

APPLICANT

LARRY COLE

PHONE

352 472-6850

ADDRESS

25370NW 8TH PLACE

NEWBERRY

FL

32669

OWNER

JEANETTE STEEDLEY

PHONE

752-4345

ADDRESS

272SW SMITH LANE

LAKE CITY

FL

32024

CONTRACTOR

TIMBERLAKE ALUMINUM

PHONE

352 472-6850

LOCATION OF PROPERTY

47S, TR ON SMITH AVE, 4TH LOT ON LEFT

TYPE DEVELOPMENT

SCREEN ENCLOSURE

ESTIMATED COST OF CONSTRUCTION

9782.00

HEATED FLOOR AREA

TOTAL AREA

HEIGHT

STORIES

FOUNDATION

WALLS

ROOF PITCH

FLOOR

LAND USE & ZONING

A-3

MAX. HEIGHT

Minimum Set Back Requirments:

STREET-FRONT

REAR

SIDE

NO. EX.D.U.

FLOOD ZONE

N/A

DEVELOPMENT PERMIT NO.

PARCEL ID

36-4S-16-03336-000

SUBDIVISION

LOT

BLOCK

PHASE

UNIT

TOTAL ACRES

SCC056710

Culvert Permit No.

Culvert Waiver

Contractor's License Number

Applicant/Owner/Contractor

EXISTING

X08-190

CS

JH

N

Driveway Connection

Septic Tank Number

LU & Zoning checked by

Approved for Issuance

New Resident

COMMENTS:

NOC ON FILE

Check # or Cash1676

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power

Foundation

Monolithic

date/app. by

date/app. by

date/app. by

Under slab rough-in plumbing

Slab

Sheathing/Nailing

date/app. by

date/app. by

date/app. by

Framing

Rough-in plumbing above slab and below wood floor

date/app. by

date/app. by

Electrical rough-in

Heat & Air Duct

Peri. beam (Lintel)

date/app. by

date/app. by

date/app. by

Permanent power

C.O. Final

Culvert

date/app. by

date/app. by

date/app. by

M/H tie downs, blocking, electricity and plumbing

Pool

date/app. by

date/app. by

Reconnection

Pump pole

Utility Pole

date/app. by

date/app. by

date/app. by

M/H Pole

Travel Trailer

Re-roof

date/app. by

date/app. by

date/app. by

BUILDING PERMIT FEE \$

50.00

CERTIFICATION FEE \$

0.00

SURCHARGE FEE \$

0.00

MISC. FEES \$

0.00

ZONING CERT. FEE \$

50.00

FIRE FEE \$

0.00

WASTE FEE \$

FLOOD DEVELOPMENT FEE \$

FLOOD ZONE FEE \$

CULVERT FEE \$

TOTAL FEE

100.00

INSPECTORS OFFICE

CLERKS OFFICE

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGRESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

**NOTICE OF COMMENCEMENT**

**STATE OF FLORIDA COUNTY OF** Columbia **CITY OF** Lake City

THE UNDERSIGNED hereby gives notice that improvement(s) will be made to certain real property, and in accordance with Chapter 713, Florida Statutes, the following information is provided in this Notice of Commencement.

**DESCRIPTION OF PROPERTY:**

LOT r/A BLOCK - SECTION - TOWNSHIP - RANGE -

TAX PARCEL # 36 42 16 03336-0004x

SUBDIVISION: -

PLATBOOK: - MAP PAGE# -

STREET ADDRESS: 272 Smith Lane  
Lake City FL 32024

**GENERAL DESCRIPTION OF IMPROVEMENT:**

TO CONSTRUCT: Screen Enclosure

**OWNER INFORMATION:**

OWNER(S) NAME: Steedley, Jeanette

ADDRESS: 272 Smith Lane PHONE 752 4345

CITY: Lake City FL STATE FL ZIP 32024

INTEREST IN THE PROPERTY: Owner

FEE SIMPLE TITLEHOLDER NAME: -

FEE SIMPLE TITLEHOLDER ADDRESS:(IF OTHER THAN OWNER) -

CONTRACTOR NAME: Timberlake Aluminum

Address: -

BONDING COMPANY: N/A ADDRESS: N/A PHONE NUMBER N/A

CITY: N/A STATE N/A ZIP CODE: N/A

LENDER NAME: None

ADDRESS :n/a PHONE N/A

CITY:N/A STATE N/A Zip: N/A

Prepared by: Peeler Pools, Inc. (Raymond Peeler)

Return to : Peeler Pools, Inc. 9878 S. US Hwy 441 Lake City, FL 32025

Persons within the State of Florida designated by Owner upon whom notices or other documents may be served as provided by Section 713.13(1) (a) 7., Florida Statutes.

NAME: None ADDRESS: N/A

In addition to himself, Owner designates: Raymond Peeler of Peeler Pools, Inc.  
9878 S US Hwy 441 Lake City, FL 32025

to receive a copy of the Lienor's Notice as provided in Section 713.13 (1) (b), Florida Statutes.

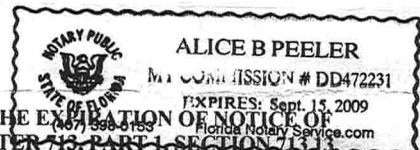
Expiration date is 1 year from date of recording unless a different date is specified.

**SIGNATURE OF OWNER** Jeanette Steedley

SWORN to and subscribed before me this 5th day of May year of 2008

Notary Public Alice B. Peeler My commission expires -

Signature: Alice B. Peeler



\*\*\*WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART 1, SECTION 713.13 FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

**Columbia County Building Permit Application**

<b>For Office Use Only</b>		Application # <u>0805-49</u>	Date Received <u>5/28/08</u>	By <u>GT</u>	Permit # <u>27067</u>
Zoning Official <u>ofs</u>	Date <u>5/28/08</u>	Flood Zone <u>N/A</u>	FEMA Map # _____	Zoning <u>A-3</u>	
Land Use <u>A-3</u>	Elevation _____	MFE _____	River _____	Plans Examiner <u>OKJTH</u>	Date <u>5-26-08</u>
Comments <u>Orig. pool built in 1990 - Rear encroachment.</u>					
<input checked="" type="checkbox"/> NOC <input checked="" type="checkbox"/> EH <input type="checkbox"/> Deed or PA <input type="checkbox"/> Site Plan <input type="checkbox"/> State Road Info <input type="checkbox"/> Parent Parcel # <u>Grandfathered in.</u>					
<input type="checkbox"/> Dev Permit # _____ <input type="checkbox"/> In Floodway <input type="checkbox"/> Letter of Authorization from Contractor _____					
<input type="checkbox"/> Unincorporated area <input type="checkbox"/> Incorporated area <input type="checkbox"/> Town of Fort White <input type="checkbox"/> Town of Fort White Compliance letter					

Septic Permit No. \_\_\_\_\_ Fax 352 472 6855

Name Authorized Person Signing Permit Larry Cole / Larry Tribanville Phone 352 472-6850

Address 25370 NW 8th PL NEWBERRY FL 32669

Owners Name Jeanette Steedley Phone 752-4345

911 Address 272 Smith Lane Lake City FL 32024

Contractors Name Timberlake Aluminum Phone 352 472 6850

Address 25370 NW 8th PLACE NEWBERRY FL 32669

Fee Simple Owner Name & Address N/A

Bonding Co. Name & Address N/A

Architect/Engineer Name & Address Lawrence E. Bennett, P.E. PO Box 214368 South Daytona FL 32121

Mortgage Lenders Name & Address N/A

Circle the correct power company - FL Power & Light - Clay Elec. - Suwannee Valley Elec. - Progress Energy

Property ID Number 36-4S-16-03336-000 HX Estimated Cost of Construction \$9782<sup>00</sup>/100

Subdivision Name N/A Lot N/A Block \_\_\_\_\_ Unit \_\_\_\_\_ Phase \_\_\_\_\_

Driving Directions \_\_\_\_\_

475, TR Smith, 4th on left

Number of Existing Dwellings on Property 4

Construction of ALUMINUM POOL ENCLOSURE Total Acreage 1.45ac Lot Size \_\_\_\_\_

Do you need a - Culvert Permit or Culvert Waiver or Have an Existing Drive Total Building Height 9'

Actual Distance of Structure from Property Lines - Front 85' Side 200' Side 200' Rear 20'

Number of Stories 1 Heated Floor Area \_\_\_\_\_ Total Floor Area 1500 sqft Roof Pitch 12/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Spoke to CRAIG  
6/4/08

**Columbia County Building Permit Application**

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**FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment**

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

**NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:**

**YOU ARE HEREBY NOTIFIED** as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

**OWNERS CERTIFICATION:** I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

  
Owners Signature

**CONTRACTORS AFFIDAVIT:** By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

  
Contractor's Signature (Permitee)

Contractor's License Number SCC056710  
Columbia County  
Competency Card Number \_\_\_\_\_

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 6<sup>th</sup> day of May 2008.  
Personally known \_\_\_\_\_ or Produced Identification Alice B. Peeler

  
State of Florida Notary Signature (For the Contractor)

SEAL:

# Columbia County Property Appraiser

DB Last Updated: 4/15/2008

## 2008 Proposed Values

[Tax Record](#)
[Property Card](#)
[Interactive GIS Map](#)
[Print](#)

Parcel: 36-4S-16-03336-000 HX

### Owner & Property Info

Owner's Name	STEEDLEY HENRY N & JEANETTE W		
Site Address	SMITH		
Mailing Address	272 SW SMITH LN LAKE CITY, FL 32024		
Use Desc. (code)	SINGLE FAM (000100)		
Neighborhood	36416.01	Tax District	3
UD Codes	MK1A01	Market Area	01
Total Land Area	1.450 ACRES		
Description	COMM NE COR OF NW1/4 OF SW1/4, RUN W 146.80 FT FOR POB, RUN S 652.55 FT TO SR-47, SW 191.53 FT, N 754.35 FT, E 146.8 FT TO POB, EX 0.85 AC AS DESC ORB 609-736. ORB 376-319, 800-1518, 852-2533,		

&lt;&lt; Prev

Search Result: 6 of 7

Next &gt;&gt;

### GIS Aerial



### Property & Assessment Values

Mkt Land Value	cnt: (1)	\$28,275.00
Ag Land Value	cnt: (0)	\$0.00
Building Value	cnt: (1)	\$84,972.00
XFOB Value	cnt: (8)	\$16,285.00
Total Appraised Value		\$129,532.00

Just Value	\$129,532.00
Class Value	\$0.00
Assessed Value	\$93,004.00
Exempt Value	(code: HX) \$25,000.00
Total Taxable Value	\$68,004.00

### Sales History

Sale Date	Book/Page	Inst. Type	Sale VImp	Sale Qual	Sale RCode	Sale Price
2/5/1998	852/2533	WD	I	U	01	\$0.00
1/18/1995	800/1518	WD	I	U	03	\$0.00

### Building Characteristics

Bldg Item	Bldg Desc	Year Blt	Ext. Walls	Heated S.F.	Actual S.F.	Bldg Value
1	SINGLE FAM (000100)	1979	Common BRK (19)	1787	3003	\$84,972.00
Note: All S.F. calculations are based on exterior building dimensions.						

