DATE 03/03/2008 Columbia County Building Permit This Permit Must Be Prominently Posted on Premises During Construction				PERMIT 000026816			
-			1 Tommenty 1 050				000020010
-	MITCHELL SA.			— FT. WHITE	PHONE	386.454.7298	FL 32038
ADDRESS		THORNE LANE			PHONE	386.454.7298	32030
-	MITCHELL SA	THORNE LANE		— FT. WHITE	HONL	300.434.7270	FL 32038
=					PHONE	386.454.7298	32000
CONTRACTOR		STORY OF STREET	S 27-S TO C-138,TF				
LOCATION OF	PROPERTY		T' TH 4TH PLACE		TEK TUK	N, TO THORNE	
TYPE DEVELO	PMENT S	CREEN ROOM A		ESTIMATED COS	ST OF CO	NSTRUCTION	122050.00
HEATED FLOO	DR AREA	1699.00	TOTAL A	AREA 2441.00		HEIGHT	STORIES
FOUNDATION		WALLS	 S	ROOF PITCH		FL	OOR
LAND USE & Z	ZONING	A-3			MAX	. HEIGHT	
Minimum Set Ba	ack Requirments	: STREET-F	RONT 30.	00 I	REAR	25.00	SIDE 25.00
NO. EX.D.U.	11	FLOOD ZONE	X	DEVELOPME	ENT PERM	MIT NO.	
PARCEL ID	30-7S-17-10058	-121	SUBDIVIS	SION SFRP			
LOT 11	BLOCK	PHASE _	UNIT		TOTA	AL ACRES	
				Î	110		
Culvert Permit No	In Culv	ert Waiver Co	ontractor's License N	- Jumbar	melle	Applicant/Owner	/Contractor
EXISTING		3-015	BLK	Nulliber		АррисаниОwner/ TH	N
Driveway Conne		ic Tank Number		oning checked by	_	roved for Issuanc	1
COMMENTS:	TTOOTABOT	E ROAD. NO IVI	PACT FEE. ADDIT	HOW TO EXISTI	vo since	Check # or C	ash ²⁹²⁰
		FOR BUI	L DING 8 701	IINO DEDAD	TAKENIT		
T		FOR BUI	LDING & ZON	IING DEPAR	IMENI		(footer/Slab)
Temporary Powe		e/app. by	Foundation	date/app. by		_ Monolithic _	data/ann bu
Under slab rough		e/app. by	CI-1			Chardeine/	date/app. by
Officer stab fough	n-m plumoing .	date/app	Slat	date/app	, by	Sneathing/	Nailing date/app. by
Framing			Rough-in plumbing		•	l floor	cate app. of
10	date/app. by		stonego on premioning	5 400 10 5140 4114 61			date/app. by
Electrical rough-			Heat & Air Duct			Peri. beam (Linte	el)
100cf		app. by		date/app. b			date/app. by
Permanent power	rdate/app	by	C.O. Final	1		Culvert	J-4-/ 1
M/H tie downs, b				date/app. by		Pool	date/app. by
Reconnection				app. by	Heller D. I		date/app. by
-	date/ap	pp. by	Pump pole	ate/app. by	Utility Pol	date/app. by	<u>'</u>
M/H Pole			el Trailer	200	_	Re-roof	date/app. by
date	Jann I						
	/app. by			date/app. by			date/app. by
BUILDING PERI			CERTIFICATION I		l	SURCHARGE	
BUILDING PERI		615.00	CERTIFICATION I	FEE \$ 12.21		SURCHARGE	
	MIT FEE \$	615.00 ONING C		FEE \$ 12.21 .00 FIRE FEE	\$ 0.00	SURCHARGE WAST	FEE \$ 12.21

PERMIT

NOTICE: IN ADDITION TO THE REQUIREMENTS OF THIS PERMIT, THERE MAY BE ADDITIONAL RESTRICTIONS APPLICABLE TO THIS PROPERTY THAT MAY BE FOUND IN THE PUBLIC RECORDS OF THIS COUNTY. AND THERE MAY BE ADDITIONAL PERMITS REQUIRED FROM OTHER GOVERNMENTAL ENTITIES SUCH AS WATER MANAGEMENT DISTRICTS, STATE AGENCIES, OR FEDERAL AGENCIES.

"WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCEMENT MAY RESULT IN YOUR PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR AN ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT."

EVERY PERMIT ISSUED SHALL BECOME INVALID UNLESS THE WORK AUTHORIZED BY SUCH PERMIT IS COMMENCED WITHIN 180 DAYS AFTER ITS ISSUANCE, OR IF THE WORK AUTHORIZED BY SUCH PERMIT IS SUSPENDED OR ABANDONED FOR A PERIOD OF 180 DAYS AFTER THE TIME THE WORK IS COMMENCED. A VALID PERMIT RECIEVES AN APPROVED INSPECTION EVERY 180 DAYS. WORK SHALL BE CONSIDERED TO BE IN ACTIVE PROGESS WHEN THE PERMIT HAS RECIEVED AN APPROVED INSPECTION WITHIN 180 DAYS.

The Issuance of this Permit Does Not Waive Compliance by Permittee with Deed Restrictions.

Columbia County Building Permit Application

For Office Use Only Application # 0802-30 Date Received 2/25 By Iw Permit # 26816
Zoning Official BLK Date 03.03.05 Flood Zone FEMA Map # MA Zoning A=3
Land Use A-3 Elevation NA MFE NA River NA Plans Examiner NA Date 3-29-38
Comments No Impact Fee, Ald to be existing Structure
□NOC ★EH Deed or PA Site Plan □ State Road Info □ Parent Parcel #
□ Dev Permit # □ In Floodway □ Letter of Authorization from Contractor
□ Unincorporated area □ Incorporated area □ Town of Fort White □ Town of Fort White Compliance letter
Septic Permit No. <u>X08-015</u> Fax 386. 365. 5827
Name Authorized Person Signing Permit Mtchell Sand Phone 386 454-7298
Address 349 SW Thorne Lane, 4 White, 7/32038
Owners Name Mitchell Sonad Phone 386-454-7398
911 Address 349 SW Thorne Lane, 72 Whole, 7/32038
Contractors Name Owner Phone 386-454-7298
Address JAME OJ Chora
Fee Simple Owner Name & Address Mitchell Soad 349 SW Thorne Lane Fortwhite
Bonding Co. Name & Address
Architect/Engineer Name & Address Michola S Paul Gersler Lake City Fl
Mortgage Lenders Name & Address Bank of America
Circle the correct power company — FL Power & Light — Clay Elec. — Suwannee Valley Elec. — Progress Energy
u \
Property ID Number 30-75-17 - 10058-121-HX Estimated Cost of Construction 430,000
Subdivision Name Sante Fe River Partation Lot Block Unit Phase
Driving Directions Southon 15 27, 8 miles S. of Ft White, Right on CR138,
First Left after Turn. Down to Thorne Lane, Right on Horne.
4th house on Right Number of Existing Dwellings on Property_
Construction of SCNEFN Gom 2ddibion Total Acreage 1.84 Lot Size 184
Do you need a - <u>Culvert Rermit</u> or <u>Culvert Waiver</u> or <u>Have an Existing Drive</u> Total Building Height 13'
Actual Distance of Structure from Property Lines - Front Side Side Side Side Rear
Number of Stories Heated Floor Area 1999 Total Floor Area 2441 Roof Pitch 412
Application is hereby made to obtain a permit to do work and installations as indicated. I certify that no work or installation has commenced prior to the issuance of a permit and that all work be performed to meet the standards of all laws regulating construction in this jurisdiction. $Cuff - 292C -$
Page 1 of 2 (Both Pages must be submitted together.)

OUNTER COURT

COLUMBIA COUNTY BUILDING DEPARTMENT

135 NE Hernando Ave.. Suite B-21

Lake City, FL 32055

Office: 386-758-1008 Fax: 386-758-2160

NOTARIZED DISCLOSURE STATEMENT

FOR OWNER/BUILDER WHEN ACTING AS THER OWN CONTRACTOR AND CLAIMING EXEMPTION OF CONTRACTOR LICENSING REQUIREMENTS IN ACCORDANCE WITH FLORIDA STATUTES, ss. 489.103(7).

State law requires construction to be done by licensed contractors. You have applied for a permit under an exemption to that law. The exemption allows you, as the owner of your property, to act as your own contractor with certain restrictions even though you do not have a license. You must provide direct, onsite supervision of the construction yourself. You may build or improve a one-family or two-family residence or a farm outbuilding. You may also build or improve a commercial building, provided your costs do not exceed \$75,000. The building or residence must be for your own use or occupancy. It may not be built or substantially improved for sale or lease. If you sell or lease a building you have built or substantially improved for yourself within 1 year after the construction is complete, the law will presume that you built or substantially improved it for sale or lease, which is a violation of this exemption. You may not hire an unlicensed person to act as your contractor or to supervise people working on your building. It is your responsibility to make sure that people employed by you have licenses required by state law and by county or municipal licensing ordinances. You may not delegate the responsibility for supervising work to a licensed contractor who is not licensed to perform the work being done. Any person working on your building who is not licensed must work under your direct supervision and must be employed by you, which means that you must deduct F.I.C.A. and withholding tax and provide workers' compensation for that employee, all as prescribed by law. Your construction must comply with all applicable laws, ordinances, building codes, and zoning regulations.

I understand that if I am not physically doing the work or physically supervising free labor from friends or relatives, that I must hire licensed contractors, i.e. electrician, plumber, mechanical (heating & air conditioning), etc. I further understand that the violation of not physically doing the work, and the use of unlicensed contractors at the construction site, will cause the project to be shut down by the inspection staff of the Columbia County Building Department. Additionally, state statutes allows for additional penalties. I also understand that if this violation does occur, that in order for the job to proceed, I will have a licensed contractor come in and obtain a new permit as taking the job over. I understand that if I hire subcontractors under a contract price, that they must be licensed to work in Columbia County, i.e. masonry, drywall, carpentry. Contractors licensed by the Columbia County Contractor Licensing Section or the State of Florida are required to have worker's compensation and liability coverage.

TYPE OF CONSTRUCTION

	THE OF CONSTRUCTION	
() Single Family Dwelling	() Two-Family Residence () Farm Outb	uilding
() Other	Maddition, Alteration, Modification or other Improv	ement
VI ADAM (CO. a)		
1 Michell XDD	, have been advised of the above disclosure stat	
from contractor licensing as an owner/bui	lder. I agree to comply with all requirem∉nts provided for	in Florida Statutes
ss.489.103(7) allowing this exception for t	he construction permitted by Columbia County Building	
Permit Number	1 thou sond	
	- Warm Ang	
	Owner Builder Signature	Date
FLORIDA NOTARY		
The above signer is personally known to m	e of produced identification FLDC	
Notary Signature Jewandle 4.	Cent Date 02-25-08 LAWANDAY. R MY COMMISSION # EXPIRES: October Bonded Thru Notary Publis	DD 710646 29, 2011
FOR BUILDING DEPARTMENT USE ONLY	William South	
I hereby certify that the above listed owner	r/builder has been notified of the disclosure statement in	Florida Statutes
ss 489.103(7). DateB	uilding Official/Representative	

< 50/est > 349 SW Mitch Saad Existing house Etisting Pool Thorne 200 Existing / 100/ Lane

NOTICE OF COMMENCEMENT	*
Tax Parcel Identification Number 30-75-17-10	County Clerk's Office Stamp or Seal
THE UNDERSIGNED hereby gives notice that improvements vibration is provided in this N	will be made to certain real property, and in accordance with Section 713.13 of the OTICE OF COMMENCEMENT.
1. Description of property (legal description):	ne lane, Fort White, FT-32038
2. General description of improvements:	he cane, Fort White, FT 32038
3. Owner Information a) Name and address: Mitchell Sound b) Name and address of fee simple titleholder (if other c) Interest in property	349 SW Thorns Lana, A White F1 32038 rthan owner) Same
	Saad 349 SWThorne have, fort white, FI
a) Name and address: b) Amount of Bond:	
c) Telephone No.:	Inst;200812004462 Date:3/4/2008 Time:4:27 PM A DC.P.DeWitt Cason Columbia County Page 1 of
b) Phone No. 7. Identity of person within the State of Florida designated by ow	
a) Name and address:	Fax No. (Opt.)
b) Telephone No.:	Fax No. (Opt.)
	Fax No. (Opt.) date is one year from the date of recording unless a different date
COMMENCEMENT ARE CONSIDERED IMPROPER PAY STATUTES, AND CAN RESULT IN YOUR PAYING TWIC COMMENCEMENT MUST BE RECORDED AND POSTEI	IE OWNER AFTER THE EXPIRATION OF THE NOTICE OF YMENTS UNDER CHAPTER 713, PART I, SECTION 713.13, FLORIDA CE FOR IMPROVEMENTS TO YOUR PROPERTY; A NOTICE OF DON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND RAN ATTORNEY BEFORE COMMENCING WORK OR RECORDING Signature of Owner or Owner's Authorized Office/Director/Partner/Manager
The foregoing instrument was acknowledged before me , a Florida No	otary, this 4th day of MAKh . 20 08, by:
Mitchell SAAD as	Owner (type of authority, e.g. officer, trustee, attorney
fact) for	(name of party on behalf of whom in the was executed).
Personally KnownOR Produced Identification Type Notary Signature	MY COMMISSION # DD 333586 EXPIRES: June 28 2008 Bonded Thru Notary Public Underwriters
11. Verification pursuant to Section 92 525. Florida Statutes. U facts stated in it are true to the best of my knowledge and b	AND Inder penalties of perjury. I declare that I have read the foregoing and that the pelief. Signature of Natural Person Signing (in line #10 above.)

