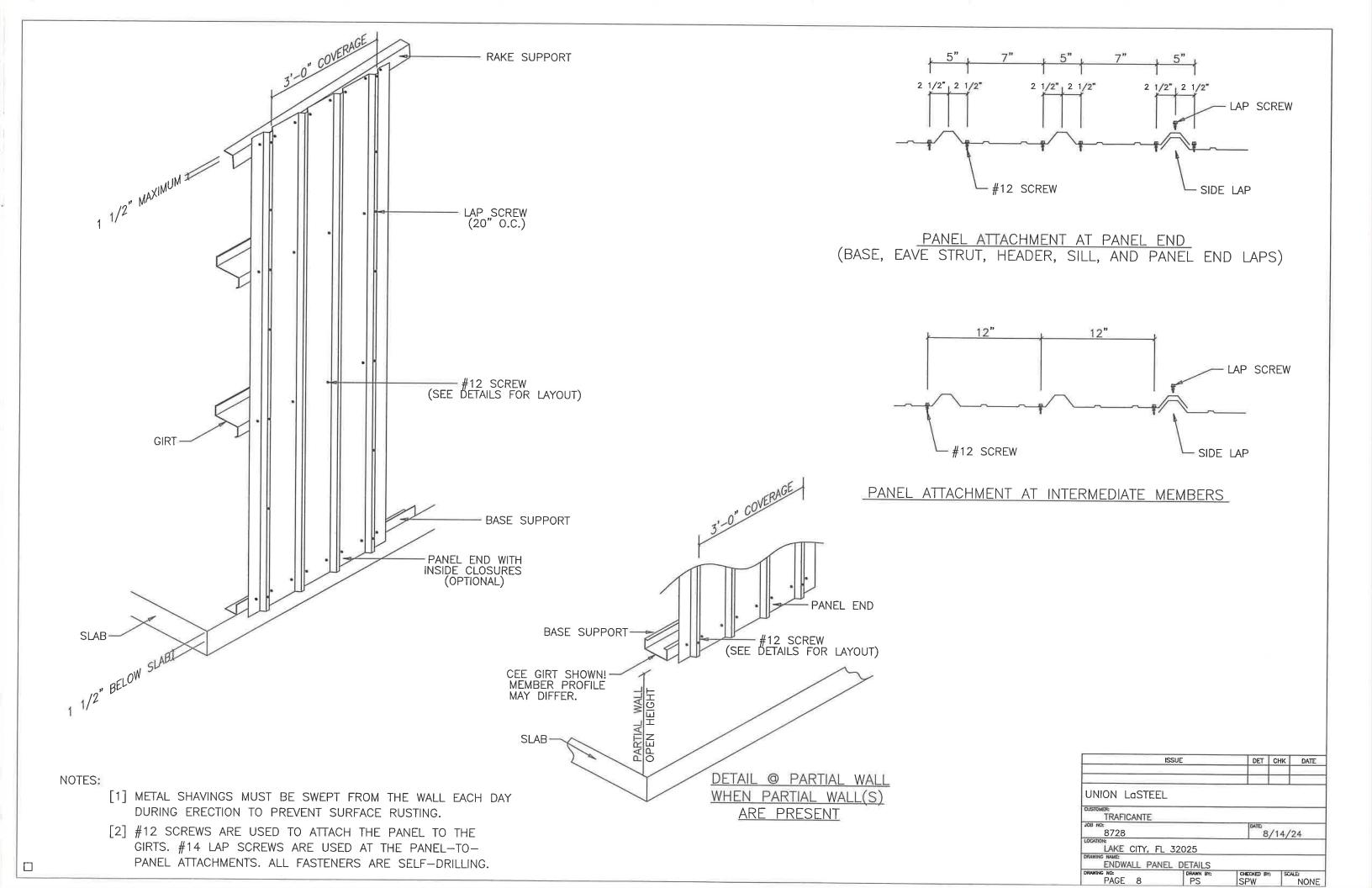
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ROOF PANELS & TRIM		
PAGE 6 PS	CHECKED BY	SCALE: NONE

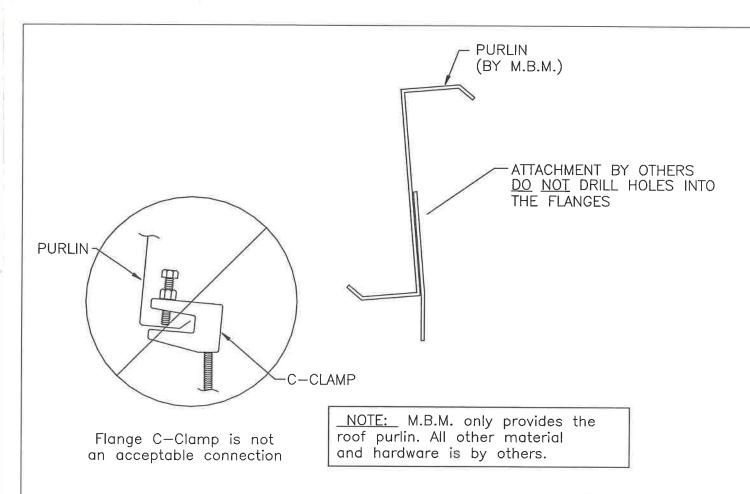


- [3] TAPE SEAL MUST BE APPLIED WITH NO GAPS OR BREAKS.
- [4] #12 SCREWS ARE USED TO ATTACH THE PANEL TO THE PURLINS. #14 LAP SCREWS ARE USED AT THE PANEL—TO—PANEL ATTACHMENTS. ALL FASTENERS ARE SELF—DRILLING.