### Extra Features & Out Buildings

Code	Desc	Year Blt	Value	Units	Dims	Condition (% Good)
0190	FPLC PF	0	\$1,600.00	1.000	0 x 0 x 0	(.00)
0021	BARN,FR AE	0	\$500.00	1.000	0 x 0 x 0	(.00)
0040	BARN,POLE	0	\$1,000.00	1.000	0 x 0 x 0	(.00)
0296	SHED METAL	0	\$500.00	1.000	0 x 0 x 0	(.00)
0280	POOL R/CON	1990	\$7,373.00	512.000	32 x 16 x 0	(.00)

### Land Breakdown

## Design Check List for Pool Enclosures (Page 1 of 4)

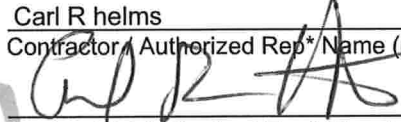
### I. Design Statement:

These plans have been designed in accordance with the Aluminum Structures Design Manual by Lawrence E. Bennett and are in compliance with the 2004 Florida Building Code Edition with 2006 Supplements, Chapter 20, ASM35 and The 2005 Aluminum Design Manual Part I-A & II-A; Exposure 'B' ☒ or 'C' ☐ or 'D' ☐; Importance Factor 0.87 for 100 MPH and 0.77 for 110 MPH and higher; Negative I.P.C. 0.00; \_\_\_ MPH Wind Zone for 3 second wind gust; Basic Wind Pressure \_\_\_; Design pressures are \_\_\_ PSF for roofs & \_\_\_ PSF for walls. (see page 1ii for wind loads and design pressures) A 300 PLF point load is also considered for screen roof members.

**Notes:** Wind velocity zones and exposure category is determined by local code. Design pressures and conversion multipliers are on page 1-ii.

### II. Host Structure Adequacy Statement:

I have inspected and verify that the host structure is in good repair and attachments made to the structure will be solid.

Carl R helms Phone: 352-472-6850  
Contractor / Authorized Rep\* Name (please print)  
  
Contractor / Authorized Rep\* Signature Date: 5/14/08

Steedley / 272 sw Smith Ln | Lake City FL  
Job Name & Address

**Note:** If the total of beam span & upright height exceeds 50' or upright height exceeds 16', site specific engineering is required.

### III. Building Permit Application Package contains the following:

	Yes	No
A. Project name & address on plans	<input checked="" type="checkbox"/>	<input type="checkbox"/>
B. Site plan or survey with enclosure location	<input checked="" type="checkbox"/>	<input type="checkbox"/>
C. Contractor's / Designer's name, address, phone number, & signature on plans	<input checked="" type="checkbox"/>	<input type="checkbox"/>
D. Site exposure form completed	<input checked="" type="checkbox"/>	<input type="checkbox"/>
E. Enclosure layout drawing @ 1/8" or 1/10" scale with the following:	<input checked="" type="checkbox"/>	<input type="checkbox"/>
1. Plan view with host structure, enclosure length, projection from host structure, and all dimensions	<input checked="" type="checkbox"/>	<input type="checkbox"/>
2. Front and side elevation views with all dimensions & heights	<input checked="" type="checkbox"/>	<input type="checkbox"/>
Note: All mansard wall drawings shall include mansard panel at the top of the wall.		
3. Beam location (show in plan & elevation view) & size (Table 1.1 & 1.6)	<input checked="" type="checkbox"/>	<input type="checkbox"/>

Roof frame member allowable span conversions from 120 MPH wind zone, "B" Exposure to \_\_\_ MPH wind zone and / or "C" or "D" Exposure for load width of \_\_\_:

**Note:** Conversion factors do not apply to members subject to point load (P).

Look up span in appropriate 120 MPH span table and apply the following formula:

$$\begin{array}{c} \text{Span} \quad \quad \quad \text{Required Converted} \\ \text{@ 120 MPH} \quad \quad \quad \text{Span / Height} \\ \quad \quad \quad \downarrow \quad \quad \quad \downarrow \\ \quad \quad \quad \text{0.00} \quad (b \text{ or } d) \times \frac{1.00}{\text{Wind Zone Multiplier}} \quad (b \text{ or } d) \times \frac{1.00}{\text{Exposure Multiplier}} \quad (b \text{ or } d) = \text{0.00} \\ \quad \quad \quad \uparrow \quad \quad \quad \uparrow \\ \quad \quad \quad \text{(see page 1ii)} \quad \quad \quad \text{(see page 1ii)} \end{array}$$

4. Upright location (show in plan & elevation view) & size (Table 1.3 & 1.6)	<input checked="" type="checkbox"/>	<input type="checkbox"/>
5. Chair rail & girt size, length, & spacing (Table 1.4)	<input checked="" type="checkbox"/>	<input type="checkbox"/>
6. Eave rail size, length, spacing and stitching of (Table 1.2)	<input checked="" type="checkbox"/>	<input type="checkbox"/>

\* Must have attended Engineer's Continuing Education Class within the past two years.

## Design Check List for Pool Enclosures (Page 2 of 4)

Wall frame member allowable span conversions from 120 MPH wind zone, "B" Exposure to \_\_\_\_\_ MPH wind zone and / or ☐ "C" or ☐ "D" Exposure for load width of 1.00 :

Look up span in appropriate 120 MPH span table and apply the following formula:

$$\begin{array}{c} \text{Span / Height} \\ \text{@ 120 MPH} \\ \text{or } \underline{\hspace{1cm}} \text{ MPH} \end{array} \quad \begin{array}{c} \swarrow \\ \text{0.00} \\ \text{Wind Zone} \\ \text{Multiplier **} \end{array} \quad \begin{array}{c} \text{(b or d) x } \frac{1.00}{\text{Wind Zone}} \\ \text{Multiplier **} \end{array} \quad \begin{array}{c} \swarrow \\ \text{1.00} \\ \text{Exposure Multiplier} \\ \text{(see page 1ii)} \end{array} \quad \begin{array}{c} \text{(b or d) x } \frac{1.00}{\text{Exposure Multiplier}} \\ \text{Exposure Multiplier} \\ \text{(see page 1ii)} \end{array} \quad \begin{array}{c} \text{(b or d) = } \underline{\hspace{1cm}} \\ \text{Required Converted} \\ \text{Span / Height} \end{array}$$

	Yes	No
7. Enclosure roof diagonal bracing in plan view .....	<input checked="" type="checkbox"/>	<input type="checkbox"/>
8. Knee braces length, location, & size .....	<input checked="" type="checkbox"/>	<input type="checkbox"/>
(Table 1.7)		
9. Wall cables or K-bracing sizes shown in wall views .....	<input checked="" type="checkbox"/>	<input type="checkbox"/>
<b>IV. Highlight details from the Aluminum Structures Design Manual:</b>	<b>Yes</b>	<b>No</b>
A. Beam & purlin tables with size, thickness, spacing, & spans / lengths .....	<input checked="" type="checkbox"/>	<input type="checkbox"/>
(Tables 1.1 & 1.2 or 1.9.1 & 1.9.2)		
B. Upright & girt tables with size, thickness, spacing, & spans / lengths .....	<input checked="" type="checkbox"/>	<input type="checkbox"/>
(Tables 1.3 & 1.4)		
C. Table 1.6 with beam & upright combination .....	<input checked="" type="checkbox"/>	<input type="checkbox"/>
D. Connection details to be use such as:		
1. Beam to upright .....	<input checked="" type="checkbox"/>	<input type="checkbox"/>
2. Beam to wall .....	<input checked="" type="checkbox"/>	<input type="checkbox"/>
3. Beam to beam .....	<input checked="" type="checkbox"/>	<input type="checkbox"/>
4. Chair rail, purlins, & knee braces .....	<input checked="" type="checkbox"/>	<input type="checkbox"/>
5. Extruded gutter connections .....	<input checked="" type="checkbox"/>	<input type="checkbox"/>
6. Angle to deck and / or sole plate .....	<input checked="" type="checkbox"/>	<input type="checkbox"/>
7. Anchors go through pavers into concrete .....	<input type="checkbox"/>	<input checked="" type="checkbox"/>
8. Minimum footing and / or knee wall details .....	<input checked="" type="checkbox"/>	<input type="checkbox"/>
9. Cable or K- brace details Section 1 .....	<input checked="" type="checkbox"/>	<input type="checkbox"/>

Wall area calculations for cables:

W = wall width, H = wall height, R = rise

W1 = width @ top of mansard, W2 = width @ top of wall

E. Select footing from examples in manual.

Example 1: Flat Roof

Front wall @ eave:  $\frac{\text{W}}{\text{H}} \text{ ft.} \times \frac{\text{H}}{\text{a}} \text{ ft.} = \frac{0.00}{\text{a}} \text{ ft.}^2 @ 100\% = \underline{\hspace{1cm}} \text{ ft.}^2$

Largest side wall:  $\frac{\text{W}}{\text{H}} \text{ ft.} \times \frac{\text{H}}{\text{b}} \text{ ft.} = \frac{0.00}{\text{b}} \text{ ft.}^2 @ 50\% = \underline{\hspace{1cm}} \text{ ft.}^2$

Total area / (233 ft.<sup>2</sup> / cable for 3/32") = 0 cable pairs

or

Total area / (445 ft.<sup>2</sup> / cable for 1/8") = 0 cable pairs

TOTAL = 0.00 ft.<sup>2</sup>

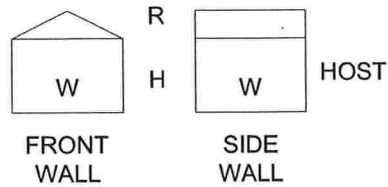
Side wall cable calculation:  $\frac{0.00}{\text{b}} \text{ ft.}^2 @ 100\% = \underline{\hspace{1cm}} \text{ ft.}^2$

Side wall area / (233 ft.<sup>2</sup> / cable for 3/32") = 0 cable(s)

or

Side wall area / (445 ft.<sup>2</sup> / cable for 1/8") = 0 cable(s)

## Design Check List for Pool Enclosures (Page 3 of 4)



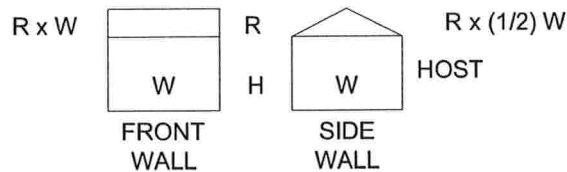
### Example 2: Gable Roof

Front wall @ eave:  $\frac{\text{ft.}}{W} \times \frac{\text{ft.}}{H} = \frac{0.00 \text{ ft.}^2}{a} @ 100\% = \underline{0.00} \text{ ft.}^2$   
 Front gable rise:  $\frac{\text{ft.}}{R} \times \frac{1}{2} \left( \frac{\text{ft.}}{W} \right) = \frac{0.00 \text{ ft.}^2}{b} @ 100\% = \underline{0.00} \text{ ft.}^2$   
 Largest side wall:  $\frac{\text{ft.}}{W} \times \frac{\text{ft.}}{H} = \frac{0.00 \text{ ft.}^2}{c} @ 50\% = \underline{0.00} \text{ ft.}^2$   
 Largest side gable rise:  $\frac{\text{ft.}}{R} \times \frac{\text{ft.}}{W} = \frac{0.00 \text{ ft.}^2}{d} @ 50\% = \underline{0.00} \text{ ft.}^2$   
 TOTAL =  $\underline{0.00} \text{ ft.}^2$

Total area / (233 ft.<sup>2</sup> / cable for 3/32") = 0 cable pairs  
 or  
 Total area / (445 ft.<sup>2</sup> / cable for 1/8") = 0 cable pairs

Side wall cable calculation:  $\frac{0.00 \text{ ft.}^2}{c} + \frac{0.00 \text{ ft.}^2}{d} = \frac{0.00 \text{ ft.}^2}{e} @ 100\% = \underline{0.00} \text{ ft.}^2$

Side wall area / (233 ft.<sup>2</sup> / cable for 3/32") = 0 cable(s)  
 or  
 Side wall area / (445 ft.<sup>2</sup> / cable for 1/8") = 0 cable(s)



