DATE 03/03/2008

Columbia County Building Permit

This Permit Must Be Prominently Posted on Premises During Construction

PERMIT
000026816

APPLICANT MITCHELL SAAD PHONE 386.454.7298

ADDRESS 349 SW THORNE LANE FT. WHITE FL 32038

OWNER MITCHELL SAAD PHONE 386.454.7298

ADDRESS 349 SW THORNE LANE FT. WHITE FL 32038

CONTRACTOR MITCHELL SAAD PHONE 386.454.7298

LOCATION OF PROPERTY 47-S TO US 27-S TO C-138,TR TO FIRST L AFTER TURN, TO THORNE TR, AND IT' TH 4TH PLACE ON R.

TYPE DEVELOPMENT SCREEN ROOM ADDITION ESTIMATED COST OF CONSTRUCTION 122050.00

HEATED FLOOR AREA 1699.00 TOTAL AREA 2441.00 HEIGHT ____ STORIES ____

FOUNDATION ____ WALLS ____ ROOF PITCH ____ FLOOR ____

LAND USE & ZONING A-3 MAX. HEIGHT ____

Minimum Set Back Requirments: STREET-FRONT 30.00 REAR 25.00 SIDE 25.00

NO. EX.D.U. 1 FLOOD ZONE X DEVELOPMENT PERMIT NO. ____

PARCEL ID 30-7S-17-10058-121 SUBDIVISION SFRP

LOT 11 BLOCK ____ PHASE ____ UNIT ____ TOTAL ACRES ____

Culvert Permit No.	Culvert Waiver	Contractor's License Number	Applicant/Owner/Contractor
			Mitchell Saad

Driveway Connection	Septic Tank Number	LU & Zoning checked by	Approved for Issuance	New Resident
EXISTING	X-08-015	BLK	JTH	N

COMMENTS: 1 FOOT ABOVE ROAD. NO IMPACT FEE. ADDITION TO EXISTING STRUCTURE

Check # or Cash 2920

FOR BUILDING & ZONING DEPARTMENT ONLY

(footer/Slab)

Temporary Power ____ date/app. by Foundation ____ date/app. by Monolithic ____ date/app. by

Under slab rough-in plumbing ____ date/app. by Slab ____ date/app. by Sheathing/Nailing ____ date/app. by

Framing ____ date/app. by Rough-in plumbing above slab and below wood floor ____ date/app. by

Electrical rough-in ____ date/app. by Heat & Air Duct ____ date/app. by Peri. beam (Lintel) ____ date/app. by

Permanent power ____ date/app. by C.O. Final ____ date/app. by Culvert ____ date/app. by

M/H tie downs, blocking, electricity and plumbing ____ date/app. by Pool ____ date/app. by

Reconnection ____ date/app. by Pump pole ____ date/app. by Utility Pole ____ date/app. by

M/H Pole ____ date/app. by Travel Trailer ____ date/app. by Re-roof ____ date/app. by

BUILDING PERMIT FEE $ 615.00 CERTIFICATION FEE $ 12.21 SURCHARGE FEE $ 12.21

MISC. FEES $ 0.00 ZONING CERT. FEE $ 50.00 FIRE FEE $ 0.00 WASTE FEE $ ____

FLOOD DEVELOPMENT FEE $ ____ FLOOD ZONE FEE $ 25.00 CULVERT FEE $ ____ **TOTAL FEE** 714.42

INSPECTORS OFFICE ____ CLERKS OFFICE ____

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0802-30 Date Received 2/25 By JW Permit # 26816

Zoning Official BLK Date 03.03.08 Flood Zone X FEMA Map # NA Zoning A-3

Land Use A-3 Elevation N/A MFE NA River NA Plans Examiner OK JH Date 3-29-08

Comments No Impact Fee, Addition to existing Structure

☐ NOC ☑ EH ☑ Deed or PA ☐ Site Plan ☐ State Road Info ☐ Parent Parcel #

☐ Dev Permit # ☐ In Floodway ☐ Letter of Authorization from Contractor

☐ Unincorporated area ☐ Incorporated area ☐ Town of Fort White ☐ Town of Fort White Compliance letter

Septic Permit No. X08-015 CELL # Fax 386.365.5827

Name Authorized Person Signing Permit Mitchell Saad Phone 386 454-7298

Address 349 SW Thorne Lane, Ft White, Fl 32038

Owners Name Mitchell Saad Phone 386-454-7298

911 Address 349 SW Thorne Lane, Ft White, Fl 32038

Contractors Name Owner Phone 386-454-7298

Address Same as above

Fee Simple Owner Name & Address Mitchell Saad 349 SW Thorne Lane Fort White Florida

Bonding Co. Name & Address

Architect/Engineer Name & Address Nicholas Paul Geisler 1758 NW Brown Rd Lake City Fl

Mortgage Lenders Name & Address Bank of America

Circle the correct power company – FL Power & Light – Clay Elec. – Suwannee Valley Elec. – Progress Energy

Property ID Number 30-7S-17-10058-121-HX Estimated Cost of Construction #30,000

Subdivision Name Santa Fe River Plantation Lot 11 Block Unit Phase

Driving Directions Southon US 27, 8 miles S. of Ft White. Right on CR 138. First left after Turn. Down to Thorne Lane. Right on Thorne. 4th house on Right

Number of Existing Dwellings on Property 1

Construction of 1 Screen Room Addition Total Acreage 1.84 Lot Size 1.84

Do you need a - ~~Culvert Permit~~ or ~~Culvert Waiver~~ or Have an Existing Drive Total Building Height 13'

Actual Distance of Structure from Property Lines - Front ~~50~~ Side ~~50'~~ Side ~~200'~~ Rear ~~40'~~

Number of Stories 1 Heated Floor Area 1699 Total Floor Area 2441 Roof Pitch 4/12

Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction.

Ck# - 2920 -

JW: Called spoke w/ M. Saad

3408

COLUMBIA COUNTY BUILDING DEPARTMENT
135 NE Hernando Ave.. Suite B-21
Lake City, FL 32055
Office: 386-758-1008 Fax: 386-758-2160

NOTARIZED DISCLOSURE STATEMENT
FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed $75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved for yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F .I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.
I understand that if I am not physically doing the work or physically supervising free labor from friends or relatives, that I must hire licensed contractors, i.e. electrician, plumber, mechanical (heating & air conditioning), etc. I further understand that the violation of not physically doing the work, and the use of unlicensed contractors at the construction site, will cause the project to be shut down by the inspection staff of the Columbia County Building Department. Additionally, state statutes allows for additional penalties. I also understand that if this violation does occur, that in order for the job to proceed, I will have a licensed contractor come in and obtain a new permit as taking the job over. I understand that if I hire subcontractors under a contract price, that they must be licensed to work in Columbia County, i.e. masonry, drywall, carpentry. Contractors licensed by the Columbia County Contractor Licensing Section or the State of Florida are required to have worker's compensation and liability coverage.

TYPE OF CONSTRUCTION

( ) Single Family Dwelling ( ) Two-Family Residence ( ) Farm Outbuilding
( ) Other _____ (X) Addition, Alteration, Modification or other Improvement

I Mitchell Laird, have been advised of the above disclosure statement for exemption from contractor licensing as an owner/builder. I agree to comply with all requirements provided for in Florida Statutes ss.489.103(7) allowing this exception for the construction permitted by Columbia County Building Permit Number _____

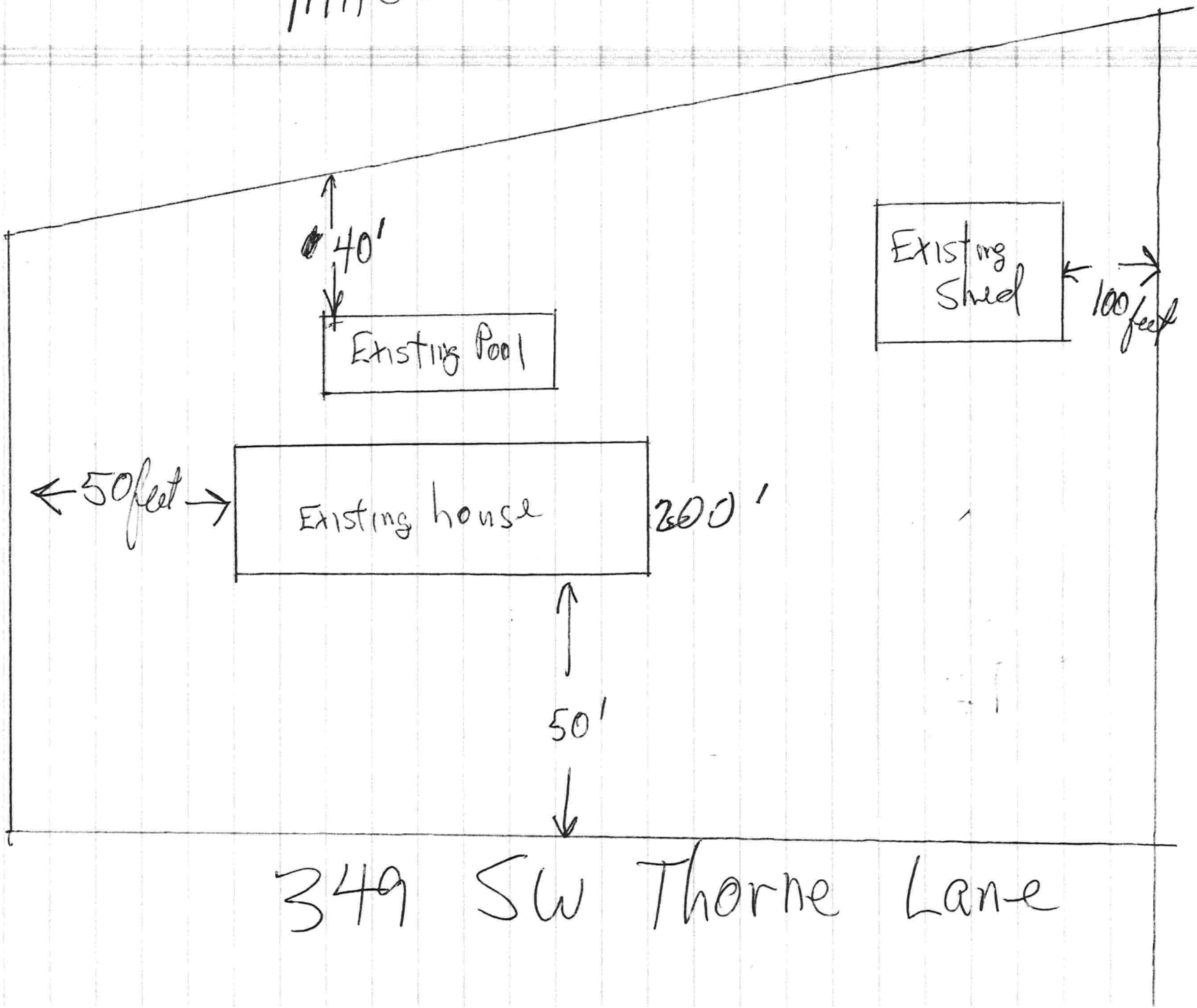
Mitchell Laird
Owner Builder Signature Date

FLORIDA NOTARY
The above signer is personally known to me or produced identification FL DL
Notary Signature Lawanda Y. Rentz Date 02-25-08

LAWANDA Y. RENTZ
MY COMMISSION # DD 710646
EXPIRES: October 29, 2011
Bonded Thru Notary Public Underwriters

FOR BUILDING DEPARTMENT USE ONLY
I hereby certify that the above listed owner/builder has been notified of the disclosure statement in Florida Statutes ss 489.103(7). Date_____ Building Official/Representative_____

Mitch Saad



349 SW Thorne Lane

NOTICE OF COMMENCEMENT

County Clerk's Office Stamp or Seal

Tax Parcel Identification Number 30-7S-17-10058-121

THE UNDERSIGNED hereby gives notice that improvements will be made to certain real property, and in accordance with Section 713.13 of the Florida Statutes, the following information is provided in this **NOTICE OF COMMENCEMENT**.

1. Description of property *(legal description)*:
   a) Street *(job)* Address: 349 SW Thorne Lane, Fort White, FL 32038
2. General description of improvements: _____
3. Owner Information
   a) Name and address: Mitchell Saad 349 SW Thorne Lane, Ft White FL 32038
   b) Name and address of fee simple titleholder (if other than owner) Same
   c) Interest in property _____
4. Contractor Information
   a) Name and address: ~~Owner~~ Mitchell Saad 349 SW Thorne Lane, Fort White, FL 32038
   b) Telephone No.: 386-454-7298 Fax No. (Opt.) _____
5. Surety Information
   a) Name and address: NA
   b) Amount of Bond: _____
   c) Telephone No.: _____
6. Lender
   a) Name and address: NA
   b) Phone No. _____

Inst:200812004462 Date:3/4/2008 Time:4:27 PM
DC,P.DeWitt Cason,Columbia County Page 1 of 1

7. Identity of person within the State of Florida designated by owner upon whom notices or other documents may be served:
   a) Name and address: _____
   b) Telephone No.: _____ Fax No. (Opt.) _____

8. In addition to himself, owner designates the following person to receive a copy of the Lienor's Notice as provided in Section 713.13(1)(b), Florida Statutes:
   a) Name and address: _____
   b) Telephone No.: _____ Fax No. (Opt.) _____

9. Expiration date of Notice of Commencement **(the expiration date is one year from the date of recording unless a different date is specified)**: _____

WARNING TO OWNER: ANY PAYMENTS MADE BY THE OWNER AFTER THE EXPIRATION OF THE NOTICE OF COMMENCEMENT ARE CONSIDERED IMPROPER PAYMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA STATUTES, AND CAN RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT YOUR LENDER OR AN ATTORNEY BEFORE COMMENCING WORK OR RECORDING YOUR NOTICE OF COMMENCEMENT.

STATE OF FLORIDA
COUNTY OF COLUMBIA

10. Mitchell Saad
Signature of Owner or Owner's Authorized Office/Director/Partner/Manager

Mitchell Saad
Print Name

The foregoing instrument was acknowledged before me, a Florida Notary, this 4th day of March, 20 08, by: Mitchell SAAD as Owner **(type of authority, e.g. officer, trustee, attorney fact)** for N/A **(name of party on behalf of whom instrument was executed)**.

Personally Known ____ OR Produced Identification ____ Type DL

Notary Signature Gale Tedder

GALE TEDDER
MY COMMISSION # DD 333586
EXPIRES: June 28 2008
Bonded Thru Notary Public Underwriters

Notary Stamp or Seal

---AND---

11. Verification pursuant to Section 92 525, Florida Statutes. Under penalties of perjury, I declare that I have read the foregoing and that the facts stated in it are true to the best of my knowledge and belief.

Mitchell Saad
Signature of Natural Person Signing (in line #10 above.)

26816

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1TGV8228Z0121095348

Truss Fabricator:	Anderson Truss Company
Job Identification:	8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk . **
Truss Count:	18
Model Code:	Florida Building Code 2004 and 2006 Supplement
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Version 7.36.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-02 -Partially Enclosed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

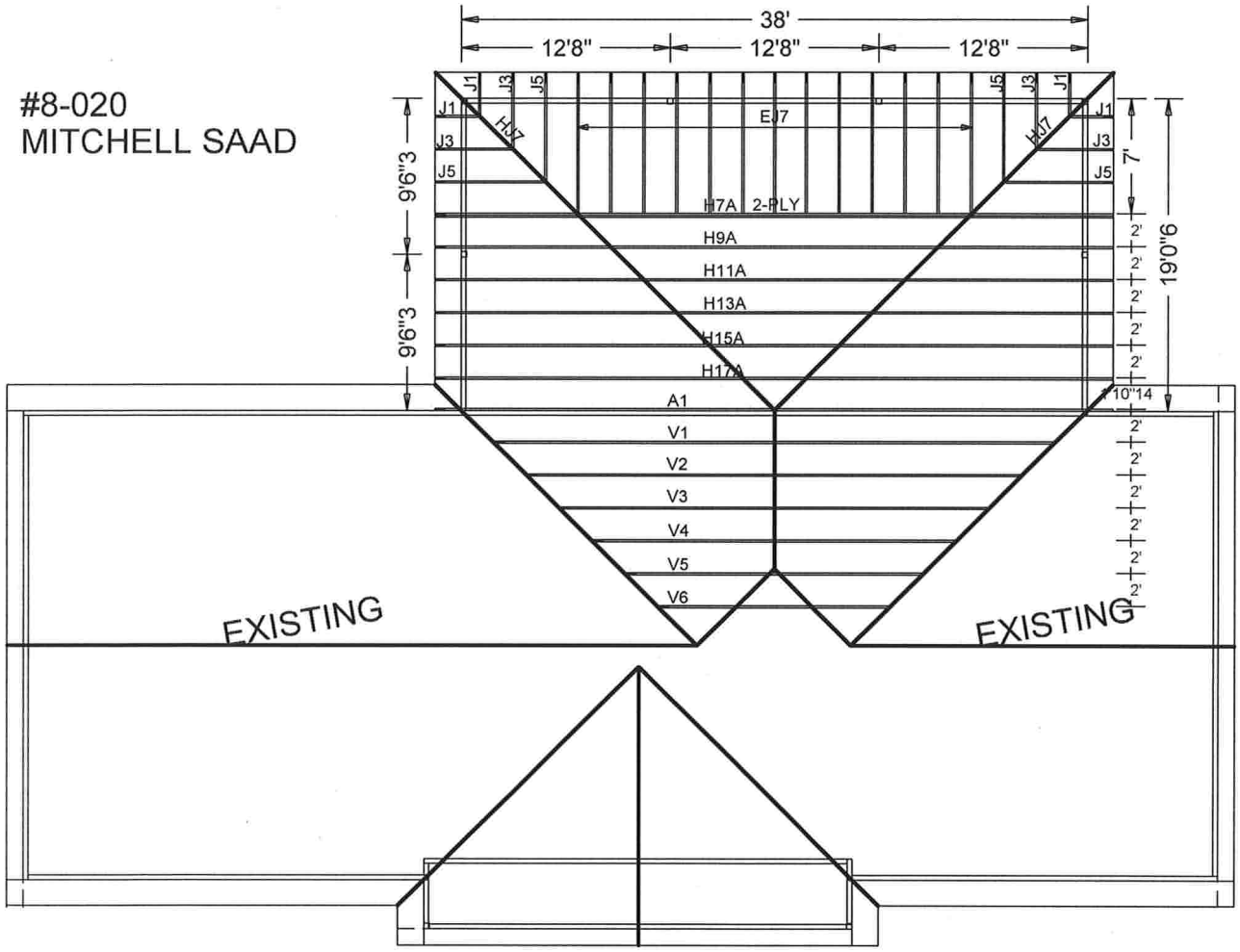
Details: VALTRUSS-

Seal Date: 04/21/2008

-Truss Design Engineer-
James F. Collins Jr.
Florida License Number: 52212
1950 Marley Drive
Haines City, FL 33844

#	Ref	Description	Drawing#	Date
1	32259	H7A	08112010	04/21/08
2	32260	H9A	08112002	04/21/08
3	32261	H11A	08112001	04/21/08
4	32262	H13A	08112003	04/21/08
5	32263	H15A	08112004	04/21/08
6	32264	H17A	08112007	04/21/08
7	32265	V1	08112001	04/21/08
8	32266	V2	08112002	04/21/08
9	32267	V3	08112003	04/21/08
10	32268	V4	08112004	04/21/08
11	32269	V5	08112005	04/21/08
12	32270	V6	08112006	04/21/08
13	32271	A1	08112007	04/21/08
14	32272	J5	08112009	04/21/08
15	32273	J3	08112005	04/21/08
16	32274	J1	08112006	04/21/08
17	32275	EJ7	08112008	04/21/08
18	32276	HJ7	08112011	04/21/08

#8-020
MITCHELL SAAD



JOB DESCRIPTION:: OWNER BUILDER
/: Mitchell Saad

JOB NO:
8-020

PAGE NO:
1 OF 1

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - H7A)

Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP #2:
Bot chord 2x4 SP #2 :B2 2x6 SP #1 Dense:
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

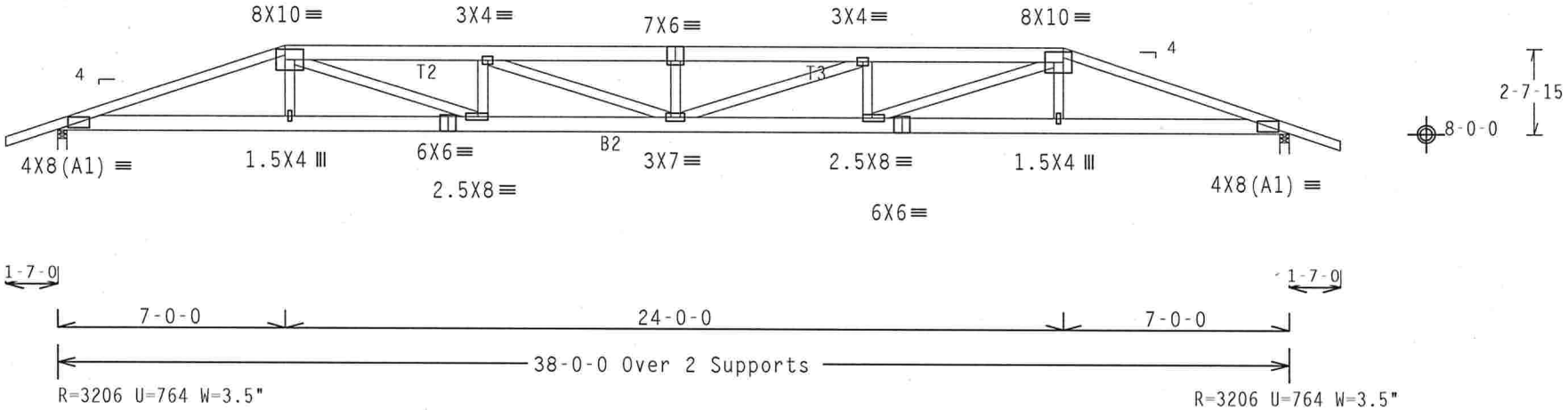
2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Roof overhang supports 2.00 psf soffit load.

#1 hip supports 7-0-0 jacks with no webs.

Calculated vertical deflection is 0.55" due to live load and 0.84" due to dead load at X = 19-0-0.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 21 08

TC LL	20.0 PSF	REF	R8228- 32259
TC DL	10.0 PSF	DATE	04/21/08
BC DL	10.0 PSF	DRW	HCUSR8228 08112010
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	83754
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGV8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - H9A)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

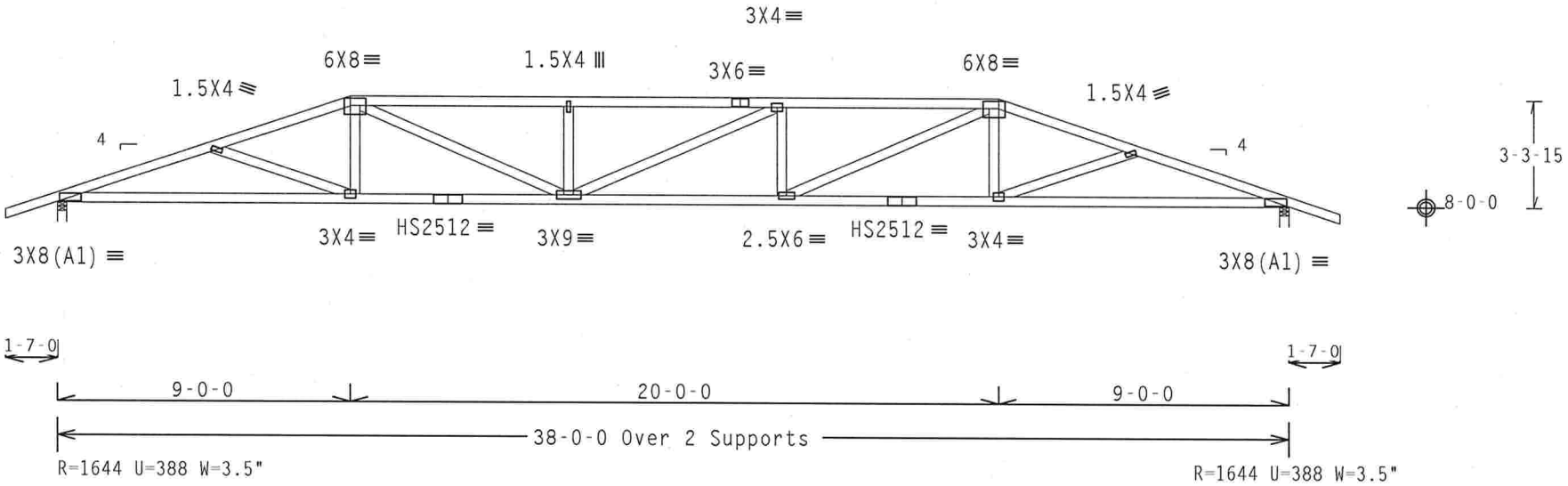
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Calculated vertical deflection is 0.42" due to live load and 0.62" due to dead load at X = 15-8-9.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424.13 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
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JAMES F. COLLINS LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Apr

TC LL	20.0 PSF	REF R8228- 32260
TC DL	10.0 PSF	DATE 04/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08112002
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 83759
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGV8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - H11A)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

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In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

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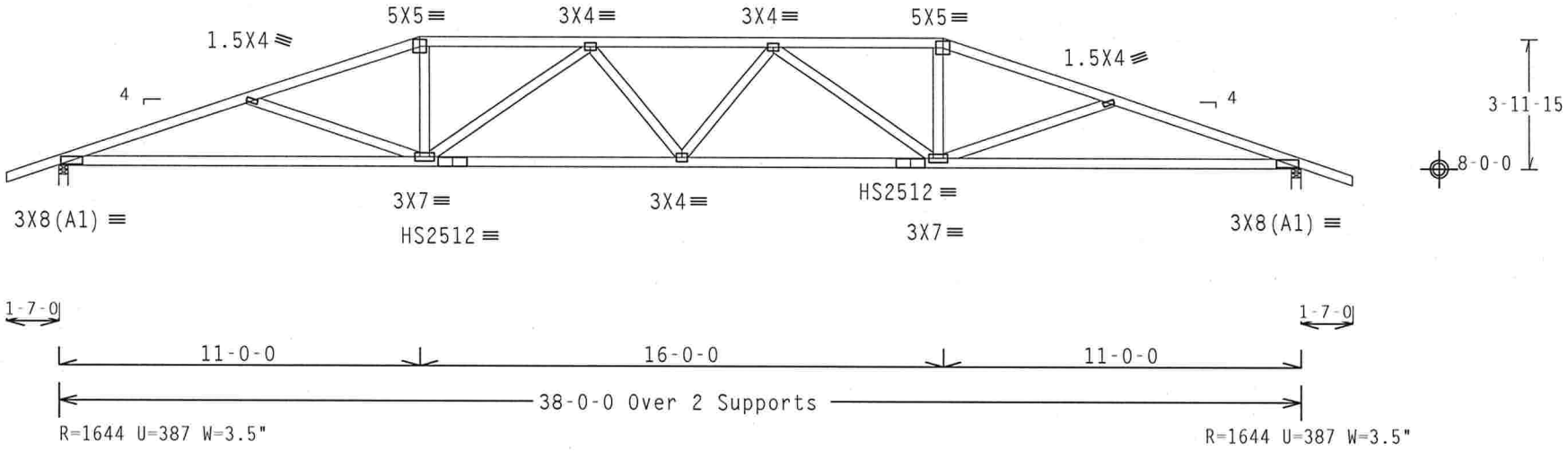

Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave | Cq/RT=1.00(1.25)/0(0) | 7.36.0424.13 | QTY:1 | FL/-/4/-/-/R/- | Scale =.1875"/Ft.