26816

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844 Florida Engineering Certificate of Authorization Number: 0 278 Florida Certificate of Product Approval # FL1999 Page 1 of 1 Document ID:1TGV8228Z0121095348

Truss Fabricator: Anderson Truss Company

Job Identification: 8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , **

Truss Count: 18

Model Code: Florida Building Code 2004 and 2006 Supplement

Truss Criteria: ANSI/TPI-2002(STD)/FBC

Engineering Software: Alpine Software, Version 7.36.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-02 -Partially Enclosed

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

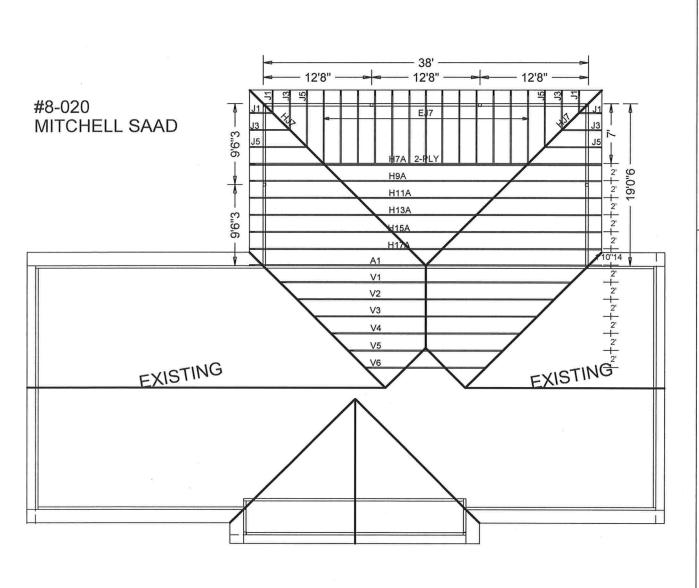
Details: VALTRUSS-

	_			
	#	Ref Description	Drawing#	Date
	1	32259 H7A	08112010	04/21/08
	2	32260 H9A	08112002	
	3	32261H11A	08112001	04/21/08
	4	32262H13A	08112001	04/21/08
	5	32263H15A		04/21/08
	6	32264H17A	08112004	04/21/08
Ì	7	32265 V1	08112007	04/21/08
			08112001	04/21/08
	8	32266V2	08112002	04/21/08
	9	32267V3	08112003	04/21/08
	10	32268 V4	08112004	04/21/08
ı	11	32269 V5	08112005	04/21/08
l	12	32270 V6	08112006	04/21/08
l	13	32271A1	08112007	04/21/08
l	14	32272 J5	08112009	04/21/08
l	15	32273J3	08112005	
	16	32274J1	08112006	04/21/08
	17	32275 EJ7	08112008	04/21/08
	18	32276HJ7		04/21/08
-		02270 1107	08112011	04/21/08

Seal Date: 04/21/2008

-Truss Design Engineer-James F. Collins Jr. Florida License Number: 52212 1950 Marley Drive Haines City, FL 33844





JOB DESCRIPTION:: OWNER BUILDER
/: Mitchell Saad

JOB NO: 8-020

PAGE NO: 1 OF 1

Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP Bot chord 2x6 SP #2 :B2 2x6 SP #1 Dense: Webs 2x4 SP #3 #2:

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures

In lieu 24" OC. of structural panels use purlins to brace all flat TC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\cdot$

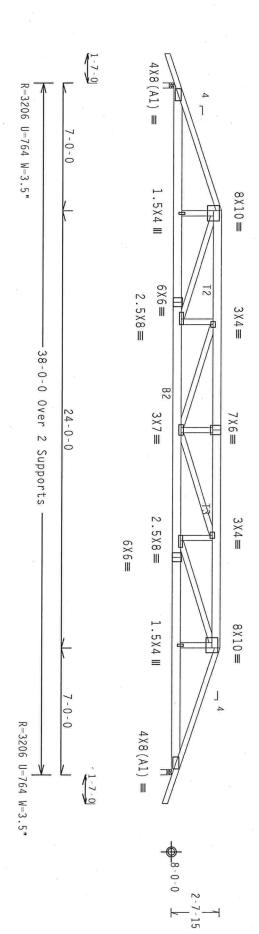
COMPLETE TRUSSES REQUIRED

Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid splitting.

Roof overhang supports 2.00 psf soffit load

#1 hip supports 7-0-0 jacks with no webs

Calculated vertical deflection is 0.55" due to live load and 0.84" due to dead load at $\rm X=19\text{-}0\text{-}0$.



Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0)

FL/-/4/-/-/R/-

R8228- 32259

04/21/08

PLT TYP.

Wave

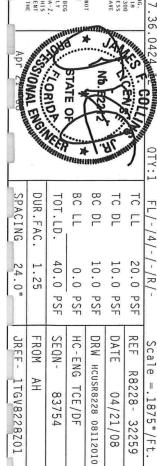
** IMPORTANT** ruberish a copy of this design to the installation contractor. It were, the shall not be responsible for any deviation from his design. Any falling to bill the truss in compondate with TPI, or familiating, inauling, shipping, installing a bracing of trusses. The component of the property of the component in Applicable Provisions of his califorat design spec, by afapa) and TPI. Design component of the provisions of his califorat design spec, by afapa) and TPI. Component of the provisions of his califoration parties are more to followed the his califoration parties are more to followed the history.

BUILDING DESIGNER PER DRAWING INDICATES UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z
BY (1) SHALL BE PER ANNEX AS OF TPI1-2002 SEC.3. A SEAL ON THIS 2 SEC.3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT G IS THE RESPONSIBILITY OF THE

Haines City, FL 33844
FI Cortificate of Authorization # 0 279

ITW Building Components Group Inc.

ALPINE



TCE / DF 83754

1TGV8228Z01

Top Bot Calculated vertical deflection is 0.42" due to live load and 0.62" due to dead load at X = 15-8-9. In lieu of structural panels use purlins to brace all flat TC $24\mbox{\ensuremath{^{\circ}}}\xspace$ 0C. Haines City, FL 33844 FL Certificate of Authorization # 0.278 PLT TYP. Roof overhang supports 2.00 8-UZU--UWNEK BUILDEK MITChell Saad --ITW Building Components Group Inc. chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 ALPINE 20 Gauge HS, Wave 1-7-0 3X8(A1) =R-1644 U-388 W-3.5' **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH TPI: OR FARRICATING, HANDLIGG, SUPPING, INSTALLING A BRACHIG OF TRUSSES, DESIGN CONTROLATION, AND TPI. IT BCG CONNECTOR THAT REPLICABLE PROVISIONS OF DIOS (MATIGNAL DESIGN SPEC, BY AFRA) AND TPI. IT BCG CONNECTOR PLATES ARE MADE OF 20/18/16/AGA (M.H.SSYA) ASTM ASS JGRADE 40/60 (M. K/M.SS) GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF FPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICALES ACCEPTANCE OF PROFESSIONAL REGIONNERS AND FPI1-5002 SEC.3. A SEAL ON THIS DRAWING INDICALES ACCEPTANCE OF PROFESSIONAL REGIONNERS AS OF FPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICALES ACCEPTANCE OF PROFESSIONAL REGIONNERS AS OF FPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICALES ACCEPTANCE OF PROFESSIONAL REGIONNERS AS OF FPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICALES ACCEPTANCE OF PROFESSIONAL REGIONNERS AS OF FPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICALES ACCEPTANCE OF PROFESSIONAL REGIONNERS AS OF FPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICALES ACCEPTANCE OF PROFESSIONAL REGIONNERS AS OF FPI1-2002 SEC.3. **WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY IMPORNATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (HODO) TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MORISON, MI 35719) FOR SAFETY PRACTICES PRIOR TO PREFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOR GHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE DESIGN SHOWN. THE SUITABILITY BUILDING DESIGNER PER ANSI/TPI 1 psf soffit load 9-0-0 1.5X4₩ 386-454-7298//397-8585wk , Design Crit: 3×4≡ 6X8≡ HS2512 ≡ TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) **@** 1.5X4 Ⅲ 38-0-0 Over 2 Supports 3 X 9 ≡ ** H9A) 20-0-0 3 X 6 ≡ 3 X 4 ≡ 2.5X6≡ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, rocated within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Wind reactions based on MWFRS pressures HS2512 ≡ 3 \ 4 ≡ €X8≡ 9-0-0 BC DL TC DL DUR.FAC. TC LL SPACING BC TOT.LD. FL/-/4/-/-/R/-R=1644 U=388 W=3.5" 3X8(A1) =40.0 10.0 20.0 1.25 24.0" 10.0 PSF 0.0 1-7-0 PSF PSF PSF PSF JREF -FROM SEQN-DATE REF HC-ENG DRW HCUSR8228 08112002 Scale =.1875"/Ft. 8-0-0 R8228- 32260 1TGV8228Z01 TCE / DF 83759 04/21/08

Bot In lieu of structural panels use purlins to brace all flat IC $24\mbox{\ensuremath{^{\circ}}}\xspace$ 0C. Roof overhang supports 2.00 psf soffit load Haines City, FL 33844
FL Cortificate of Authorization # 0 278 PLT TYP. ITW Building Components Group Inc. 8-020--OWNER BUILDER Mitchell Saad -chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 ALPINE 20 Gauge HS, Wave 1-7-0 3X8(A1) =R=1644 U=387 W=3.5" **IMPORTANT**GURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL HOT BE RESPONSIBLE FOR ANY DEVLATION FROM THIS DESIGN FOR TALIDUR FOR BUILD THE TRUSS IN COMPORNANCE WITH TOT: OR FARBICATING, MINDIVING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF HOS (MATIONAL DESIGN SPEC, BY AFAPA) AND TPI.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF HOS (MATIONAL DESIGN SPEC, BY AFAPA) AND TPI.

CONNECTOR PLATES ARE MADE OF 20/18/166A (M.H/SS/K) ASTM A653 GRADE 40/50 (H. K.H.SS) GAVE. STEEL, APPLY **WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.

REFER TO BEST (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 219

MORTH LEE STREET, SUITE 312, ALEXAMBRIA, VA, 22314) AND WICA (MODD TRUSS COUNCIL OF AMERICA, 6300

ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PEFFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDIVIALID TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING. DRAWING INDICATES ACCEPTANCE OF PROFESSION DESIGN SHOWN. THE SUITABILITY AND USE OF BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. 11-0-0 1.5 \ 4 \ ■ 386-454-7298//397-8585wk , Design Crit: 5 X 5 = 3 X 7 = OF THERMISE LOCATED ON THIS DESIGN, POSITION PER RORAINGS 160A-Z SINALL BE PER ANNEX A3 OF TP11-2002 SEC.3. A SEAL ON THIS ORAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE HS2512 ≡ TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) (0) 38-0-0 Over 2 Supports 3 X 4 ≡ * H11A) 3 X 4 ≡ 0-0-0 3 X 4 == 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Wind reactions based on MWFRS pressures HS2512 ≡ 3 X 7 = 5 X 5 ≡ ONAL EMORIE TATE OF .5X4 ≢ [1-0-0]BC LL BC DL TC DL SPACING DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/-R=1644 U=387 W=3.5" 3X8(A1) =40.0 24.0" 20.0 1.25 10.0 PSF 10.0 PSF 0.0 1-7-0 PSF PSF PSF DATE REF JREF -FROM SEQN-DRW HCUSR8228 08112001 HC-ENG יים שטביינונים בי וונסשט ווויו. Scale =.1875"/Ft. 8-0-0 R8228- 32261 1TGV8228Z01 TCE / DF 3-11-15 83764 04/21/08

Top In lieu of structural panels use purlins to brace all flat TC @ $24\mbox{\ensuremath{^{\circ}}}$ Roof overhang supports 2.00 psf soffit load Haines City, FL 33844 FL Certificate of Authorization # 0 278 PLT TYP. 8-020--OWNER BUILDER Mitchell Saad --ITW Building Components Group Inc. webs 2x4 SP ALPINE 20 Gauge HS, Wave 1-7-0 3X8(A1) =#2 Dense #2 Dense #3 R=1644 U=386 W=3.5' **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, ING. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN: AFFAILURE TO BUILD THE TRUSS IN COMPORMANCE HITH TPI: OR FABRICATION, INANLING, SHAPPING, INSTALLING A BRACTIC OF TRUSSES.

DESIGN COMPORMS HITH APPLICABLE PROVISIONS OF DDS (MATIONAL DESIGN SEC. B. *AFRAN) AND TPI. ITM BCG CONNECTOR PLATES ARE MADE OF 20/18/16/36 (M.H/SS/M) ASIM A653 GRADE A0/500 (M. K/M.SS) GALY. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERNISE LOCATED ON THIS DESIGN, POSITION PER BRANTHES 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE FER ANKEX A3 OF TPII: 200E YEC. 3. ASIAL ON THIS BUSINGS MACHINES AGRICADED BY (I) SHALL BE FER ANKEX A3 OF TPII: 200E YEC. 3.

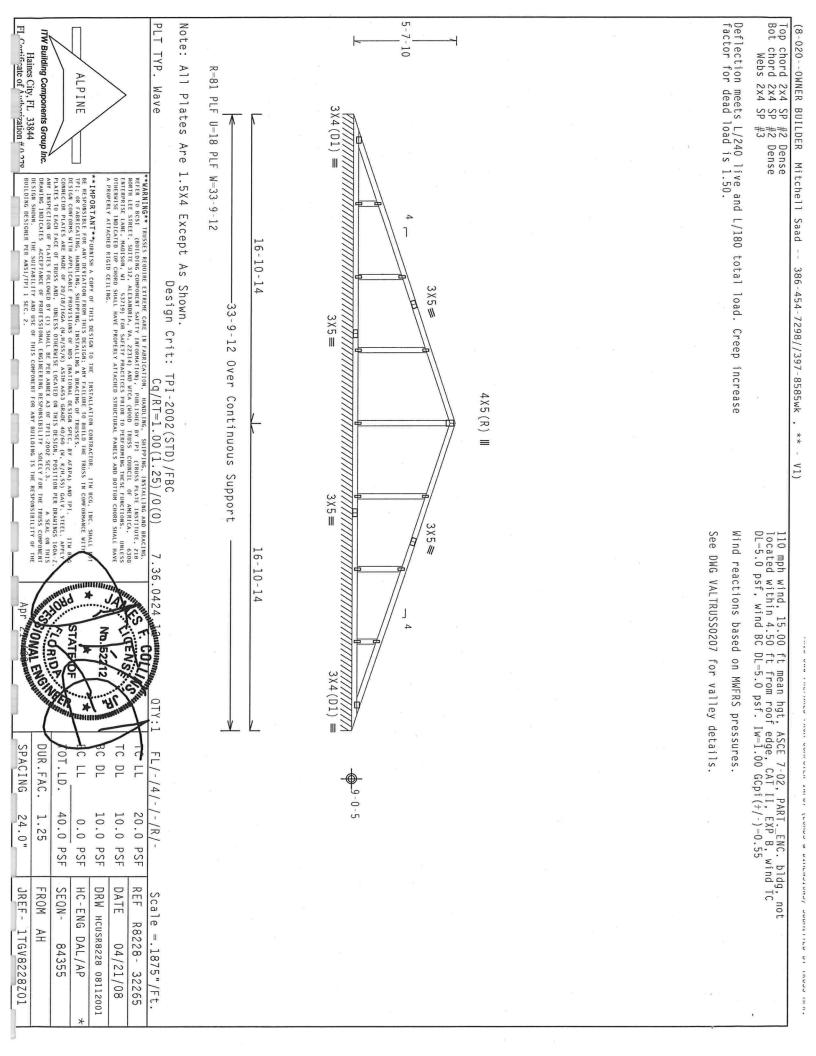
BRANTING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUSINGS IS THE RESPONSIBILITY OF THE OTHERWISE INDICATED TOP CHORD SHALL A PROPERLY ATTACHED RIGID CEILING 1.5X4 Ⅲ 13-0-0 3X4= 386-454-7298//397-8585wk , Design Crit: HS2512 ≡ 3 \ 4 ≡ 5 X 8 = TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) 38-0-0 Over 2 Supports ** H13A 1.5X4 III 3 X 7 ≡ .2-0-0 Wind reactions based on MWFRS pressures 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. I_W =1.00 GCpi(+/-)=0.55 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 5 X 8 = 3 X 4 ≡ HS2512 ≡ 1.5X4 Ⅲ 3 \ 4 ₩ 13-0-0 BC LL BC DL TC DL SPACING DUR.FAC. C TOT.LD. FL/-/4/-/-/R/-R=1644 U=386 W=3.5" 3X8(A1) =40.0 20.0 1.25 10.0 PSF 24.0" 10.0 PSF 0.0 1-7-0 PSF PSF PSF FROM SEQN-DATE REF JREF -DRW HCUSR8228 08112003 HC-ENG Scale =.1875"/Ft 8-0-0 R8228- 32262 1TGV8228Z01 TCE / DF 83769 04/21/08