### Example 3: Transverse Gable Roof

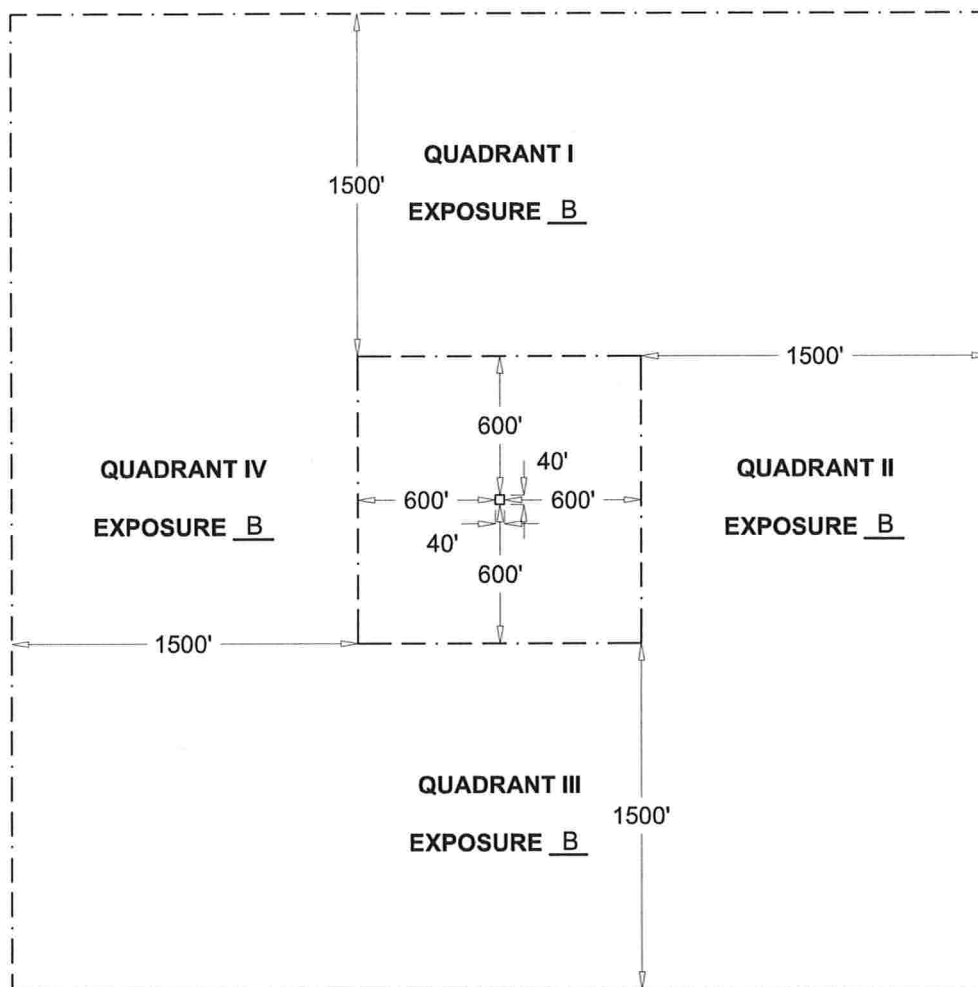
Front wall @ eave:  $\frac{59.75 \text{ ft.}}{W} \times \frac{9.00 \text{ ft.}}{H} = \frac{537.75 \text{ ft.}^2}{a} @ 100\% = \underline{537.75} \text{ ft.}^2$   
 Front gable rise:  $\frac{4.00 \text{ ft.}}{R} \times \frac{59.75 \text{ ft.}}{W} = \frac{239.00 \text{ ft.}^2}{b} @ 100\% = \underline{239.00} \text{ ft.}^2$   
 Largest side wall:  $\frac{24.66 \text{ ft.}}{W} \times \frac{9.00 \text{ ft.}}{H} = \frac{221.94 \text{ ft.}^2}{c} @ 50\% = \underline{110.97} \text{ ft.}^2$   
 Largest side gable rise:  $\frac{4.00 \text{ ft.}}{R} \times \frac{1}{2} \left( \frac{24.66 \text{ ft.}}{W} \right) = \frac{49.32 \text{ ft.}^2}{d} @ 50\% = \underline{24.66} \text{ ft.}^2$   
 TOTAL =  $\underline{912.38} \text{ ft.}^2$

Total area / (233 ft.<sup>2</sup> / cable for 3/32") = 4 cable pairs  
 or  
 Total area / (445 ft.<sup>2</sup> / cable for 1/8") = 2 cable pairs

Side wall cable calculation:  $\frac{221.94 \text{ ft.}^2}{c} + \frac{49.32 \text{ ft.}^2}{d} = \frac{271.26 \text{ ft.}^2}{e} @ 100\% = \underline{271.26} \text{ ft.}^2$

Side wall area / (233 ft.<sup>2</sup> / cable for 3/32") = 1 cable(s)  
 or  
 Side wall area / (445 ft.<sup>2</sup> / cable for 1/8") = 1 cable(s)

## SITE EXPOSURE EVALUATION FORM



**NOTE:** ZONES ARE MEASURED FROM STRUCTURE OUTWARD

### SITE

SCALE: 1" = 800'

USING THE FOLLOWING CRITERIA, EVALUATE EACH QUADRANT AND MARK IT AS 'B', 'C', OR 'D' EXPOSURE. 'C' OR 'D' EXPOSURE IN ANY QUADRANT MAKE THE SITE THAT EXPOSURE.

EXPOSURE C: 1. OPEN TERRAIN FOR MORE THAN 1,500 FEET IN ANY QUADRANT.

2. ANY 'C' EXPOSURE FOR GREATER THAN 600 FEET IN ANY QUADRANT.

3. NO SHORT TERM CHANGES IN 'B', 2 YEARS BEFORE SITE EVALUATION AND BUILD OUT WITHIN 3 YEARS, SITE WILL BE 'B'.

4. FLAT, OPEN COUNTRY, GRASSLANDS, PONDS AND OCEAN OR SHORELINES IN ANY QUADRANT FOR GREATER THAN 1,500 FEET.

EXPOSURE D: FLAT, UNOBSTRUCTED AREAS THAT ARE 1,500 FT INLAND FROM THE SHORE LINE AND ARE EXPOSED TO WIND FLOWING OVER WATER FOR A DISTANCE OF AT LEAST 1 MILE.

SITE IS EXPOSURE: B EVALUATED BY: Carl R Helms DATE: 5-14-08

SIGNATURE: *Carl R Helms* LICENSE #: scc056710

↑ 200' to P/L

59 ft. 9 in.

20' to P/L  
→

24 ft. 8 in.

44 ft.

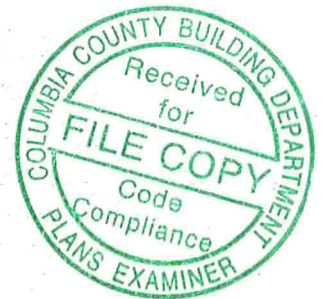
↓ 200' to P/L

5 ft. 6 in.

← 85' to P/L

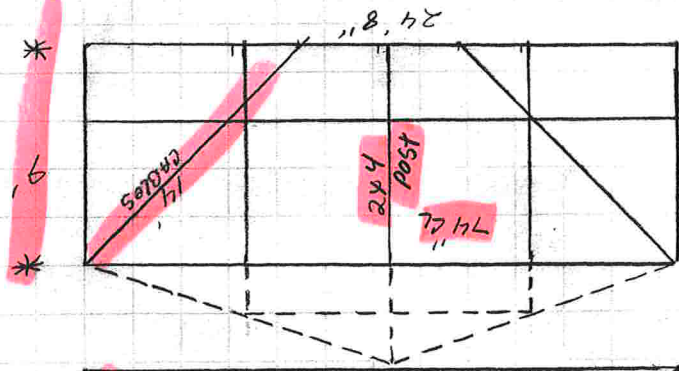
16 ft.

Steedley Screen Enclosure  
Site Plan

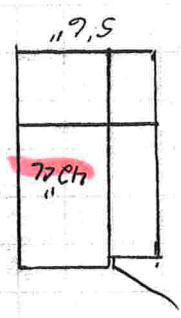
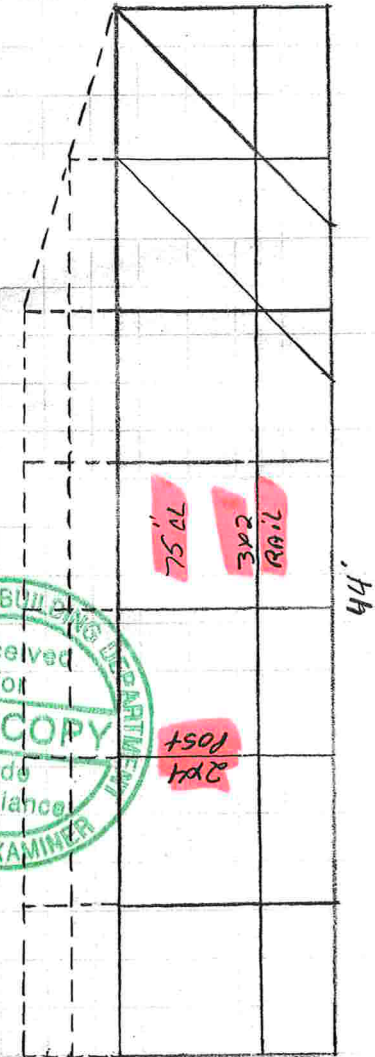
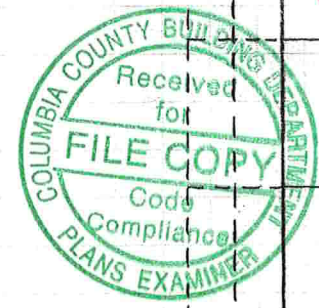
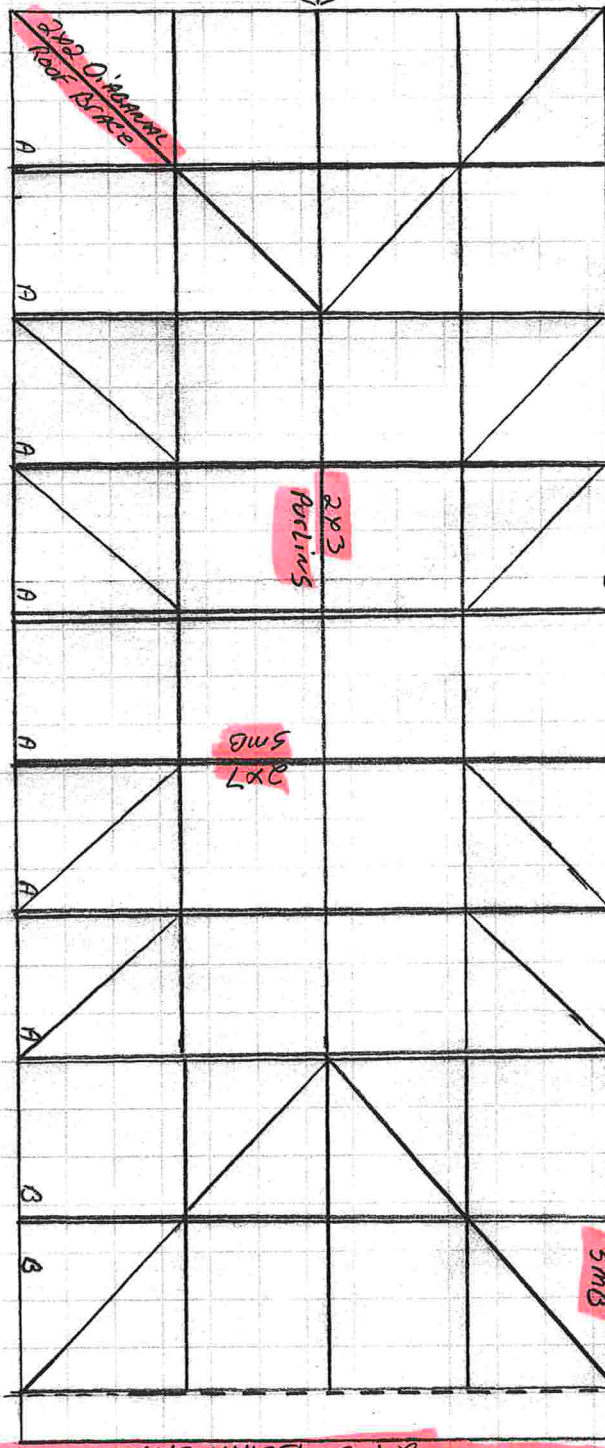


SCALE: 1/8" = 1'

CHAIR RAIL INTERP.  
 $5'6" = 6'6" \times 1.10 = 7'2"$



2x7.5MB INTERPOLATION  
 $7'2" = 8'11" \times 1.13 = 24' 9"$   
 6005.75  
 ALLOY



STEEDLEY/PELER RAILS #  
 272 SW SMITH LN  
 LAKE CITY 32024

1MBEALKE ALUMINUM  
 25370 AND 801  
 NEWBERRY 32669

2x4.050 INTERPOLATION  
 $6'4" = 7'9"$   
 $7'2" = 7'11"$   
 $5'11" + 7'11" = 7'6" \times 1.13 = 8'6" \times 1.10 = 9'2"$   
 18x14  
 6005.75  
 ALLOY

24' 5" BEAM SPAN

## Section 1 Design Statement:

The structures designed for Section 1 are framing systems with screen roofs & walls and loads have been determined by wind tunnel test that include any negative internal pressure coefficient. Since these structures are open, the negative internal pressure coefficient is considered to be 0.00. The design loads used are from Chapter 20 of the 2004 Florida Building Code w/ 2006 Supplements. The loads assume a mean roof height of less than 30'; roof slope of 0° to 20°;  $I = 0.87$  for 100 MPH and 0.77 for 110 or higher. All loads are based on 20 / 20 screen or larger. Multiply wall heights by 1.10 for members controlled by bending(b) and 1.07 for members controlled by deflection(d) when using 18 / 14 screen. All pressures shown in the below table are in PSF (#/SF). All framing components are considered to be 6063-T6 alloy. For components of 6005-T5 and 6061-T6 multiply spans by 1.13.