ALPINE
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FL Certificate of Authorization # 0 279

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IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W.H/SS/K) ASTM A653 GRADE 40/60 (W. K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

JAMES F. COLLINS, JR. LICENSED No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Apr

TC LL	20.0 PSF	REF	R8228- 32261
TC DL	10.0 PSF	DATE	04/21/08
BC DL	10.0 PSF	DRW	HCUSR8228 08112001
BC LL	0.0 PSF		HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN-	83764
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGV8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - H13A)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

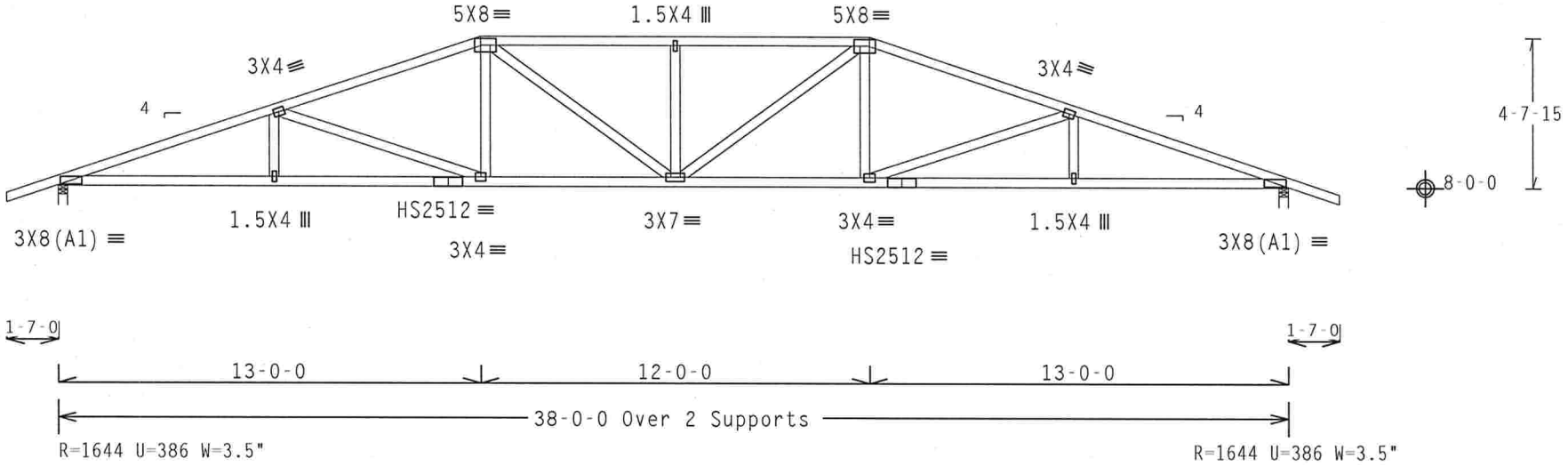
Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 1 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 24,2008

TC LL	20.0 PSF	REF	R8228- 32262
TC DL	10.0 PSF	DATE	04/21/08
BC DL	10.0 PSF	DRW	HCUSR8228 08112003
BC LL	0.0 PSF	HC-ENG	TCE/DF *
TOT.LD.	40.0 PSF	SEQN-	83769
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGV8228Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

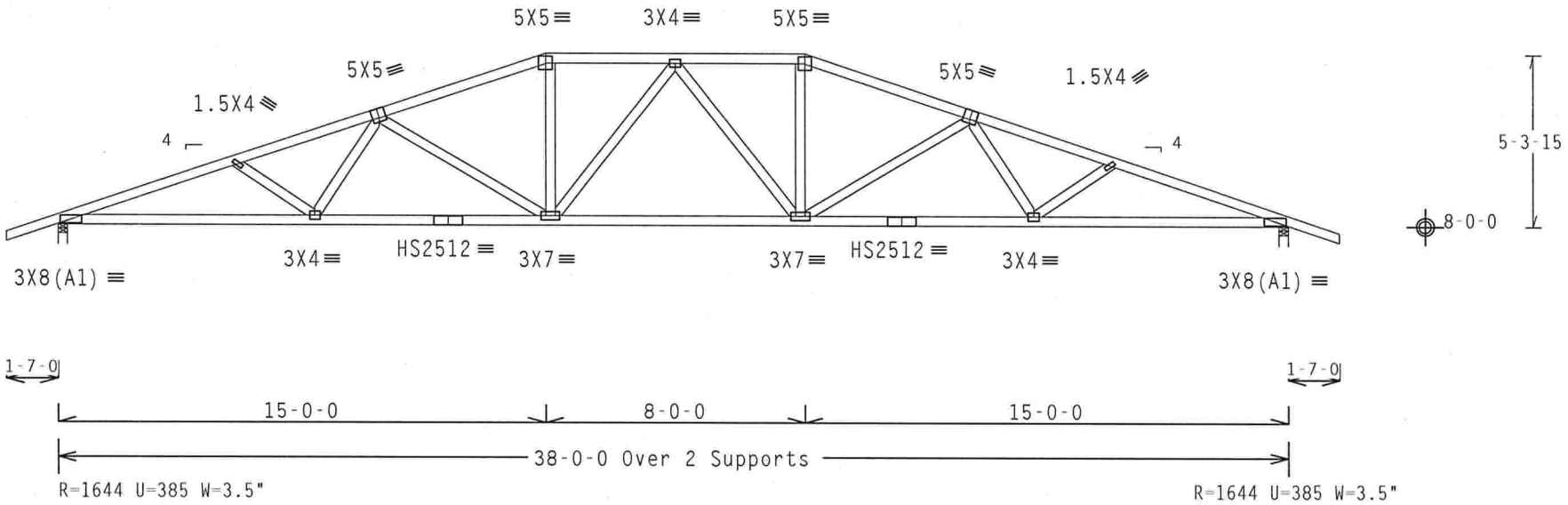
Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 1 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 21,2008

TC LL	20.0 PSF	REF R8228- 32263
TC DL	10.0 PSF	DATE 04/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08112004
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 83774
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGV8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - H17A)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

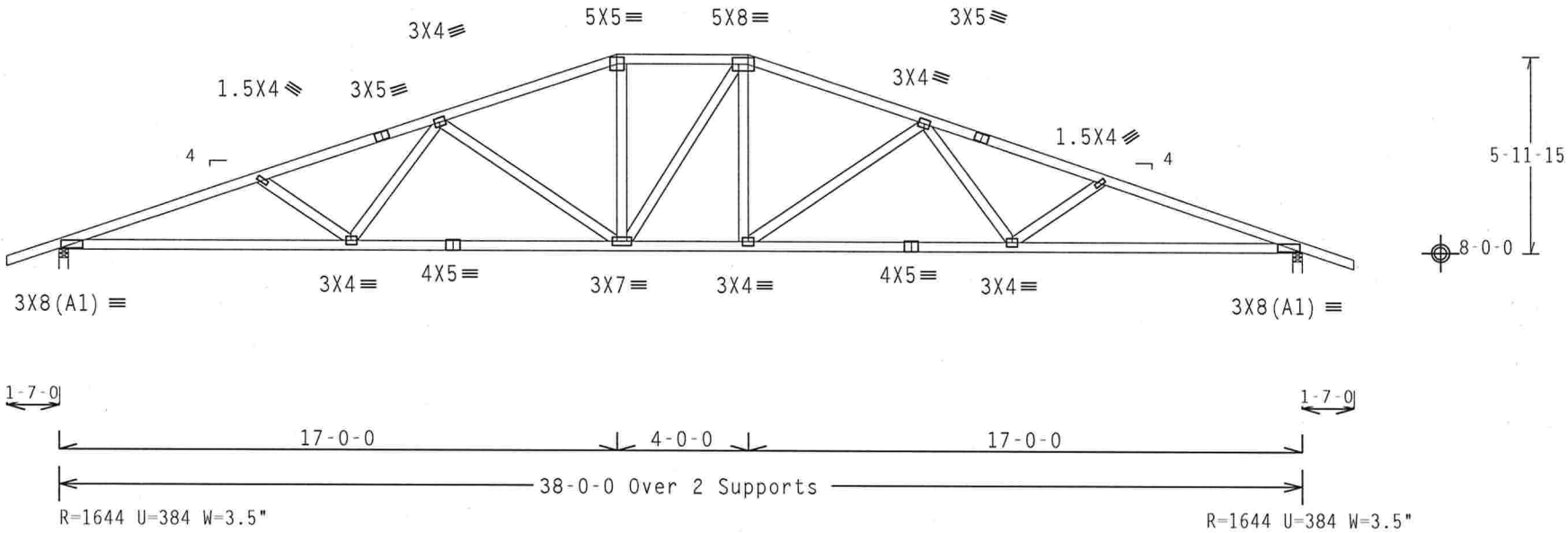
Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424.13 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS LICENSE No 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Apr

TC LL	20.0 PSF	REF R8228- 32264
TC DL	10.0 PSF	DATE 04/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08112007
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 83779
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGV8228Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

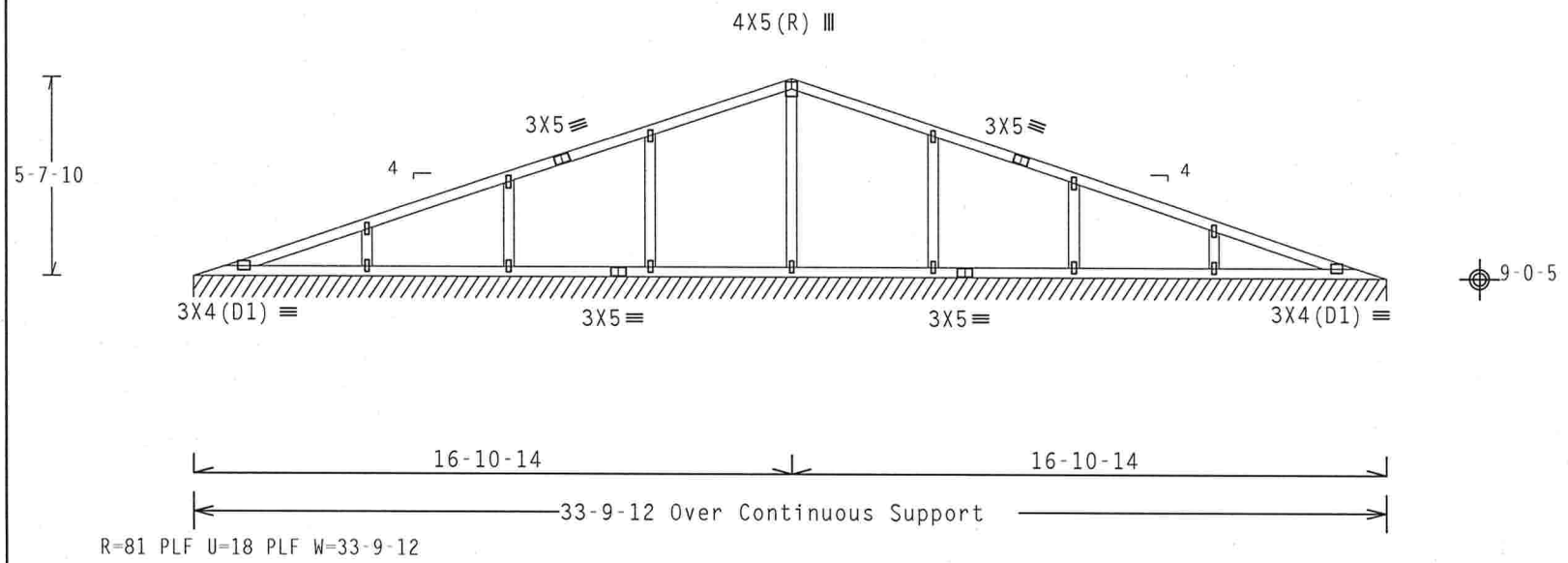
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424.10 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Apr 21,2008

TC LL	20.0 PSF	REF R8228- 32265
TC DL	10.0 PSF	DATE 04/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08112001
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 84355
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGV8228Z01

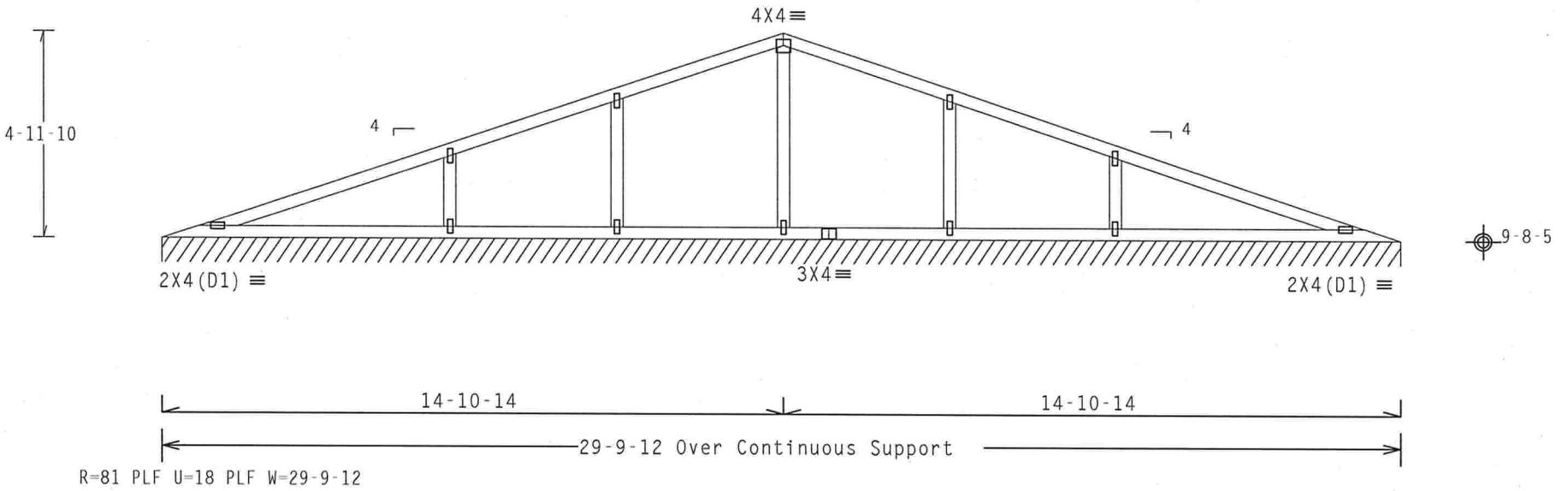
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 2 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 21 '08

TC LL	20.0 PSF	REF R8228- 32266
TC DL	10.0 PSF	DATE 04/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08112002
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 84360
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGV8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - V3)

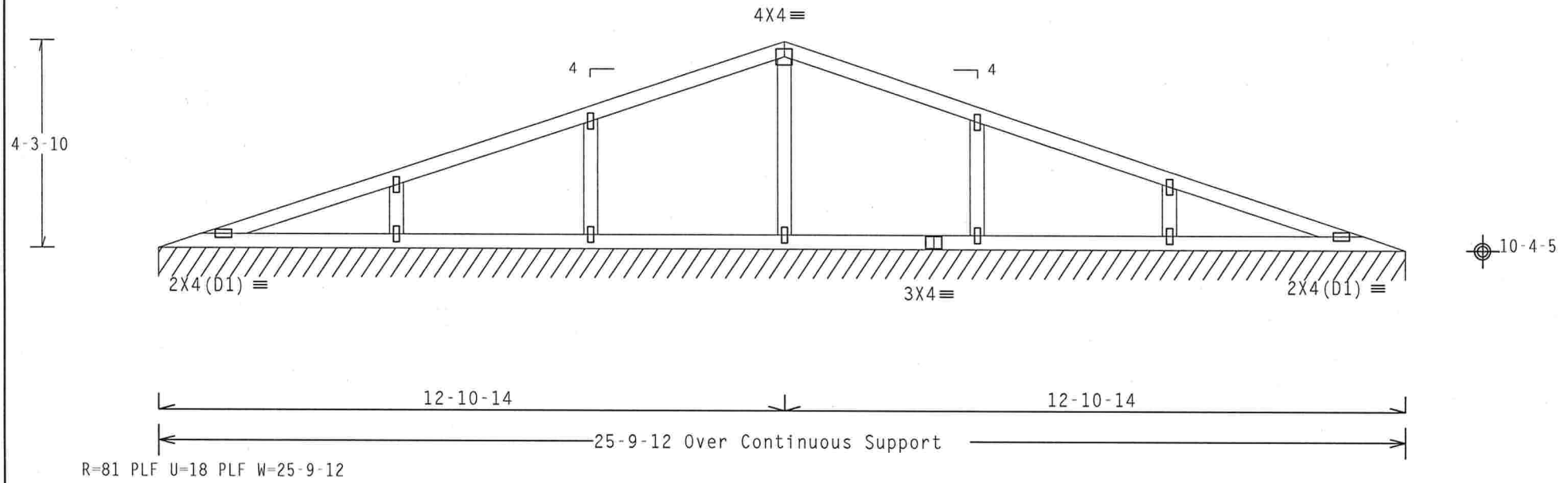
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 1 QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 21 08

TC LL	20.0 PSF	REF	R8228- 32267
TC DL	10.0 PSF	DATE	04/21/08
BC DL	10.0 PSF	DRW	HCUSR8228 08112003
BC LL	0.0 PSF	HC-ENG	DAL/AP *
TOT.LD.	40.0 PSF	SEQN-	84365
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGV8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - V4)

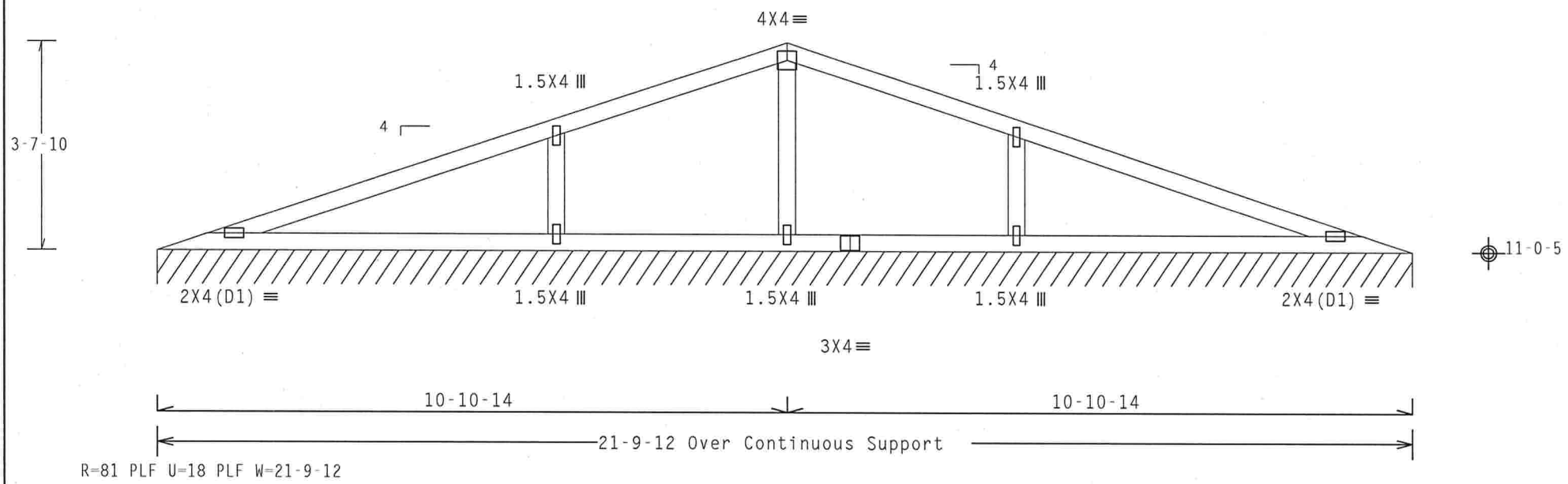
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave | Cq/RT=1.00(1.25)/0(0) | 7.36.0424 | QTY:1 | FL/-/4/-/-/R/- | Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W.H/SS/K) ASTM A653 GRADE 40/60 (W. K/H.SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 21 '08

TC LL	20.0 PSF	REF	R8228- 32268
TC DL	10.0 PSF	DATE	04/21/08
BC DL	10.0 PSF	DRW	HCUSR8228 08112004
BC LL	0.0 PSF	HC-ENG	DAL/AP *
TOT.LD.	40.0 PSF	SEQN-	84370
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGV8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - V5)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

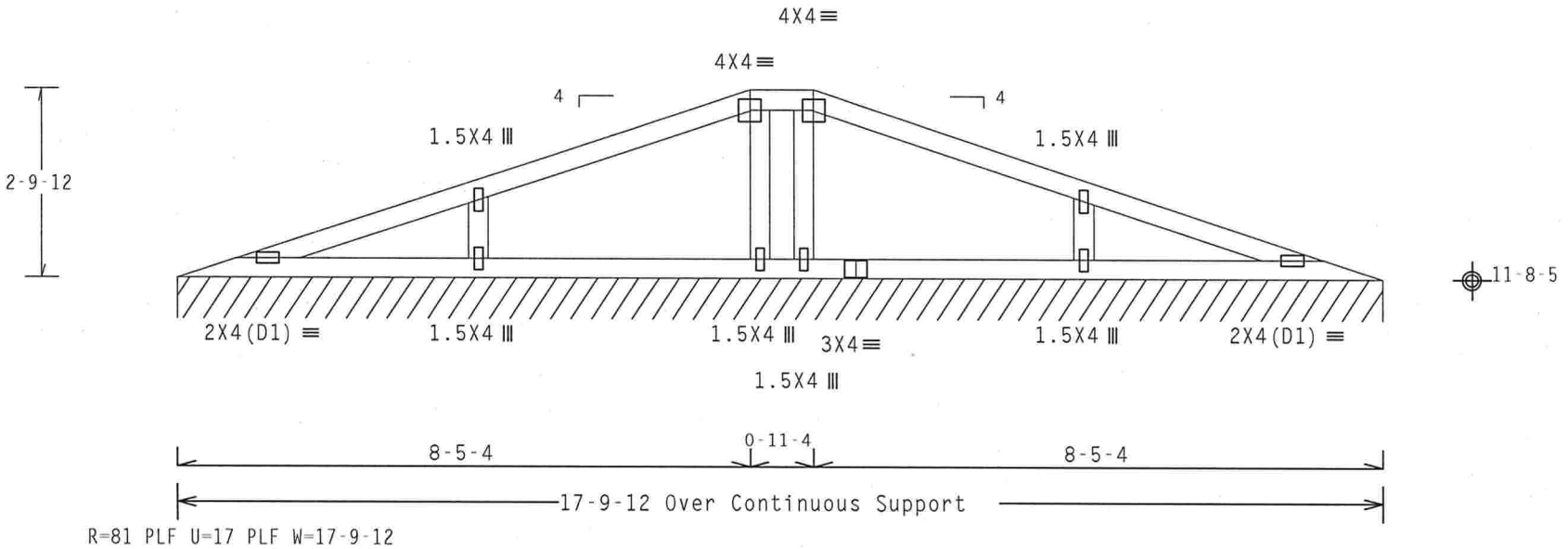
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

See DWG VALTRUSS0207 for valley details.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.36.0424.10 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES F. COLLINS LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Apr

TC LL	20.0 PSF	REF	R8228- 32269
TC DL	10.0 PSF	DATE	04/21/08
BC DL	10.0 PSF	DRW	HCUSR8228 08112005
BC LL	0.0 PSF	HC-ENG	DAL/AP *
TOT.LD.	40.0 PSF	SEQN-	84375
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGV8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - V6)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

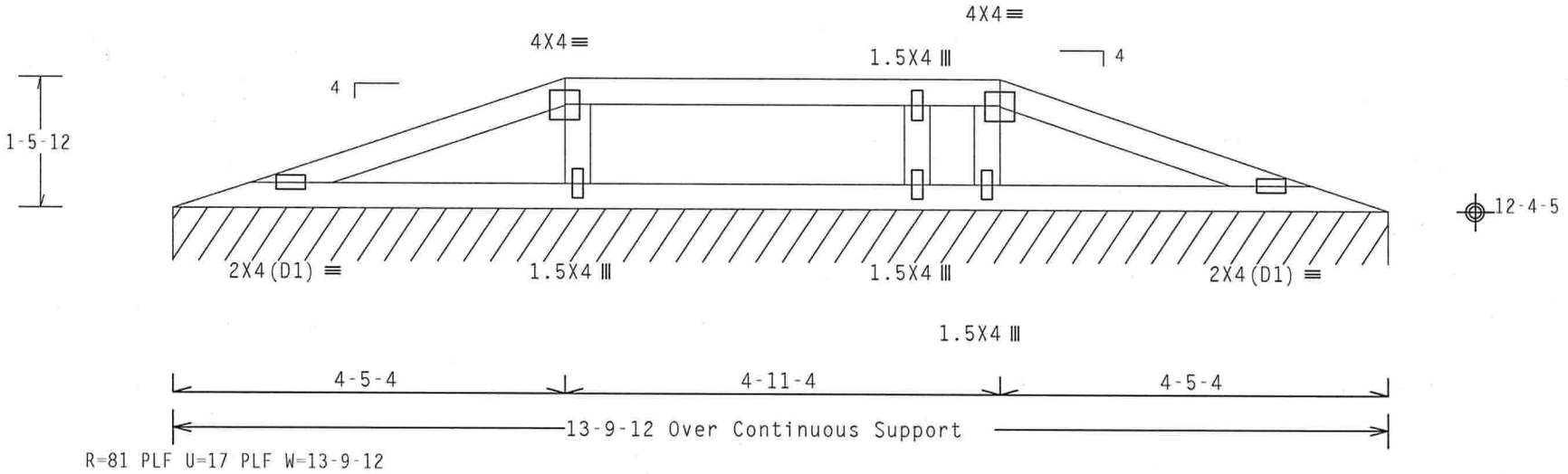
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

See DWG VALTRUSS0207 for valley details.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424.12 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Apr

TC LL	20.0 PSF	REF R8228- 32270
TC DL	10.0 PSF	DATE 04/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08112006
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 84379
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGV8228Z01

WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment
According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:
YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

Mitchell Sard
Owners Signature

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Mitchell Sard
Contractor's Signature (Permitee)

Contractor's License Number_____
Columbia County
Competency Card Number_____

Affirmed under penalty of perjury to by the Contractor and subscribed before me this 25 **day of** Feb. **20**08.