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PANEL DETA	AILS			
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Recommended Connection Detail

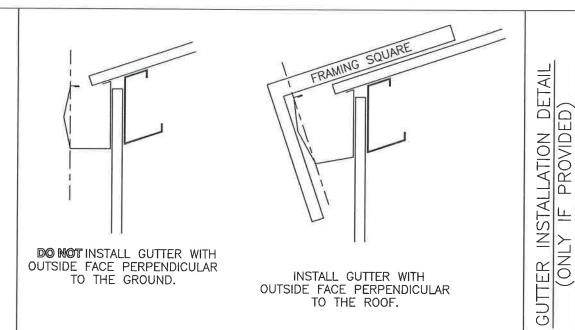
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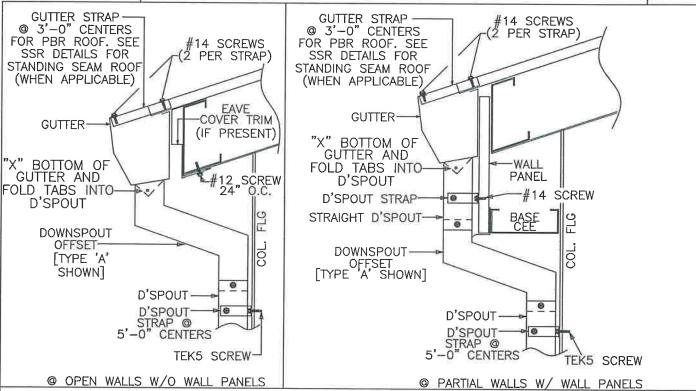
MANY FACTORS BEYOND THE CONTROL OF THE METAL BUILDING SUPPLIER AFFECT THE ABILITY OF A PURLIN TO SAFELY SUPPORT HANGING LOADS COMBINED WITH OTHER REQUIRED ROOF LOADS. DUE TO THE VARIABLES INVOLVED IN HANGING LOADS AND THEIR ATTACHMENTS TO THE PURLINS, THE METAL BUILDING SUPPLIER CANNOT ASSURE THAT THE PURLINS FOR A PARTICULAR BUILDING PROJECT CAN SAFELY SUPPORT THE MAXIMUM ALLOWABLE HANGING LOADS IN COMBINATION WITH OTHER ROOF LOADS.

IT IS THE RESPONSIBILITY OF THE HANGER SYSTEM INSTALLER TO COORDINATE WITH THE ENGINEER OF RECORD FOR THE OVERALL PROJECT TO ENSURE A SAFE HANGING LOAD INSTALLATION. THE METAL BUILDING ENGINEER IS NOT THE ENGINEER OF RECORD FOR THE OVERALL PROJECT. WITHOUT SPECIFIC CERTIFICATION FOR INDIVIDUAL HANGING LOADS, THE NET EFFECTS OF APPLIED HANGER LOADS INSTALLED ON A PARTICULAR PURLIN SHALL NOT EXCEED THE NET EFFECTS OF THE CERTIFIED UNIFORMLY APPLIED DESIGN COLLATERAL LOAD.

HANGING LOADS SHOULD NOT BE APPLIED TO THE PURLIN LIP. WHERE PERMISSIBLE, THE BEST PRACTICE FOR HANGING LOADS IS TO ATTACH TO THE PURLIN WEB USING A BOLT AND NUT, OR SELF-DRILLING SCREWS.

HANGING UNIFORM LOADS SUCH AS SPRINKLER MAINS OR HVAC EQUIPMENT SHOULD BE DISTRIBUTED OVER SEVERAL PURLINS, AND SHOULD NEVER EXCEED THE COLLATERAL LOAD ALLOWANCE FOR THE ROOF SYSTEM. FOR UNIFORM LOADS THAT RUN PARALLEL TO THE PURLINS, IT MAY BE NECESSARY TO USE TRANSVERSE SUPPORT CHANNELS (A.KA. TRAPEZE BEAMS) ATTACHED TO THE WEBS OR FLANGES OF ADJACENT PURLINS TO SPREAD THE LOAD BETWEEN TWO OR MORE PURLINS. IN SUCH CASES, CONTACT THE BUILDING MANUFACTURER OR A LOCAL PROFESSIONAL ENGINEER PRIOR TO ATTEMPTING TO HANG LOADS FROM THE PURLINS





NOTE: REGARDLESS OF DOWNSPOUT OFFSET SCENARIO, TEK5 SCREWS MUST BE USED TO ATTACH DOWNSPOUT STRAPS TO PEMB FRAMING. WHEN WALL PANELS SPAN FROM GROUND TO EAVE (FULL SPAN), #14 SCREWS WILL BE USED TO ATTACH DOWNSPOUT STRAPS TO WALL PANELS.

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