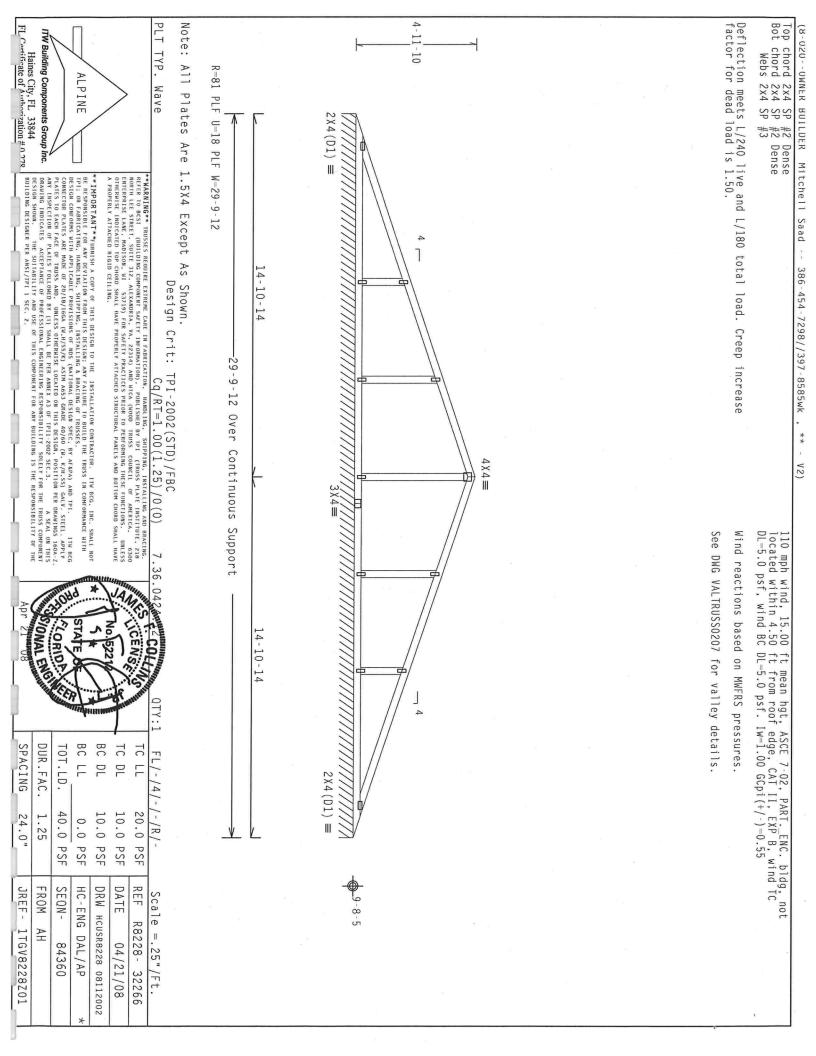
Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 In lieu of structural panels use purlins to brace all flat TC @ $24\,\text{{\tt "}}$ OC. Roof overhang supports 2.00 psf soffit load Haines City, FL 33844 FL Certificate of Authorization # 0 278 PLT TYP. 8-020--OWNER BUILDER Mitchell Saad --ITW Building Components Group Inc. ALPINE 20 Gauge HS, Wave 1-7-0 $3X8(A1) \equiv$ R-1644 U-385 W-3.5" **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR.
BE RESCONSIBLE FOR MAY DEVIATION FROM THIS DESIGN: MAY FAILURE TO BUILD THE TRUS
TPI: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACITIGO FE TRUSSES.
DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (HATIONAL DESIGN SPEC, BY AFAR
CONNECTOR PLATES ARE MADE OF 20/18/166A (M.H/55/K) ASTM A653 GRADE 40/60 (M.K/H **WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BESS! (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TIP! (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREET, SUITE 312. ALEXANDRIA, VA, AZSIJA) AND HICA (MODO TRUSS COUNCIL OF AMERICA, 6300 CHIERRETS LAME, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP GUORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PAWELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING. DRAWING INDICATES 1.5X4 15-0-0 3 X 4 ≡ 386-454-7298//397-8585wk , Design Crit: PAJGGA, (H.MYSS/K) ASTM AGS3 GRADE 40/60 (H. K/M'SS) GALV, STEEL, APPLY UNLESS OHTEGHISE LOCATED ON THIS DESIGNE POSITION PER DRAWINGS 160A-Z WY (1) SMALL BE PER AMBEX AS OF TPI1-2002 FEC. 3. A SEAL ON THIS PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE 5X5# HS2512 ≡ N TO THE INSTALLATION COMPRACTOR. ITW BGG, INC., SHALL NOT SLOH, ANY FALURE TO BUILD THE TRUSS IN COMFORMANCE MITH AULING & BRACHIG OF TRUSSES.

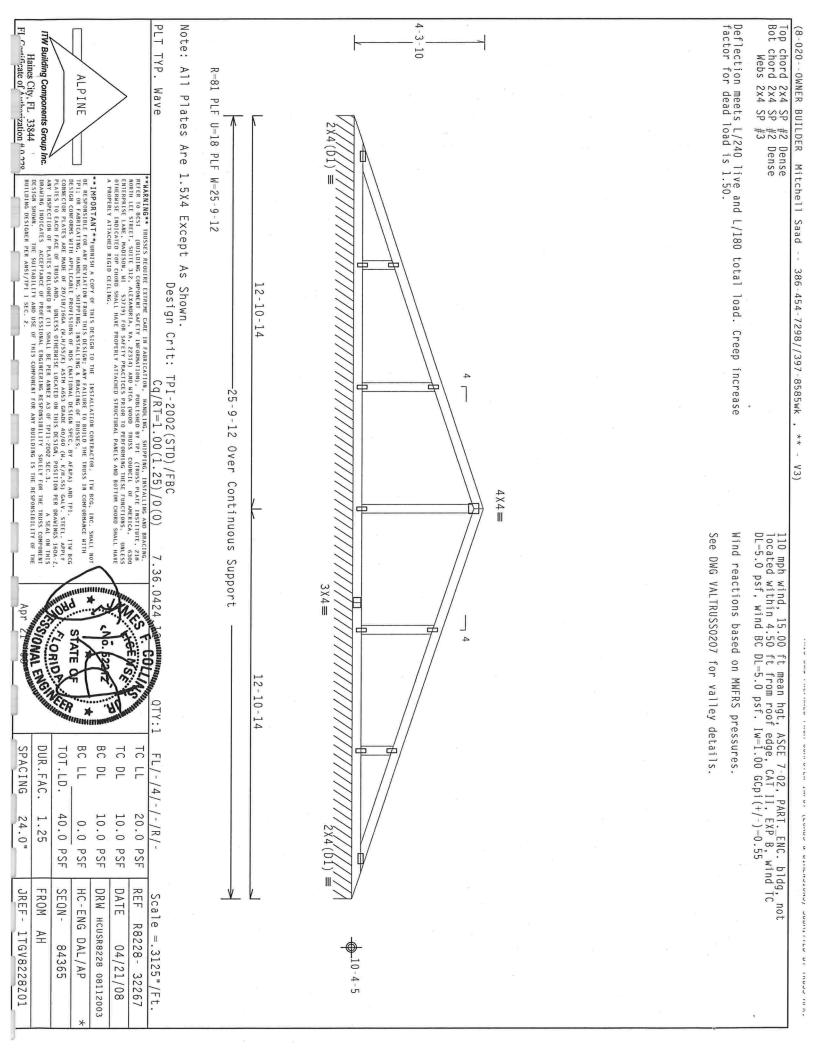
MICHING & BRACHIG OF TRUSSES.

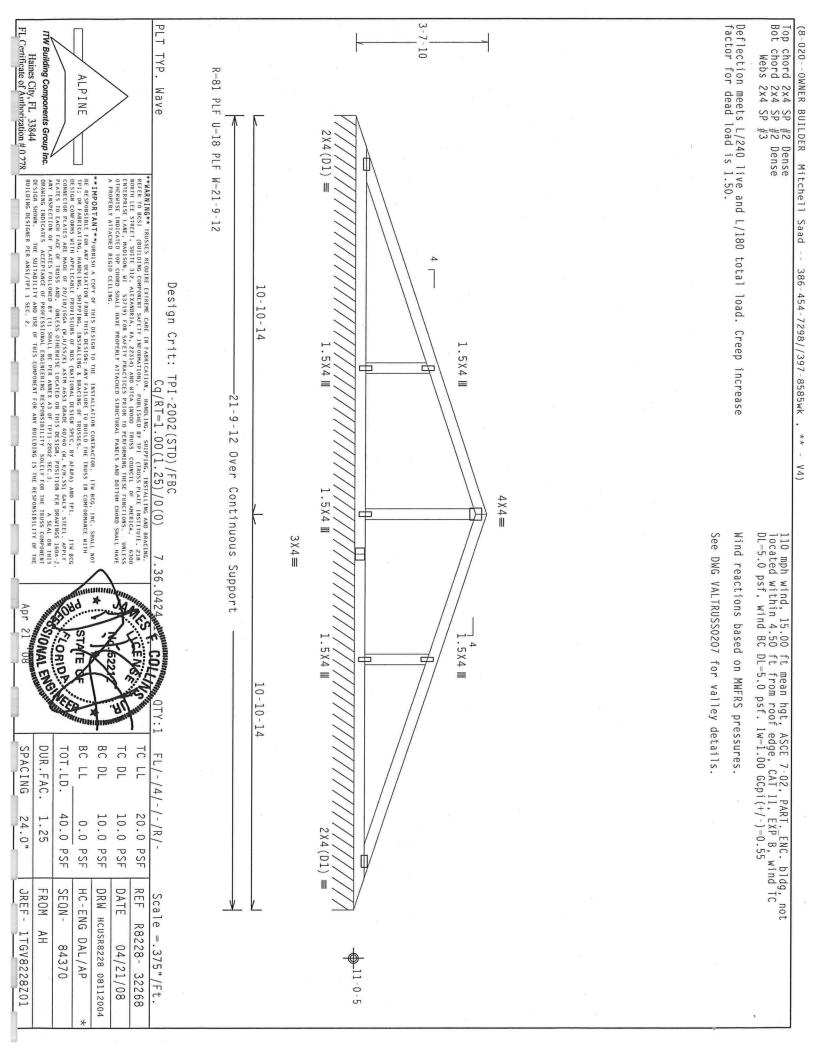
MICHAEL AND THE TRUSSES. TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0) 5 X 5 = 3 X 7 ≡ 38-0-0 Over 2 Supports ** H15A) 3 X 4 ≡ 8-0-0 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 Wind reactions based on MWFRS pressures Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 3 X 7 **≡** 5 X 5 == HS2512 ≡ CORIDE 5 X 5 3 \ 4 ≡ [5-0-0 QTY:1 1.5X4 / BC DL BC LL TC DL SPACING DUR.FAC. TC TOT.LD. FL/-/4/-/-/R/-הירט דעומי לרחשמת פ מדוירעתידמעת למחוידוורה חי ועחתת נעשי R=1644 U=385 W=3.5" 3X8(A1) = 40.0 10.0 20.0 1.25 10.0 PSF 24.0" 0.0 1-7-0 PSF PSF PSF PSF FROM SEQN-DATE REF JREF -HC-ENG DRW HCUSR8228 08112004 Scale =.1875"/Ft R8228- 32263 1TGV8228Z01 TCE / DF 83774 04/21/08

Bot In 24" Roof overhang supports 2.00 psf soffit load Haines City, FL 33844
FL Certificate of Authorization # 0 278 PLT TYP. 8-020--OWNER BUILDER Mitchell Saad --TW Building Components Group Inc. lieu of structural panels use purlins to brace all flat TC @ " 0C.chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 ALPINE Wave 1-7-0 $3X8(A1) \equiv$ R=1644 U=384 W=3.5" **IMPORTANT**FURMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR NAW DEVIATION FROM THIS DESIGN, NAY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH TPI: OR FARRICATING, HANDLING, SHEPPING, INSTALLING A BRACING OF TRUSSES, BUSICAL CONTROLS HITH APPLICABLE PROVISIONS OF HOS (MATICHAL DESIGN SPEC, BY ATRPA) AND TPI. ITH BCG CONNECTOR PLATES ARE MADE OF 20/18/1668 (M.H/SS/K). ASTH AG53 GRADE 40/160 (M.K/M.SS) GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSS, AND, LINESS OTHERHISE LOCATED ON THIS DESIGN. POSITION PER DRAWHENG 160A-Z ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A.3 OF TPI1-200Z SEC.3. A SEAL ON THIS A PROPERLY ATTACHED RIGID CEILING DRAWING INDICATES ACCEPTANCE 1.5X4 // 386-454-7298//397-8585wk . Design Crit: 17-0-0 3 X 4 **≡** AND USE OF THIS COMP. SEC. 2. 3X5■ 3X4≢ 4 X 5 = TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) 38-0-0 Over 2 Supports 5 X 5 ≡ ** 3 X 7 = 2 SEC.3. A SEAL ON THIS SOLELY FOR THE TRUSS COMPONENT G IS THE RESPONSIBILITY OF THE H17A 4-0-0 5 X 8 ≡ 3 X 4 ≡ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Wind reactions based on MWFRS pressures 4 X 5 ≡ 3 \ 4 # STONAL ENGINEE 3 × 5 ₩ 3 X 4 **≡** 17-0-0 1.5X4 BC LL BC DL TC DL DUR.FAC. TC LL SPACING TOT.LD. FL/-/4/-/-/R/-R=1644 U=384 W=3.5" 3X8(A1) = 1.25 40.0 20.0 10.0 PSF 10.0 PSF 24.0" 0.0 1-7-0 PSF PSF PSF JREF -FROM SEQN-DATE REF HC-ENG DRW HCUSR8228 08112007 Scale = .1875"/Ft. 8-0-0 R8228-1TGV8228Z01 TCE / DF 83779 04/21/08 32264