Personally known____ **or Produced Identification** FL DL

Lawanda Y. Rentz
State of Florida Notary Signature (For the Contractor)

SEAL:

LAWANDA Y. RENTZ
MY COMMISSION # DD 710646
EXPIRES: October 29, 2011
Bonded Thru Notary Public Underwriters

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - A1)

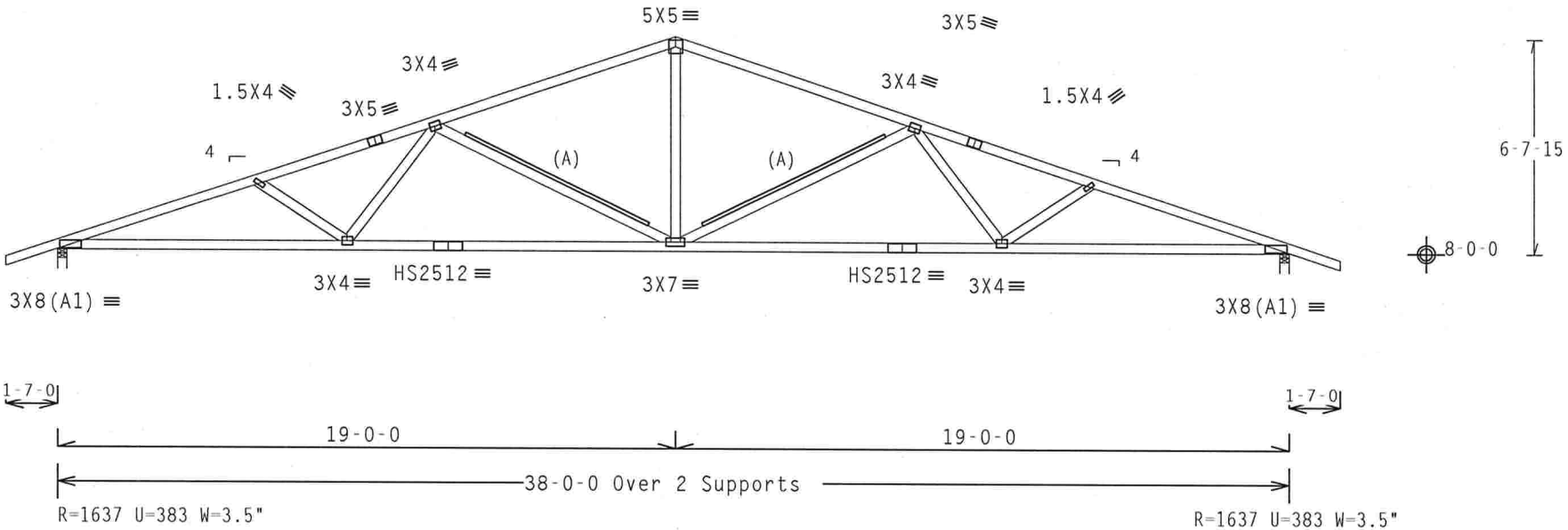
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

(A) 1x4 #3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 21 '08

TC LL	20.0 PSF	REF R8228- 32271
TC DL	10.0 PSF	DATE 04/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08112007
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 84348
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGV8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - J5)

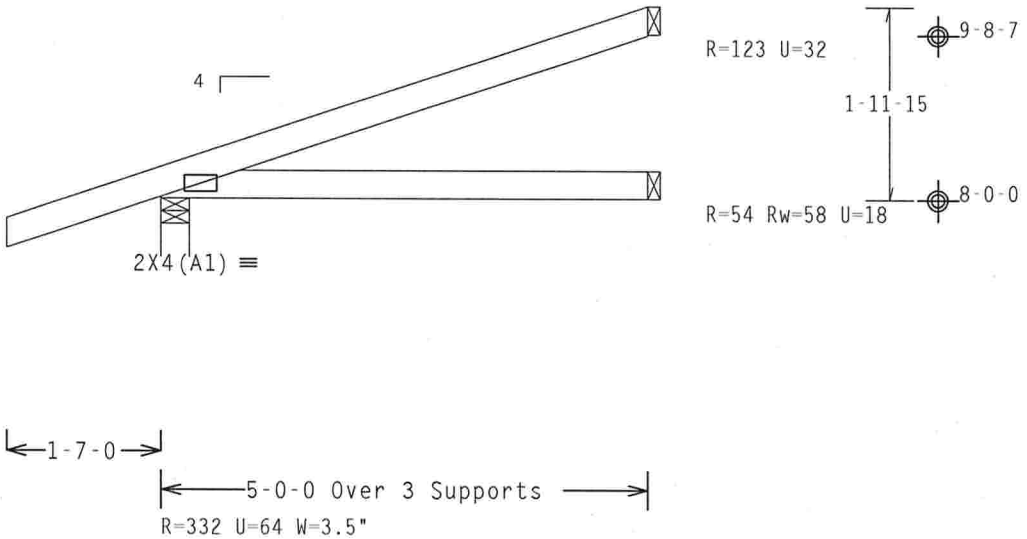
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 21 '08

TC LL	20.0 PSF	REF	R8228- 32272
TC DL	10.0 PSF	DATE	04/21/08
BC DL	10.0 PSF	DRW	HCUSR8228 08112009
BC LL	0.0 PSF	HC-ENG	DAL/AP *
TOT.LD.	40.0 PSF	SEQN-	71766
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGV8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - J3)

THIS WAS PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

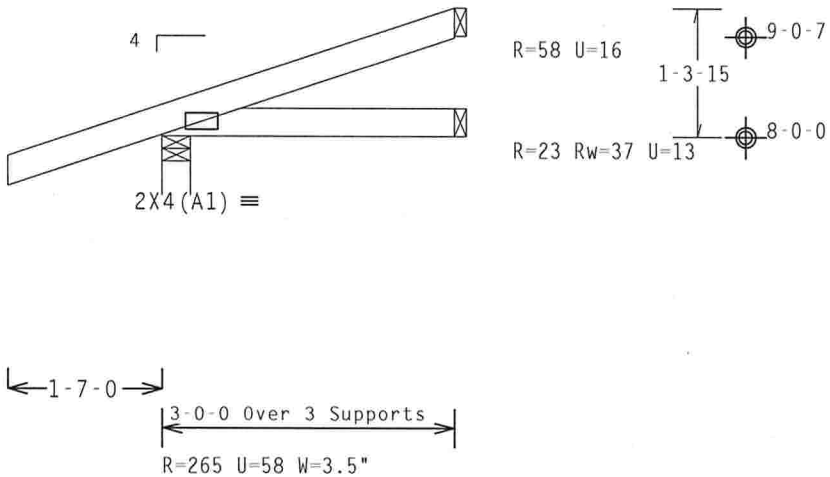
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 21 '08

TC LL	20.0 PSF	REF R8228- 32273
TC DL	10.0 PSF	DATE 04/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08112005
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 71774
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGV8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - J1)

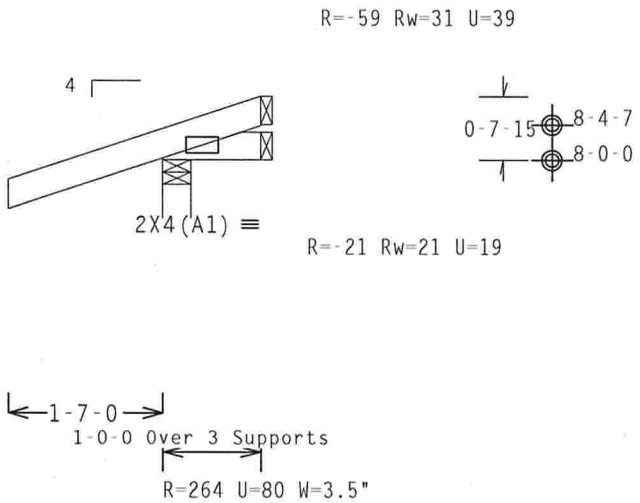
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 21 '08

TC LL	20.0 PSF	REF	R8228- 32274
TC DL	10.0 PSF	DATE	04/21/08
BC DL	10.0 PSF	DRW	HCUSR8228 08112006
BC LL	0.0 PSF	HC-ENG	DAL/AP
TOT.LD.	40.0 PSF	SEQN-	71757
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGV8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - EJ7)

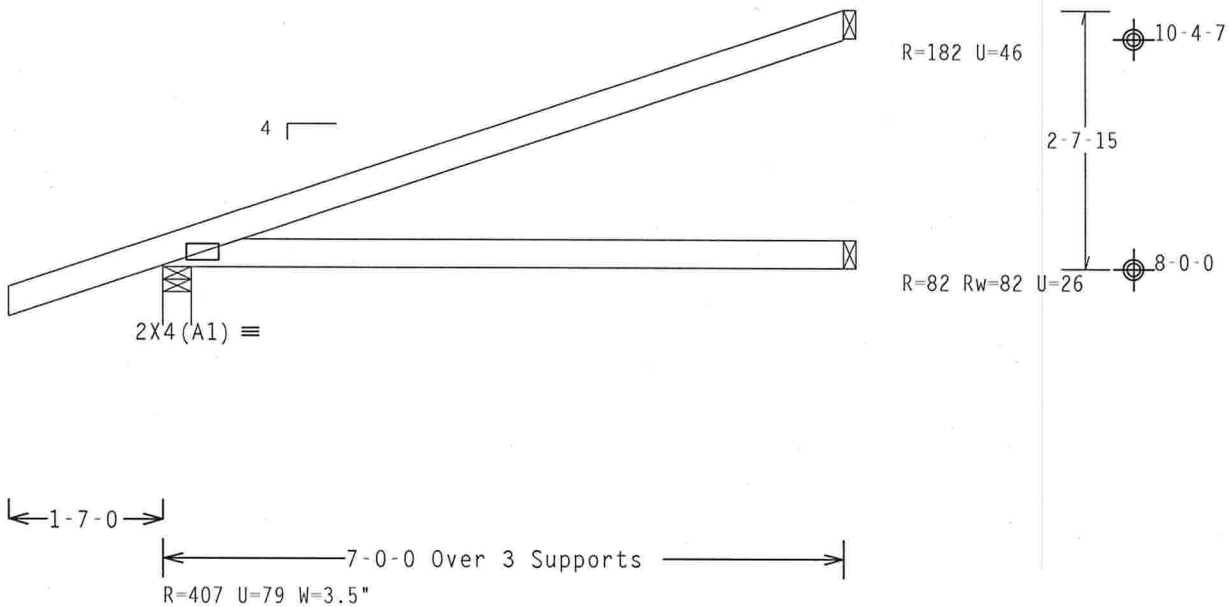
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W. K/H.SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 21 08

TC LL	20.0 PSF	REF	R8228- 32275
TC DL	10.0 PSF	DATE	04/21/08
BC DL	10.0 PSF	DRW	HCUSR8228 08112008
BC LL	0.0 PSF	HC-ENG	DAL/AP *
TOT.LD.	40.0 PSF	SEQN-	71760
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGV8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - HJ7)

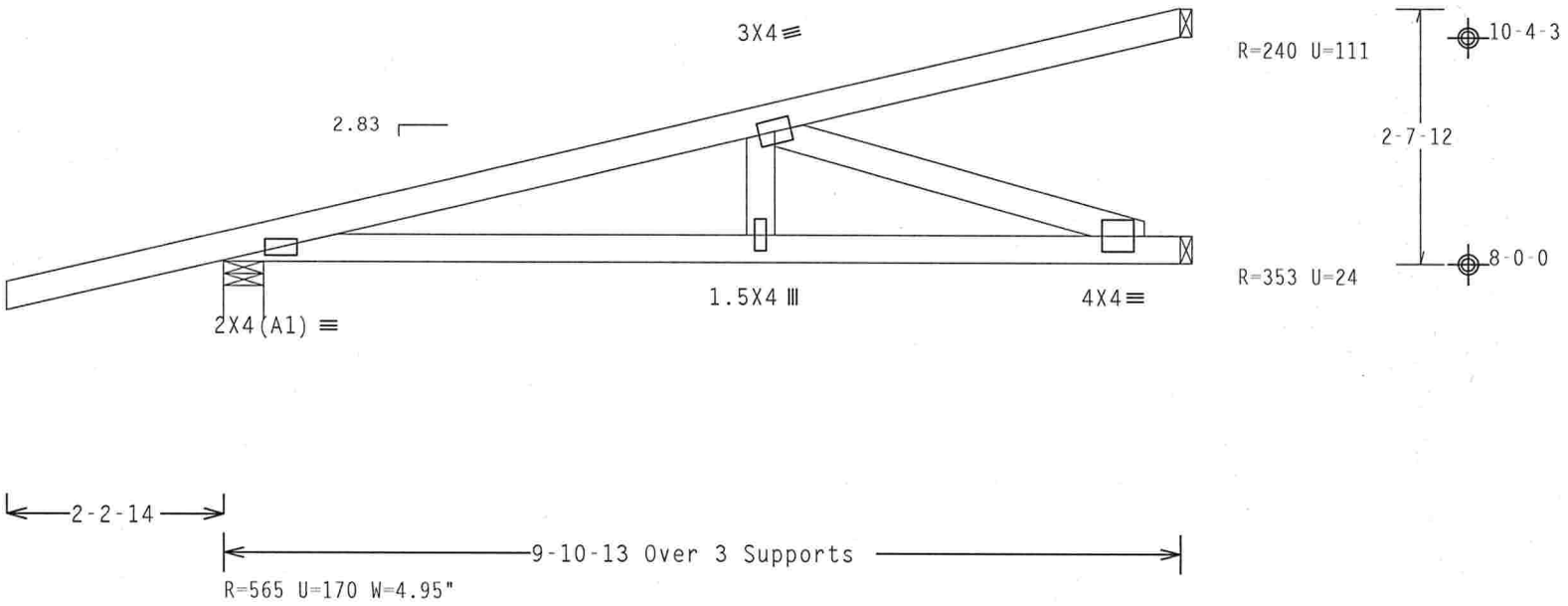
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Hipjack supports 7-0-0 setback jacks with no webs.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424.11 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 24,08

TC LL	20.0 PSF	REF	R8228- 32276
TC DL	10.0 PSF	DATE	04/21/08
BC DL	10.0 PSF	DRW	HCUSR8228 08112011
BC LL	0.0 PSF	HC-ENG	DAL/AP
TOT.LD.	40.0 PSF	SEQN-	71770
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGV8228Z01

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR SBC 110 MPH, ASCE 7-93 110 MPH OR ASCE 7-98, ASCE 7-02 OR ASCE 7-05 130 MPH. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

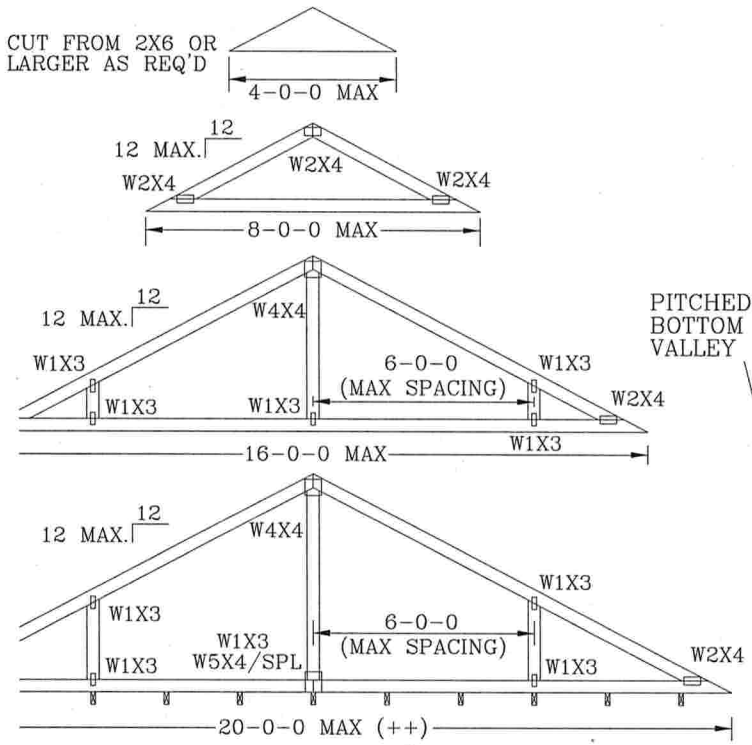
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

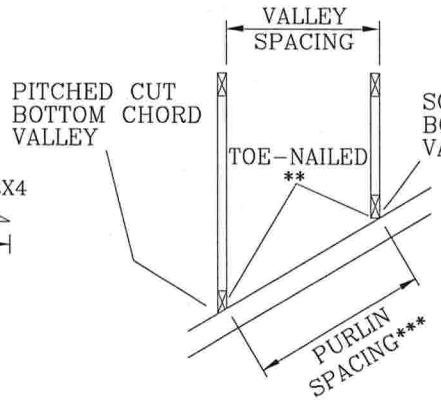
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

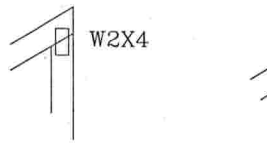
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



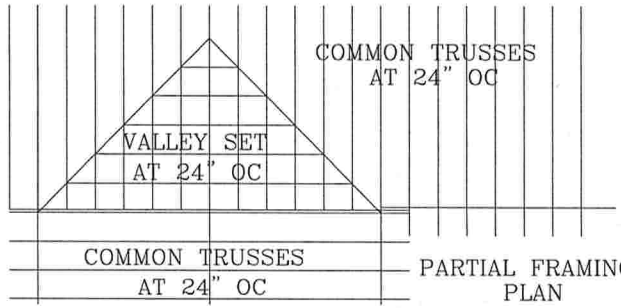
SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.





OPTIONAL STUB END DETAIL

OPTIONAL HIP JOINT DETAIL



PARTIAL FRAMING PLAN

THIS DRAWING REPLACES DRAWING A105

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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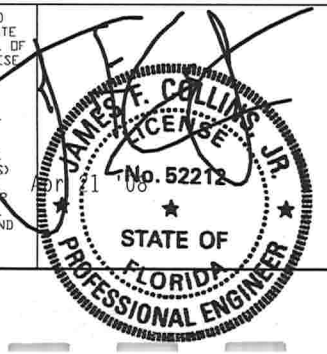


TC LL	30	30	40 PSF	REF VALLEY DETAIL
TC DL	20	15	7 PSF	DATE 2/23/07
BC DL	10	10	10 PSF	DRWG VALTRUSS0207
BC LL	0	0	0 PSF	-ENG MLH/KAR
TOT. LD.	60	55	57 PSF	
DUR.FAC.	1.25/1.33	1.15	1.15	
SPACING		24"		

2008.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY. MAKE PROVISION FOR ADEQUATE DRAINAGE.
7. PROVIDE LATERAL BRACING FOR THE TOP EDGE AT EACH END OF COMPONENT.
8. PROVIDE LATERAL BRACING FOR THE BOTTOM EDGE AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 12" OC, STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD		FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	BEAM	WEIGHT		10	PLF	00-00-00	19-00-06		0.90
CONCENTRATED	ROOF	LIVE	TOP	1603	LBS	07-02-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	1603	LBS	07-02-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	LIVE	TOP	822	LBS	09-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	822	LBS	09-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	DEAD	TOP	822	LBS	11-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	LIVE	TOP	822	LBS	13-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	822	LBS	13-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	DEAD	TOP	822	LBS	15-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	LIVE	TOP	822	LBS	17-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	822	LBS	17-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	LIVE	TOP	822	LBS	15-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	LIVE	TOP	822	LBS	11-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	LIVE	TOP	166	LBS	05-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	166	LBS	05-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	LIVE	TOP	133	LBS	03-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	133	LBS	03-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	LIVE	TOP	132	LBS	01-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	132	LBS	01-01-00	MINBRG=2.50"		0.90

2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

DESIGN CRITERIA : MSI: 0.52 VSI: 0.54 RSI: 0.96

LIVE LOAD	=	20 PSF
DEAD LOAD	=	20 PSF
TOTAL LOAD	=	40 PSF

ROOF LEFT SPAN CARR. : 0.00 FT
ROOF RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 360
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICC-ES	ESR-1254
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP LSL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

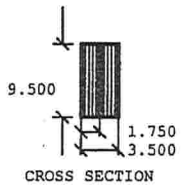
PROJECT DESIGNER TO PROVIDE ADEQUATE UPLIFT AND/OR LATERAL ANCHORAGE FOR THIS BEAM AND THE MEMBERS ATTACHING TO IT.

SUPPORT REACTIONS (LBS):

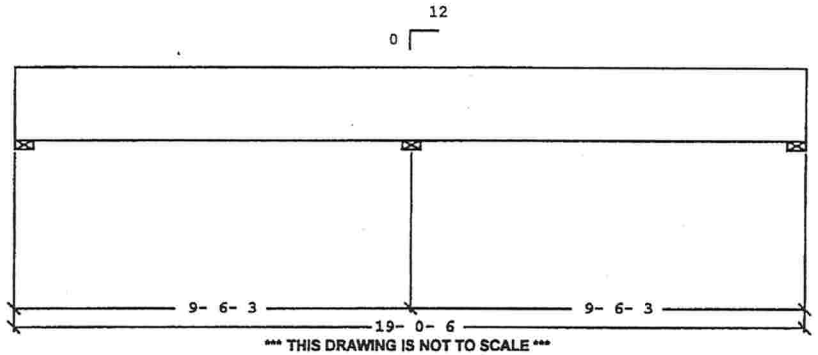
MAXIMUM BEARING NUMBER	1	2	3
DOWN	737	9483	2531
UPLIFT	---	---	---

MIN BEARING SIZES (IN-SX)
3- 0 3-12 3- 0

	MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE LOAD	0.11"	0.31"
*DEAD LOAD	0.12"	
TOTAL LOAD	0.18"	0.47"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL, LP LSL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL, LP LSL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL, LP LSL, CTR and LPI to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL, LP LSL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL, LSL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

04/25/08 IBC 2008.1

DWG # 0804-092

SHEET # 1A of 4

File: L:\Engineering\Wood-E 2008\APR 08\0804-092\Beam-A DownLoads.SPX

2008.1 Allowable Stress Design

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LOAD TABLE

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(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD		FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	BEAM	WEIGHT		10	PLF	00-00-00	19-00-06		0.90
CONCENTRATED	ROOF	LIVE	TOP	-764	LBS	07-02-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-388	LBS	09-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-387	LBS	11-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-386	LBS	13-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-385	LBS	15-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-384	LBS	17-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-80	LBS	01-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-64	LBS	05-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-58	LBS	03-01-00	MINBRG=2.50"		1.00

2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

DESIGN CRITERIA : MSI: 0.14 VSI: 0.15 RSI: 0.02

LIVE LOAD	=	20 PSF
DEAD LOAD	=	20 PSF
TOTAL LOAD	=	40 PSF

ROOF LEFT SPAN CARR. : 0.00 FT
ROOF RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 360
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICC-ES	ESR-1254
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

WARNING NOTES:

THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP LSL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL.

PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

PROVIDE ANCHORAGE FOR UPLIFT AT SUPPORTS. ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT DESIGNER.

ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

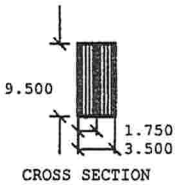
PROJECT DESIGNER TO PROVIDE ADEQUATE UPLIFT AND/OR LATERAL ANCHORAGE FOR THIS BEAM AND THE MEMBERS ATTACHING TO IT.

SUPPORT REACTIONS (LBS):

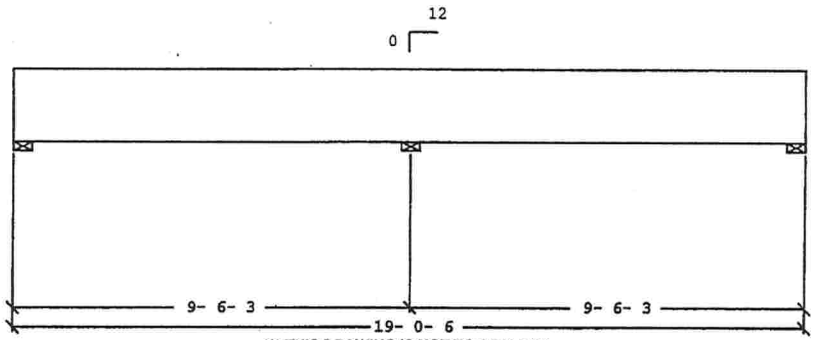
MAXIMUM BEARING	NUMBER 1	2	3
DOWN	147	113	96
UPLIFT	163	2090	592

MIN BEARING SIZES (IN-SX)
3- 0 3-12 3- 0

		MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE	LOAD	-0.05"	0.31"
*DEAD	LOAD	0.00"	
TOTAL	LOAD	-0.05"	0.47"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL, LP LSL and CTR, LP I-Joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.