## General Notes and Specifications for Section 1 Tables:

### SECTION 1 Uniform Loads for Structures with Screen Roof & Walls

Wind Velocity MPH	Basic Wind Pressure	Exposure 'B'			Exposure 'C'		
		Roofs	Windward Walls	Leeward Walls	Roofs	Windward Walls	Leeward Walls
100	13	3	12	10	5	17	13
110	14	4	13	9	5	18	14
120	17	4	15	13	6	21	17
123	18	4.3	15.9	13.3	6.3	22.2	17.6
130	20	5	18	14	7	25	19
1401 & 2	23	6	21	15	8	29	23
150	26	7	24	18	9	33	27

Loads per table 2002.4

Multipliers only apply to members when spans / heights are controlled by wind pressure, not by point load.

### Conversion Table 1A

#### Wind Zone Conversion Factors for Screen Roof or Wall Frame Members

From 120 MPH Wind Zone to Others; Exposure 'B'

Wind Zone MPH	Roofs		Walls	
	Applied Load #/ SF	Conversion Factor	Applied Load #/ SF	Conversion Factor
100	3	1.15	12	1.12
110	4	1.00	13	1.07
120	4	1.00	15	1.00
123	4.3	0.96	15.9	0.97
130	5	0.89	18	0.91
1401 & 2	6	0.82	21	0.85
150	7	0.76	24	0.79

Note:

Multipliers are for wall loads only.

Multipliers only apply to members when spans / heights are controlled by wind pressure, not by point load.

### Conversion Table 1B

#### Load Conversion Factors Based on Mean Roof Height from Exposure "B" to "C" & "D"

Mean Roof Height*	Exposure "B" to "C"			Exposure "B" to "D"		
	Load Conversion Factor	Span Multiplier		Load Conversion Factor	Span Multiplier	
		Bending	Deflection		Bending	Deflection
0 - 15'	1.21	0.91	0.94	1.47	0.83	0.88
15' - 20'	1.29	0.88	0.92	1.54	0.81	0.87
20' - 25'	1.34	0.86	0.91	1.60	0.79	0.86
25' - 30'	1.40	0.85	0.89	1.66	0.78	0.85
30' - 40'	1.37	0.85	0.90	1.61	0.79	0.85

\* Use larger mean roof height of host structure or enclosure

Values are from ASCE 7-02

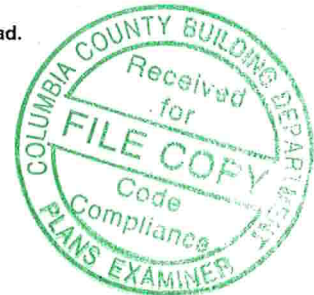
Multipliers only apply to members when spans / heights are controlled by wind pressure, not by point load.

#### Conversion Example (Convert span for Exposure "B" to "C"):

If max span found from span tables for Exposure "B" = 31'-11" = 31.92'

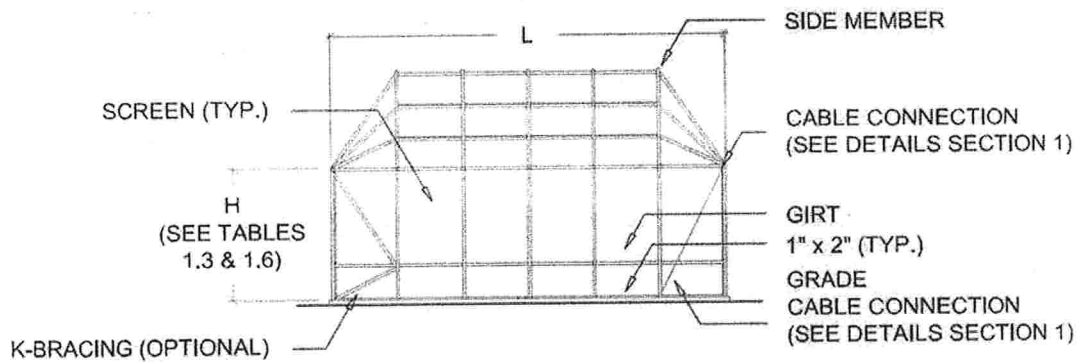
and the mean roof height of the structure is 0-15' then multiply span by 0.91

the span for Exposure "C" is 31.92' \* 0.91 = 29.05' = 29'-1"



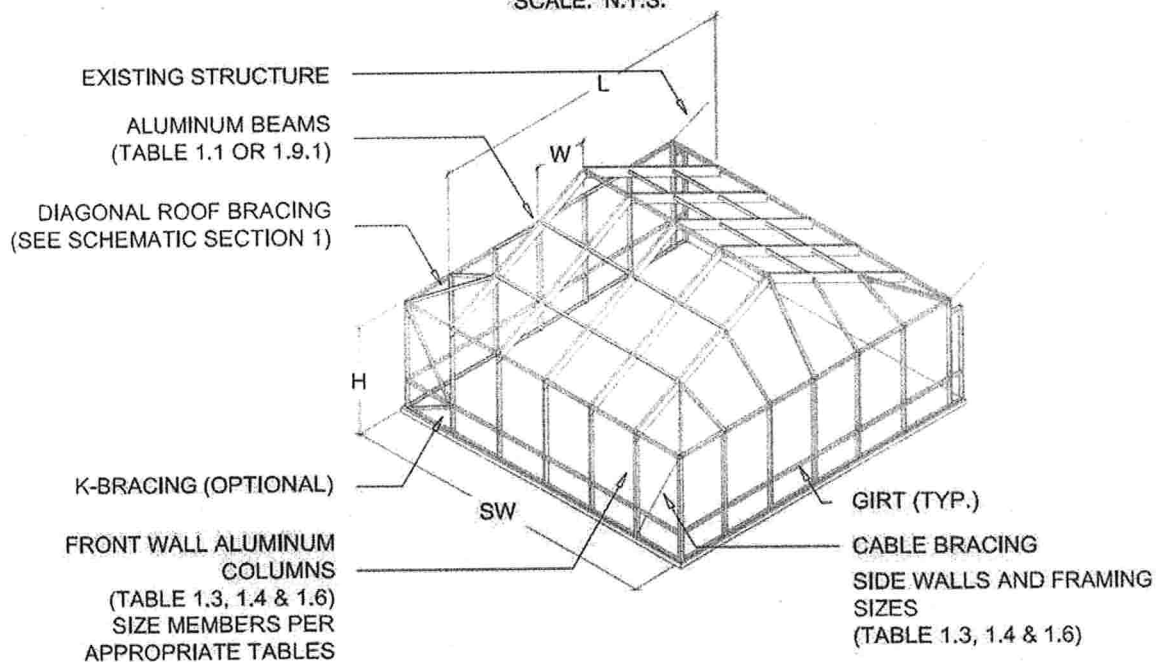
## SECTION 1

## SCREENED ENCLOSURES



### TYPICAL MODIFIED HIP ROOF - FRONT WALL ELEVATION

SCALE: N.T.S.



### TYPICAL MODIFIED HIP ROOF - ISOMETRIC

SCALE: N.T.S.

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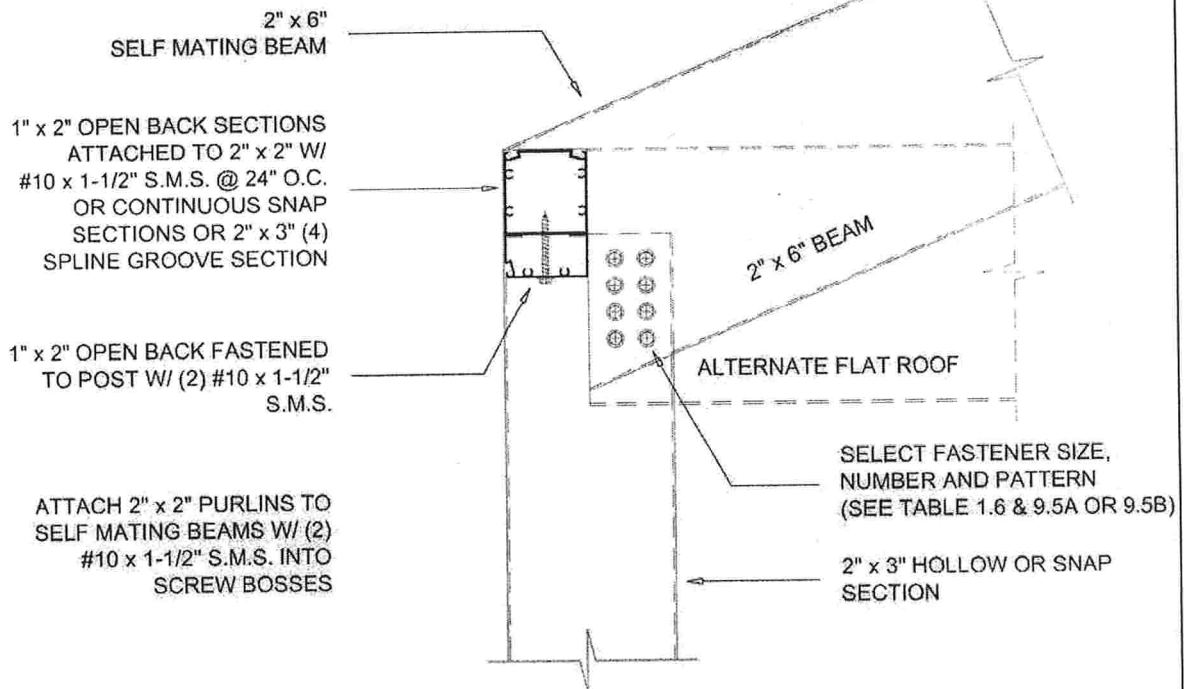
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# SCREENED ENCLOSURES

## SECTION 1

MINIMUM POST SIZES  
REQUIRED FOR EACH BEAM  
SIZE (SEE TABLE 1.6)



### SLOPING BEAM TO UPRIGHT CONNECTION DETAIL (PARTIAL LAP)

SCALE: 3" = 1'-0"

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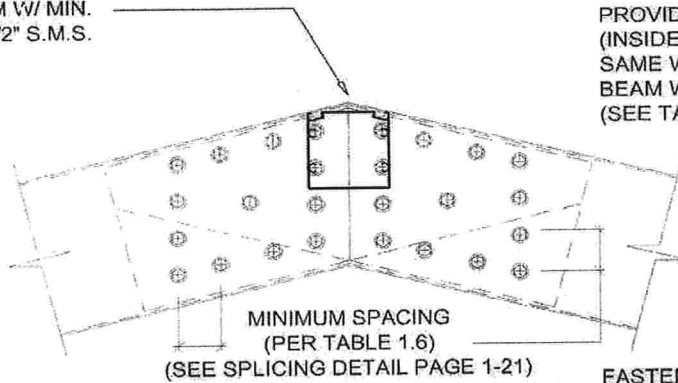
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# SECTION 1

# SCREENED ENCLOSURES

2" x 2" PURLINS ATTACHED  
TO BEAM W/ MIN.  
(3) #10 x 1-1/2" S.M.S.