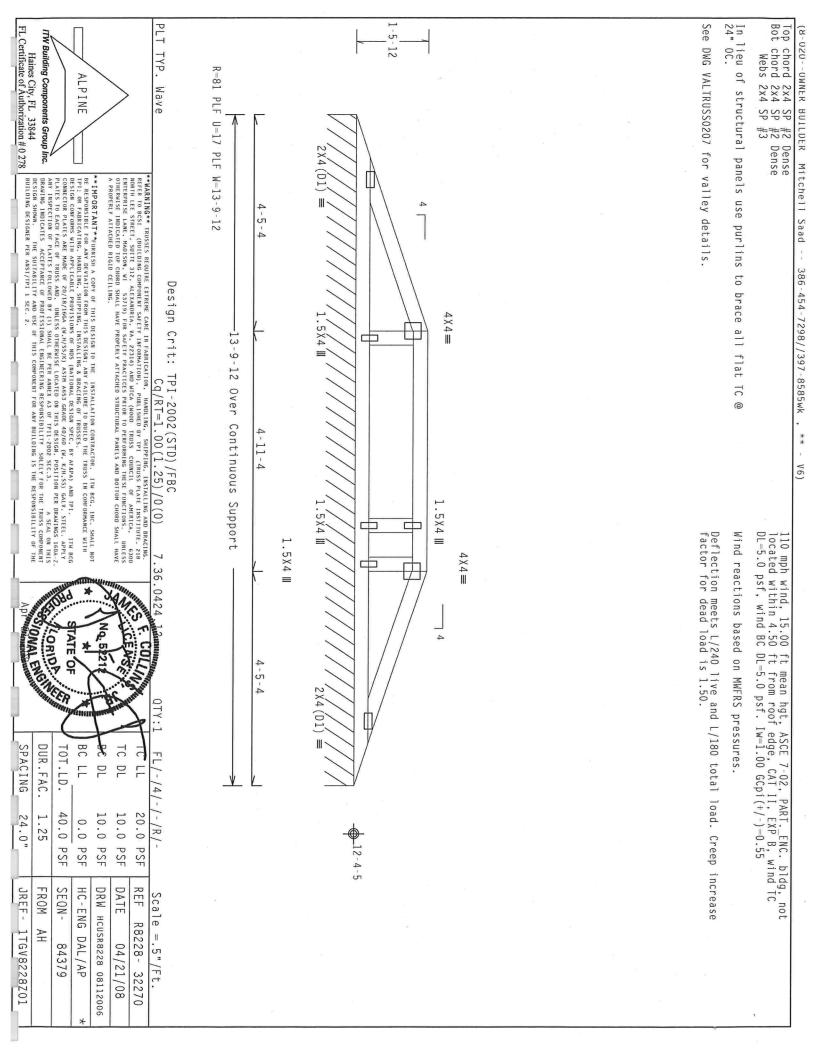
Top chord 2x4 SP # Bot chord 2x4 SP # Webs 2x4 SP # In lieu of structural panels use purlins to brace all flat TC 24" $\,$ OC. PLT TYP. 2-9-12 See DWG VALTRUSS0207 for valley details. Haines City, FL 33844 FL Certificate of Authorization # 0 278 (8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ITW Building Components Group Inc. R=81 PLF U=17 PLF W=17-9-12 ALPINE Wave #2 Dense #2 Dense #3 2X4(D1 **IMPORTANT**CHEMIS A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE EGG. HIC. SHALL NOT THE RESPONSEME FOR ANY PERIATION FROM HIS DESIGN. ANY FALUNE TO BUILD THE TRUSS IN COMPORMACE WITH TPT: OR FARELCATING. IMMULHA. SHIPPING. INSTALLING & BRACING OF TRUSSES. TO THE TRUSS IN COMPORMS OF THE PROPERTY OF THE BESIGN COMPORES HITH APPLICABLE PROPISIONS OF MSS (MATIDMAL DESIGN SPEC, BY MEAPA) AND TPT.

CONNECTOR PLATES AND MODE TO 20/18/1660A (M:M/SS/N) ASTH MSS GRADE MODE OF THE MSS.) GALV. STEEL, APPLY **#ARANING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, IMADILING, SHIPPING, INSTALLING AND BRACING.
REFER TO BESS! (BUILDING COMPONENT SAFETY IMPORANION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218
MORH LEE STREE, SUITE 312, ALEXANDRIA, VA, 2231) AND NICA (MODD TRUSS COUNCIL OF AMERICA, 6300
ENTERPLISE LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PEFFORMING THESE FUNCTIONS. UNLESS
OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED REGIO CEILING. CONNECTOR PLATES ARE MADE OF 20/18/16GA (H.H/SS/K) PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISI ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE I 8-5-4 1.5X4 Ⅲ 5 X 4 Ⅲ Design Crit: 17-9-12 Over Continuous Support OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z
SHALL BE PER ANNEX A3 OF TPI1-ZDO2 SEC.3. A SEAL ON THIS TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0) $4 \times 4 =$ 5×4 **Ⅲ** 1.5X4 III * 4 X 4 ≡ 3 X 4 ≡ V5) A SEAL ON THIS
OR THE TRUSS COMPONENT
RESPONSIBILITY OF THE 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Wind reactions based on MWFRS pressures 1.5X4 Ⅲ . 5×4 8-5-4 CORIDE 2X4(D1) BC DL TC DL DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/-דעו היו לרהטחים מ הדוורעוסדהעם! יחמוודוורה מו ועמים ווועי 40.0 10.0 20.0 24.0" 1.25 10.0 PSF 0.0 PSF PSF PSF PSF FROM SEQN-DATE REF HC-ENG DRW HCUSR8228 08112005 Scale =.375"/Ft. R8228- 32269 DAL/AP 84375 04/21/08

SPACING

JREF -

1TGV8228Z01



WARNING TO OWNER: YOUR FAILURE TO RECORD A NOTICE OF COMMENCMENT MAY RESULT IN YOU PAYING TWICE FOR IMPROVEMENTS TO YOUR PROPERTY. A NOTICE OF COMMENCEMENT MUST BE RECORDED AND POSTED ON THE JOB SITE BEFORE THE FIRST INSPECTION. IF YOU INTEND TO OBTAIN FINANCING, CONSULT WITH YOUR LENDER OR ATTORNEY BEFORE RECORDING YOUR NOTICE OF COMMENCEMENT.

FLORIDA'S CONSTRUCTION LIEN LAW: Protect Yourself and Your Investment

According to Florida Law, those who work on your property or provide materials, and are not paid-in-full, have a right to enforce their claim for payment against your property. This claim is known as a construction lien. If your contractor fails to pay subcontractors or material suppliers or neglects to make other legally required payments, the people who are owed money may look to your property for payment, even if you have paid your contractor in full. This means if a lien is filed against your property, it could be sold against your will to pay for labor, materials or other services which your contractor may have failed to pay.

NOTICE OF RESPONSIBILITY TO BUILDING PERMITEE:

YOU ARE HEREBY NOTIFIED as the recipient of a building permit from Columbia County, Florida, you will be held responsible to the County for any damage to sidewalks and/or road curbs and gutters, concrete features and structures, together with damage to drainage facilities, removal of sod, major changes to lot grades that result in ponding of water, or other damage to roadway and other public infrastructure facilities caused by you or your contractor, subcontractors, agents or representatives in the construction and/or improvement of the building and lot for which this permit is issued. No certificate of occupancy will be issued until all corrective work to these public infrastructures and facilities has been corrected.

OWNERS CERTIFICATION: I hereby certify that all the foregoing information is accurate and all work will be done in compliance with all applicable laws and regulating construction and zoning. I further understand the above written responsibilities in Columbia County for obtaining this Building Permit.

CONTRACTORS AFFIDAVIT: By my signature I understand and agree that I have informed and provided this written statement to the owner of all the above written responsibilities in Columbia County for obtaining this Building Permit.

Contractor's License Number

Affirmed under penalty of perjury to by the <u>Contractor</u> and subscribed before me this <u>25</u> day of <u>Feb.</u> 20<u>08</u>

Personally known____ or Produced Identification_<u>FLDC</u>______

Columbia County

Competency Card Number

Yawanda Y. Kent SEAL:

State of Florida Notary Signature (For the Contractor)

Owners Signature

Contractor's Signature (Permitee)

Bot PLT TYP. (A) 1x4~#3 or better "T" brace. 80% length of web member. Attach with 8d Box or Gun (0.113"x2.5",min.)nails @ 6" OC. Haines City, FL 33844 FL Certificate of Authorization # 0 278 (8-020--OWNER BUILDER Mitchell Saad --ITW Building Components Group Inc. chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 ALPINE 20 Gauge HS, Wave 3X8(A1) = R=1637 U=383 W=3.5" **IMPORTANT**FURMISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN: ANY FAILURE TO BUILD THE TRUSS IN COMFORMANCE WITH TP1: OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.
DESIGN COMPORENS WITH APPLICABLE PROVISIONS OF BUDS (WAITIONAL DESIGN SPEC, SY AF&PA) AND TP1. ITH BCG CONNECTOR PLATES ARE MADE OF 20/18/166A (W.H/SS/K) ASTM A653 GRADE 40/60 (M.K/H.SS) GALV. SITEEL. APPLY DRAING HOME ACCOUNT OF THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY DUILDING IS THE SUITABILITY OF THE **WARNING.** TRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. REFER TO BEST. (BUILDING COMPONENT SAFETY INFORMATION), PRUBLISHED BY TPT (TRUSS PLATE INSTITUTE, 21B MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND NTGA (400D TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PREFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TO PROBE SAMEL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE 1.5X4// 386-454-7298//397-8585wk , Design Crit: 3×4≡ 19-0-0 3×5# HS2512 ≡ 3×4 # TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0) 38-0-0 Over 5×5= 3 X 7 ≡ 2 Supports 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART_ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 Wind reactions based on MWFRS pressures Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. HS2512 ≡ Apr 3X4# YONAL ENGIN CORIDE 3×5₩ .9-0-0 3 X 4 ≡ 1.5X4 / QTY:1BC DL BC LL TC DL SPACING DUR.FAC. TC LL TOT.LD. FL/-/4/-/-/R/-R=1637 U=383 W=3.5" 3X8(A1) =40.0 10.0 20.0 10.0 PSF 24.0" 1.25 0.0 1-7-0 PSF PSF PSF PSF במיניים ביים ונות מו נתמים נחת. FROM DATE REF HC-ENG JREF -SEQN-DRW HCUSR8228 08112007 Scale =.1875"/Ft. R8228- 32271 1TGV8228Z01 DAL/AP 84348 04/21/08

PLT TYP. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Roof overhang supports 2.00 psf soffit load. Top chord 2x4 SP Bot chord 2x4 SP Haines City, FL 33844 FL Certificate of Authorization # 0 278 8-020--OWNER BUILDER Mitchell Saad --ITW Building Components Group Inc. ALPINE Wave #2 Dense #2 Dense **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE HITH FD: OR FABRICATHOR. HANDLING, SHEPPING, HISTALLING & BRACHEN OF TRUSSES; DESIGN COMPORES HITH APPLICABLE PROVISIONS OF MDS (MATIONAL DESIGN SPEC, BY AFRAY) AND TPI. THE GC COMMECTOR PA LARES ARE MODE OF 20/18/16/06 (H.H/SS/M) ASTA MASSI GRADE 40/56 (H. K/H.SS) GALL SIELE, APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION OF PROBABINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF TPIL-2002 SEC. 3. A SLAL ON THIS DRAINING INDICATES ACCEPTANCE OF PROFESSIONAL REGIONEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SHITABLLITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE **WARNING** IRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. RETER TO BCSI (BUILDING COMPONENT SAFETY IMPORATION), PUBLISHED BY TPI (TRUSS PLATE HISTITUTE, ZIB MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, ZZ314) AND NTCA (HOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRESE LANE, MADISON, NI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE BUILDING DESIGNER PER ANSI/TPI I SEC. 2. 386-454-7298//397-8585wk , 2X'4('A1) =Design Crit: R=332 U=64 W=3.5" MX -5-0-0 Over 3 Supports TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0) * Wind reactions based on MWFRS pressures. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, rocated within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 R=54 Rw=58 U=18 R=123 U=32 Apr CORION 9-8-7 8-0-0 BC DL TC LL SPACING DUR.FAC. BC LL TC DL TOT.LD. FL/-/4/-/-/R/-40.0 24.0" 1.25 10.0 PSF 10.0 PSF 20.0 PSF 0.0 PSF PSF DATE REF JREF -FROM SEQN-HC-ENG DRW HCUSR8228 08112009 Scale =.5"/Ft. not R8228- 32272 1TGV8228Z01 DAL/AP 04/21/08 71766

Top chord 2x4 SP Bot chord 2x4 SP 8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , #2 Dense #2 Dense J3)

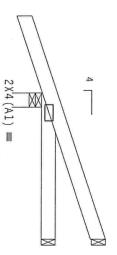
בחו פו (בפחפי ש פנות מינות מופחים ושפות וובם פו ומפסי ווומ.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures

Roof overhang supports 2.00 psf soffit load.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.



R=58 U=16

 $R=23 Rw=37 U=\overline{13}$ 8-0-0

1-7-0-R=265 U=58 W=3.5" 3-0-0 Over 3 Supports

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

.36.042

FL/-/4/-/-/R/-

Scale =.5"/Ft.

PLT TYP.

Wave

A PROPERLY ATTACHED RIGID CEILING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BGG, ING. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH IP: OR FARRICATING, HANDLIGG, SUPPIG, HISTALLING & BRACING OF TRUSSES.

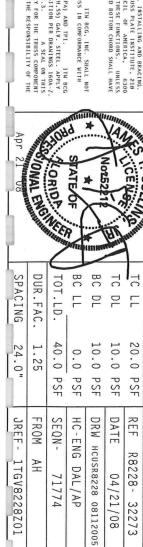
DESIGN COMPORES WITH APPLICABLE PROVISIONS OF DUDS (MATIONAL DESIGN SPEC, BY ATEN) AND TP!. ITH BGG COMMECTOR PALTES ARE MADE OF 207/18/196A (M. H/SSY) ASTM A653 GRADE 40/60 (M. K/M.SS) GALV. STEEL APPLY PLATES TO EACH FAGE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN. POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF FILL-2002 SEC.3. A SEAL ON THIS DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE DESIGN SHOWN.

Haines City, FL 33844 FL Certificate of Authorization # 0 278 ITW Building Components Group Inc.

BUILDING DESIGNER PER ANSI/TPI 1

ALPINE

Apr



8-020--OWNER BUILDER Mitchell Saad --386-454-7298//397-8585wk , * JI

Hop Bot chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense

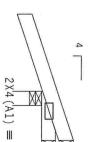
Roof overhang supports 2.00 psf soffit load.

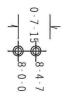
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.55 wind

Wind reactions based on MWFRS pressures

R=-59 Rw=31 U=39





R=-21 Rw=21 U=19

1-0-0 Over 3 Supports R-264 U-80 W-3.5"

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

PLT

TYP.