LP LVL, LP LSL and CTR, LP I-Joist Specifications

* Supports and connections for LP LVL, LP LSL, CTR and LPI to be specific applications.
* Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d.
* Do not cut, notch, drill or alter LP LVL, LP LSL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL, LSL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

* A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR

LP is a registered trademark of Louisiana-Pacific Corporation.

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

04/25/08 IBC 2008.1

DWG # 0804-092

SHEET # 2A of 4

Anderson Truss / Saad / Beam-B **REGULAR DOWNLOADS** FL Woodford Plywood, Inc. 1504 South Mock Road Albany, GA 31705 (800) 342-6400

2008.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY. MAKE PROVISION FOR ADEQUATE DRAINAGE.
7. PROVIDE LATERAL BRACING FOR THE TOP EDGE AT EACH END OF COMPONENT.
8. PROVIDE LATERAL BRACING FOR THE BOTTOM EDGE AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	BEAM	WEIGHT		10 PLF	00-00-00	38-00-00		0.90
CONCENTRATED	ROOF	LIVE	TOP	204 LBS	07-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	LIVE	TOP	204 LBS	19-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	LIVE	TOP	204 LBS	21-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	LIVE	TOP	204 LBS	23-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	LIVE	TOP	204 LBS	25-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	LIVE	TOP	204 LBS	27-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	LIVE	TOP	204 LBS	29-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	LIVE	TOP	204 LBS	31-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	204 LBS	31-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	DEAD	TOP	204 LBS	29-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	DEAD	TOP	204 LBS	27-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	DEAD	TOP	204 LBS	25-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	DEAD	TOP	204 LBS	23-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	DEAD	TOP	204 LBS	21-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	DEAD	TOP	204 LBS	19-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	DEAD	TOP	204 LBS	17-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	DEAD	TOP	204 LBS	07-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	LIVE	TOP	204 LBS	09-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	LIVE	TOP	204 LBS	11-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	204 LBS	11-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	LIVE	TOP	204 LBS	13-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	LIVE	TOP	204 LBS	15-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	204 LBS	15-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	LIVE	TOP	204 LBS	17-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	204 LBS	13-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	DEAD	TOP	204 LBS	09-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	LIVE	TOP	166 LBS	05-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	166 LBS	33-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	LIVE	TOP	166 LBS	33-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	166 LBS	05-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	LIVE	TOP	133 LBS	03-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	133 LBS	35-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	DEAD	TOP	133 LBS	03-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	LIVE	TOP	133 LBS	35-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	LIVE	TOP	132 LBS	01-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	132 LBS	01-01-00	MINBRG=2.50"		0.90
CONCENTRATED	ROOF	LIVE	TOP	132 LBS	37-01-00	MINBRG=2.50"		1.25
CONCENTRATED	ROOF	DEAD	TOP	132 LBS	37-01-00	MINBRG=2.50"		0.90

DESIGN CRITERIA : MSI: 0.23 VSI: 0.18 RSI: 0.33

LIVE LOAD	=	20 PSF
DEAD LOAD	=	20 PSF
TOTAL LOAD	=	40 PSF

ROOF LEFT SPAN CARR. : 0.00 FT
ROOF RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 360
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICC-ES	ESR-1254
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

WARNING NOTES:

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PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

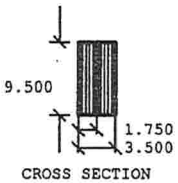
PROJECT DESIGNER TO PROVIDE ADEQUATE UPLIFT AND/OR LATERAL ANCHORAGE FOR THIS BEAM AND THE MEMBERS ATTACHING TO IT.

SUPPORT REACTIONS (LBS):

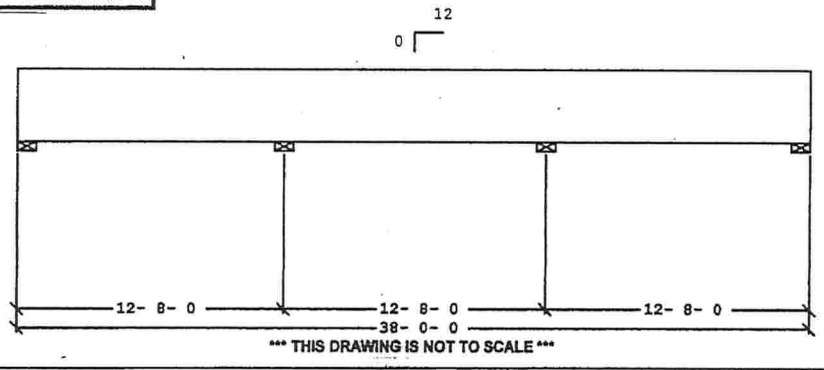
MAXIMUM BEARING	NUMBER 1	2	3	4
DOWN	790	2994	3003	818
UPLIFT	---	---	---	---

MIN BEARING SIZES (IN-SX)
3- 0 3- 8 3- 8 3- 0

	MAXIMUM DEFLECTIONS CALCULATED	ALLOWABLE
LIVE LOAD	0.06"	0.42"
*DEAD LOAD	0.09"	
TOTAL LOAD	0.12"	0.63"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection

Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.

Design Criteria

The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.

Miscellaneous Information

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LP LVL, LP LSL and CTR, LP I-Joist Specifications

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* Do not cut, notch, drill or alter LP LVL, LP LSL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL, LSL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use.

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LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

04/25/08 IBC 2008.1

DWG # 0804-092

SHEET # 3B of 4

File: L:\Engineering\Wood-E 2008\APR 08\0804-092\Beam-B DownLoads.SPX

2008.1 Allowable Stress Design

NOTE:
1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS. VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT. I DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE BUILDING DESIGN.
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING LP LVL TO SIZE.
6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY.
MAKE PROVISION FOR ADEQUATE DRAINAGE.
7. PROVIDE LATERAL BRACING FOR THE TOP EDGE AT EACH END OF COMPONENT.
8. PROVIDE LATERAL BRACING FOR THE BOTTOM EDGE AT EACH END OF COMPONENT.

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-1/2") NAILS AT 12" OC. STAGGER ROWS. NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-1/4") MAY BE USED, BUT HALF MUST BE DRIVEN FROM EACH FACE.

CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED.

LOAD TABLE

NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1). OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED.
(DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.)

2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E
DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES).

DISTRIBUTION	SOURCE	TYPE	TOP/SIDE	LOAD	FROM FT-IN-SX	TO FT-IN-SX	LOAD	LDF
UNIFORM	BEAM	WEIGHT		10 PLF	00-00-00	38-00-00		0.90
CONCENTRATED	ROOF	LIVE	TOP	-80 LBS	01-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-80 LBS	37-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-79 LBS	07-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-79 LBS	09-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-79 LBS	31-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-79 LBS	11-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-79 LBS	13-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-79 LBS	25-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-79 LBS	15-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-79 LBS	19-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-79 LBS	17-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-79 LBS	23-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-79 LBS	27-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-79 LBS	29-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-79 LBS	21-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-64 LBS	33-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-64 LBS	05-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-58 LBS	35-01-00	MINBRG=2.50"		1.00
CONCENTRATED	ROOF	LIVE	TOP	-58 LBS	03-01-00	MINBRG=2.50"		1.00

DESIGN CRITERIA : MSI: 0.05 VSI: 0.03 RSI: 0.01

LIVE LOAD	=	20 PSF
DEAD LOAD	=	20 PSF
TOTAL LOAD	=	40 PSF

ROOF LEFT SPAN CARR. : 0.00 FT
ROOF RIGHT SPAN CARR. : 0.00 FT

DEFLECTION CRITERIA :
LIVE LOAD DEFL: L / 360
TOTAL LOAD DEFL: L / 240

CODE COMPLIANCES :

	REPORT #
ICC-ES	ESR-1254
L.A. City	RR 25167
CCMC	11518-R
WISCONSIN	200124-W
N.Y. CITY	MEA 97-94-E
HUD	MR 1214D

WARNING NOTES:

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PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY.

MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS.

PROVIDE ANCHORAGE FOR UPLIFT AT SUPPORTS. ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT DESIGNER.

ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS.

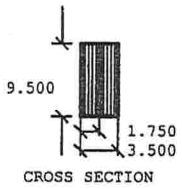
PROJECT DESIGNER TO PROVIDE ADEQUATE UPLIFT AND/OR LATERAL ANCHORAGE FOR THIS BEAM AND THE MEMBERS ATTACHING TO IT.

SUPPORT REACTIONS (LBS):

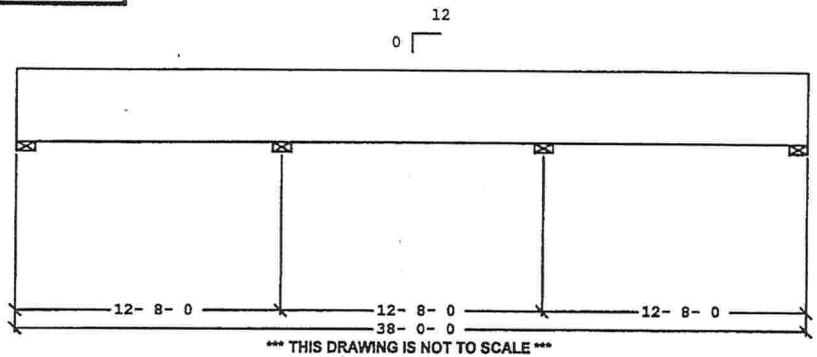
MAXIMUM BEARING NUMBER	1	2	3	4
DOWN	73	133	133	73
UPLIFT	126	451	452	132

MIN BEARING SIZES (IN-SX)			
3- 0	3- 8	3- 8	3- 0

MAXIMUM DEFLECTIONS	CALCULATED	ALLOWABLE
LIVE LOAD	-0.02"	0.42"
*DEAD LOAD	0.01"	
TOTAL LOAD	-0.02"	0.63"



CROSS SECTION



*** THIS DRAWING IS NOT TO SCALE ***

Handling & Erection	Miscellaneous Information	LP LVL, LP LSL and CTR, LP I-Joist Specifications
Temporary and permanent bracing for holding component plumb and for resisting lateral forces shall be designed and installed by others. No loads are to be applied to the component until after all the framing and fastening are completed. At no time shall loads greater than design loads be applied to the component.	The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code compliance approval and instructions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building code requirements, do not use this design. When this drawing is signed and sealed, the structural design is approved as shown in this drawing based on data provided by the customer. LP LVL, LP LSL and CTR, LP I-joists are made without camber and will deflect under load. Wood in direct contact with concrete must be protected as required by code. Continuous lateral support is assumed (wall, floor beam, etc.). LP does not provide on-site inspection. This drawing must have an Architect's or Engineer's seal affixed to be considered an Engineering document.	* Supports and connections for LP LVL, LP LSL, CTR and LPI to be specific applications. * Common nails driven parallel to glue lines shall be spaced a minimum of 4" for 10d and 3" for 8d. * Do not cut, notch, drill or alter LP LVL, LP LSL and CTR, LP I-Joists except as shown in published material from LP any use of LP LVL, LSL and CTR, LP I-Joists contrary to the limits set forth hereon,negates any express warranty of the product and LP disclaims all implied warranties including the implied warranties of merchantability and fitness for a particular use. * A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louisiana-Pacific Corporation.
Design Criteria		
The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous.		

LP Engineered Wood Products
2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.9878
National Wats 800.999.9105

04/25/08 IBC 2006.1

DWG # 0804-092

SHEET # 4B of 4

Permit # 26816

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1TGH8228Z0107132019

Truss Fabricator:	Anderson Truss Company
Job Identification:	8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , **
Truss Count:	18
Model Code:	Florida Building Code 2004 and 2006 Supplement
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Version 7.36.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-02 -Partially Enclosed

Seal Date: 04/07/2008

-Truss Design Engineer-
Doug Fleming
Florida License Number: 66648
1950 Marley Drive
Haines City, FL 33844

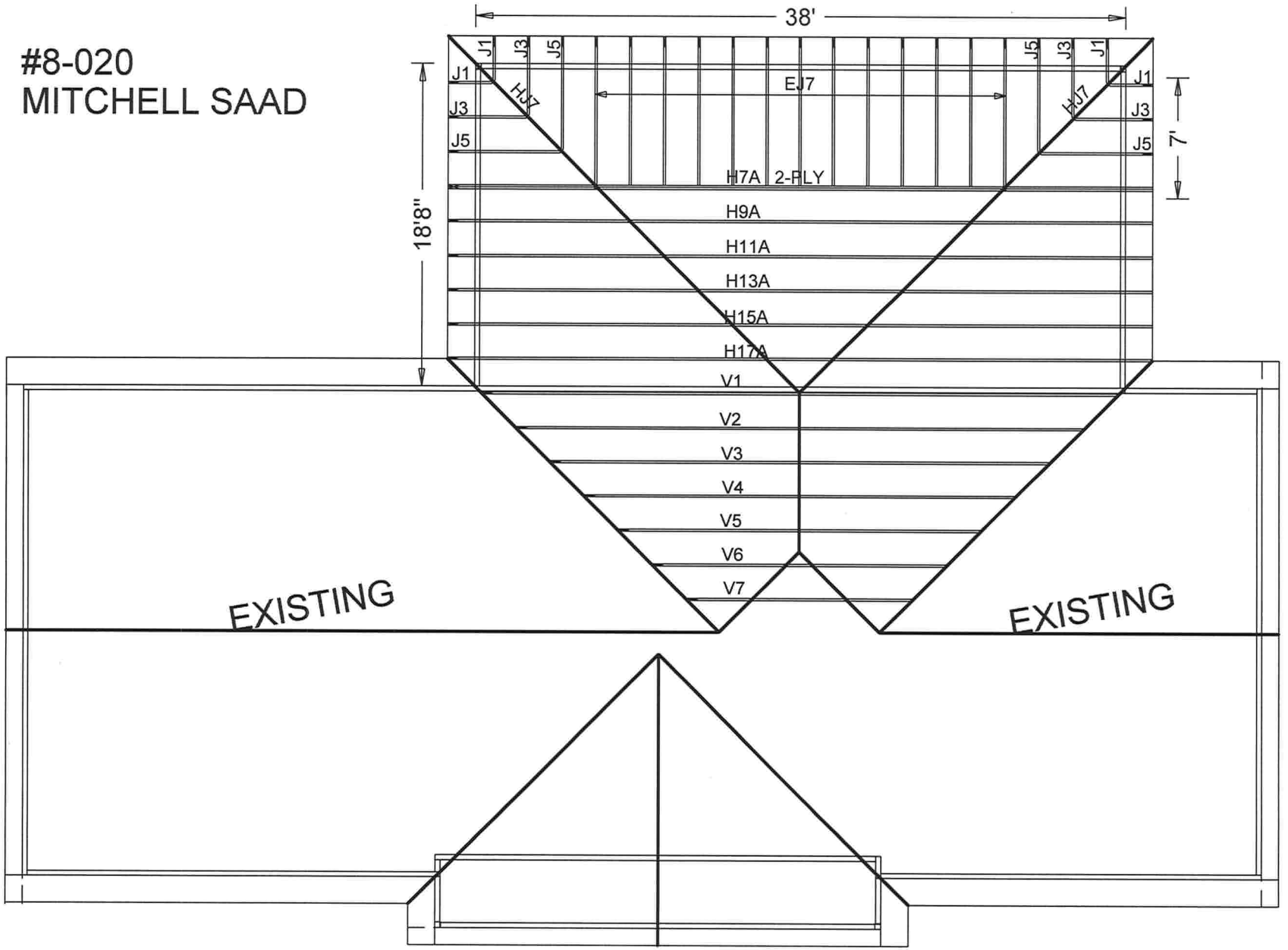
Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: VALTRUSS-

#	Ref	Description	Drawing#	Date
1	71153	H7A	08098001	04/07/08
2	71154	H9A	08098002	04/07/08
3	71155	H11A	08098003	04/07/08
4	71156	H13A	08098004	04/07/08
5	71157	H15A	08098005	04/07/08
6	71158	H17A	08098006	04/07/08
7	71159	V1	08098007	04/07/08
8	71160	V2	08098008	04/07/08
9	71161	V3	08098009	04/07/08
10	71162	V4	08098010	04/07/08
11	71163	V5	08098011	04/07/08
12	71164	V6	08098012	04/07/08
13	71165	V7	08098013	04/07/08
14	71166	HJ7	08098005	04/07/08
15	71167	J5	08098004	04/07/08
16	71168	J3	08098001	04/07/08
17	71169	J1	08098002	04/07/08
18	71170	EJ7	08098003	04/07/08

#8-020
MITCHELL SAAD



JOB DESCRIPTION:: OWNER BUILDER
/: Mitchell Saad

JOB NO:
8-020

PAGE NO:
1 OF 1

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - H7A)

Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP #2:
Bot chord 2x6 SP #2 :B2 2x6 SP #1 Dense:
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

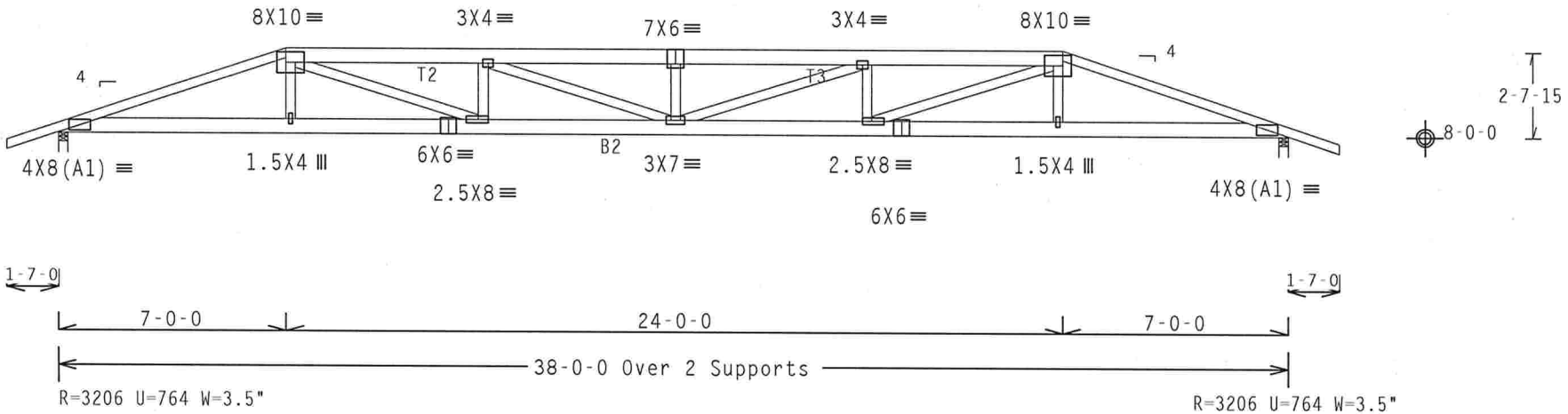
2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Roof overhang supports 2.00 psf soffit load.

#1 hip supports 7-0-0 jacks with no webs.

Calculated vertical deflection is 0.55" due to live load and 0.84" due to dead load at X = 19-0-0.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.36.04 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
07 '08

TC LL	20.0 PSF	REF	R8228- 71153
TC DL	10.0 PSF	DATE	04/07/08
BC DL	10.0 PSF	DRW	HCUSR8228 08098001
BC LL	0.0 PSF	HC-ENG	TCE/DF
TOT.LD.	40.0 PSF	SEQN-	83754
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGH8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - H9A)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

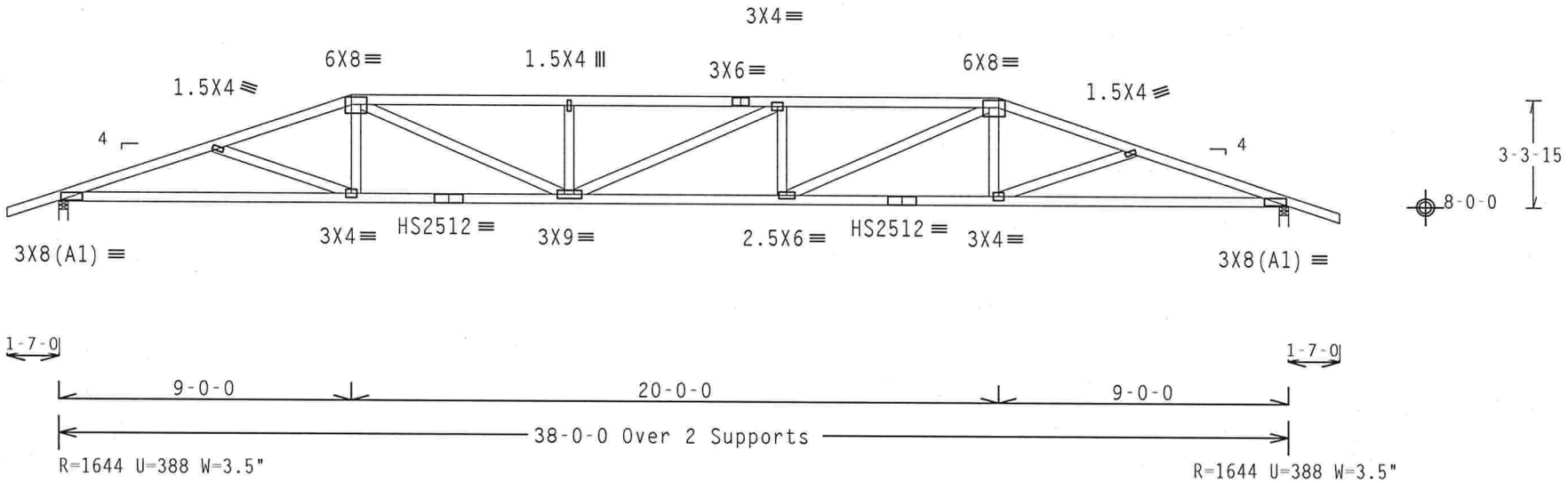
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Calculated vertical deflection is 0.42" due to live load and 0.62" due to dead load at X = 15-8-9.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/0(0) 7.36.04 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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07 '08

TC LL	20.0 PSF	REF R8228- 71154
TC DL	10.0 PSF	DATE 04/07/08
BC DL	10.0 PSF	DRW HCUSR8228 08098002
BC LL	0.0 PSF	HC-ENG TCE/DF
TOT.LD.	40.0 PSF	SEQN- 83759
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGH8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - H11A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

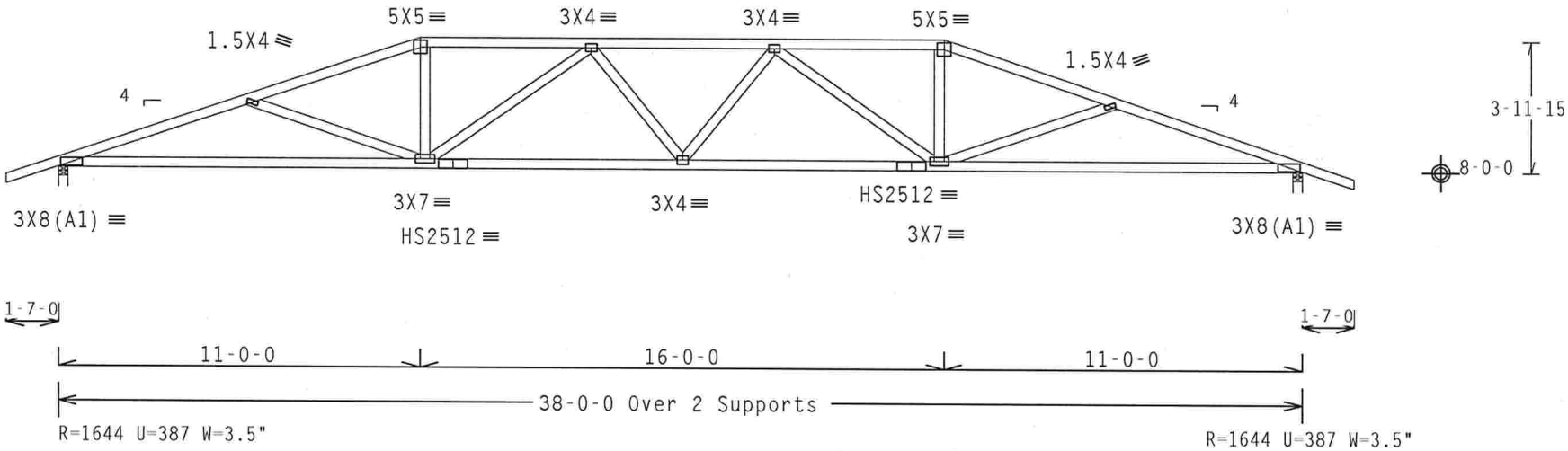
Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/0(0) 7.36.042 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
07 '08

TC LL	20.0 PSF	REF	R8228- 71155
TC DL	10.0 PSF	DATE	04/07/08
BC DL	10.0 PSF	DRW	HCUSR8228 08098003
BC LL	0.0 PSF	HC-ENG	TCE/DF *
TOT.LD.	40.0 PSF	SEQN-	83764
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGH8228Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