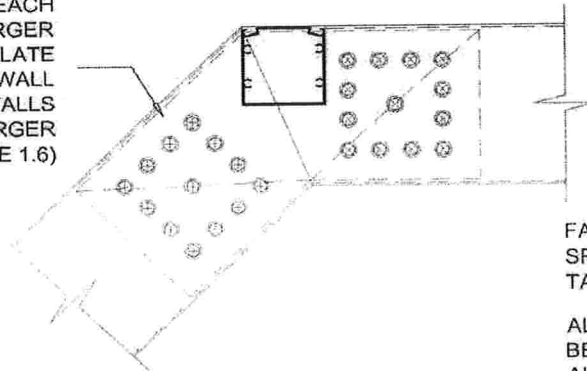
CUT 2" x 4", 2" x 5", OR 2" x 6"  
BEAMS TO SLIDE OVER EACH  
OTHER 2" x 7" & LARGER  
PROVIDE GUSSET PLATE  
(INSIDE OR OUTSIDE BEAM)  
SAME WALL THICKNESS AS  
BEAM WALLS OR LARGER  
(SEE TABLE 1.6)

FASTENER SIZE, NUMBER AND  
SPACING PER PAGE 1-20(SEE  
TABLE 1.6)

## **ALTERNATE SIDE PLATE CONNECTION DETAIL** **GUSSET PLATE MOUNTED INTERNALLY**

SCALE: 3" = 1'-0"

CUT 2" x 4", 2" x 5", OR 2" x 6"  
BEAMS TO SLIDE OVER EACH  
OTHER 2" x 7" & LARGER  
PROVIDE GUSSET PLATE  
(INSIDE BEAM) SAME WALL  
THICKNESS AS BEAM WALLS  
OR LARGER  
(SEE TABLE 1.6)



FASTENER SIZE, NUMBER AND  
SPACING PER PAGE 1-20(SEE  
TABLE 1.6)

ALL GUSSET PLATES SHALL  
BE A MINIMUM OF 5052 H-32  
ALLOY OR HAVE AN ULTIMATE  
YIELD STRENGTH OF 30 KSI

## **ALTERNATE SIDE PLATE CONNECTION DETAIL - MANSARD ROOF** **GUSSET PLATE MOUNTED INTERNALLY**

SCALE: 3" = 1'-0"

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# SCREENED ENCLOSURES

## SECTION 1

1/4" x 2" LAG SCREWS @ 24"  
O.C. OR #10 x 2" SCREWS @  
12" O.C. MIN. AND (2) @ EACH  
STRAP  
OPTIONAL 1" x 2" OR 2" x 2"  
FOR SCREEN

SELF-MATING  
BEAM  
(SIZE VARIES)

SUPER OR  
EXTRUDED  
GUTTER

ANGLE, INTERIOR OR  
EXTERIOR RECEIVING  
CHANNEL (SEE SECTION 9)

2" x     " x 0.050" STRAP  
@ EACH BEAM CONNECTION  
AND @ 1/2 BEAM SPACING W/  
(2) #8 x 1/2" S.M.S. PER STRAP

MAX. DISTANCE FROM FASCIA  
TO HOST STRUCTURE WALL  
(SEE TABLE 1.11)

### ALTERNATE SELF MATING BEAM CONNECTION TO SUPER OR EXTRUDED GUTTER

SCALE: 3" = 1'-0"

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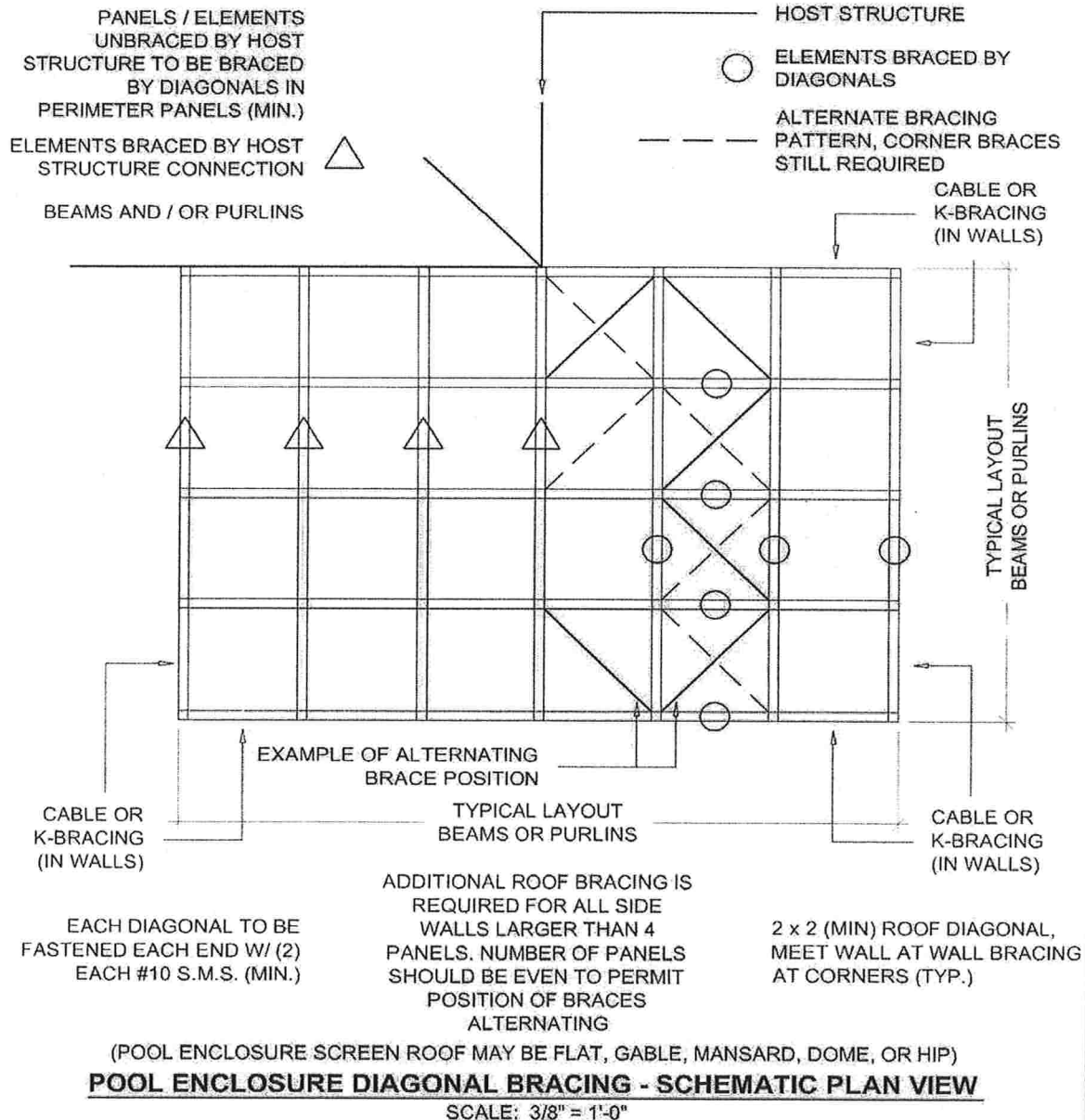
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# SECTION 1

# SCREENED ENCLOSURES



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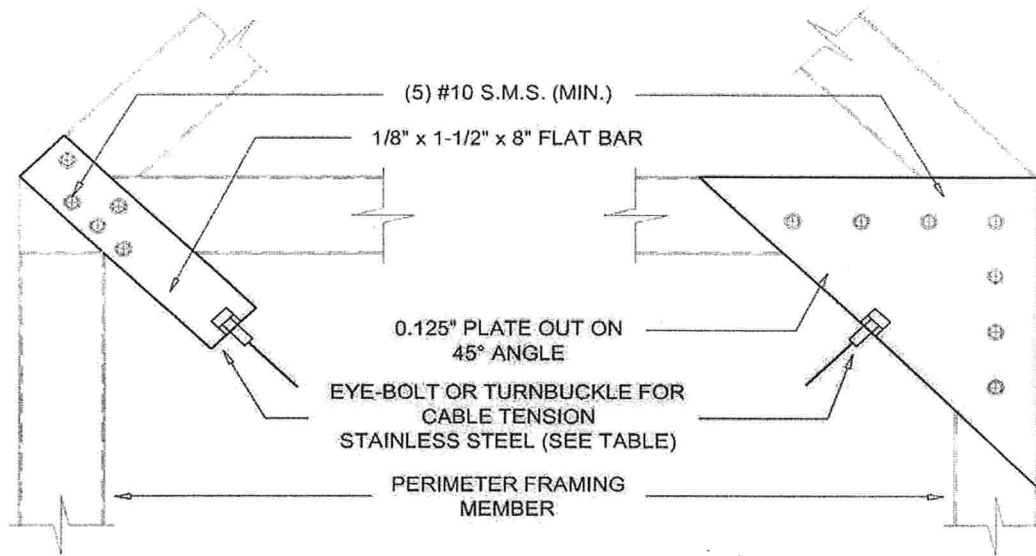
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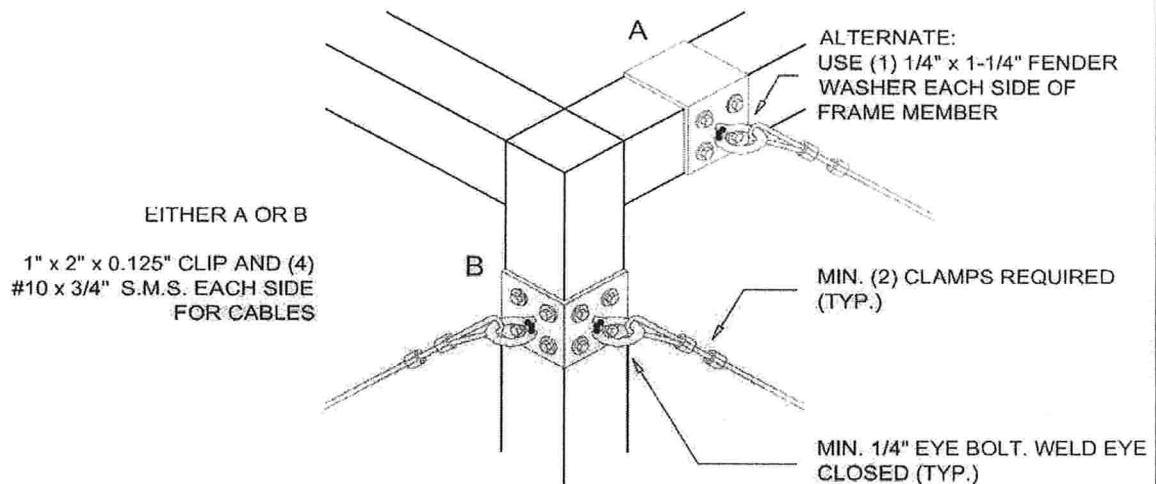
# SCREENED ENCLOSURES

## SECTION 1



**TYPICAL CABLE CONNECTIONS AT CORNER - DETAIL 1**

SCALE: 3" = 1'-0"



**ALTERNATE TOP CORNER OF CABLE CONNECTION - DETAIL 1A**

SCALE: 3" = 1'-0"