Wave

WARNING IRUSSES REDUIRE EXTREME CARE IN FARRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST (BUILDING COMPONENT SAFETY IMPORATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND MICA (MODO) TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PEFFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TO CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE

IMPORTANTSURMISM A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG. INC. SMALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMPONANCE WITH IP: OR FARRICATHG. MANDLING. INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF HOS (MATIONAL DESIGN SPEC, BY AERDA) AND TPI. THE BCG CONNECTOR PLATES ARE MADE OF ZOJBATICAG (M.H.SSPA) ASTH AGS BRADE 40/50 (M.K.M.SS) SALV. SIEE. APPLY PLATES TO GACH FACE OF TRUSS SAND, DURESS OTHERWISE COCKIED ON THIS DESIGN. POSITION PER DRAWINGS 160A-Z. APPLY PRACES TO GACH FACE OF TRUSS SAND, DURESS OTHERWISE COCKIED ON THIS DESIGN. POSITION PER DRAWINGS 160A-Z. PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERMISE LOCATED ON HILD ESSIGN, POSITION PER DRAWINGS 16DA-Z. ANY INSPECTION OF PLATES FOLLOHED BY (1) SHALL BE PER ANNEX AS OF 1P11-200Z SEC.3.

BRAWING INDICARES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

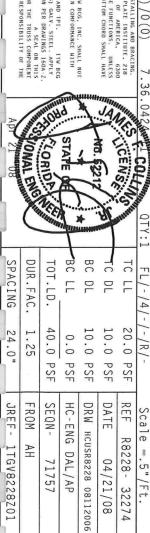
FL Certificate of Authorization # 0 278

Haines City, FL 33844

ITW Building Components Group Inc.

ALPINE

BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



PLT TYP. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Roof overhang supports 2.00 psf soffit load. Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense FL Certificate of Authorization # 0 278 8-020--OWNER BUILDER Mitchell Saad --TW Building Components Group Inc. Haines City, FL 33844 ALPINE Wave **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BEG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, ANY FALLURE TO BUILD THE TRUSS IN COMPORNANCE WITH TPI: OR FARRICATING, HANGLING, SHEPPING, HISTALLING A BRACING OF TRUSSES, DESIGN CONTROLATION, AND TRI. THE BEG. THE STORE ARE THE OPENING THE STORE ARE STORED BY (1) SHALL BE FER ANNEX AS OF FRITZONG SEC. 3. SEAL ON THIS BESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANNEX AS OF FRITZONG SEC. 3. SEAL ON THIS BRAING THOLORIES ACCOMPONENT FOR THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE BESPONSIBILITY OF THE **WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING. SHIPPING, INSTALLING AND BRACING. RETER TO BEST (BUILDING COMPONENT SAFETY IMPORATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 2718 MORTH LEE STREET, SUITE 317, ALEXANDRIA, VA, 22314) AND MICA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CORDS SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTON CHORD SHALL HAVE 386-454-7298//397-8585wk . 2X'4('A1) =Design Crit: R-407 U-79 W-3.5" TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) -7-0-0 Over 3 Supports * EJ7 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 Wind reactions based on MWFRS pressures CLORID R=182 U=46 R=82 Rw=82 U=26 BC LL BC DL DUR.FAC. TC DL TC LL SPACING TOT.LD. FL/-/4/-/-/R/-⊕10-4-7 8-0-0 40.0 24.0" 1.25 10.0 PSF 20.0 PSF 10.0 PSF 0.0 PSF PSF FROM DATE REF JREF -SEQN-HC-ENG DAL/AP DRW HCUSR8228 08112008 Scale =.5"/Ft. R8228-1TGV8228Z01 04/21/08 71760 32275

PLT Bot Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Hipjack supports 7-0-0 setback jacks with no webs Haines City, FL 33844 FL Certificate of Authorization # 0 278 8-020--UWNER BUILDER MITCHEII Saad -- 386-454-7298/7397-8585wk , ** - HJ7) ITW Building Components Group Inc. chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 TYP. ALPINE Wave **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN; AVE FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH TP: OR FARRICATHO. HANDLING, SHEPPING, INSTALLING A BRACTING OF TRUSSES, DESIGN CONFORMS, HITH APPLICABLE PROVISIONS OF DNDS. (MATIONAL DESIGN SECE, BY AREA), AND TPI. BCG CONNECTOR PLATES ARE MADE OF ZO/18/16/6A (M.H/SS/K) ASIM A653 GRADE 40/60 (M. K/M.SS) GALV. SIEEL. APPLICABLE PROVISIONS OF THIS DESIGN SECE, BY ASIM ASS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A OF TPI1-ZOOZ SEC.3. A SEAL ON THIS DESIGN SECO. BY A SEAL ON THIS DESIGN SECO. BY THE TRUSS CORPORENT DESIGN SHOWN. THE SUITABLILITY AND USE OF THIS COMPONENT FOR MAY BUILDING IS THE RESPONSIBILITY OF THE **WARNING** RUSSES REQUIRE EXTREME CARE IN FARRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY IMPORNATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND HTCA (MODO TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PEFFORNING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TO PUBDO SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE $2X4(A1) \equiv$ R=565 U=170 W=4.95" 2.83 Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0) -9-10-13 Over 3 Supports 1.5X4 Ⅲ \Box Wind reactions based on MWFRS pressures 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, BC DL=5.0 psf, Iw=1.00 GCpi (+/-)=0.55 STATE O 4 X 4 ≡ R=353 U=24 R=240 U=111 BC LL TC DL BC DL TC LL SPACING DUR.FAC. TOT.LD. FL/-/4/-, 2-7-12 20.0 24.0" 1.25 40.0 10.0 PSF 10.0 PSF 0.0 PSF PSF PSF 8-0-0 JREF -FROM DATE SEQN-REF HC-ENG DRW HCUSR8228 08112011 Scale =.5"/Ft. wind R8228- 32276 1TGV8228Z01 DAL/AP 04/21/08 71770

VALLEYTRUSS DETAIL

CHORD CHORD 2X3(*) 2X4 SP 2X4 SP #2 OR SPF #1/#2 OR BETTER) OR 2X4 P #3 OR SP #2N OR SPF #1/#2 OR BETTER.

- 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: ASCE 7-02 OR ASCE 7-05 130 MPH. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, SBC 110 MPH, ASCE 7-93 110 MPH OR ASCE 7-98, WIND TC DL=5 PSF 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9'

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY INSTALLATION TRUSS

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON

*** NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF CHORD OF THE TRUSS THE TOP CHORD.

ENGINEERS'

SEALED DESIGN

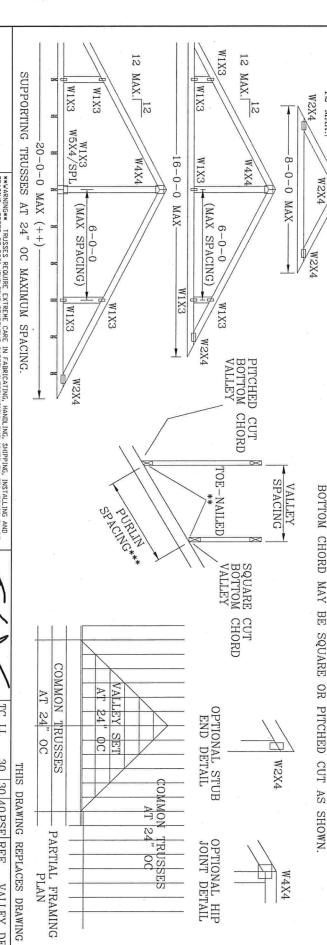
CUT FROM 2X6 OR LARGER AS REQ'D

4-0-0 MAX

12 MAX.

++ LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0".





THIS DRAWING REPLACES DRAWING A105

MIT BE RESEMBLES FOR ANY BEFAILUR FROM TO INSTALLATION CONTRACTOR. ITTY BG, ING.

NOT BE RESEMBLES FOR ANY BEVAILUR FROM THIS USEGN. ANY FALLURE ID BILLD THE TRUSS
CONFERNANCE WITH THIS DE FARRICATION FROM THIS USEGN. ANY FALLURE ID BILLD THE TRUSS
BESIGN CONTENENCE WITH APPLICABLE PROPUSIONS OF DUSC ANY BILLDAGE USEGN. ANY BASES AND ANY BASES OF THE WASHE AND ANY BOOK ON THE ABOUT ANY BEFORE ANY BOOK ON THE ABOUT ANY BEFORE ANY BOOK ON THE ABOUT ANY BOOK ON THE BUILDING BESIGNER, ANSI/TPI I SEC. 2. **VARRING** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BRSI (BULLING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI CTRUSS CLUATE INSTITUTE, 218 MORTH LEE ST., SUITE 312, ALEXANDRIA, VA. 22314) AND WTGA (VODD TRUSS COUNCI, BUSINITUTE, COUNCIL MORTH LEE ST., SUITE 312, ALEXANDRIA, VA. 22314) AND WTGA (VODD TRUSS COUNCI, WARRICA, 6300 ENTERPRISE LN, MADISON, WI 33719) FOR SAFETY PRACTICES PRIDE TO PEREDRING THESE FUNCTIONS. UNLESS THIREXISE INDICATED, TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANCE AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CELLING.

ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE

SONAL ENGINE ₩o. 5221% 11 Ju SPACING

DUR.FAC. 1.25/1.33 1.15 1.15	TOT. LD.	ВС	ВС	-	0.15
.FAC.			C 2	TC	TC
1.2	Þ	E	DL	DL	T
5/1.33	60	0	10	20	30
1.15	55	0	10	15	30
1.15	55 57 PSF	0 PSF	10 PSF	7 PSF DATE	30 40 PSF REF
-		-ENG	DRWG	DATE	REF
201		OPSF -ENG MLH/KAR	10 10 PSF DRWG VALTRUSS0207	2/23/07	VALLEY DETAIL
			ų		C

24

BUILDING DESIGN.

2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE
LATERAL STABILITY.

3. DO NOT CUT, NOTCH OR DRILL LP LVL.

4. SHM ALL BEARINGS FOR FULL CONTACT.

5. VERHY DIMENSIONS BEFORE CUTTING
LP LVL. TO SIZE.
LP LVL. TO SIZE. DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 168 (3-12") NAILS AT 12" CC, STAGGER ROWS. NAILS CAN BE CRIVEN FROM ONE FACE OR HALF FROM EACH FACE ON HALF FROM ONE FACE ON HALF FROM SHAWK DIAMETER OF 0.131". 148 SINKERS (2-14") MAY BE USED, BUT HALF MUST BE DRIVEN FROM 1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS, VERFIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT, LDISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONIENT INTO THE The design and material specified are in substantial conformity with the latest revisions of NDS and ATC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous. SUPPORT REACTIONS (LBS):
MAXIMUM B E A R I N G
1 2 6. THIS LP LVL IS TO BE USED AS A ROOF BEAM ONLY. CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES, ADDITIONAL FASTENERS MAY BE REQUIRED. AT EACH END OF COMPONENT.
PROVIDE LATERAL BRACING FOR THE BOTTOM
EDGE AT EACH END OF COMPONENT. MAKE PROVISION FOR ADEQUATE DRAINAGE.
PROVIDE LATERAL BRACING FOR THE TOP EDGE by and for residing idental forces shall be designed and complete structure. Obtain all the necessary code compliance approved and Common nalls driven penalted to give he shall be designed and complete structure before using the potent until after all the framing and fastening are pleted. At no time shall be specified by the design refer to the complete structure before using the potent with after all the framing and fastening are pleted to the component. If the design refer is additionable to the component. If the design refer is a design refer 2008.1 Allowable Stress Design MAXIMOM DEFLECTIONS CALCULATED ALLOWABLE AD 0.11" 0.31" AD 0.12" AD 0.18" 0.47" SIZES (IN-SX) 3-12 3-0 9483 2531 ALLOWABLE NUMBER lateral support is assumed (wail, floor beam, etc.), on-site inspection. This drawing must have an Arc affixed to be considered an Engineering document Miscellaneous Information MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT GRUSHING OF THE LP LVL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP LSL OR LP I-JOISTS IS STRICTLY PROMIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL ANCHORAGE FOR THIS BEAM AND THE MEMBERS ATTACHING TO IT. PROJECT DESIGNER TO PROVIDE ADEQUATE UPLIFT AND/OR LATERAL PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY. WARNING NOTES: ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS. DISTRIBUTION CONCENTRATED CONCENTRATED CONCENTRATED CONCENTRATED CONCENTRATED CONCENTRATED CONCENTRATED NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1), OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) CROSS SECTION SOURCE TYPE TOP/SIDE TOP LOAD TABLE LP LVL, LP LSL and CTR, LP I-Joist Specifications * A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR LP is a registered trademark of Louislana-Pacific Corporation. TI-IN-SX FI-IN-SX
10 PLF 00-00-00 19-00-06
13 IBS 07-02-00MINERG=2.50"
13 IBS 07-02-00MINERG=2.50"
22 IBS 09-01-00MINERG=2.50" 38 09-01-00MINERG=2.50"
38 11-01-00MINERG=2.50"
38 13-01-00MINERG=2.50"
38 13-01-00MINERG=2.50"
38 17-01-00MINERG=2.50"
38 17-01-00MINERG=2.50"
38 17-01-00MINERG=2.50"
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38 03-01-00MINERG=2.50"
38 03-01-00MINERG=2.50"
38 03-01-00MINERG=2.50" 9- 6- 3 *** THIS DRAWING IS NOT TO SCALE *** TO A 2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). 19-HON 2706 Highway 421 North
Wilmington, NC 28401
Local 910.762.8878
National Wats 800.999.9105 DWG SHEET LP Engineered Wood Products 9 6 WISCONSIN N.Y. CITY HUD DEFLECTION CRITERIA LIVE LOAD DEFL: TOTAL LOAD DEFL: DEAD TOTAL DESIGN CRITERIA : CODE COMPLIANCES ROOF LEFT SPAN CARR. ICC-ES 1A of 4 0804-092 City I LOAD LOAD 04/25/08 200124-W MER 97-94-E MR 1214D ESR-1254 . . . IBC WSI: 0.52 VSI: 0.54 RSI: 0.96 0.00 200 **`** HS4 HS4 FF

DOWN

737

MIN BEARING 3- 0

TOTAL LOAD

ndling & Erection

DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 16d (3-12") NAILS AT 12" OC. STAGGER ROWS.

NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DIAMETER OF 0.131". 16d SINKERS (3-14") MAY BE USED BUT HALF MUST BE DRIVEN FROM 1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS, VERTIFICATION OF LOADING, DETECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONIENT INTO THE 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE LATERAL STABILITY.
3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
6. VERIFY DIMENSIONS BEFORE CUTTING The design and material specified are in substantial conformity with the latest revisions of NDS and ATC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous. 6. THIS LP LVL IS TO BE USED AS A MONTHAN SUPPORT REACTIONS (LBS): andling & Erection DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED. MAKE PROVISION FOR ADEQUATE DRAINAGE.
- PROVIDE LATERAL BRACING FOR THE TOP EDGE
AT EACH END OF COMPONENT.
- PROVIDE LATERAL BRACING FOR THE BOTTOM
EDGE AT EACH END OF COMPONENT. sign Criteria MIN BEARING SIZES (IN-SX) 3-0 3-12 3-0 porary and permanent bracing for holding component the and for resisting lateral forces shall be designed and allied by others. No loads are to be applied to the ponent until after all the framing and fastening are pretent. At no time shall loads greater than design loads 2008.1 Allowable Stress Design BEARING 163 RATED LOADS MUST BE EQUALLY 147 MAXIMOM DEFLECTIONS
CALCULATED ALLOWABLE
AD -0.05" 0.31"
AD 0.00"
AD -0.05" 0.47" 113 2090 592 NUMBER The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code completence approval and Common nails driven parallel to glue lines shall be specifie applications. Instinctions from the designers of the complete structure before using this component. If the design criteria listed above does not meet local building component. If the design criteria listed above does not meet local building to the requirements, do not use this design. When this drawing is signed to the structural design is approved as shown in this drawing name and the structural design is approved as shown in this drawing name and the structural design is approved as shown in this drawing national design. The provided shall be structural design is approved as shown in this drawing national design. The provided shall be structural design in approved as shown in the drawing of the provided by the castomer, L.P.U., L.P.L. S. L. ACCIR., L.P. Losts a continuous lateral support is assumed (wait. floor beam, etc.). LP does not provide on-side texpection. This drawing nurst have an Architectra or Engineer's used.

**A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR and the structure of the provided of the support of the provided of the support of the provided of the support of the provided and the support is assumed (wait. floor beam, etc.). LP does not provide on-side texpection. This drawing nurst have an Architectra or Engineer's used.

**A COPY OF THIS DRAWING IS TO BE GIVEN TO THE INSTALLING CONTRACTOR. Miscellaneous Information PROJECT DESIGNER TO PROVIDE ADEQUATE UPLIFT AND/OR LATERAL PROVIDE ANCHORAGE FOR UPLIFT AT SUPPORTS. ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT DESIGNER. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LYL BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP LSL OR LP L-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL ANCHORAGE FOR THIS BEAM AND THE MEMBERS ATTACHING TO IT. PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY. ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS WARNING NOTES: CONCENTRATED CONCENTRATED CONCENTRATED CONCENTRATED DISTRIBUTION CONCENTRATED CONCENTRATED NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1), OTHER LOAD CASES FOR PATTERN LYBE LOAD GARE CHECKED AS REQUIRED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) CROSS SECTION SOURCE TYPE TOP/SIDE LOAD ROOF ROOF ROOF ROOF ROOF ROOF WEIGHT LIVE LIVE LIVE LIVE LIVE LIVE TOP TOP TOP LOAD TABLE LP LVL, LP LSL and CTR, LP I-Joist Specifications LP is a registered trademark of Louisians-Pacific Corporation ND FROM TO TOAD

PT-IN-SX PT-IN-SX

10 PLF 00-00-00 19-00-06

44 LBS 07-02-00MINERGE-2.50"

88 LBS 09-01-00MINERGE-2.50"

87 LBS 11-01-00MINERGE-2.50"

87 LBS 11-01-00MINERGE-2.50"

86 LBS 13-01-00MINERGE-2.50"

86 LBS 13-01-00MINERGE-2.50"

86 LBS 13-01-00MINERGE-2.50"

88 LBS 13-01-00MINERGE-2.50" 9-6-3-*** THIS DRAWING IS NOT TO SCALE *** TOAD TOAD 2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - FLIES FASTENED TOGETHER (REFER TO NOTES). -19-0-6-E 0 DWG 2706 Highway 421 North Wilmington, NC 28401 SHEET National Wats LP Engineered Wood Products 9- 6- 3 910.762.9878 800.999.9105 LIVE LOAD DEAD LOAD TOTAL LOAD WISCONSIN DESIGN CRITERIA: CODE COMPLIANCES : DEFIECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL: ROOF LEFT SPAN CARR. ICC-ES 0804-092 ATTO 04/25/08 ESR-1254 RR 25167 200124-W MEA 97-94-E MER 1214D IBC : 0.00 FT rr // VSI: 360 2008.1 ESE ESE ESE

BUILDING DESIGN.

2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE
2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE
3. DO NOT CUT, NOTCH OR DRILL LP LYL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
5. VERIFY DIMENSIONS BEFORE CUTTING
1. LYL LYL TO SIZE.
8. THIS LP LYL IS TO BE USED AS A
ROOF BEAM ONLY. DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LVL, SUCH THAT LOAD IS DISTRIBUTED BEOLALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 164 (3-127) NAILS AT 12" OC, STAGGER ROWS.

NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE OR HALF FROM EACH FACE OR HALF ROW SALLS WITH A MINIMUM SHANK DIAMETER OF 0.131", TAG SINKERS (3-147) MAY BE USED, BUT HALF MUST BE DRIVEN FROM 1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS, VERFIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAY'S REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT; DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE Dead load deflection includes adjustm Total load deflection is instantaneous. The design and material specified are in substantial conformity with the latest revisions of NDS and AITC. * Dead load deflection includes adjustment factor for creep. *DEAD MAXIMUM B E A R I N G

1 2 Jesign Criteria THITTO DOWN CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED. MAKE PROVISION FOR ADEQUATE DRAINAGE.
PROVIDE LATERAL BRACING FOR THE TOP EDGE
AT EACH END OF COMPONENT.
PROVIDE LATERAL BRACING FOR THE BOTTOM
EDGE AT EACH END OF COMPONENT. npleted. At no time shall loads greater than design loads nporary and permanent bracing for holding component mb and for resisting lateral forces shall be designed and lateral by others. No loads are to be applied to the nponent until after all the framing and fastening are andling & Erection MIN BEARING SIZES (IN-SX) ppiled to the component 2008.1 Allowable Stress Design 790 MAXIMUM DEFILECTIONS
CALCULATED ALLOWABLE
AD 0.06" 0.42"
AD 0.09"
AD 0.12" 0.63" 2994 3003 NUMBER 818 The use of this component shall be specified by the designer of the complete structure. Obtain all the necessary code completes applications, complete structure. Obtain all the necessary code completes applications from the designer of the component. If the design of the component is the design of the component is the design of the component is the design of the component. If the design of the component is the design of the component is the design of the component is design of the component is design. When this drawing to the product and provided by the outcomer, LP LVL, LP LSI, and CITA, LP LSI, and LTA, LP LSI, and CITA, LP LSI, and Miscellaneous Information n. This drawing must have an Architect's or Engineer's sea sidered an Engineering document. DISTRIBUTION SOURCE TYPE TOP/SIDE LOAD CONCENTRATED CONCENTRATED CONCENTRATED CONCENTRATED CONCENTRATED CONCENTRATED CONCENTRATEI CONCENTRATEI CONCENTRATED CONCENTRATED CONCENTRATED CONCENTRATED CONCENTRATED CONCENTRATED PROJECT DESIGNER TO PROVIDE ADEQUATE UPLIFT AND/OR LATERAL ANCHORAGE FOR THIS BEAM AND THE MEMBERS ATTACHING TO IT. NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (1), OTHER LOAD CASES FOR PATTERN LIVE LOADING ARE CHECKED AS REQUIRED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CAVITLEVER.) 9.500 CROSS SECTION DEAD DEAD DEAD DEAD DEAD DEAD LOAD TABLE LP LVL, LP LSL and CTR, LP I-Joist Specifications LP is a registered trademark of Louisiana-Pacific Corporation FROM TO LO FT-IN-SX FT-IN-SX FLF 00-00-00 38-00-00 LBS 07-01-00MINERG=2.50" \$ 11-01-00HINERG=2.5 \$ 11-01-00HINERG=2.5 \$ 11-01-00HINERG=2.5 \$ 15-01-00HINERG=2.5 \$ 15-01-00HINERG=2.5 \$ 15-01-00HINERG=2.5 \$ 17-01-00HINERG=2.5 \$ 05-01-00HINERG=2.5 S 29-01-00MINBRG=2. S 31-01-00MINBRG=2. S 31-01-00MINBRG=2. S 29-01-00MINBRG=2. 21-01-00MINBRG=2 19-01-00MINBRG=2 23-01-00MINBRG=2 27-01-00MINERG=2 25-01-00MINERG=2 23-01-00MINERG=2. 25-01-00MINERG=2. 27-01-00MINERG=2. -12- 8- 0 -01-00MINBRG=2 *** THIS DRAWING IS NOT TO SCALE *** DAD 2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). -38- 0- 0 LDE THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRÓDUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP LSL OR LP I-JOISTS IS STRICTLY PROHIBITED. ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LYL
BEAM AS DESIGNED. IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER,
ARCHITECT OR DESIGNER TO VERIEY THAT THE SUPPORT STRUCTURE FOR THIS
BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY. WARNING NOTES: ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS 2706 Highway 421 North Wilmington, NC 28401 Local 910,762,9878 National Wats 800,988,9105 DWG SHEET LP Engineered Wood Products 12- 8- 0 DEAD WISCONSIN DEFLECTION CRITERIA LIVE LOAD DEFL: TOTAL LOAD DEFL: DESIGN CRITERIA : BOB F.A. CODE COMPLIANCES ROOF RIGHT SPAN CARR. ROOF LEFT SPAN ICC-ES 0804-092 3B of 4 City LOAD 04/25/08 11518-R 200124-W MEA 97-94-E MR 1214D ESR-1254 RR 25167 REPORT CARR. IBC MSI: 0.23 VST: 0.18 RSI: 0.33 HH 0.00 `` 360 2006.1 ESE Abd 111

1. THIS COMPONENT IS DESIGNED TO SUPPORT ONLY THE VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS, VERTICAL LOADS SHOWN AS DETERMINED BY OTHERS, VERTICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND AND SEISMIC BRACING, AND OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED IS THE RESPONSIBILITY OF THE PROJECT ENGINEER OR ARCHITECT; DISCLAIM ALL RESPONSIBILITY FOR ALL PLANS, SPECIFICATIONS OR OTHER DOCUMENTS THAT MAY BE USED TO INCORPORATE THIS COMPONENT INTO THE The design and material specified are in substantial conformity with the latest revisions of NDS and ATC. * Dead load deflection includes adjustment factor for creep. Total load deflection is instantaneous. 3. DO NOT CUT, NOTCH OR DRILL LP LVL.
4. SHIM ALL BEARINGS FOR FULL CONTACT.
6. VERIFY DIMENSIONS BEFORE CUTTING
LP LVL. TO SIZE.
6. THIS LP LVL IS TO BE USED AS A
ROOF BEAM ONLY. 2. PROVIDE RESTRAINT AT SUPPORTS TO ENSURE Anderson Truss / Saad / Beam-B MAXIMUM BEARING NAILS CAN BE DRIVEN FROM ONE FACE OR HALF FROM EACH FACE. NAILS MAY BE COMMON OR BOX NAILS WITH A MINIMUM SHANK DAMETER OF 0.131". 164 SINKERS (3-114") MAY BE USED, BUT HALF MUST BE DRIVEN FROM DESIGN ASSUMES COMPONENTS CARRIED ARE APPLIED TO TOP EDGE OF LP LYL, SUCH THAT LOAD IS DISTRIBUTED EQUALLY TO EACH PLY. ATTACH THE TWO PLIES WITH 2 ROWS OF 184 (3-1/2") NAILS AT 12" OC. STAGGER ROWS. LATTAN DOWN CONCENTRATED LOADS MUST BE EQUALLY DISTRIBUTED TO ALL PLIES. ADDITIONAL FASTENERS MAY BE REQUIRED. *Supports and permanent bracing for holding component in the use of this component shall be specified by the designer of the component as the processor code completes of component and the processor code completes of component and the processor code completes of component and as the processor code completes of component and a five nonexestable of the component of the component.

*Supports and connections for LP LVL, LP LSL and CTR, LP LSL TOTAL MAKE PROVISION FOR ADEQUATE DRAINAGE.
PROVIDE LATERAL BRACING FOR THE TOP EDGE
AT EACH END OF COMPONENT.
PROVIDE LATERAL BRACING FOR THE BOTTOM ndling & Erection EDGE AT EACH END OF COMPONENT. MIN BEARING EQA B 2008.1 Allowable Stress Design 126 MAXIMIM DEFLECTIONS
CALCULATED ALLOWABLE
AD -0.02" 0.42"
AD -0.01" 0.63" 73 3-8 133 451 8 -E (xs-NI) NUMBER 3 133 3-0 73 132 **UPLIFTS** Miscellaneous Information fixed to be considered an Engineering THIS COMPONENT DESIGN IS SPECIFICALLY FOR L-P ENGINEERED WOOD PRODUCTS. USE OF THIS DESIGN FOR ANYTHING OTHER THAN LP LVL OR LP LSL OR LP L-JOISTS IS STRICTLY PROPUBITED, ANY MODIFICATION OF THIS DOCUMENT REQUIRES REVIEW BY A DESIGN PROFESSIONAL ANCHORAGE FOR THIS BEAM AND THE MEMBERS ATTACHING TO IT. PROJECT DESIGNER TO PROVIDE ADEQUATE UPLIFT AND/OR LATERAL PROVIDE ANCHORAGE FOR UPLIFT AT SUPPORTS, ANCHORAGE DETAIL TO BE PROVIDED BY PROJECT DESIGNER. MINIMUM BEARING SIZES ARE SUFFICIENT TO PREVENT CRUSHING OF THE LP LYL BEAM AS DESIGNED, IT IS THE RESPONSIBILITY OF THE PROJECT ENGINEER, ARCHITECT OR DESIGNER TO VERIFY THAT THE SUPPORT STRUCTURE FOR THIS BEAM IS CAPABLE OF SUPPORTING THE REACTIONS. PROVIDE RESTRAINT AT CONCENTRATED LOAD TO ENSURE LATERAL STABILITY. ANCHOR LP LVL ROOF BEAM SECURELY TO BEARINGS OR HANGERS WARNING NOTES: CONCENTRATED CONCENTRATED DISTRIBUTION CONCENTRATED CONCENTRATED CONCENTRATED CONCENTRATED CONCENTRATED NOTE: LOADS SHOWN ARE FOR INPUT LOAD CASE (), OTHER LOAD CASES FOR PAITERN LIVE LOADING ARE CHECKED AS REQUIRED. (DIMENSIONS MEASURED FROM LEFT END OF SPAN OR CANTILEVER.) 9.500 CROSS SECTION SOURCE TYPE TOP/SIDE LOAD 20,17 3.500 LOAD TABLE LP LVL, LP LSL and CTR, LP I-Joist Specifications LP is a registered trademark of Louislana-Pacific Corporation FIF 00-00-00 38-00-00 LBS 01-01-00MINBRG=2. LBS 37-01-00MINBRG=2. 38 01-01-00MINBRG=2.50"
38 07-01-00MINBRG=2.50"
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38 33-01-00MINBRG=2.50"
38 33-01-00MINBRG=2.50" FROM -12- 8- 0 7 C Woodford Plywood, Inc. 1504 South Mock Road Albany, GA 31705 (800) 342-6400 *** THIS DRAWING IS NOT TO SCALE *** 2 BEAMS 1.75 X 9.500 LP LVL2650Fb-1.9E DESIGN CONSISTS OF 2 - PLIES FASTENED TOGETHER (REFER TO NOTES). -12- 8- 0 **-**F 0 DWG 2706 Highway 421 North Wilmington, NC 28401 SHEET Local 910.762.9878 National Wats 800.989.9105 LP Engineered Wood Products 12- 8- 0 ICC-ES L.A. City WISCONSIN N.Y. CITY DEFLECTION CRITERIA: LIVE LOAD DEFL: TOTAL LOAD DEFL: DEAD DESIGN CRITERIA : 250 CODE COMPLIANCES ROOF LEFT SPAN CARR. ROOF RIGHT SPAN CARR 0804-092 4B of 4 LOAD LOAD 04/25/08 11518-R 200124-W MEA 97-94-E MR 1214D ESR-1254 RR 25167 REPORT # CARR. 8 8 8 IBC WSI: 0.05 VSI: 0.03 RSI: 0.01 0.00 FT 20 2000.1 360 PSF