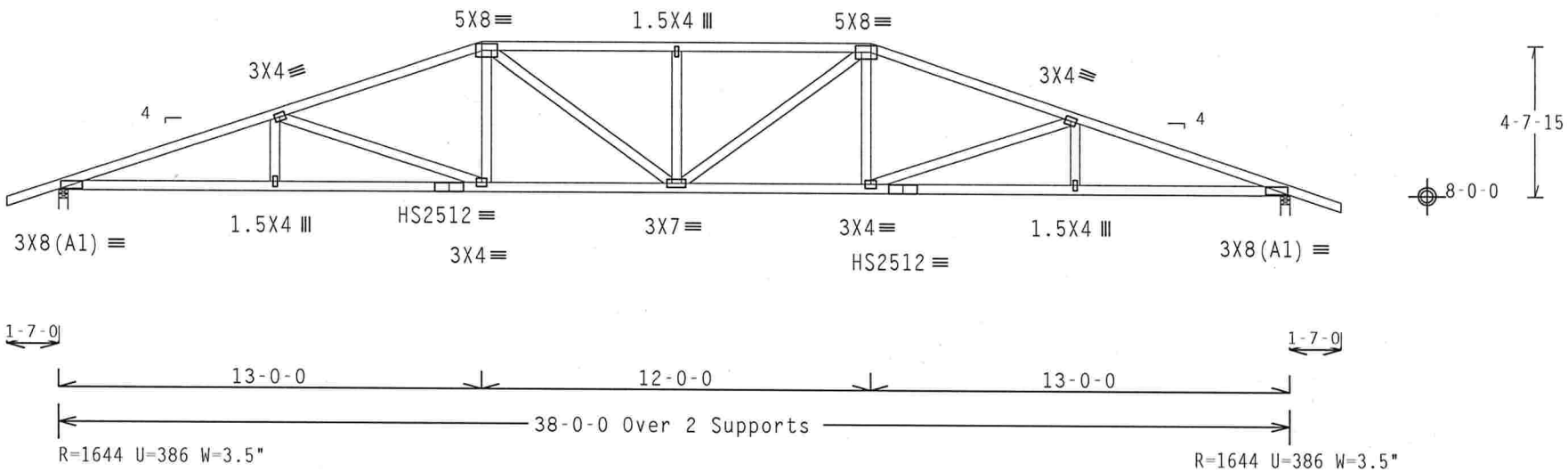
Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave

Cq/RT=1.00(1.25)/0(0) 7.36.042 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
07 '08

TC LL	20.0 PSF	REF R8228- 71156
TC DL	10.0 PSF	DATE 04/07/08
BC DL	10.0 PSF	DRW HCUSR8228 08098004
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 83769
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGH8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - H15A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

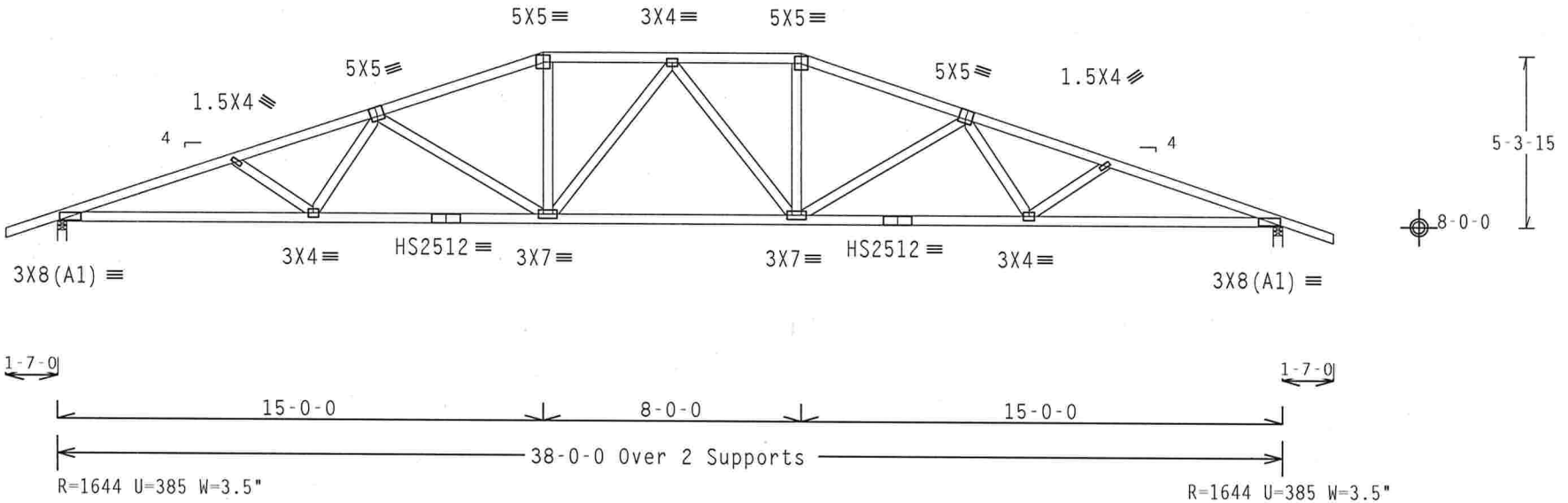
Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/0(0) 7.36.042 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
07 '08

TC LL	20.0 PSF	REF	R8228- 71157
TC DL	10.0 PSF	DATE	04/07/08
BC DL	10.0 PSF	DRW	HCUSR8228 08098005
BC LL	0.0 PSF	HC-ENG	TCE/DF *
TOT.LD.	40.0 PSF	SEQN-	83774
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGH8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - H17A)

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Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

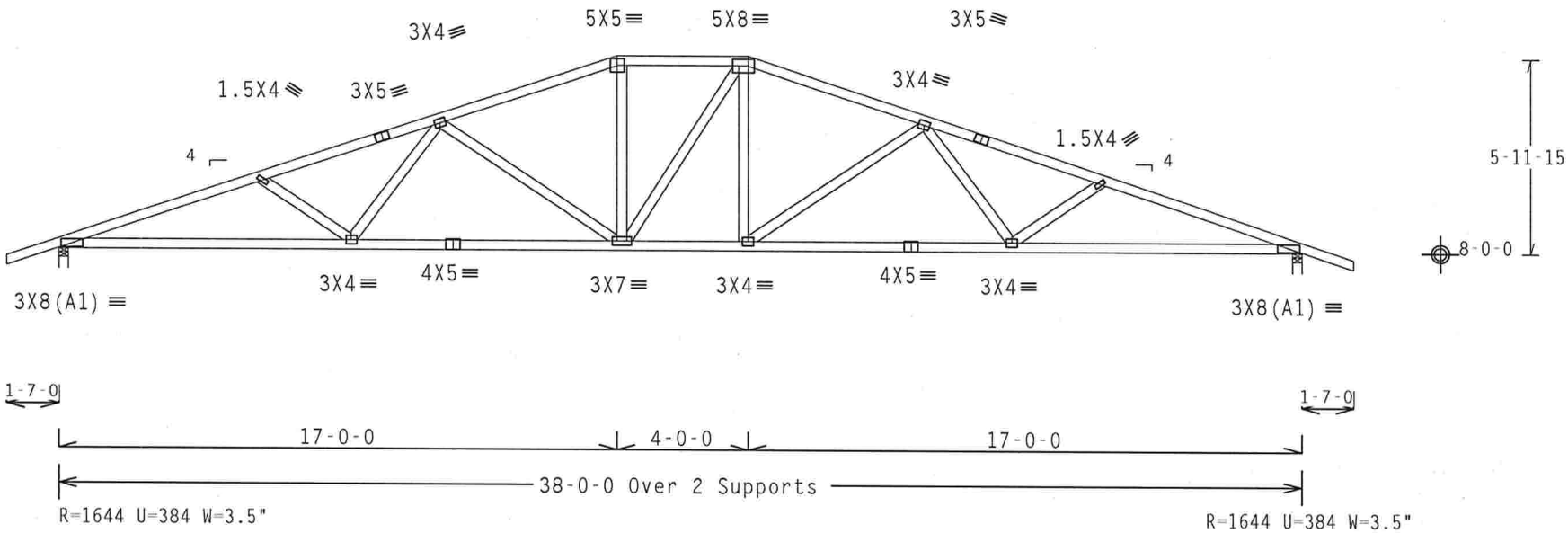
Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.36.04 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

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Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W.H/SS/K) ASTM A653 GRADE 40/60 (W. K/H.SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
07 '08

TC LL	20.0 PSF	REF R8228- 71158
TC DL	10.0 PSF	DATE 04/07/08
BC DL	10.0 PSF	DRW HCUSR8228 08098006
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 83779
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGH8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - V1)

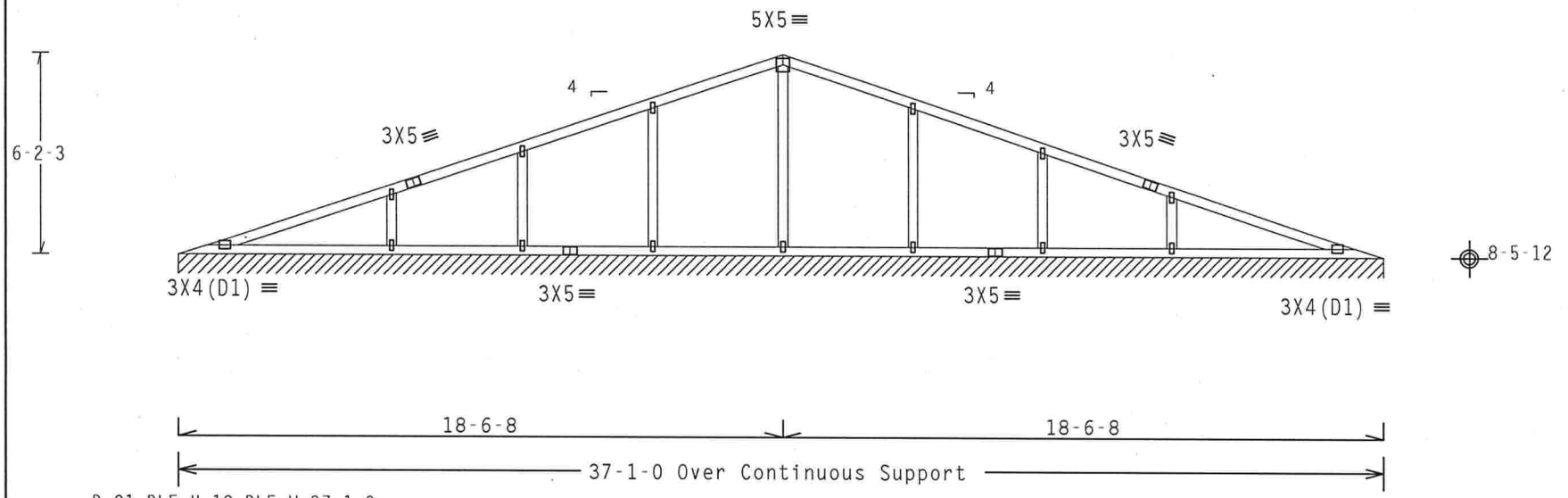
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
Apr 07 '08

TC LL	20.0 PSF	REF R8228- 71159
TC DL	10.0 PSF	DATE 04/07/08
BC DL	10.0 PSF	DRW HCUSR8228 08098007
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 83685
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGH8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - V2)

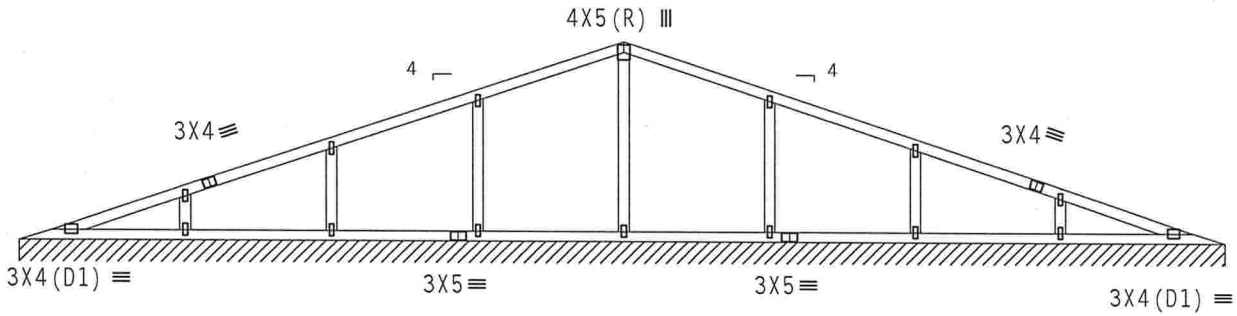
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

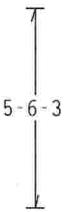
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.





9-1-12

16-6-8 16-6-8

33-1-0 Over Continuous Support

R=81 PLF U=18 PLF W=33-1-0

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.042 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
07 '08

TC LL	20.0 PSF	REF R8228- 71160
TC DL	10.0 PSF	DATE 04/07/08
BC DL	10.0 PSF	DRW HCUSR8228 08098008
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 83697
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TGH8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - V3)

THIS END PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

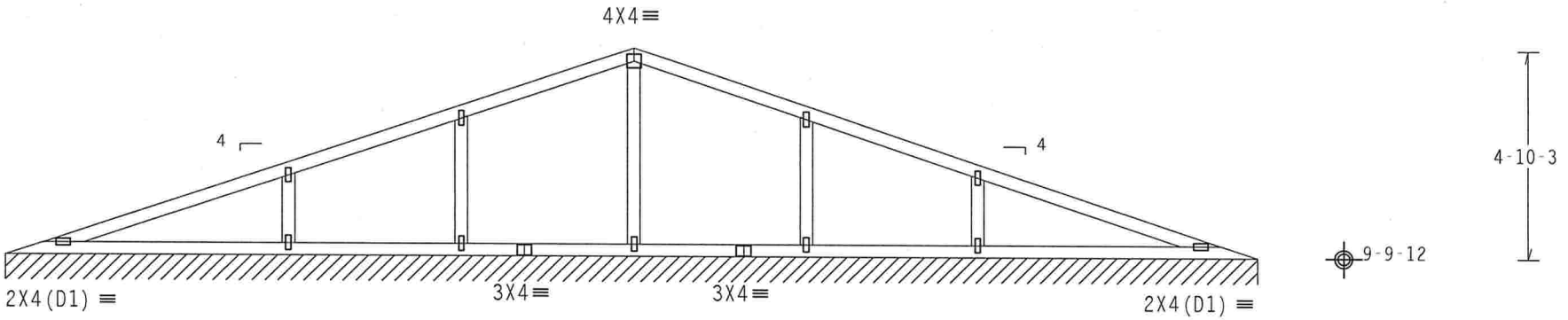
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.36.042 VBS QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
07 '08

TC LL	20.0 PSF	REF R8228- 71161
TC DL	10.0 PSF	DATE 04/07/08
BC DL	10.0 PSF	DRW HCUSR8228 08098009
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 83702
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGH8228Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

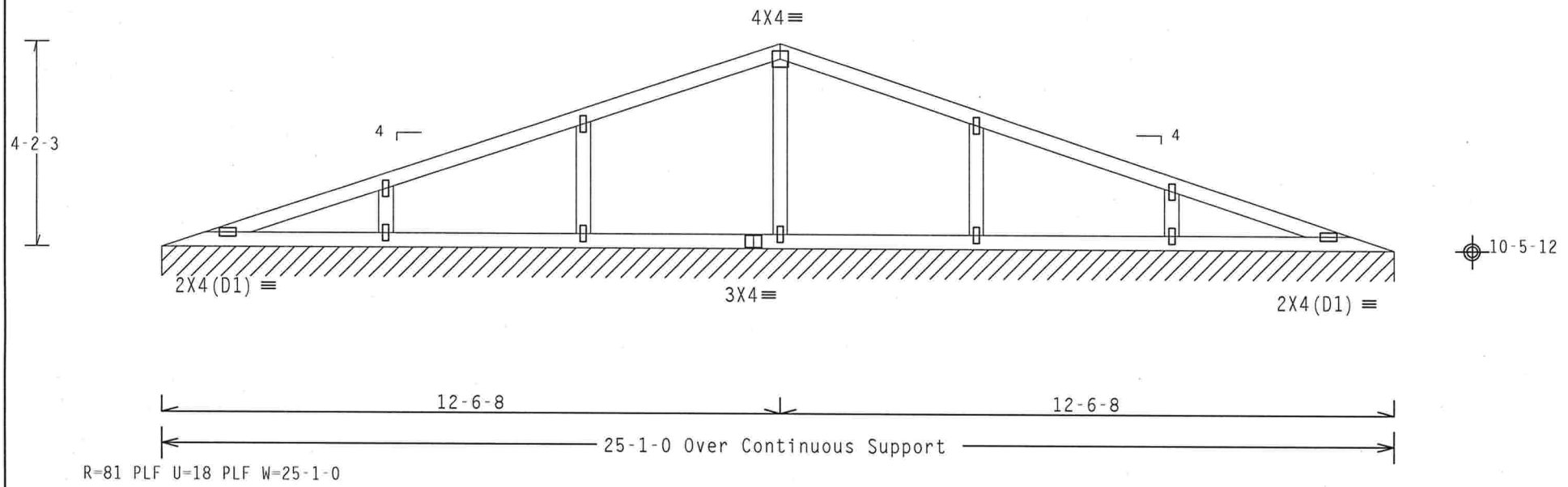
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.36.042 QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
07 '08

TC LL	20.0 PSF	REF R8228- 71162
TC DL	10.0 PSF	DATE 04/07/08
BC DL	10.0 PSF	DRW HCUSR8228 08098010
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 83707
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGH8228Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

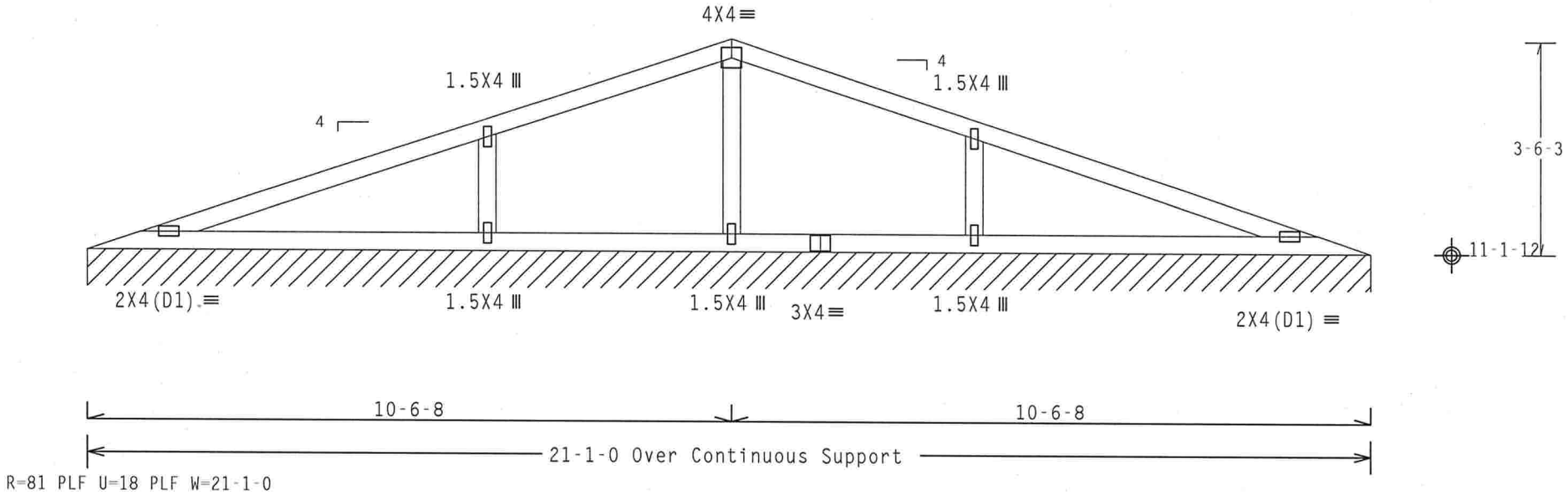
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.04.3

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
APR 07 '08

TC LL	20.0 PSF	REF R8228- 71163
TC DL	10.0 PSF	DATE 04/07/08
BC DL	10.0 PSF	DRW HCUSR8228 08098011
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 83712
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGH8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - V6)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

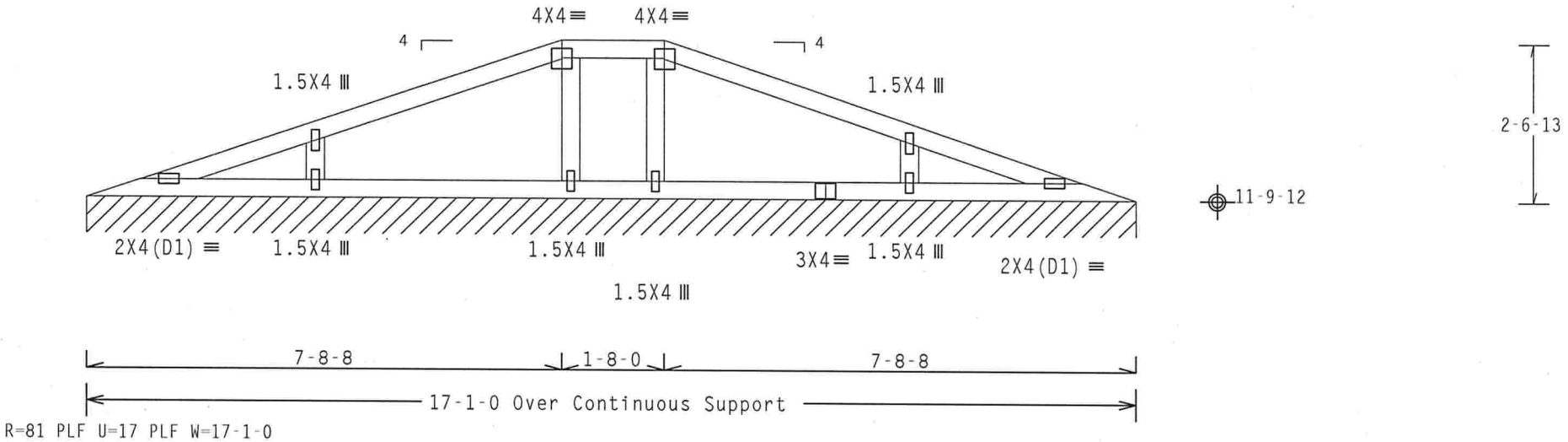
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

See DWG VALTRUSS0207 for valley details.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.36.042 QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
07 '08

TC LL	20.0 PSF	REF	R8228- 71164
TC DL	10.0 PSF	DATE	04/07/08
BC DL	10.0 PSF	DRW	HCUSR8228 08098012
BC LL	0.0 PSF	HC-ENG	TCE/DF *
TOT.LD.	40.0 PSF	SEQN-	83717
DUR.FAC.	1.25	FROM	AH
SPACING	24.0"	JREF-	1TGH8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - V7)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

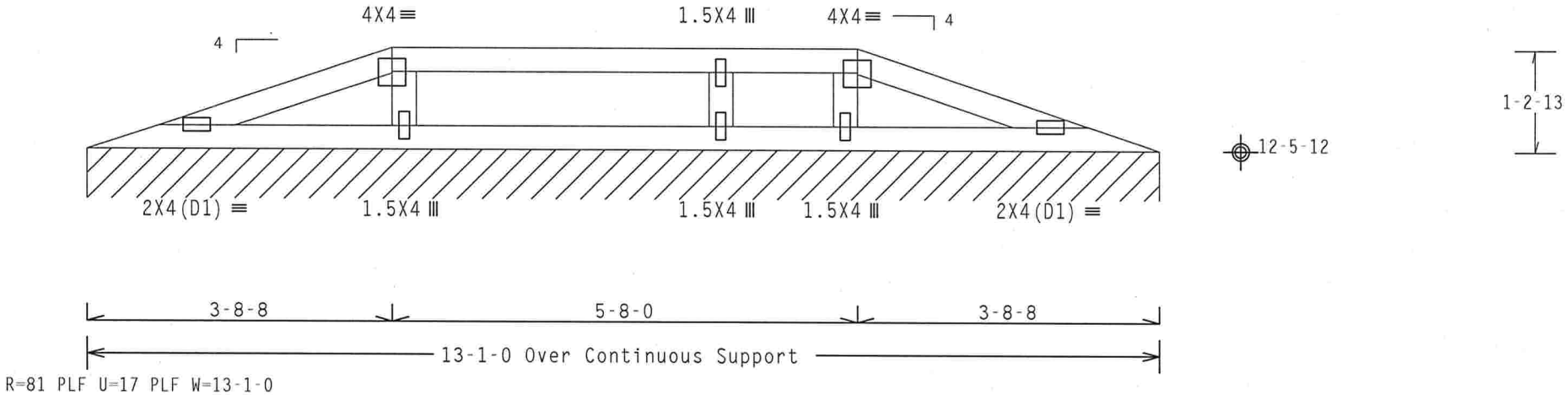
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

See DWG VALTRUSS0207 for valley details.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.04 [illegible] QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

PLT TYP. Wave

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING. INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA. 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
APR 07 '08

TC LL	20.0 PSF	REF R8228- 71165
TC DL	10.0 PSF	DATE 04/07/08
BC DL	10.0 PSF	DRW HCUSR8228 08098013
BC LL	0.0 PSF	HC-ENG TCE/DF *
TOT.LD.	40.0 PSF	SEQN- 83721
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGH8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - HJ7)

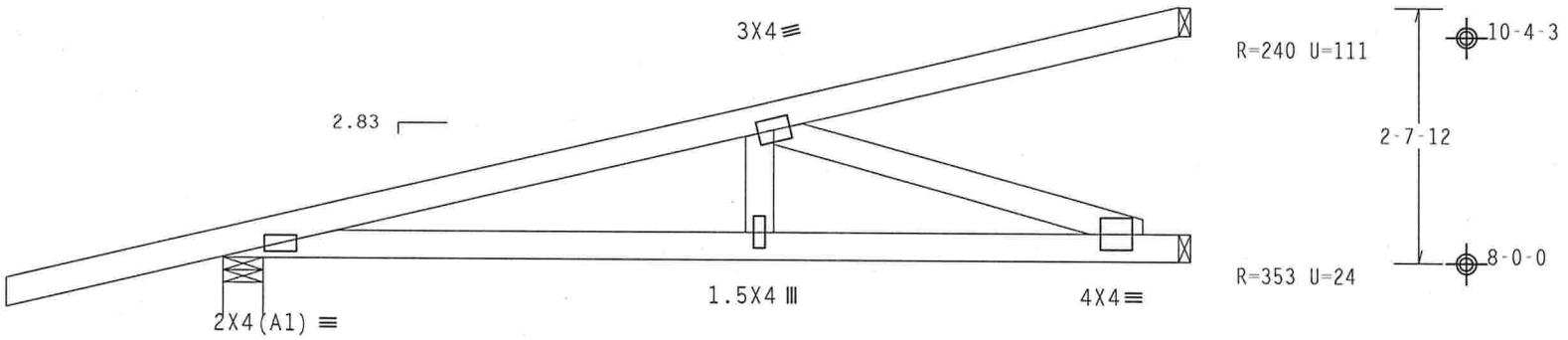
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Hipjack supports 7-0-0 setback jacks with no webs.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