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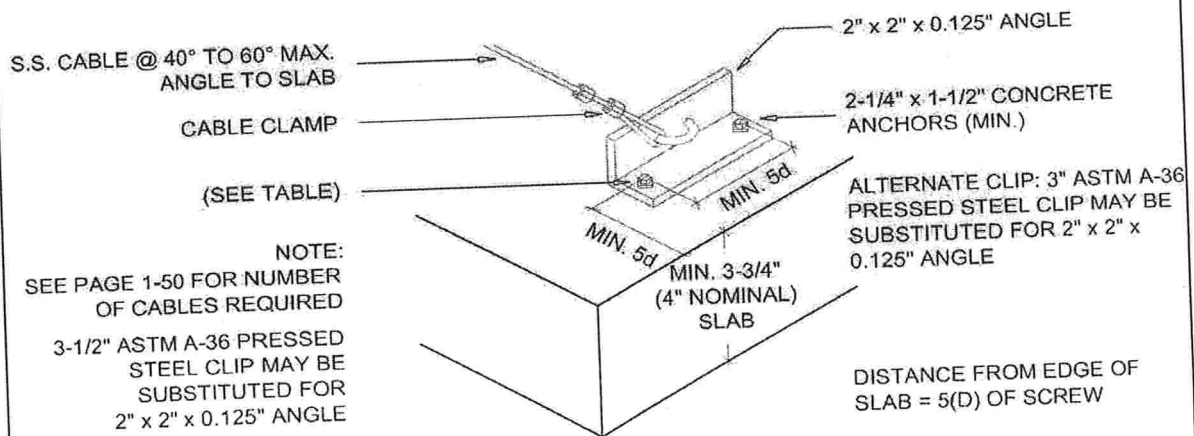
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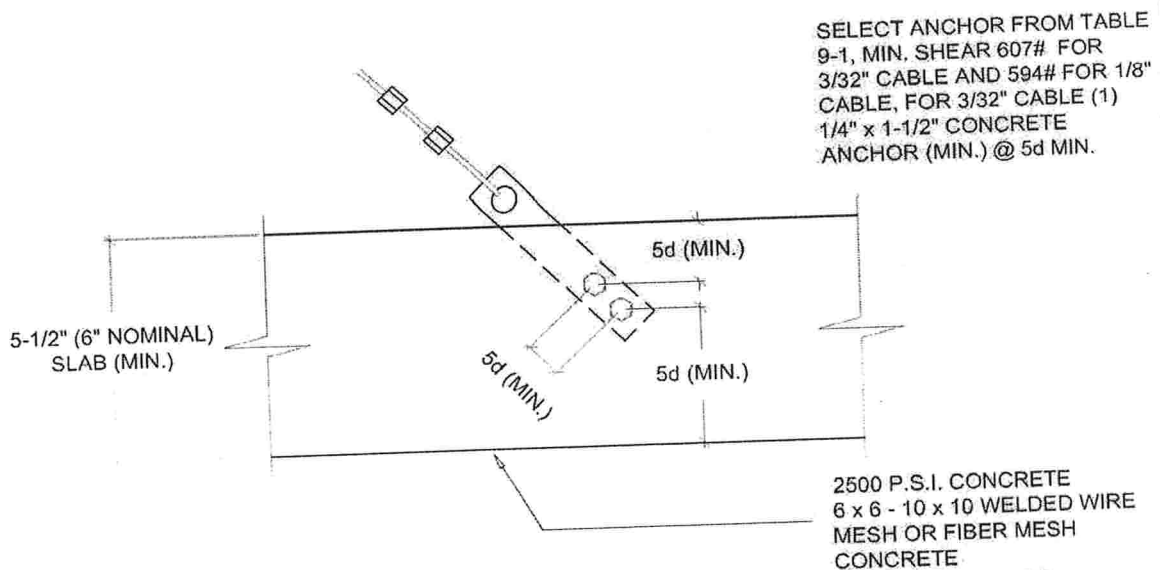
1-51

# SCREENED ENCLOSURES

## SECTION 1



**ALTERNATE CABLE CONNECTION AT SLAB DETAIL - DETAIL 2B**  
SCALE: 3" = 1'-0"



**ALTERNATE CABLE CONNECTIONS AT FOUNDATION - DETAIL 2C**  
SCALE: 3" = 1'-0"

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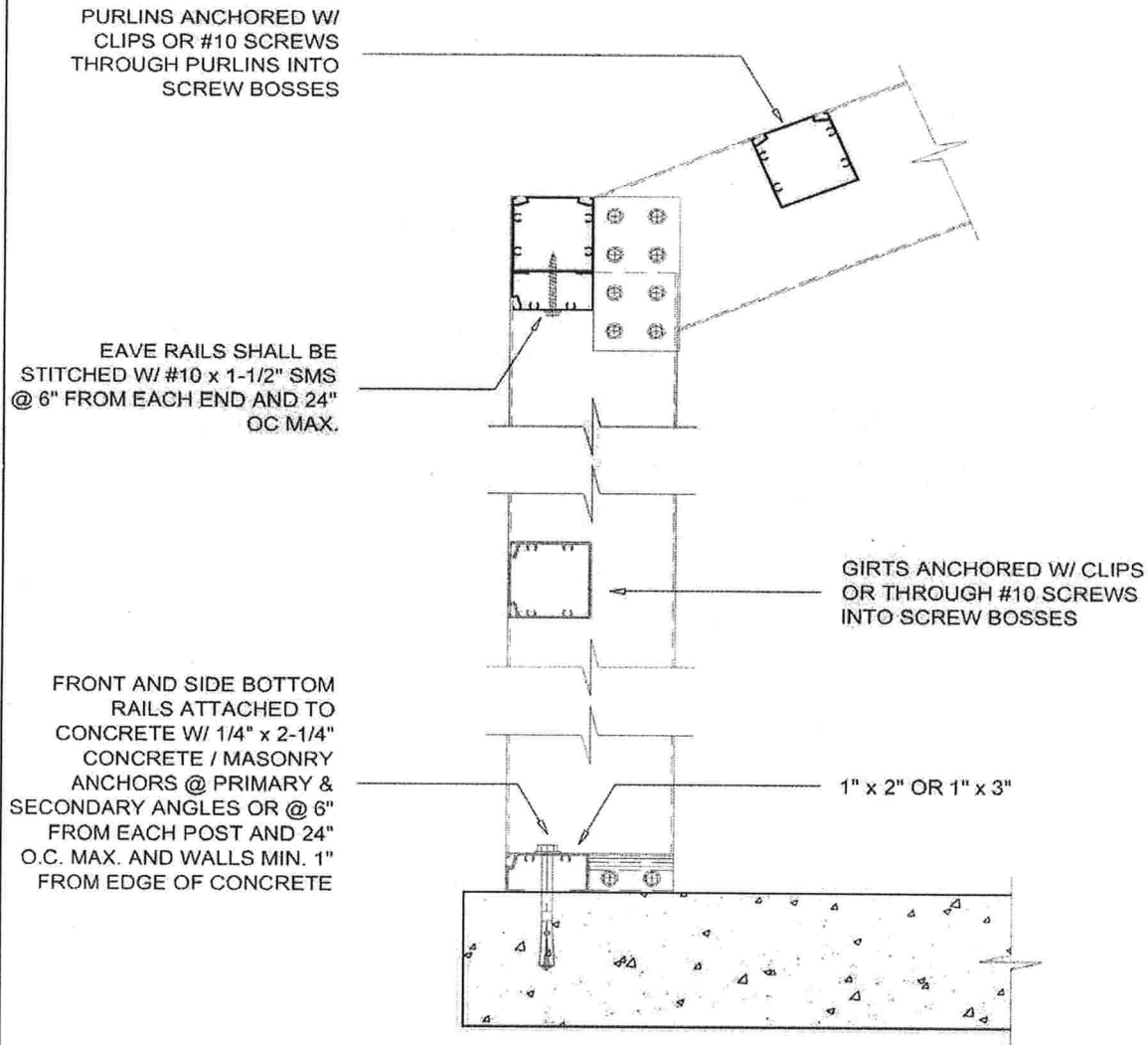
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# SECTION 1

# SCREENED ENCLOSURES



**PURLIN & CHAIR RAIL DETAIL**

SCALE: 3" = 1'-0"

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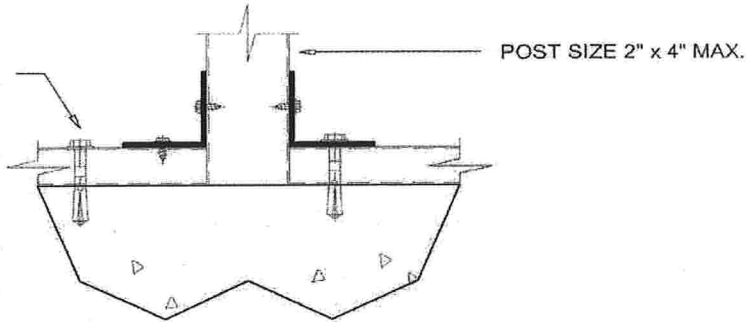
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**SCREENED ENCLOSURES****SECTION 1**

1" x 2" EXTRUSION ANCHOR  
TO CONCRETE W/ CONCRETE  
ANCHORS OR THRU PRIMARY  
ANGLE 6" MAX. EACH SIDE OF  
EACH POST AND @  
24" O.C. MAX.  
SELECT CONCRETE ANCHORS  
FROM SECTION 9

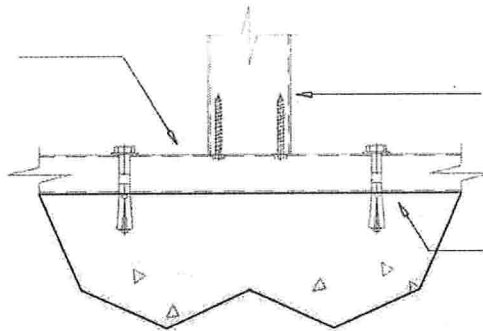
MIN. 3-1/2" SLAB 2500 P.S.I.  
CONCRETE 6 x 6 - 10 x 10  
WELDED WIRE MESH OR  
FIBER MESH CONCRETE

**SIDE WALL POST TO PLATE TO CONCRETE DETAIL**

SCALE: 3" = 1'-0"

1" x 2" EXTRUSION ANCHOR  
TO CONC. W/ CONC. ANCH. 6"  
MAX. EA. SIDE OF EA. POST  
AND @ 24" O.C. MAX. SELECT  
CONCRETE ANCHORS FROM  
SECTION 9

MIN. 3-1/2" SLAB 2500 P.S.I.  
CONC. 6 x 6 - 10 x 10 W.W.M.  
OR FIBER MESH CONC.



2" x 2", 2" x 3" OR 2" x 4"  
HOLLOW SECTION  
(SEE TABLES)

MIN. (3) #10 x 1-1/2" S.M.S. INTO  
SCREW BOSSES

MASONRY ANCHOR @ 6" EA.  
SIDE OF POST AND @ 24" O.C.  
MAX. SELECT CONCRETE  
ANCHORS FROM SECTION 9

**SIDE WALL HOLLOW POST TO BASE DETAIL**

SCALE: 3" = 1'-0"

**POOL ENCLOSURE UPRIGHT TO DECK ANCHOR REQUIREMENTS****General Notes and Specifications:**

1. The uplift load on a pool enclosure upright is calculated as 1/2 the beam span x the beam spacing x the screen load of 7# / Sq. Ft.

**EXAMPLE:**

FOR A 2" x 6" BEAM WITH A SPAN OF 23' AND A BEAM & UPRIGHT SPACING  
OF 7' USE: 1/2 x 17'-11" x 7' x 10# / Sq. Ft. = 627.2# UPLIFT

2. Table 1.6 of this manual uses the worst case loads for all cases.
3. In all cases there must be a primary anchor within 6" of each side of the upright.
4. For attachment to wood deck (min. 2" nominal thickness) use wood anchors with details shown above (min. 1-3/8" embedment).