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1TGH8228Z0107132019

Truss Fabricator: Anderson Truss Company

Job Identification: 8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , **

Truss Count: 18

Model Code: Florida Building Code 2004 and 2006 Supplement

Truss Criteria: ANSI/TPI-2002(STD)/FBC

Engineering Software: Alpine Software, Version 7.36.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-02 -Partially Enclosed

Description

Notes:

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

3. As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: VALTRUSS-

	#	ker bescription	Drawing#	Date
	1	71153H7A	08098001	04/07/08
	2	71154H9A	08098002	04/07/08
	3	71155H11A	08098003	04/07/08
	4	71156H13A	08098004	04/07/08
	5	71157H15A	08098005	04/07/08
ij	6	71158H17A	08098006	04/07/08
3	7	71159V1	08098007	04/07/08
	8	71160V2	08098008	04/07/08
	9	71161V3	08098009	04/07/08
	10	71162V4	08098010	04/07/08
4	11	71163V5	08098011	04/07/08
	12	71164 V6	08098012	04/07/08
1	13	71165 V7	08098013	04/07/08
1	14	71166HJ7	08098005	04/07/08
	15	71167 J5	08098004	04/07/08
	16	71168J3	08098001	04/07/08
1	17	71169J1	08098002	04/07/08
	18	71170EJ7	08098003	04/07/08

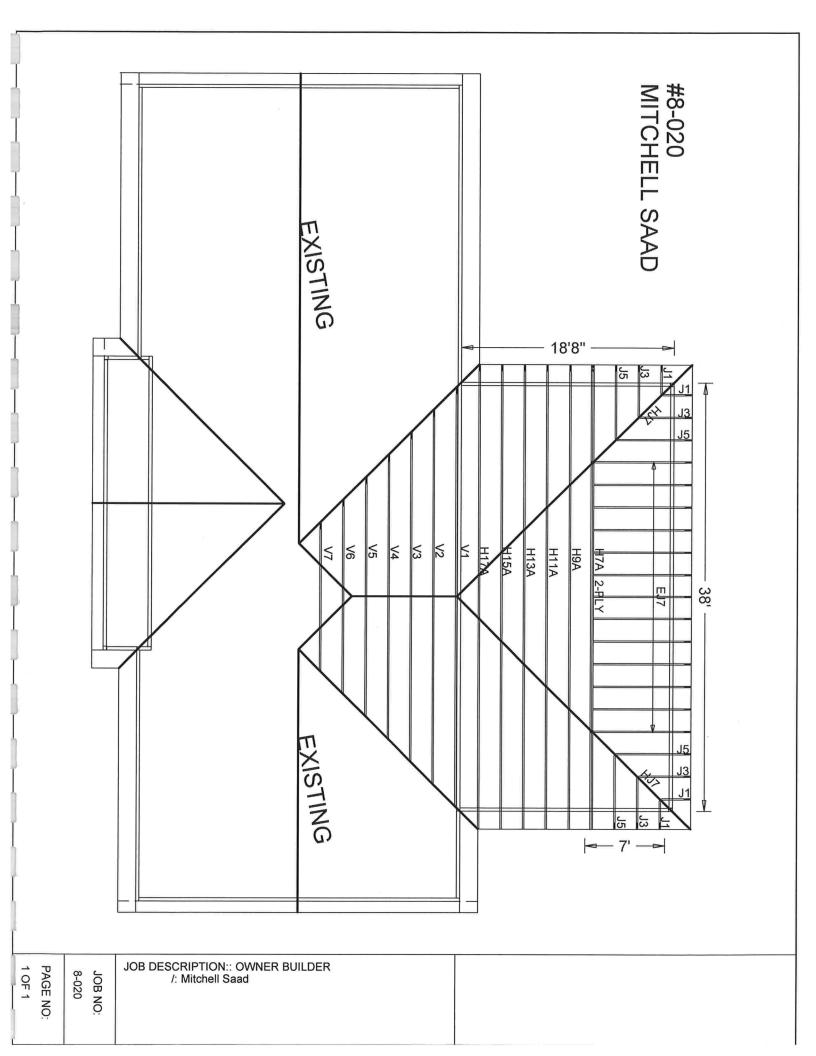
Drawing#

Date

Seal Date: 04/07/2008

-Truss Design Engineer-Doug Fleming Florida License Number: 66648 1950 Marley Drive Haines City, FL 33844





chord 2x4 SP #2 Dense :T2, T3 2x6 SP chord 2x6 SP #2 :B2 2x6 SP #1 Dense: Webs 2x4 SP #3

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures

In lieu of structural panels use purlins to brace all flat $24\mbox{"}$ OC.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\mathrm{.}$

COMPLETE TRUSSES REQUIRED

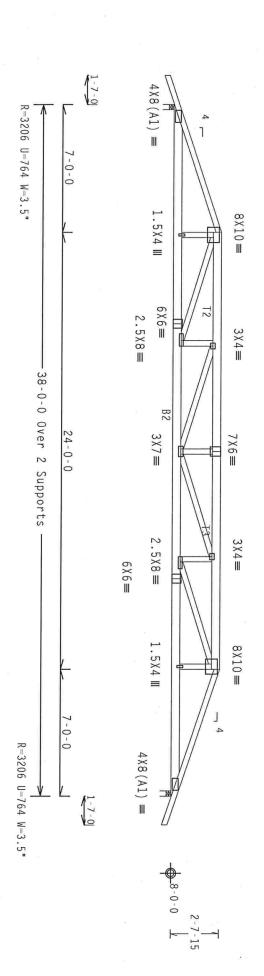
Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.

Use equal spacing between rows and stagger nails in each row to avoid splitting.

Roof overhang supports 2.00 psf soffit load

#1 hip supports 7-0-0 jacks with no webs

Calculated vertical deflection is 0.55" due to live load and 0.84" due to dead load at $\rm X=19\text{-}0\text{-}0$.



MARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BOSI (BUILDING COMPONENT SAFETY IMPORATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, ZIB HORTH LEE SIREET, SUITE 31Z, ALEXANDRIA, VA, ZEJIA) AND MICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ERITEPRISE LAME, MADISON, NI 5371B) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERNISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

OUGUAS FLE

CENS

TC LL

10.0 20.0

PSF PSF

DATE REF

04/07/08

10.0 PSF 0.0

DRW HCUSR8228 08098001

TCE/DF 83754

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

R8228- 71153

Design Crit:

PLT TYP.

Wave

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEFLATION FROM THIS DESIGN; ANY FAILURE TO BELLO THE TRUSS IN COMPORNANCE WITH TOT; OR FARRICATING, HANDLING, SHIPFING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONTORNS WITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC, BY AFAPA) AND TPI. ITH BCG CONNECTOR PLATES ARE MADE OF 20/18/166A (W.H/SS/K) ASTM A653 ORADE 40/60 (M.K/M.SS) GAV. STEEL, APPLY IS OF BIOS (MATIONAL DESIGN SPEC, BY AFRA) AND TPI. ITH REG. (M.H.YSS/K) ACKI. YEEL, APPY S OTHERWISE LOCATED ON THIS DESIGN, POSITION FER DRAMINGS 160A-Z. STALL BE FER MANKEX AS OF TPI1-2002 SEC.3. A SEAL ON THIS OBSIGN FOR THE TRUSS COMPONENT OF THIS COMPONENT FOR AMY BUILDING IS THE RESPONSIBILITY OF THE

DRAWING INDICATES ACCEPTANCE

Haines City, FL 33844
FI Cortificate of Authorization # 0.278

ITW Building Components Group Inc.

ALPINE

80 DUR.FAC. SPACING 24.0" 1.25 FROM JREF -1TGH8228Z01

BC LL BC DL TC DL

PSF PSF

TOT.LD.

40.0

SEQN-HC-ENG

PLT Bot Calculated vertical deflection is 0.42" due to 0.62" due to dead load at X=15-8-9. Roof overhang supports 2.00 psf soffit load In lieu of structural panels use purlins to brace all flat TC @ $24\mbox{''}$ OC. Haines City, FL 33844
FL Certificate of Authorization # 0 278 (8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , TW Building Components Group Inc. t chord 2x4 SP t chord 2x4 SP Webs 2x4 SP TYP. ALPINE 20 Gauge HS 1-7-0 3X8(A1) =#2 Dense #2 Dense #3 R=1644 U=388 W=3.5' , Wave ***IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR NAW DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH P1: OR FABRICATING, MANUFURG, SHEPPING, INSTALLING & BRACHEN OF TRUSSES, DESIGN COMPORES WITH APPLICABLE PROVISIONS OF DIDS (MATIONAL DESIGN SPEC, BY AFREA) AND TPI. ITH BCG CONNECTOR PLATES ARE MADE OF 20/183/166A (M.H.YSKEX) ASTH AGE (M.K.Y.) SAS, GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERNISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS BGOA-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE FER ANRY XA OF TPI1-2002 SEC 3. A SEAL ON THIS DRAWING HOLGATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT A PROPERLY ATTACHED RIGID CEILING 9-0-0 1.5 \ 4 \ Design Crit: 3 \ 4 ≡ €X8≡ live load and HS2512 = TPI-2002 (STD) /FBC Cq/RT=1.00(1.25) /0(0) 1.5X4 Ⅲ 38-0-0 Over 2 Supports 3 X 9 ≡ H9A) 20-0-0 3 X 6 ≡ 2.5X6≡ 3 X 4 ≡ 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Wind reactions based on MWFRS pressures COUSTAS FLEE HS2512 ≡ CENS No. 66648 3 X 4 ≡ 6X8≡ 80 9-0-0 BC DL TC DL TC LL DUR.FAC. SPACING TOT.LD. FL/-/4/-/-/R/-R=1644 U=388 W=3.5" 3X8(A1) = 40.0 24.0" 1.25 10.0 PSF 10.0 PSF 20.0 0.0 1-7-0 PSF PSF PSF יוות ביים מו יוונים מו יוונים וווות. DATE REF JREF -FROM SEQN-HC-ENG DRW HCUSR8228 08098002 Scale =.1875"/Ft. 8-0-0 R8228- 71154 1TGH8228Z01 TCE / DF 83759 04/07/08

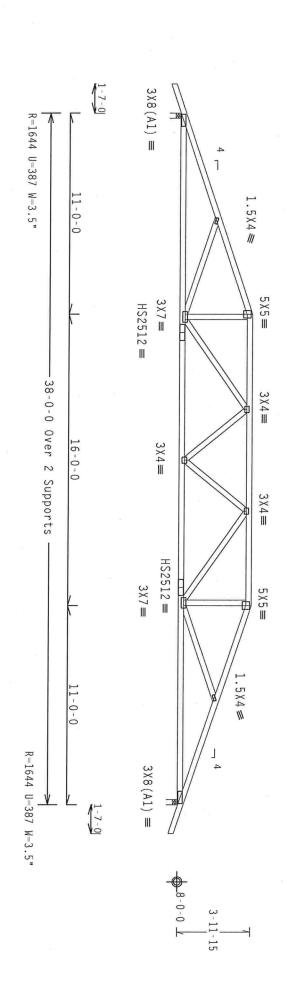
Wind reactions based on MWFRS pressures

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

In lieu of structural panels use purlins to brace all flat TC $24\mbox{\tt "}0\text{\tt C}.$

@

Roof overhang supports 2.00 psf soffit load



Haines City, FL 33844 FI Certificate of Amborization # 0 278

DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ITW Building Components Group Inc.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, MY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH TPI; OR FARRICATING, MADILING. SHEPPING, INSTALLING A BRACHING OF TRUSSES, DESIGN CONTROLATING, MADILING, SHEPPING, INSTALLING A BRACHING OF TRUSSES, DESIGN COMPORES WITH APPLICABLE PROVISIONS OF BIDS (MATIONAL DESIGN SPEC, BY ATERA) AND TPI. BCG CONNECTOR PLATES ARE MADE OF 20/18/16/66, (W.H.YSK), ASTM A653 GRADE 40/60 (W.K./M.SS) GALV. STEEL APPLY DATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWHOS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER WHERE AS OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWHOS HOLD AND THE TRUSS COMPONENT OF THE STATE OF THE TRUSS COMPONENT FOR MAY BUILDING IS THE RESPONSIBILITY OF THE

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BOSI. (BUILDING COMPONENT SAFETY IMFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND HICA (HONDE TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53729) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE

A PROPERLY ATTACHED RIGID CEILING.

ALPINE

PLT TYP.

20

Gauge HS, Wave

Design Crit:

TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

SOUICENSE

FL/-/4/-/-/R/-

Scale =.1875"/Ft.

R8228- 71155

20.0

PSF

DATE REF

04/07/08

No. 66648

BC DL

10.0 PSF 10.0 PSF

DRW HCUSR8228 08098003

TC DL TC LL

BC LL

0.0

PSF

HC-ENG

TCE/DF 83764

80

DUR.FAC.

FROM SEQN-

TOT.LD.

40.0

PSF

SPACING

24.0" 1.25

JREF -

1TGH8228Z01

Haines City, FL 33844
FL Cartificate of Anthorization # 0.279

ITW Building Components Group Inc.