PLT TYP. Wave

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.043 [illegible] QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
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Haines City, FL 33844
FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
07 '08

TC LL	20.0 PSF	REF R8228- 71166
TC DL	10.0 PSF	DATE 04/07/08
BC DL	10.0 PSF	DRW HCUSR8228 08098005
BC LL	0.0 PSF	HC-ENG DAL/AP
TOT.LD.	40.0 PSF	SEQN- 71770
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGH8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - J5)

THIS DAG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

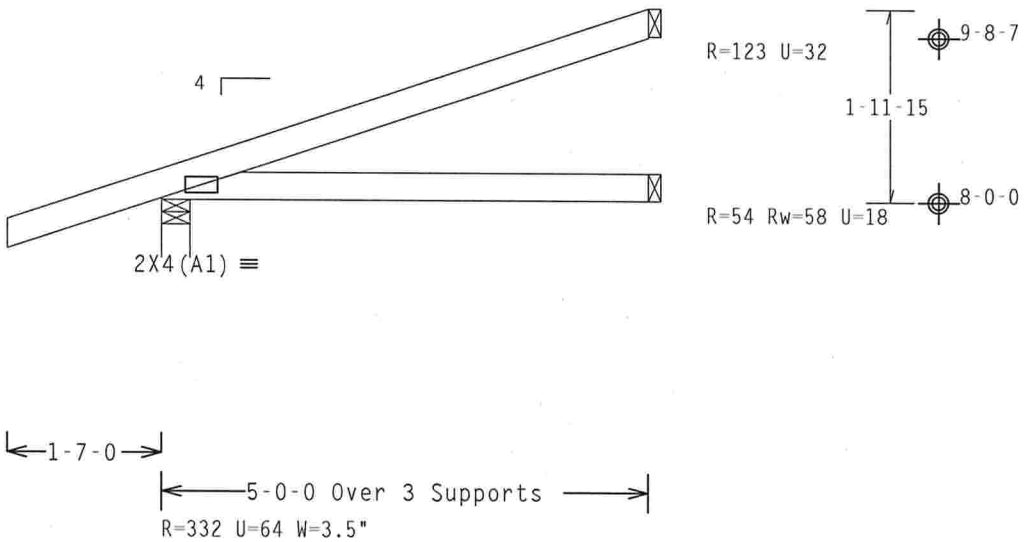
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.042

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
07 '08

TC LL	20.0 PSF	REF R8228- 71167
TC DL	10.0 PSF	DATE 04/07/08
BC DL	10.0 PSF	DRW HCUSR8228 08098004
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 71766
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGH8228Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

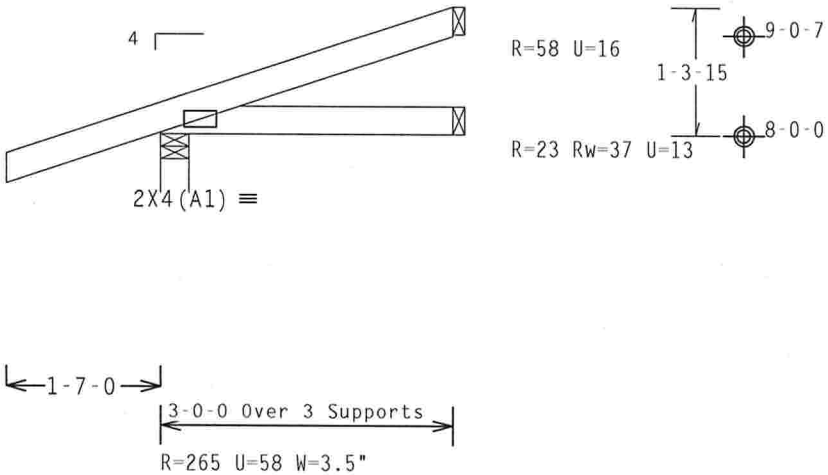
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.042 [illegible] QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

PLT TYP. Wave

ALPINE
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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
[illegible] 07 '08

TC LL	20.0 PSF	REF R8228- 71168
TC DL	10.0 PSF	DATE 04/07/08
BC DL	10.0 PSF	DRW HCUSR8228 08098001
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 71774
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGH8228Z01

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - J1)

THIS JOB PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

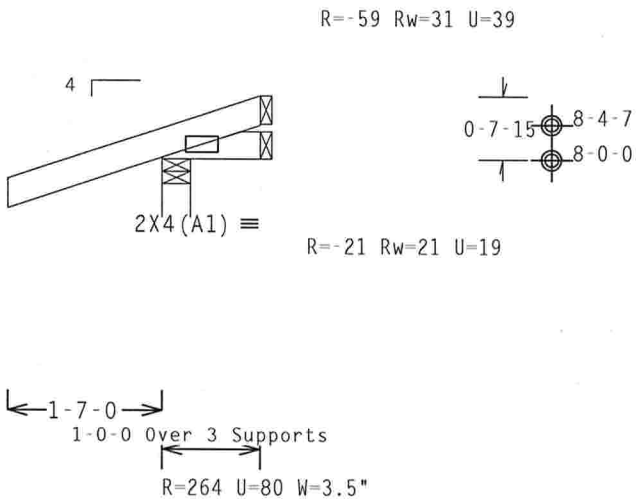
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.0

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group Inc.
Haines City, FL 33844
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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER

Apr 07 '08

TC LL	20.0 PSF	REF R8228- 71169
TC DL	10.0 PSF	DATE 04/07/08
BC DL	10.0 PSF	DRW HCUSR8228 08098002
BC LL	0.0 PSF	HC-ENG DAL/AP
TOT.LD.	40.0 PSF	SEQN- 71757
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGH8228Z01

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

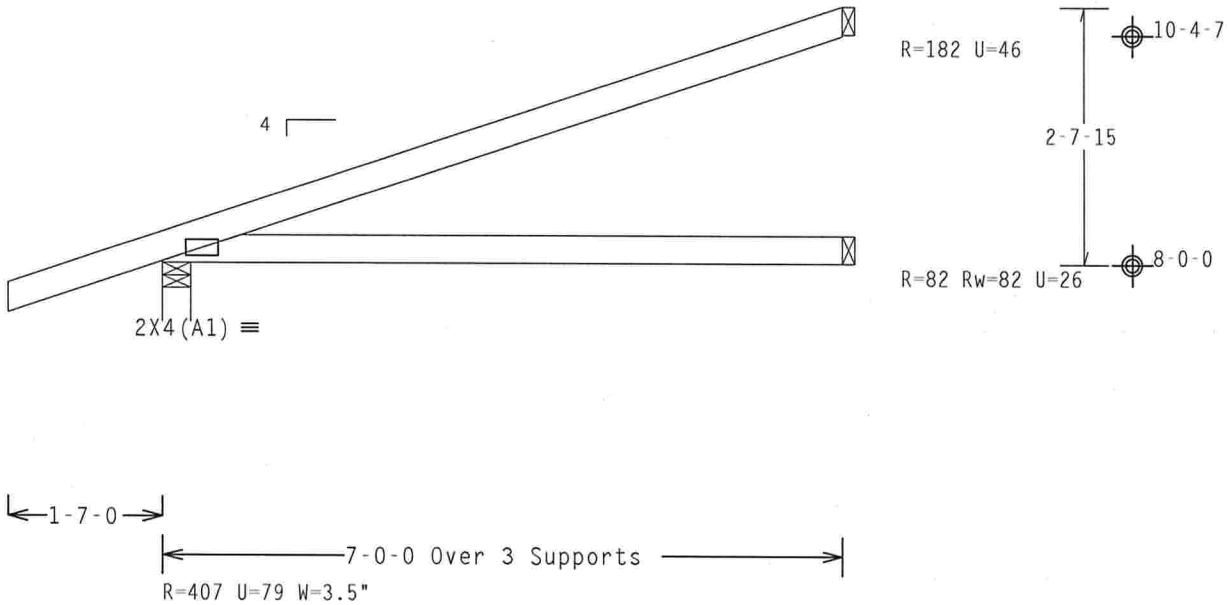
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.04

PLT TYP. Wave QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

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FL Certificate of Authorization # 0 278

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DOUGLAS FLEMING LICENSE No. 66648 STATE OF FLORIDA PROFESSIONAL ENGINEER
07 '08

TC LL	20.0 PSF	REF R8228- 71170
TC DL	10.0 PSF	DATE 04/07/08
BC DL	10.0 PSF	DRW HCUSR8228 08098003
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 71760
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TGH8228Z01

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR SBC 110 MPH, ASCE 7-93 110 MPH OR ASCE 7-98, ASCE 7-02 OR ASCE 7-05 130 MPH. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

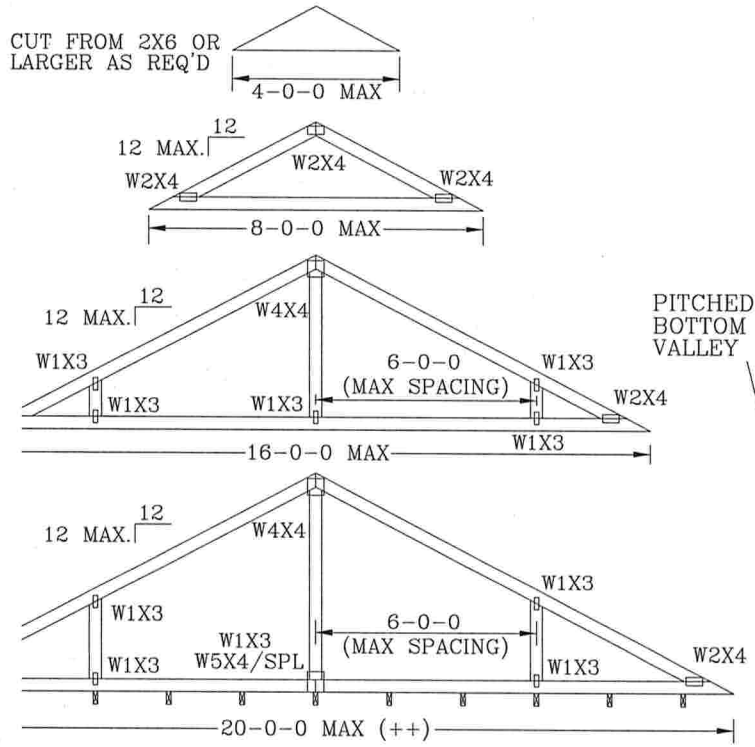
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

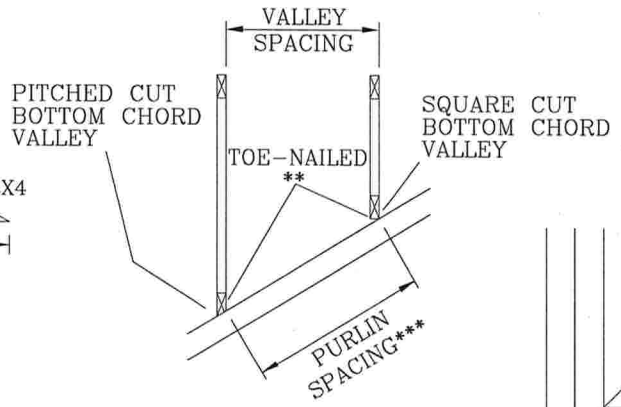
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

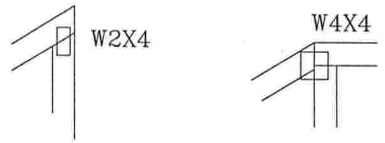
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

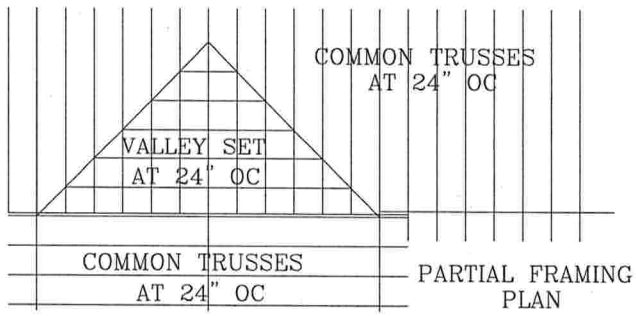
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.







THIS DRAWING REPLACES DRAWING A105

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

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DOUGLAS FLEMING LICENSE No. 66648 07-08 STATE OF FLORIDA PROFESSIONAL ENGINEER

TC LL	30	30	40 PSF	
TC DL	20	15	7 PSF	
BC DL	10	10	10 PSF	
BC LL	0	0	0 PSF	
TOT. LD.	60	55	57 PSF	
DUR.FAC.	1.25/1.33	1.15	1.15	
SPACING		24"		

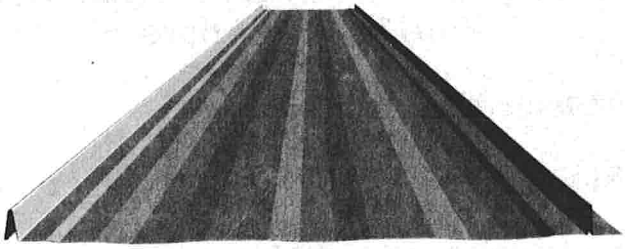
REF	VALLEY DETAIL
DATE	2/23/07
DRWG	VALTRUSS0207
-ENG	MLH/KAR

GULF-LOK PANEL STANDING SEAM

Gulf Coast Supply & Mfg. Inc.

State of Florida Approved

Residence ~ Carrabelle, Florida



The Gulf-Lok standing seam roofing system is the perfect choice in top-of-the-line roofing for residential and commercial applications and is our most popular, as well as our most cost-effective, standing seam roofing panel. Gulf-Lok features a 1" rib with slotted screw strip on the under-lap side for concealed fasteners, and comes in either Galvalume or any of over 20 colors of 24 gauge steel. Panels are available with either 12- or 16-inch coverage, with on-site manufacturing as the most popular option for delivery. For details and information about Gulf-Lok (including color availability), contact your Gulf Coast representative.

Colors are representative of colors offered and are not intended for matching purposes.

Patina Green
Surf Beige
Dark Bronze
Regal White
Matte Black
Patriot Bronze
Sandstone
Evergreen
Hartford Green
Roman Blue
Regal Blue
Colonial Red
Brite Red
Brandywine
Mansard Brown
Slate Gray
Ash Gray
Charcoal
Copper Penny
Medium Bronze

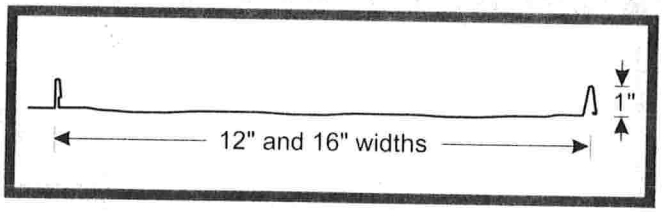
4020 SW 449th St., Horseshoe Beach, FL 32648 • 352-498-0778 • 352-498-7852 fax • 888-393-0335 toll free

Gulf Coast Supply & Mfg. Inc.

GULF-LOK PANEL STANDING SEAM

Features:

Colors: 20+ Colors Available
Color Chart Available Upon Request. Also Available in Mill Finished Galvalume.



Coverage: 12" & 16" Net Coverage

Gauge: 24 Gauge Steel

Substrate: AZ-50 Galvalume (Painted)
AZ-55 Galvalume (Mill Finish)

Warranty: 40 years

Testing: UL580/UL1897 – Uplift Test
Florida State Approval

Minimum slope: 3:12

Substructure: 15/32" CDX (minimum)

Installation: Detail Manual / Installation Guide – Available Upon Request

Colors are representative of colors offered and are not intended for matching purposes.

Pat Gre
Sur Bla
Da Bro
Reg Whi
Mat Bla
Patri Bro
San Ston
Evergr
Hartf Gree
Rom Blu
Rega Blue
Colon Red
Brit Red
Brandy wine
Mansa Brow
Sl Gr
A Gra
Charco
Copper Penny
Mediu Bronze

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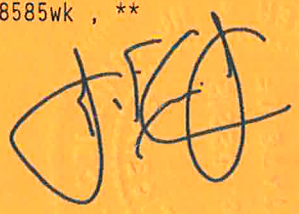
ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
 Document ID:1TE98228Z0521075904

Truss Fabricator:	Anderson Truss Company
Job Identification:	8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , **
Truss Count:	18
Model Code:	Florida Building Code 2004 and 2006 Supplement
Truss Criteria:	ANSI/TPI-2002(STD)/FBC
Engineering Software:	Alpine Software,Version 7.36.
Structural Engineer of Record:	The identity of the structural EOR did not exist as of
Address:	the seal date per section 61G15-31.003(5a) of the FAC
Minimum Design Loads:	Roof - 40.0 PSF @ 1.25 Duration
	Floor - N/A
	Wind - 110 MPH ASCE 7-02 -Closed



Seal Date: 01/21/2008

-Truss Design Engineer-
James F. Collins Jr.
Florida License Number: 52212
1950 Marley Drive
Haines City, FL 33844

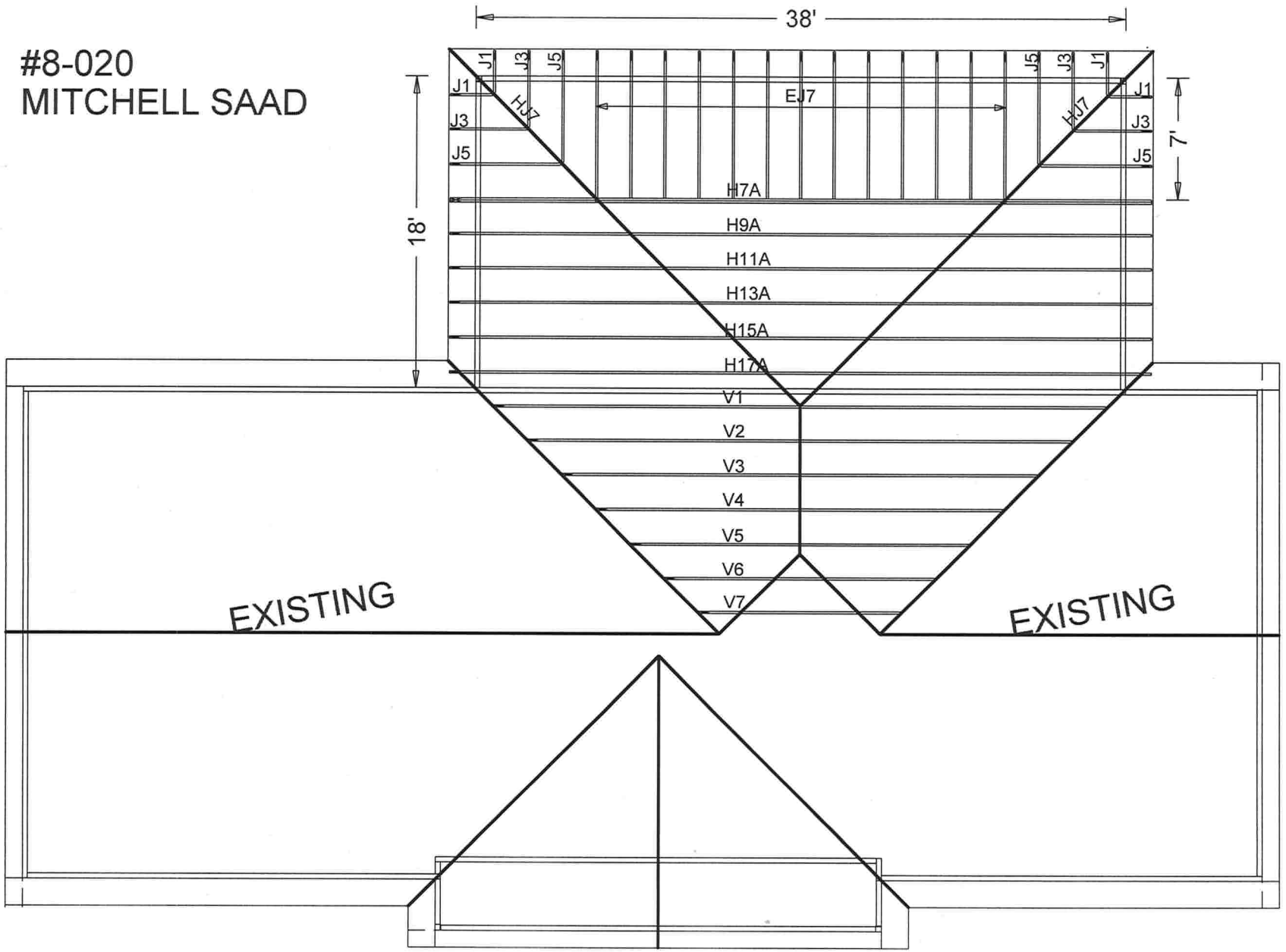
Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.
3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: VALTRUSS-

#	Ref Description	Drawing#	Date
1	26384--H7A	08021005	01/21/08
2	26385--H9A	08021006	01/21/08
3	26386--H11A	08021007	01/21/08
4	26387--H13A	08021008	01/21/08
5	26388--H15A	08021009	01/21/08
6	26389--H17A	08021010	01/21/08
7	26390--V1	08018044	01/18/08
8	26391--V2	08018040	01/18/08
9	26392--V3	08018041	01/18/08
10	26393--V4	08018042	01/18/08
11	26394--V5	08018043	01/18/08
12	26395--V6	08021003	01/21/08
13	26396--V7	08021004	01/21/08
14	26397--EJ7	08021011	01/21/08
15	26398--J5	08021012	01/21/08
16	26399--HJ7	08021013	01/21/08
17	26400--J3	08021014	01/21/08
18	26401--J1	08021015	01/21/08

#8-020
MITCHELL SAAD



JOB DESCRIPTION:: OWNER BUILDER
/: Mitchell Saad

JOB NO:
8-020

PAGE NO:
1 OF 1

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - H7A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP #2:
Bot chord 2x6 SP #2 :B2 2x6 SP #1 Dense:
Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

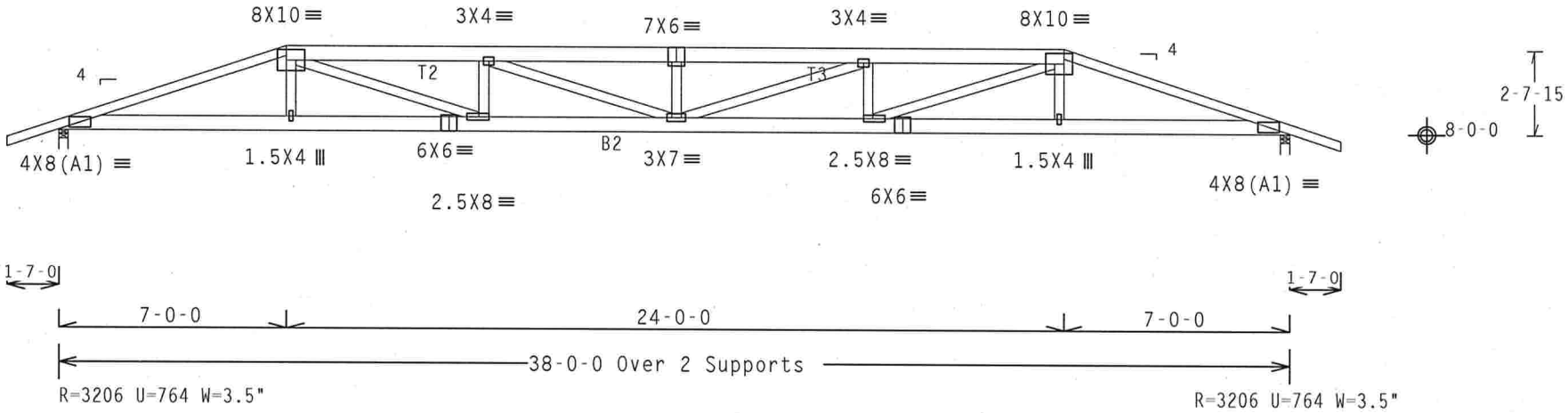
2 COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Roof overhang supports 2.00 psf soffit load.

#1 hip supports 7-0-0 jacks with no webs.

Calculated vertical deflection is 0.55" due to live load and 0.84" due to dead load at X = 19-0-0.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

JAMES F. COLLINS, P.E. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 21 08

TC LL	20.0 PSF	REF R8228- 26384
TC DL	10.0 PSF	DATE 01/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08021005
BC LL	0.0 PSF	HC-ENG DAL/AP
TOT.LD.	40.0 PSF	SEQN- 71763
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JRFF- 1TE98228Z05

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Roof overhang supports 2.00 psf soffit load.