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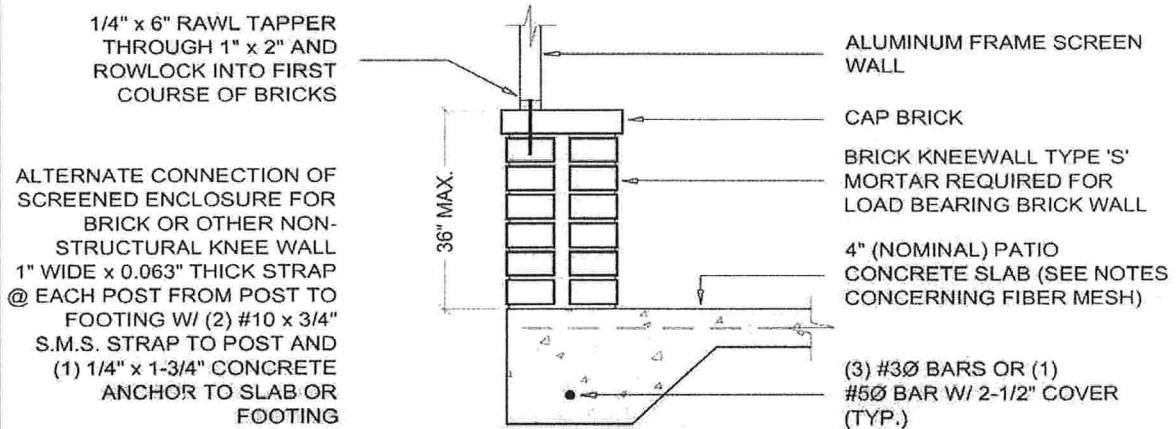
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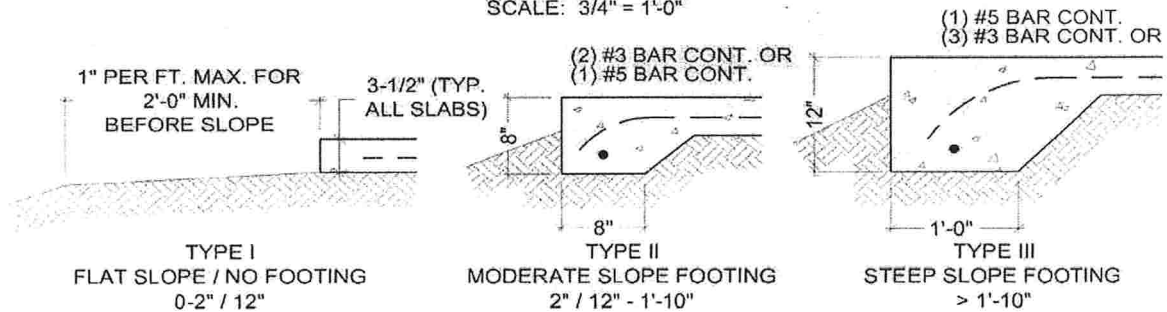
## SCREENED ENCLOSURES

## SECTION 1



### BRICK KNEEWALL AND FOUNDATION FOR SCREEN WALLS

SCALE: 3/4" = 1'-0"



#### Notes for all foundation types:

- The foundations shown are based on a minimum soil bearing pressure of 1,500 PSF. Bearing capacity of soil shall be verified prior to placing slab by field soil test (soil penetrometer) or a soil testing lab.
- The slab / foundation shall be cleared of debris, roots and compacted prior to placement of concrete.
- No footing is required except when addressing erosion until the slab width in the direction of the primary beams exceeds the span per table on page 1-69, then a type II slab is required under the load bearing wall only unless the side wall exceeds 16' in height or the enclosure is in a "C" exposure category in which case a type II footing is required.
- Monolithic slabs and footings shall be minimum 2,500 psi concrete with 6 x 6 - 10 x 10 welded wire mesh or crack control fiber mesh; Fibermesh® Mesh, InForce™ e3™ (Formerly Fibermesh MD) per manufacturer's specification may be used in lieu of wire mesh. All slabs / footings shall be allowed to cure for 7 days before installing anchors.
- If local codes require a minimum footing use Type II footing or footing section required by local code. Local codes govern.

### SLAB-FOOTING DETAILS

SCALE: 3/4" = 1'-0"

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## SECTION 1

## SCREENED ENCLOSURES

Table 1.1 120 Allowable Spans for Primary Screen Roof Frame Members  
Aluminum Alloy 6063 T-6

For Wind Zones up to 120 M.P.H., Exposure "B" and Latitudes Below 30°-30'-00" North (Jacksonville, FL)  
Uniform Load = 4 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered

Hollow Sections	Tributary Load Width 'W' = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb
2" x 2" x 0.050"	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb
2" x 2" x 0.090"	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb	7'-6"	Pb
2" x 3" x 0.045"	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb	7'-7"	Pb
2" x 4" x 0.050"	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb
2" x 5" x 0.062"	20'-5"	Pb	20'-5"	Pb	20'-5"	Pb	20'-4"	Ud	19'-4"	Ud	18'-6"	Ud	17'-9"	Ud

Self Mating Sections	Tributary Load Width 'W' = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 4" x 0.044 x 0.100"	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd	11'-8"	Pd
2" x 5" x 0.050" x 0.100"	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	16'-1"	Pd	15'-9"	Ud	15'-1"	Ud
2" x 6" x 0.050" x 0.120"	20'-4"	Pd	20'-4"	Pd	20'-4"	Pd	20'-3"	Ud	19'-3"	Ud	18'-5"	Ud	17'-8"	Ud
2" x 7" x 0.055" x 0.120"	24'-9"	Pd	24'-9"	Pd	24'-6"	Ud	23'-1"	Ud	21'-11"	Ud	20'-11"	Ud	20'-2"	Ud
2" x 8" x 0.072" x 0.224"	34'-2"	Pd	32'-9"	Ud	30'-5"	Ud	28'-7"	Ud	27'-2"	Ud	25'-11"	Ud	24'-11"	Ud
2" x 9" x 0.072" x 0.224"	39'-3"	Pd	35'-11"	Ud	33'-4"	Ud	31'-5"	Ud	29'-10"	Ud	28'-6"	Ud	27'-5"	Ud
2" x 9" x 0.082" x 0.310"	42'-5"	Ud	38'-7"	Ud	35'-10"	Ud	33'-8"	Ud	31'-11"	Ud	30'-7"	Ud	29'-5"	Ud
2" x 10" x 0.092" x 0.369"	49'-3"	Ud	44'-9"	Ud	41'-7"	Ud	39'-1"	Ud	37'-2"	Ud	35'-6"	Ud	34'-2"	Ud

Snap Sections	Tributary Load Width 'W' = Beam Spacing													
	3'-0"		4'-0"		5'-0"		6'-0"		7'-0"		8'-0"		9'-0"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd	4'-10"	Pd
2" x 3" x 0.045"	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd	7'-6"	Pd
2" x 4" x 0.045"	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd	10'-8"	Pd
2" x 6" x 0.062"	22'-2"	Pd	22'-2"	Pd	22'-2"	Pd	21'-5"	Ud	20'-5"	Ud	19'-6"	Ud	18'-9"	Ud
2" x 7" x 0.062"	26'-8"	Pd	26'-8"	Pd	25'-9"	Ud	24'-3"	Ud	23'-0"	Ud	22'-0"	Ud	21'-2"	Ud

## Note:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
  2. The structures designed using this section shall be limited to a maximum combined span and upright height of 50' and a maximum upright height of 16'. Structures larger than these limits shall have site specific engineering.
  3. Span is measured from center of beam and upright connection to fascia or wall connection.
  4. Above spans do not include length of knee brace. Add horizontal distance from upright to center of brace to beam connection to the above spans for total beam spans.
  5. Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable. Other conditions may offer better spans w/ enclosure site specific engineering.
  6. Spans may be interpolated.
  7. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.
- Example: Max. 'L' for 2" x 4" x 0.050" hollow section with 'W' = 5'-0" = 9'-1"

2x7 5mb INTERPOLATION

$$7'6" = 21'11" \times 1.13 = 24'9"$$

6005-T5  
ALLOY

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# SCREENED ENCLOSURES

# SECTION 1

**Table 1.2 120 Allowable Spans for Secondary Screen Roof Frame Members**

Aluminum Alloy 6063 T-6

For Wind Zones up to 120 M.P.H., Exposure "B", and Latitudes Below 30°-30'-00" North (Jacksonville, FL)

Uniform Load = 4 #/SF, a Point Load of 300 #/SF over (1) linear ft. is also considered

A. Sections Fastened To Beams With Clips

Hollow Sections	Tributary Load Width 'W' = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb	4'-5"	Pb
2" x 2" x 0.050"	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb	5'-2"	Pb
2" x 2" x 0.090"	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd
3" x 2" x 0.045"	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb	5'-8"	Pb
3" x 2" x 0.070"	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd	7'-8"	Pd
2" x 3" x 0.045"	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd	7'-4"	Pd
2" x 4" x 0.050"	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb	9'-1"	Pb
2" x 5" x 0.062"	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd	14'-1"	Pd

Snap Sections	Tributary Load Width "W" = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb	4'-11"	Pb
2" x 3" x 0.045"	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd	7'-3"	Pd
2" x 4" x 0.045"	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd	9'-2"	Pd

B. Sections Fastened Through Beam Webs Into Screw Bosses

Hollow Sections	Tributary Load Width 'W' = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span 'L' / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 3" x 0.050"	11'-5"	Pb	11'-5"	Pb	11'-5"	Pb	11'-4"	Ud	10'-11"	Ud	10'-8"	Ud	10'-3"	Ud
2" x 4" x 0.050"	13'-8"	Pb	13'-8"	Pb	13'-8"	Pb	13'-8"	Pb	13'-8"	Pb	13'-8"	Pb	13'-8"	Pb
2" x 5" x 0.062"	22'-4"	Pd	22'-4"	Pd	22'-4"	Pd	21'-7"	Ud	20'-11"	Ud	20'-4"	Ud	19'-7"	Ud

Snap Sections	Tributary Load Width "W" = Purlin Spacing													
	3'-6"		4'-0"		4'-6"		5'-0"		5'-6"		6'-0"		6'-8"	
	Allowable Span "L" / Point Load (P) or Uniform Load (U), bending (b), deflection (d)													
2" x 2" x 0.044"	4'-4"	Pb	4'-4"	Pb	4'-4"	Pb	4'-4"	Pb	4'-4"	Pb	4'-4"	Pb	4'-4"	Pb

## Notes:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Span is measured from center of beam and upright connection to fascia or wall connection.
3. Tables are based on a maximum wall height of 16' including a 4' max. mansard or gable. Other conditions may offer better spans w/ enclosure site specific engineering.
4. Spans may be interpolated.
5. 2" x 4" & 2" x 5" Hollow Girts shall be connected w/ an internal or external 1-1/2" x 1-1/2" x 0.044" angle.
6. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

CHECK TABLE 1.6 FOR MINIMUM UPRIGHT SIZE FOR BEAMS.

Example: Max. 'L' for 2" x 4" x 0.050" hollow section fastened to beam with clips with 'W' = 5'-0" = 9'-1"

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## SECTION 1

## SCREENED ENCLOSURES

Table 1.3 110 Allowable Post / Upright Heights for Primary Screen Wall Frame Members  
Aluminum Alloy 6063 T-6

For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 #/sq. ft.