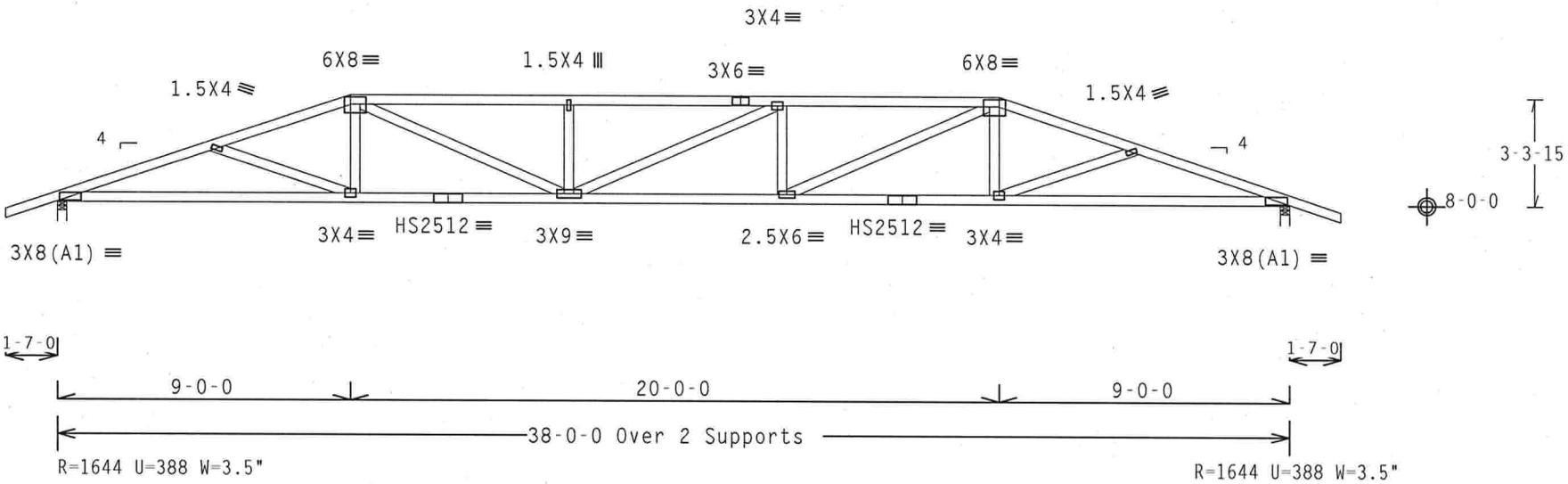
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Calculated vertical deflection is 0.42" due to live load and 0.62" due to dead load at X = 15-8-9.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave | Cq/RT=1.00(1.25)/0(0) | 7.36.0424.11 | QTY:1 | FL/-/4/-/-/R/- | Scale =.1875"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES E COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan

TC LL	20.0 PSF	REF R8228- 26385
TC DL	10.0 PSF	DATE 01/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08021006
BC LL	0.0 PSF	HC-ENG DAL/AP
TOT.LD.	40.0 PSF	SEQN- 71777
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TE98228Z05

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - H11A)

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

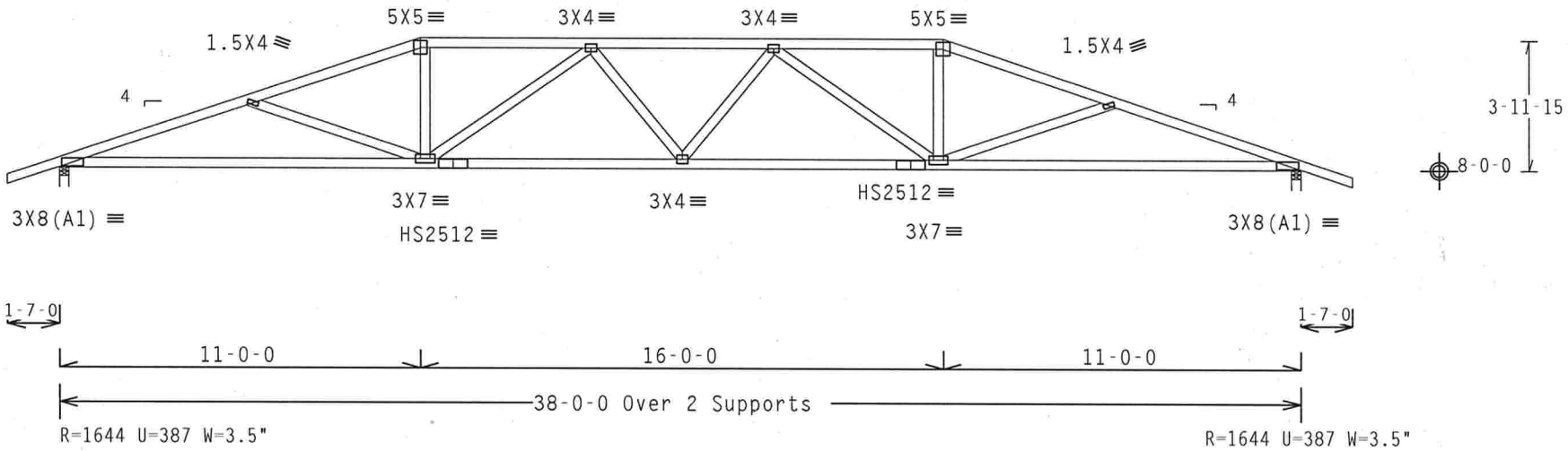
Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424.11 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan

TC LL	20.0 PSF	REF R8228- 26386
TC DL	10.0 PSF	DATE 01/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08021007
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 71794
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TE98228Z05

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - H13A)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

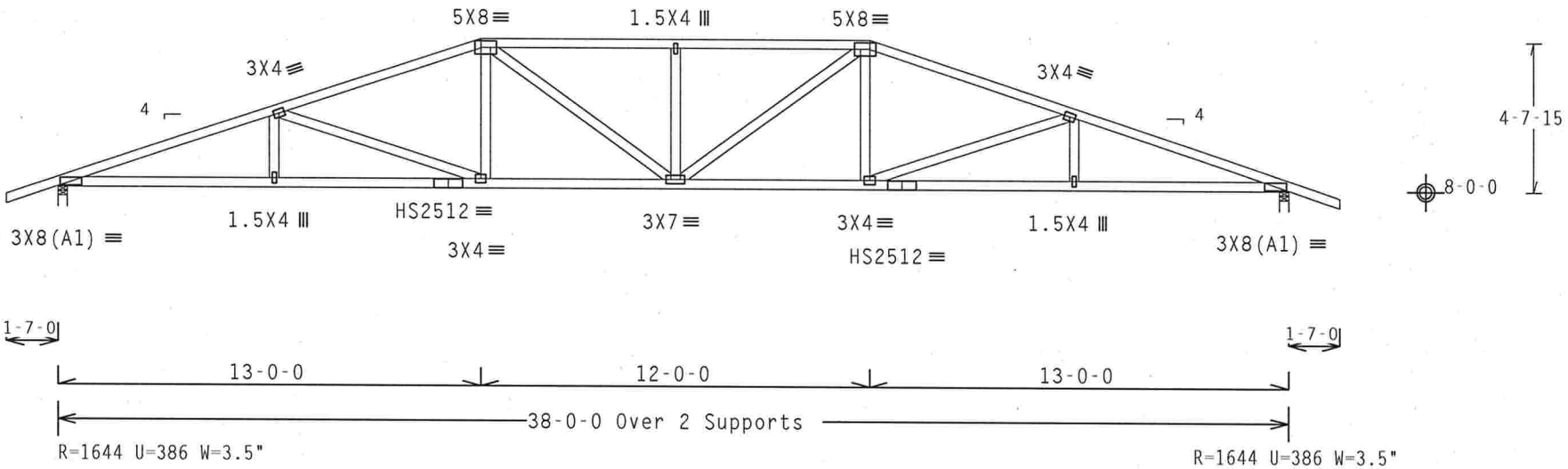
Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 11 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan 21

TC LL	20.0 PSF	REF R8228- 26387
TC DL	10.0 PSF	DATE 01/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08021008
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 71799
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TE98228Z05

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - H15A)

THIS DESIGN PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

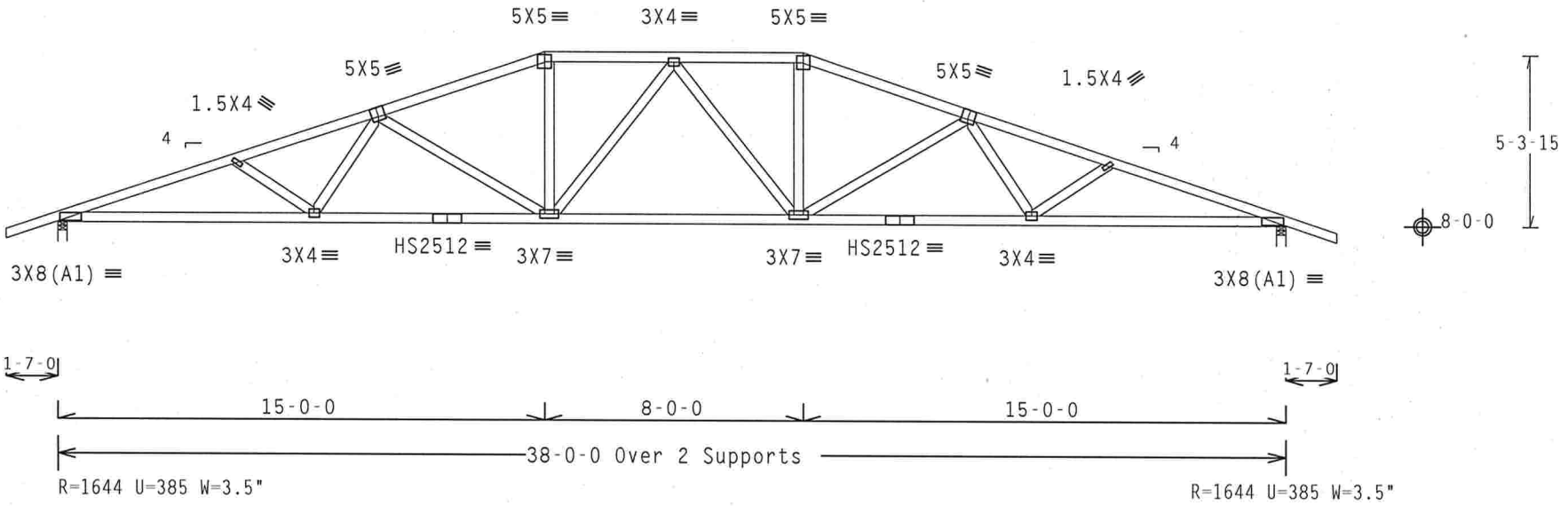
Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. 20 Gauge HS,Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan 21 08

TC LL	20.0 PSF	REF R8228- 26388
TC DL	10.0 PSF	DATE 01/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08021009
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 71802
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TE98228Z05

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - H17A)

THIS END PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

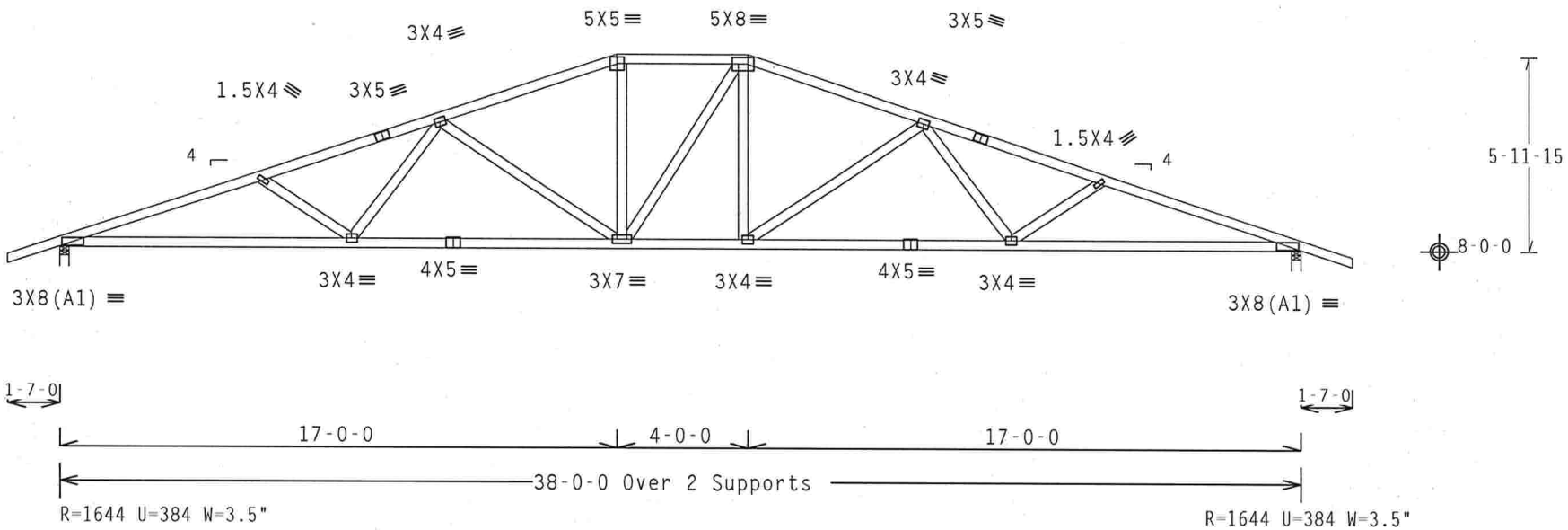
Roof overhang supports 2.00 psf soffit load.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

JAMES F. COLLINS JR LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan 21 '08

TC LL	20.0 PSF	REF R8228- 26389
TC DL	10.0 PSF	DATE 01/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08021010
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 71805
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TE98228Z05

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

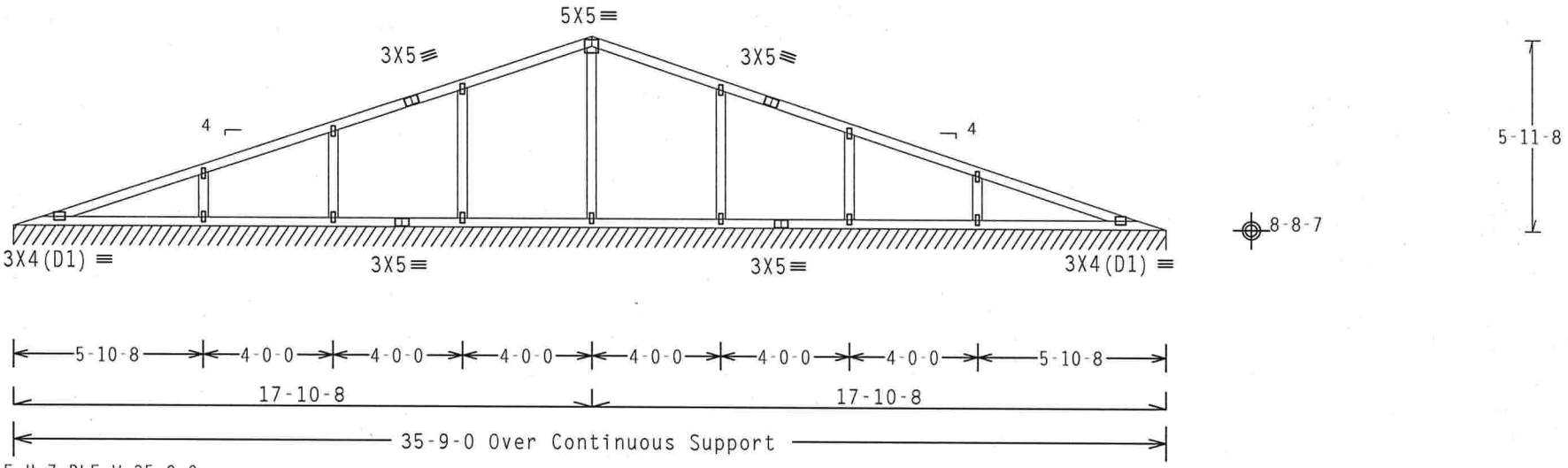
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



R=81 PLF U=7 PLF W=35-9-0

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.1875"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 21 '08

TC LL	20.0 PSF	REF R8228- 26390
TC DL	10.0 PSF	DATE 01/18/08
BC DL	10.0 PSF	DRW HCUSR8228 08018044
BC LL	0.0 PSF	HC-ENG DLJ/DLJ *
TOT.LD.	40.0 PSF	SEQN- 71552
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TE98228Z05

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

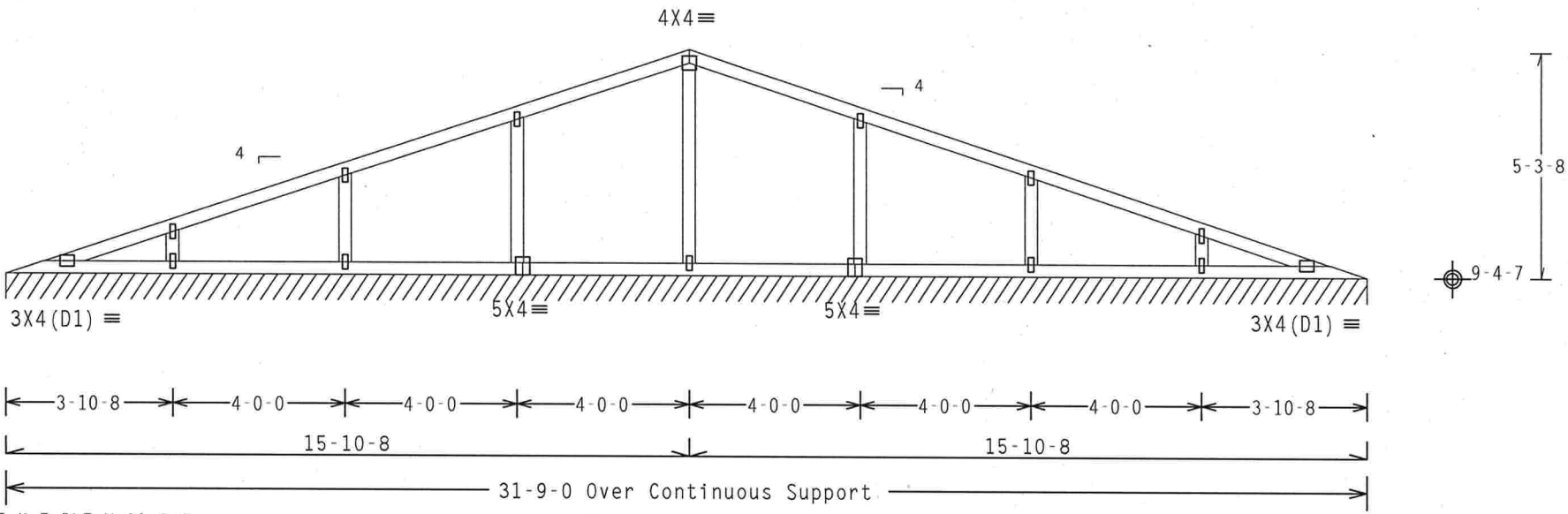
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



R=81 PLF U=7 PLF W=31-9-0

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 11 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan 2

TC LL	20.0 PSF	REF R8228- 26391
TC DL	10.0 PSF	DATE 01/18/08
BC DL	10.0 PSF	DRW HCUSR8228 08018040
BC LL	0.0 PSF	HC-ENG DLJ/DLJ *
TOT.LD.	40.0 PSF	SEQN- 71557
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TE98228Z05

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

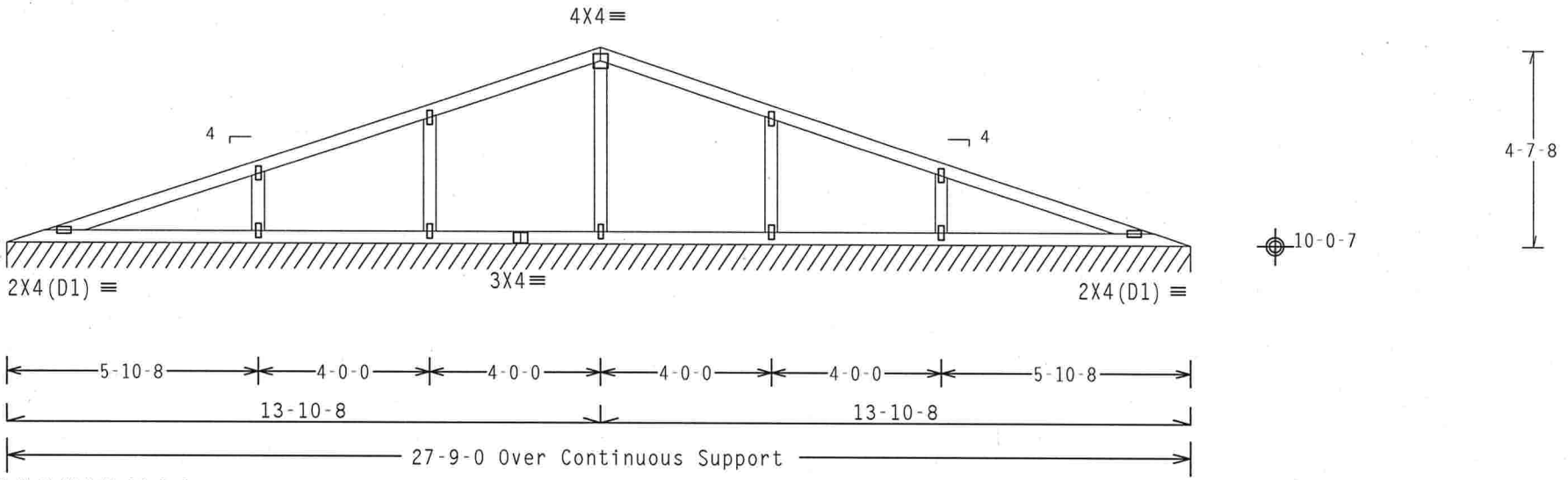
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



R=81 PLF U=7 PLF W=27-9-0

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 F1 QTY:1 FL/-/4/-/-/R/- Scale =.25"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 21 '08

TC LL	20.0 PSF	REF R8228- 26392
TC DL	10.0 PSF	DATE 01/18/08
BC DL	10.0 PSF	DRW HCUSR8228 08018041
BC LL	0.0 PSF	HC-ENG DLJ/DLJ *
TOT.LD.	40.0 PSF	SEQN- 71564
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TE98228Z05

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

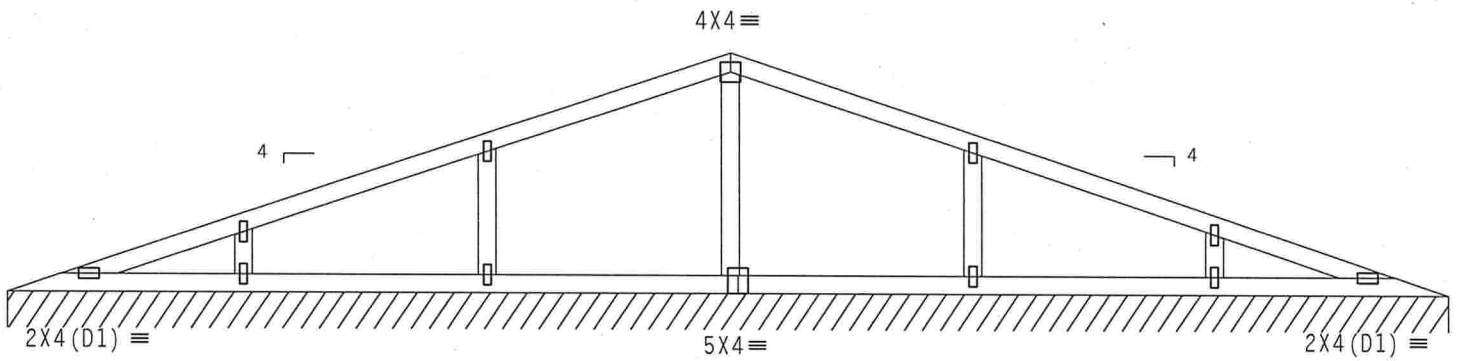
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



R=81 PLF U=7 PLF W=23-9-0

Note: All Plates Are 1.5X4 Except As Shown.