Hollow Sections	Tributary Load Width "W" = Upright Spacing						
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"
	Allowable Height "H" / bending (b), deflection (d)						
2" x 2" x 0.044"	7'-5" d	6'-5" b	5'-8" b	5'-1" b	4'-8" b	4'-3" b	3'-11" b
2" x 2" x 0.050"	7'-10" d	7'-1" b	6'-3" b	5'-8" b	5'-2" b	4'-9" b	4'-5" b
2" x 2" x 0.090"	8'-11" d	8'-2" d	7'-10" d	7'-1" b	6'-7" b	6'-1" b	5'-9" b
2" x 3" x 0.045"	8'-4" d	7'-7" d	7'-9" d	6'-11" d	6'-5" d	5'-11" b	5'-6" b
2" x 4" x 0.050"	11'-2" b	9'-7" b	8'-6" b	7'-9" b	7'-1" b	6'-7" b	6'-1" b
2" x 5" x 0.062"	17'-3" b	14'-10" b	13'-2" b	11'-11" b	11'-0" b	10'-3" b	9'-7" b
Self Mating Sections	Tributary Load Width "W" = Upright Spacing						
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"
	Allowable Height "H" / bending (b), deflection (d)						
2" x 4" x 0.044 x 0.100"	11'-11" d	10'-10" d	10'-0" d	9'-5" b	8'-8" b	8'-0" b	7'-6" b
2" x 5" x 0.050" x 0.100"	14'-9" d	13'-5" d	12'-5" d	11'-7" b	10'-8" b	9'-11" b	9'-4" b
2" x 6" x 0.050" x 0.120"	17'-3" d	15'-8" d	14'-4" b	13'-1" b	12'-0" b	11'-3" b	10'-6" b
2" x 7" x 0.055" x 0.120"	19'-8" d	17'-6" b	15'-7" b	14'-2" b	13'-1" b	12'-2" b	11'-5" b
2" x 8" x 0.072" x 0.224"	24'-4" d	22'-1" d	20'-6" d	19'-4" d	18'-4" d	17'-6" d	16'-10" d
2" x 9" x 0.072" x 0.224"	26'-8" d	24'-3" d	22'-6" d	21'-2" d	20'-1" d	19'-3" d	18'-2" b
2" x 9" x 0.082" x 0.310"	28'-8" d	26'-0" d	24'-2" d	22'-9" d	21'-7" d	20'-8" d	19'-10" d
2" x 10" x 0.092" x 0.369"	33'-3" d	30'-3" d	28'-1" d	26'-5" d	25'-1" d	23'-11" d	23'-1" d
Snap Sections	Tributary Load Width "W" = Upright Spacing						
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"
	Allowable Height "H" / bending (b), deflection (d)						
2" x 2" x 0.044"	6'-7" d	5'-11" d	5'-7" d	5'-3" d	4'-10" b	4'-5" b	4'-1" b
2" x 3" x 0.045"	8'-10" d	8'-1" d	7'-6" d	6'-11" b	6'-3" b	5'-9" b	5'-3" b
2" x 4" x 0.045"	11'-2" d	10'-2" d	9'-2" b	8'-2" b	7'-5" b	6'-9" b	6'-2" b
2" x 6" x 0.062"	18'-3" d	16'-7" d	15'-5" d	14'-6" d	13'-9" d	13'-2" d	12'-8" d
2" x 7" x 0.062"	20'-7" d	18'-9" d	17'-5" d	16'-4" d	15'-7" d	14'-10" d	14'-2" b

## Notes:

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Using screen panel width "W" select upright length "H".
3. Above heights do not include length of knee brace. Add vertical distance from upright to center of brace to beam connection to the above spans for total beam spans.
4. Site specific engineering required for pool enclosures over 30' in mean roof height.
5. Height is to be measured from center of beam and upright connection to fascia or wall connection.
6. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height are designed to be residential guardrails provided they are attached with min. (3) #10 x 1-1/2" S.M.S. into the screw bosses and do not exceed 8'-0" in span.
7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"
8. Spans may be interpolated.
9. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

$$\frac{6'}{4} = 7'9" - \frac{7'}{4} = 7'1" = 10' \div 2 = 5' + 7'1" = 7'6"$$

$$7'6" \times 1.13 = 8'6" \times 1.10 = 9'2"$$

2x4.050 Interpolation

T5  
Alloy

18x14  
56000

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## SECTION 1

## SCREENED ENCLOSURES

**Table 1.4 110 Allowable Post / Girt / Chair Rail Spans, Header Spans & Upright Heights for Secondary Screen Wall Frame Members**  
Aluminum Alloy 6063 T-6

For 3 second wind gust at a velocity of 110 MPH, Exposure "B" or an applied load of 13 # / sq. ft.

A. Sections As Horizontals Fastened To Posts With Clips

Hollow Sections	Tributary Load Width 'W' = Upright Spacing						
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"
	Allowable Height "H" or Span "L" / bending (b), deflection (d)						
2" x 2" x 0.044"	7'-5" d	6'-5" b	5'-8" b	5'-1" b	4'-8" b	4'-3" b	3'-11" b
2" x 2" x 0.050"	7'-10" d	7'-1" b	6'-3" b	5'-8" b	5'-2" b	4'-9" b	4'-5" b
2" x 2" x 0.090"	8'-11" d	8'-2" d	7'-10" d	7'-1" b	6'-7" b	6'-1" b	5'-9" b
3" x 2" x 0.045"	8'-4" d	7'-4" b	6'-6" b	5'-10" b	5'-4" b	4'-11" b	4'-7" b
3" x 2" x 0.070"	9'-5" d	8'-6" d	7'-9" b	7'-0" b	6'-5" b	5'-11" b	5'-7" b
2" x 3" x 0.045"	8'-4" d	7'-7" d	7'-9" d	6'-11" d	6'-5" d	5'-11" b	5'-6" b
2" x 4" x 0.050"	11'-2" b	9'-7" b	8'-6" b	7'-9" b	7'-1" b	6'-7" b	6'-1" b
2" x 5" x 0.062"	17'-3" b	14'-10" b	13'-2" b	11'-11" b	11'-0" b	10'-3" b	9'-7" b

Snap Sections	Tributary Load Width 'W' = Upright Spacing						
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"
	Allowable Height "H" or Span "L" / bending (b), deflection (d)						
2" x 2" x 0.044"	6'-7" d	5'-11" d	5'-7" d	5'-3" d	4'-10" b	4'-5" b	4'-1" b

B. Sections As Horizontals Fastened To Posts Through Side Into Screw Bosses

Hollow Sections	Tributary Load Width 'W' = Upright Spacing						
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"
	Allowable Height "H" or Span "L" / bending (b), deflection (d)						
2" x 2" x 0.044"	8'-4" b	7'-2" b	6'-4" b	5'-8" b	5'-2" b	4'-9" b	4'-5" b
3" x 2" x 0.045"	9'-7" b	8'-3" b	7'-3" b	6'-6" b	5'-11" b	5'-6" b	5'-1" b
3" x 2" x 0.070"	11'-5" b	9'-10" b	8'-8" b	7'-10" b	7'-2" b	6'-8" b	6'-3" b
2" x 3" x 0.045"	11'-2" d	9'-9" b	8'-8" b	7'-10" b	7'-2" b	6'-8" b	6'-2" b
2" x 4" x 0.050"	12'-6" b	10'-9" b	9'-6" b	8'-7" b	7'-11" b	7'-4" b	6'-10" b
2" x 5" x 0.062"	19'-3" b	16'-7" b	14'-9" b	13'-5" b	12'-4" b	11'-6" b	10'-9" b

Snap Sections	Tributary Load Width 'W' = Upright Spacing						
	3'-0"	4'-0"	5'-0"	6'-0"	7'-0"	8'-0"	9'-0"
	Allowable Height "H" or Span "L" / bending (b), deflection (d)						
2" x 2" x 0.044"	8'-10" d	7'-8" b	6'-9" b	6'-0" b	5'-5" b	4'-11" b	4'-7" b

**Note:**

1. Thicknesses shown are "nominal" industry standard tolerances. No wall thickness shall be less than 0.040".
2. Using screen panel width "W" select girt lengths.
3. Site specific engineering required for pool enclosures over 30' in mean roof height.
4. Span/height is to be measured from center of beam and upright connection to fascia or wall connection.
5. Chair rails of 2" x 2" x 0.044" min. and set @ 36" in height are designed to be residential guardrails provided they are attached with min. (3) #10 x 1-1/2" s.m.s. into the screw bosses and do not exceed 8'-0" o.c.
6. Girt spacing shall not exceed 6'-8".
7. Max. beam size for 2" x 5" is 2" x 7" x 0.055" x 0.120"
8. 2" x 4" & 2" x 5" hollow girts shall be connected w/ an internal or external 1-1/2" x 1-1/2" x 0.044" angle.
9. Spans/heights may be interpolated.
10. To convert spans to "C" and "D" exposure categories see exposure multipliers and example on page 1-ii.

$$5'6" = 6'6" \times 1.10 = 7'2" \quad \text{CHAIR RAIL INTERPOLATION}$$

↑  
18x14  
SCM

REVISED APRIL 2007

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## SECTION 1

## SCREENED ENCLOSURES

**Table 1.6 Minimum Upright Sizes and Number of Screws for Connection of Roof Beams To Wall Uprights or Beam Splicing**

Beam/Upright or Post	Upright or Post/Beam	Minimum Purlin, Girt & Knee Brace Size	Notes	Minimum Number of Screws*			Beam Stitching Screw at 24" OC
				#8 x 1/2"	#10 x 1/2"	#12 x 1/2"	
2 x 4 SMB	2 x 3 SMB or H	2" x 2" x 0.044"	Partial Lap	8	6	4	#10
2 x 5 SMB	2 x 3 SMB or H	2" x 2" x 0.044"	Partial Lap	8	6	4	#8
2 x 6 SMB	2 x 3 SMB or H	2" x 2" x 0.044"	Partial Lap	10	8	6	#10
2 x 7 SMB	2 x 4 SMB or H	2" x 3" x 0.044"	Full Lap	14	12	10	#12
2 x 8 SMB	2 x 5 SMB or H	2" x 3" x 0.044"	Full Lap	16	14	12	#14
2 x 9 SMB	2 x 6 SMB	2" x 3" x 0.045"	Full Lap	18	16	14	#14**
2 x 9 SMB *	2 x 7 SMB	2" x 4" x 0.050"	Full Lap	20	18	16	#14**
2 x 10 SMB	2 x 8 SMB	2" x 5" x 0.050"	Full Lap	20	18	16	#14**

Screw Size	Minimum Distance and Spacing of Screws		Gusset Plate Thickness	
	Edge To Center	Center To Center	Beam Size	Thickness
#8	5/16"	5/8"	2" x 7" x 0.055" x 0.120"	0.063"
#10	3/8"	3/4"	2" x 8" x 0.072" x 0.224"	0.125"
#12	1/2"	1"	2" x 9" x 0.072" x 0.224"	0.125"
#14 or 1/4"	3/4"	1-1/2"	2" x 9" x 0.082" x 0.306"	0.190"
5/16"	7/8"	1-3/4"	2" x 10" x 0.092" x 0.369"	0.250"
3/8"	1"	2"		

\* 0.082" wall thickness, 0.310" flange thickness

\*\* (1) Stitching screw at 16" O.C. max.

**Connection Example:**

2" x 7" beam & 2" x 5" at beam & gusset plate, (14) #8 x 1/2" sms & upright & gusset plate  
(14) #8 x 1/2" sms ea. side of beam & upright.

**Note:**

1. Connection of 2" x 6" to 2" x 4" shall use a full lap cut or 1/16" gusset plate.
2. For beam splice connections the number of screws shown is the total for each splice with 1/2 the screws on each side of the cut.
3. The number of screws is based on the maximum allowable moment of the beam.
4. The number of deck anchors is based on RAWL R Tapper allowable load data for 2,500 psi concrete and / or equal anchors may be used. The number shown is the total use 1/2 per side.
5. Hollow splice connections can be made provided the connection is approved by the engineer.
6. If a larger than minimum upright is used the number of screws is the same for each splice with 1/2 the screws on each side of the cut.
7. The side wall upright shall have a minimum beam size as shown above, i.e., a 2" x 4" upright shall have a 2" x 3" beam.
8. For minimum girt size read upright size as a beam and purlin size is minimum girt size. (i.e. 2" x 9" x 0.072" x 0.224" s.m.b. w/ 2" x 6" x 0.050 x 0.120" s.m.b. upright requires a 2" x 3" x 0.045" girt / chair rail.)

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