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424.11 QTY:1 FL/-/4/-/-/R/- Scale =.3125"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan

TC LL	20.0 PSF	REF R8228- 26393
TC DL	10.0 PSF	DATE 01/18/08
BC DL	10.0 PSF	DRW HCUSR8228 08018042
BC LL	0.0 PSF	HC-ENG DLJ/DLJ *
TOT.LD.	40.0 PSF	SEQN- 71569
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TE98228Z05

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

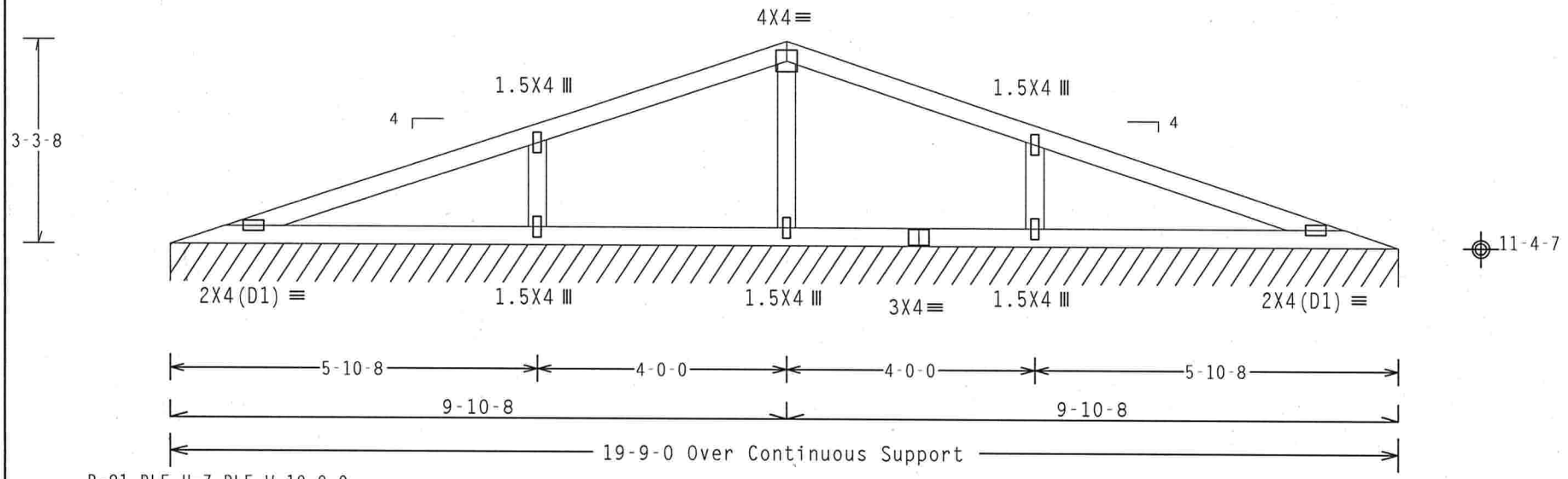
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, CLOSED bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.18

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



R=81 PLF U=7 PLF W=19-9-0

Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.0424.11

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/- Scale =.375"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS, JR. LICENSE No. 22219 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 18,08

TC LL	20.0 PSF	REF R8228- 26394
TC DL	10.0 PSF	DATE 01/18/08
BC DL	10.0 PSF	DRW HCUSR8228 08018043
BC LL	0.0 PSF	HC-ENG DLJ/DLJ *
TOT.LD.	40.0 PSF	SEQN- 71574
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TE98228Z05

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

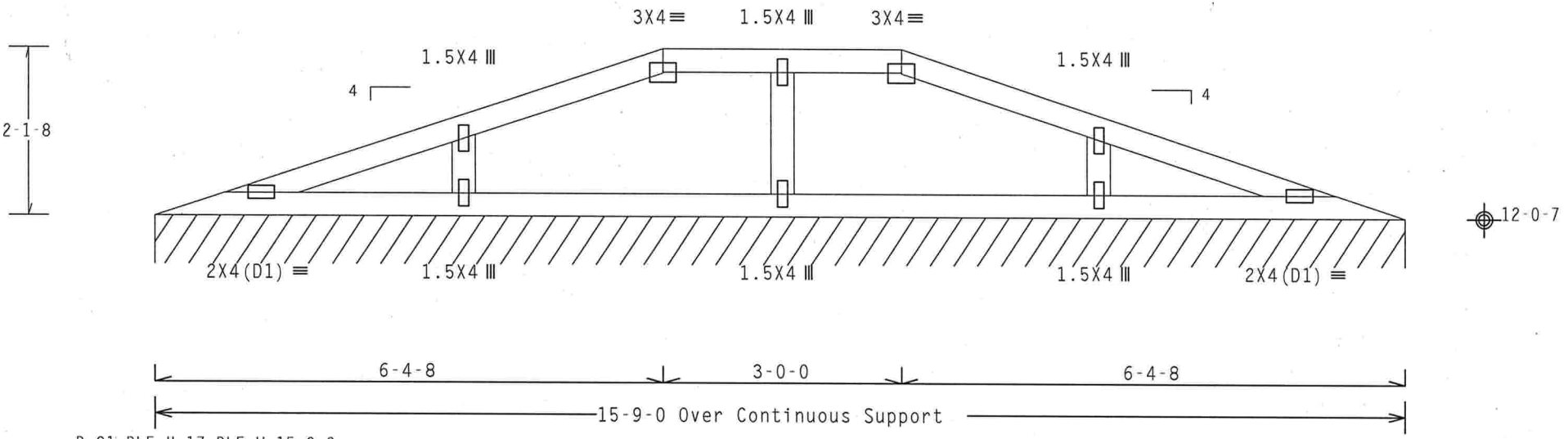
In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

See DWG VALTRUSS0207 for valley details.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=81 PLF U=17 PLF W=15-9-0

Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 11 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS JR. LICENSE NO. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan 2[illegible]

TC LL	20.0 PSF	REF R8228- 26395
TC DL	10.0 PSF	DATE 01/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08021003
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 71748
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TE98228Z05

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - V7)

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

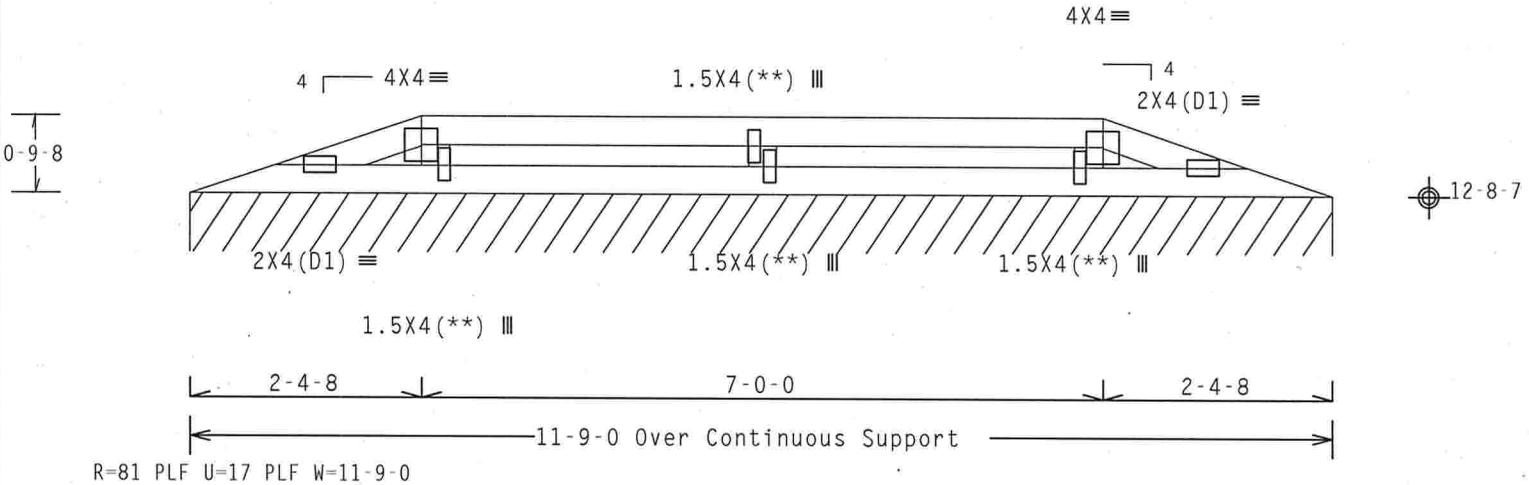
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

(**) 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.

See DWG VALTRUSS0207 for valley details.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

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JAMES F. COLLINS, JR. LICENSE No. 52972 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 21 '08

TC LL	20.0 PSF	REF R8228- 26396
TC DL	10.0 PSF	DATE 01/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08021004
BC LL	0.0 PSF	HC-ENG DAL/AP
TOT.LD.	40.0 PSF	SEQN- 71752
DUR.FAC.	1.25	
SPACING	24.0"	JREF- 1TE98228Z05

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ** - EJ7)

THIS DAT PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

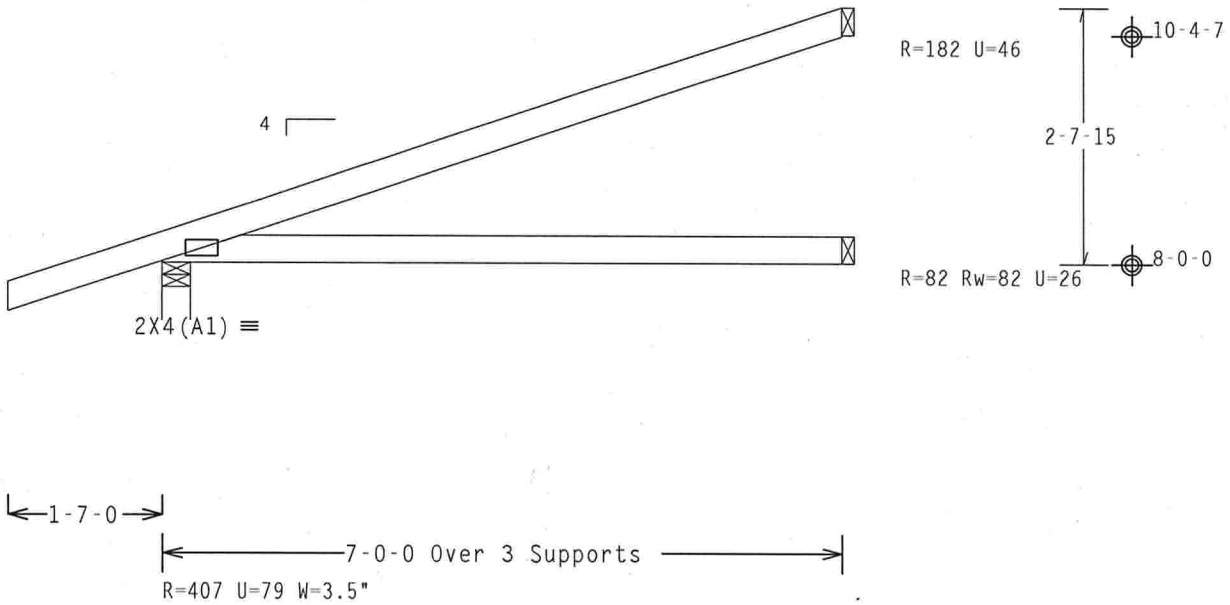
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.0424

PLT TYP. Wave QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 21 08

TC LL	20.0 PSF	REF R8228- 26397
TC DL	10.0 PSF	DATE 01/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08021011
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 71760
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TE98228Z05

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

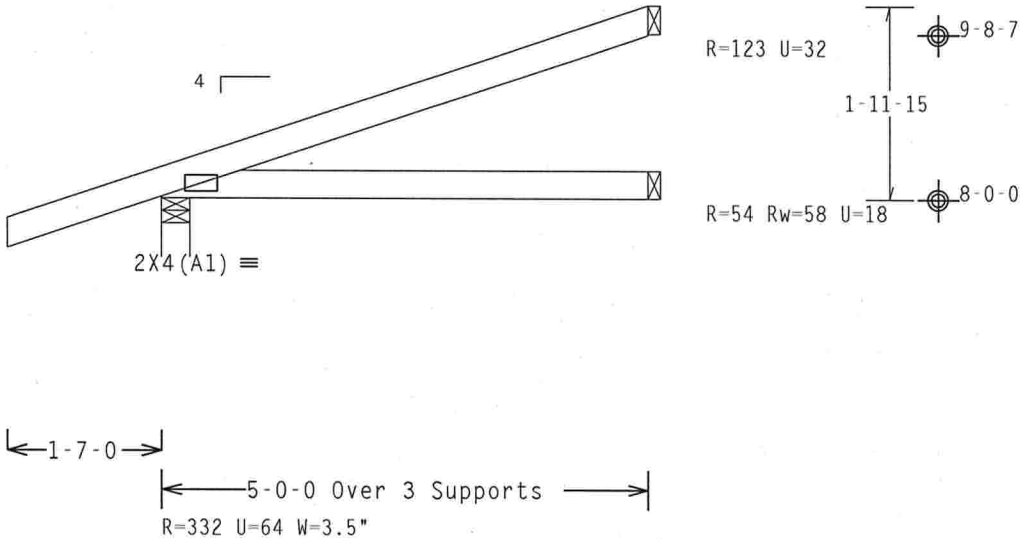
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.0424 11

PLT TYP. Wave

QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan

TC LL	20.0 PSF	REF R8228- 26398
TC DL	10.0 PSF	DATE 01/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08021012
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 71766
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TE98228Z05

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

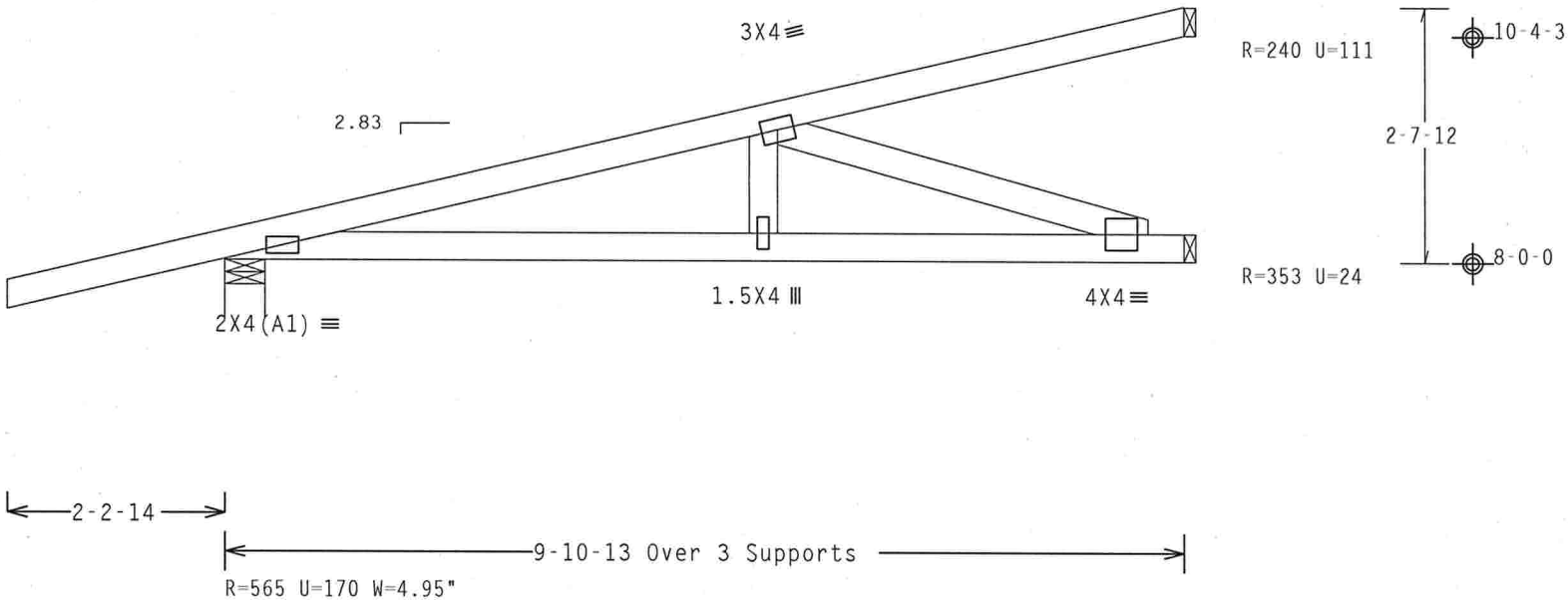
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense
Webs 2x4 SP #3

Hipjack supports 7-0-0 setback jacks with no webs.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC
PLT TYP. Wave Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE
ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 21 08

TC LL	20.0 PSF	REF R8228- 26399
TC DL	10.0 PSF	DATE 01/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08021013
BC LL	0.0 PSF	HC-ENG DAL/AP
TOT.LD.	40.0 PSF	SEQN- 71770
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TE98228Z05

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

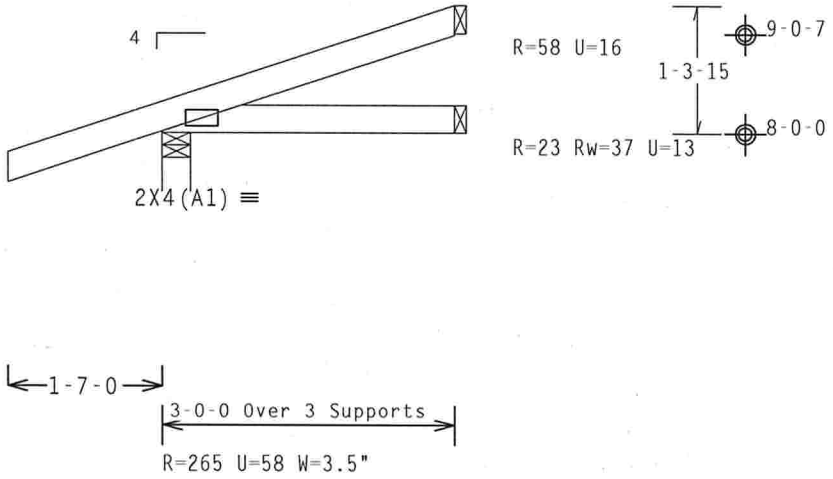
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC

PLT TYP. Wave

Cq/RT=1.00(1.25)/0(0) 7.36.0424.11 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES E. COLLINS, JR. LICENSE No. 52212 STATE OF FLORIDA PROFESSIONAL ENGINEER

Jan

TC LL	20.0 PSF	REF R8228- 26400
TC DL	10.0 PSF	DATE 01/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08021014
BC LL	0.0 PSF	HC-ENG DAL/AP *
TOT.LD.	40.0 PSF	SEQN- 71774
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TE98228Z05

THIS DWG PREPARED FROM COMPUTER INPUT (LOADS & DIMENSIONS) SUBMITTED BY TRUSS MFR.

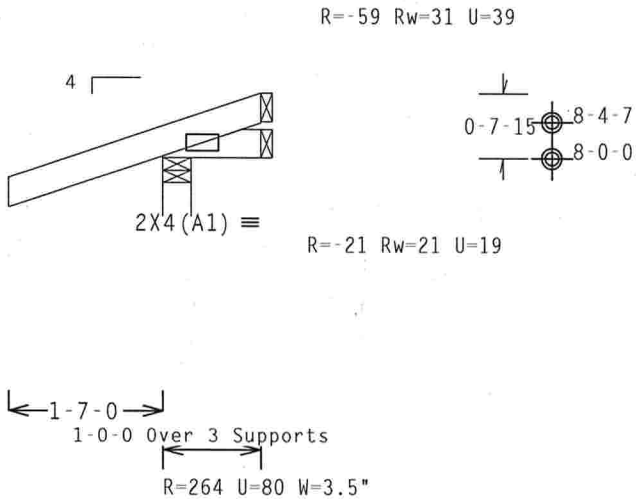
Top chord 2x4 SP #2 Dense
Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures.



Design Crit: TPI-2002(STD)/FBC
Cq/RT=1.00(1.25)/0(0) 7.36.0424 QTY:1 FL/-/4/-/-/R/- Scale =.5"/Ft.

PLT TYP. Wave

ALPINE

ITW Building Components Group, Inc.
Haines City, FL 33844
FL Certificate of Authorization # 0 278

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JAMES F. COLLINS, JR. LICENSE No. 22212 STATE OF FLORIDA PROFESSIONAL ENGINEER
Jan 21 '08

TC LL	20.0 PSF	REF R8228- 26401
TC DL	10.0 PSF	DATE 01/21/08
BC DL	10.0 PSF	DRW HCUSR8228 08021015
BC LL	0.0 PSF	HC-ENG DAL/AP
TOT.LD.	40.0 PSF	SEQN- 71757
DUR.FAC.	1.25	FROM AH
SPACING	24.0"	JREF- 1TE98228Z05

VALLEY TRUSS DETAIL

TOP CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER.
BOT CHORD 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER.
WEBS 2X4 SP #3 OR BETTER.

* 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).

** ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR SBC 110 MPH, ASCE 7-93 110 MPH OR ASCE 7-98, ASCE 7-02 OR ASCE 7-05 130 MPH. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

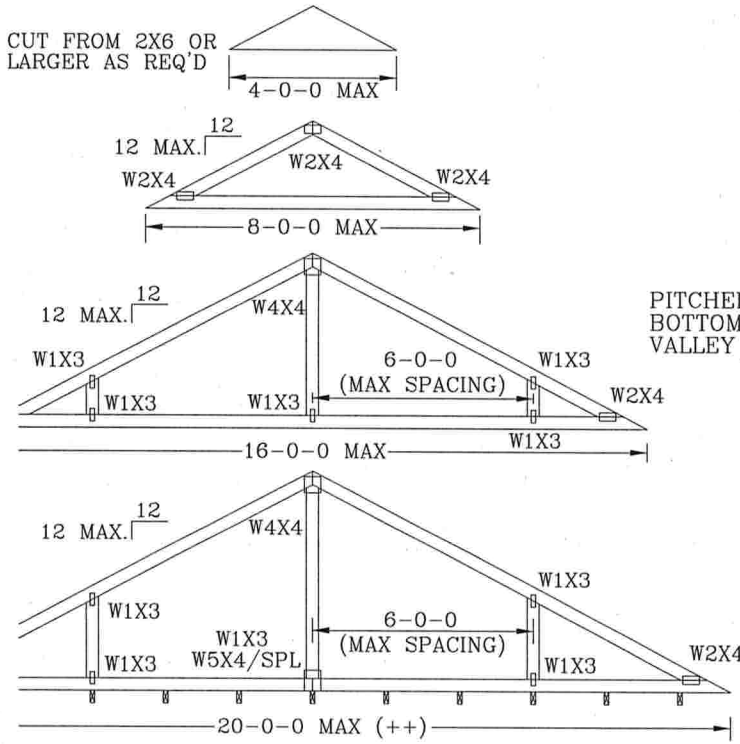
MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0".

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH:
PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS INSTALLATION
OR
PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN
OR
BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON ENGINEERS' SEALED DESIGN.

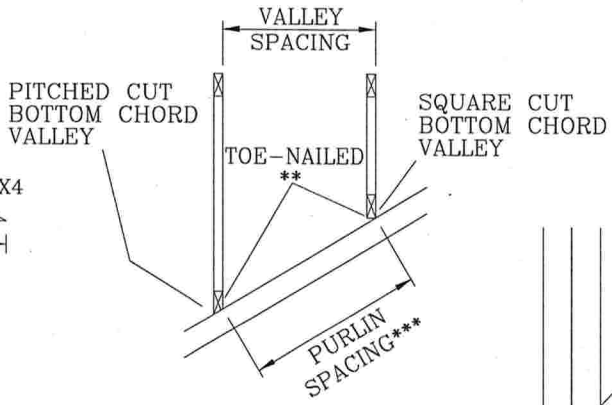
*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

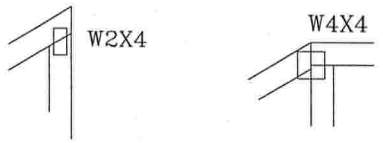
++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".

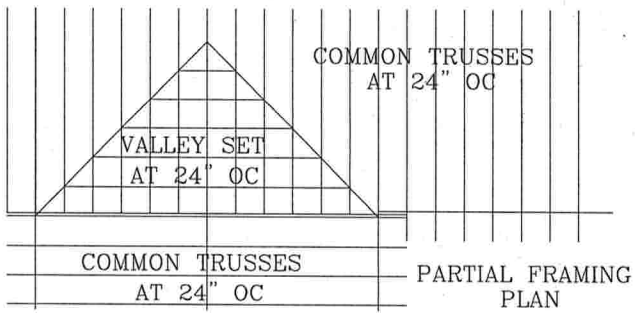
BOTTOM CHORD MAY BE SQUARE OR PITCHED CUT AS SHOWN.



SUPPORTING TRUSSES AT 24" OC MAXIMUM SPACING.







THIS DRAWING REPLACES DRAWING A105

ALPINE
ITW BUILDING COMPONENTS GROUP, INC.
POMPANO BEACH, FLORIDA

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANDRIA, VA. 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

IMPORTANT FURNISH COPY OF THIS DESIGN TO INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ITW, BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W,K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI 1-2002 SEC. 3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER, PER ANSI/TPI 1 SEC. 2.

TC LL	30	30	40 PSF	REF VALLEY DETAIL
TC DL	20	15	7 PSF	DATE 2/23/07
BC DL	10	10	10 PSF	DRWG VALTRUSS0207
BC LL	0	0	0 PSF	-ENG MLH/KAR
TOT. LD.	60	55	57 PSF	
DUR.FAC.	1.25/1.33	1.15	1.15	
SPACING		24"		

Lake City • (386) 755-3633
Fax • (386) 752-5456

Jacksonville • (904) 381-8901
Fax • (904) 381-8902

Quincy • (850) 442-3495
Fax • (850) 442-4008

CAL-TECH TESTING, INC.

ENGINEERING & TESTING LABORATORY

P.O. Box 1625, Lake City, FL 32056-1625
4784 Rosselle St. • Jacksonville, FL 32254
2230 Greensboro Hwy., Quincy, FL 32351

JOB NO.: 08-205
DATE TESTED: 04-08-08

REPORT OF IN-PLACE DENSITY TEST

#26816

ASTM METHOD: ✓ (D-2922) Nuclear _____ (D-2937) Drive Cylinder _____ Other

PROJECT: MITCHELL SAAD

CLIENT: MITCHELL SAAD

GENERAL CONTRACTOR: SAC EARTHWORK CONTRACTOR: SAC

SOIL USE (SEE NOTE): _____ SPECIFICATION REQUIREMENTS: 95%

TECHNICIAN: S. OSTEEN

MODIFIED (ASTM D-1557): ✓ STANDARD (ASTM D-698): _____

TEST NO.	TEST LOCATION	TEST: ✓DEPTH ___ELEV. ___LIFT	PROCTOR NO.	WET DENS. LBS.CU.FT.	DRY DENS. LBS.CU.FT.	MOIST PERCENT	% MAX. DENS.
4.	12' FROM SE CORNER	12"	1	107.6	103.5	4.0	100.4
5	CENTER OF PORCH PAD	12"	1	113.8	108.1	5.3	104.8

REMARKS: _____

PROCTOR NO.	SOIL DESCRIPTION	PROCTOR VALUE	OPT. MOIST.
1	RICHARDSON'S FORT WHITE PIT	103.1	10.8

NOTE: 1. Building Fill 2. Trench Backfill 3. Base Course 4. Subbase/Stabilized Subgrade 5. Embankment 6. Subgrade/Natural Soil 7. Other

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test location and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.

NICHOLAS
PAUL
GEISLER
ARCHITECT
N.C.A.R.B. Certified

1758 NW Brown Road
Lake City, FL 32055
386/755-9021

13 MAY 2008

JOHNNY KEARSE, BUILDING OFFICIAL
COLUMBIA COUNTY, BUILDING DEPT.
COLUMBIA COUNTY COURTHOUSE ANNEX
LAKE CITY, FLORIDA 32055

RE: SAAD RESIDENCE
PERMIT Nr.: # 26816

DEAR SIR:

PLEASE BE ADVISED OF THE FOLLOWING CHANGE TO THE CONSTRUCTION DOCUMENTS FOR THE ABOVE REFERENCED PROJECT:

1. IN LIEU OF THE TRUSS ANCHORS SPECIFIED IN THE PLANS, IT IS PERMISSIBLE TO USE "SIMPSON" H16 ANCHORS OR OTHER "SIMPSON" ANCHORS, EQUAL OR EXCEEDING THE DESIGN LOADS AS INDICATED IN THE ENGINEERED TRUSS SHOP DRAWINGS.

SHOULD YOU HAVE ANY FURTHER QUESTIONS WITH THIS, PLEASE CALL FOR ASSISTANCE.

YOURS TRULY,
NICHOLAS PAUL GEISLER, ARCHITECT AR0007005

NOTICE OF TREATMENT

Applicator Name Mccall Service

Address 415 NW 250 St Suite I

City Newberry

Time 10:46 Date 5-29-08

b 26816

SITE LOCATION

Lot # _____ Block # _____ Permit # 00026816

Subdivision _____

Address 349 SW Thorn LN

Name of Chemical Applied Premice Pro Used .05 %

Area Treated 1288 sq ft 136 LN ft

Gallons Used 213 gals

Remarks Soil Treatment on Patio Pool Addition

Applicator - White Permit File - Canary Permit Holder - Pink

M-1043

NOTICE OF TREATMENT

Applicator Name: Mc Call Service

Address: 415 NW 250th ST Suite 1

City: Newberry

Time: 7:48 AM Date: 4-8-08

26816

SITE LOCATION

#26816

Lot # _____ Block # _____ Permit # 200026816

Subdivision _____

Address: 349 SW Thorne Lane

Name of Chemical Applied: Premise Used: 05 %

Area Treated: Soil Pretreat on Patio Addition

Gallons Used: 150

Remarks: 684 sq ft 110 LN ft

Stem Wall

Mitchell Saad

Applicator - White Permit File - Canary Permit Holder - Pink

M-1043