ANY INSPECTION OF PLATES FOLLOWED BY (1)
DRAWING INDICATES ACCEPTANCE OF PROFESSI

ASTM A653 GRADE 40/60 (N. K/H.SS) GALV. STEEL. APPLY SE LOCATED ON THIS DESIGN. POSITION PER DRAWHEGS 160A-Z. PR ANNEX A3 OF TPIL-2002 SEC.3. A SEAL ON THIS IMEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

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DUR.FAC.

FROM SEQN-

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SPACING

24.0" 1.25

JREF -

1TGH8228Z01

BC LL

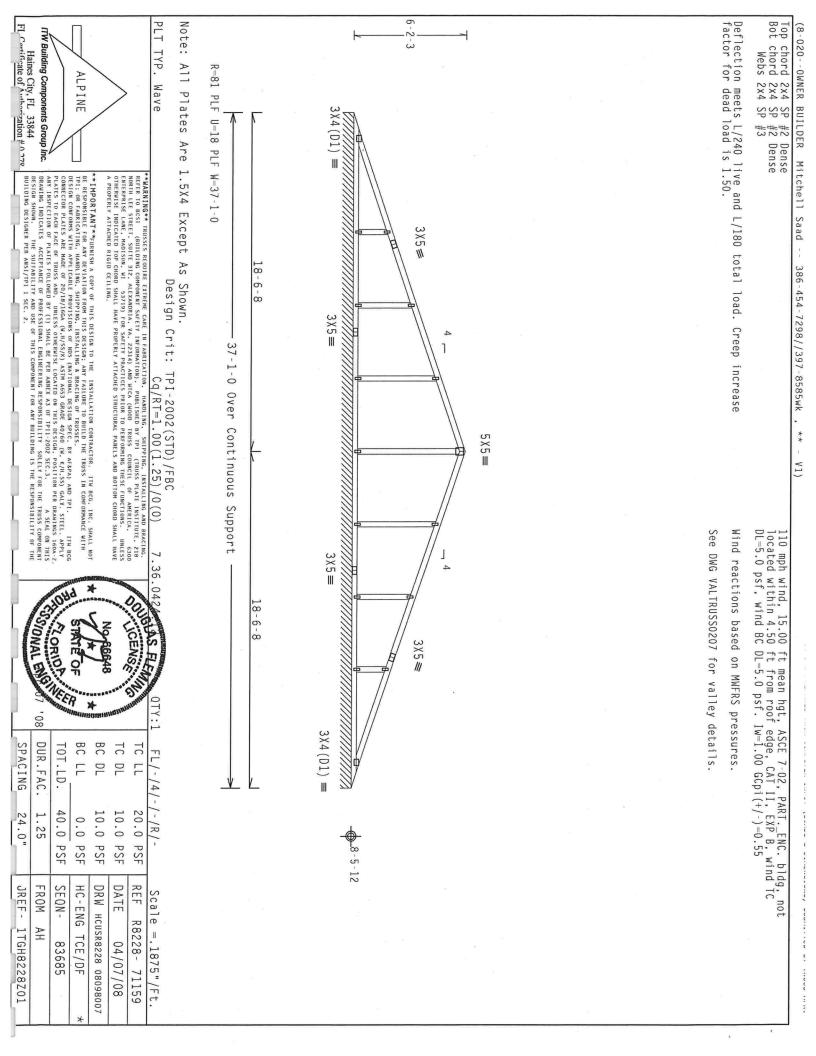
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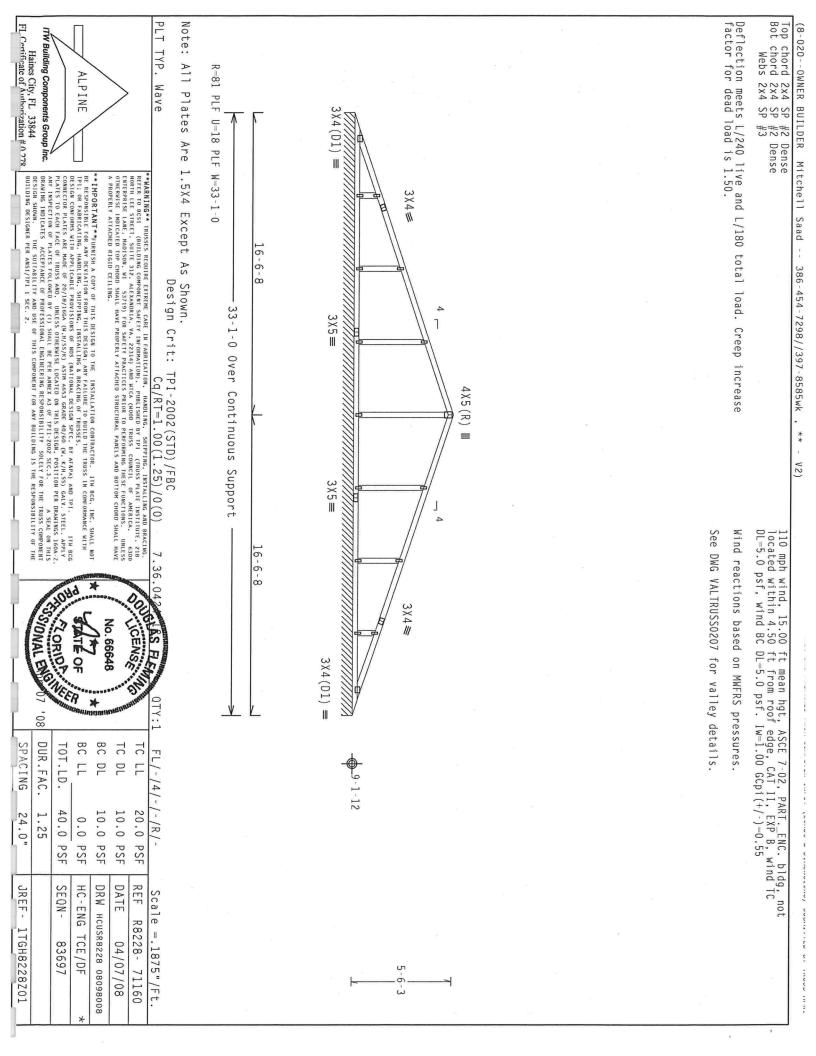
PSF

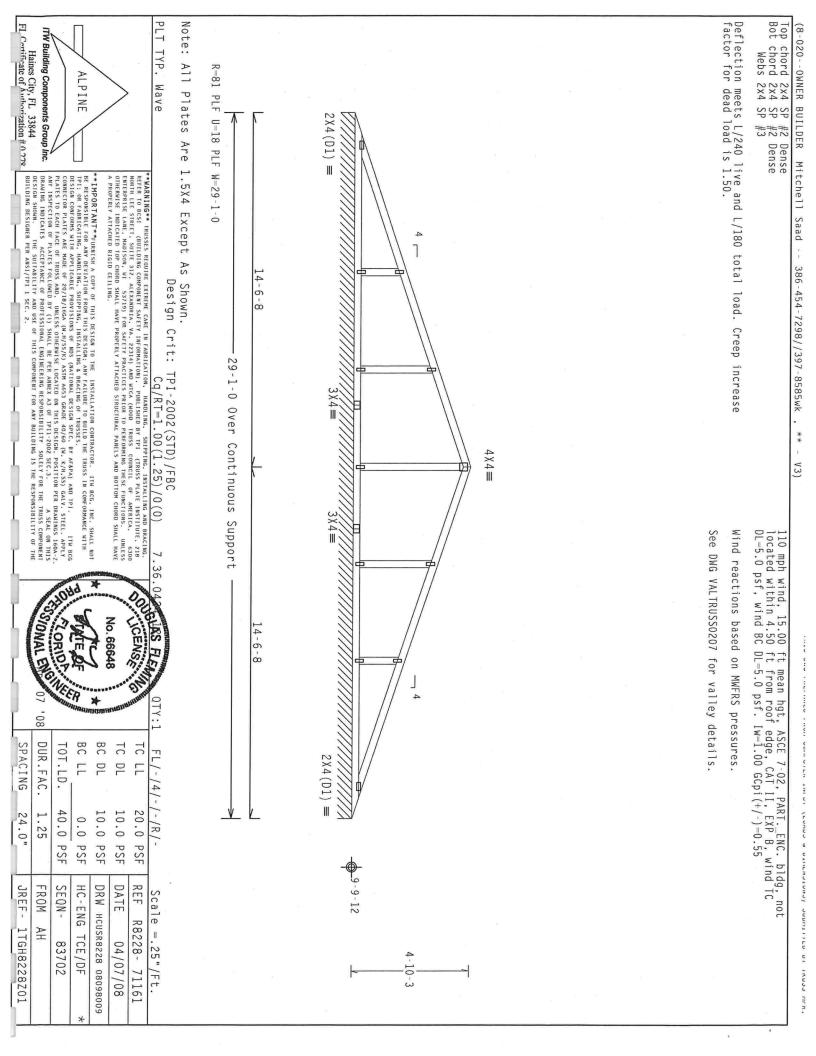
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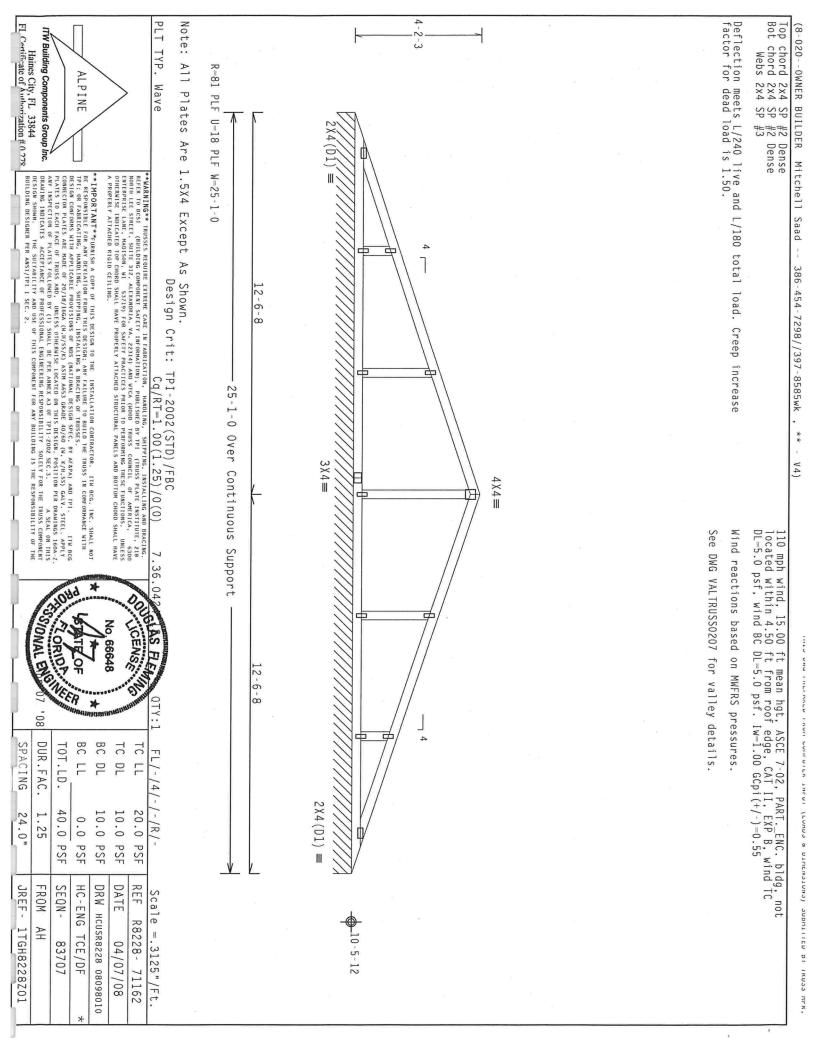
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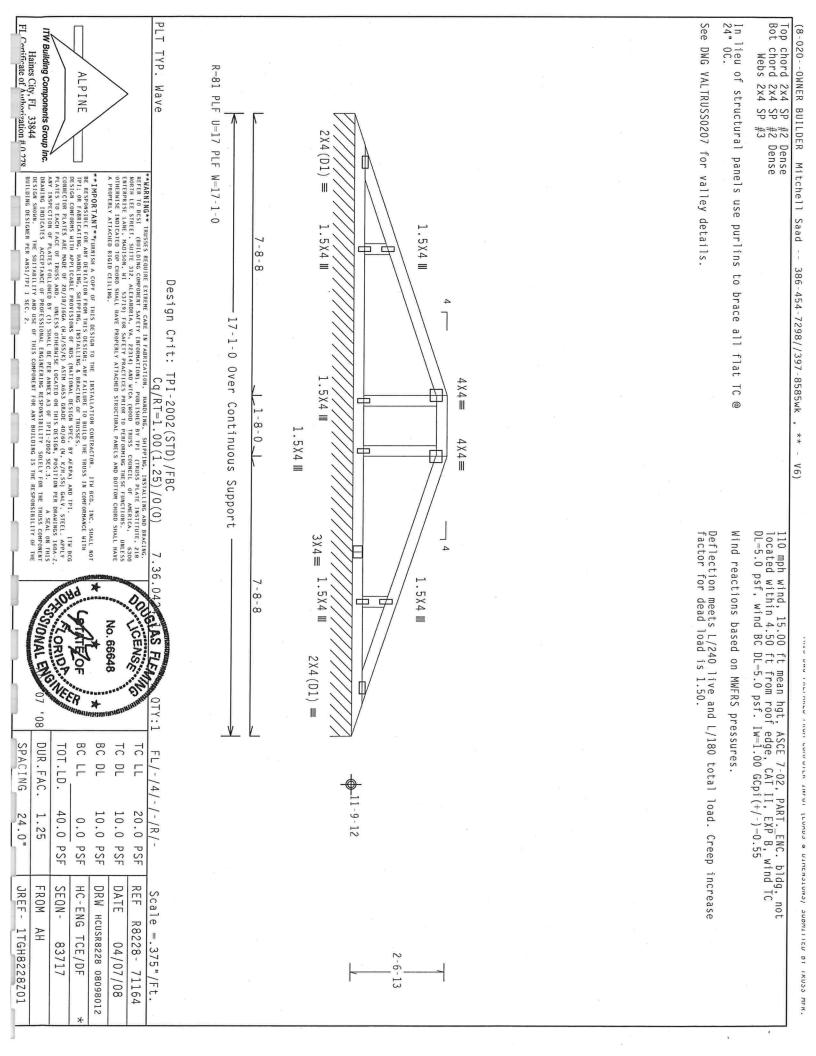
ALPINE











Hipjack supports 7-0-0 setback jacks with no webs Haines City, FL 33844 FL Certificate of Authorization # 0.278 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. TW Building Components Group Inc. 8-020--OWNER BUILDER Mitchell Saad chord 2x4 SP t chord 2x4 SP Webs 2x4 SP TYP. ALPINE Wave #2 Dense #2 Dense #3 -2-2-14-**IMPORTANT**GURHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG. INC. SHALL HOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FALLURE TO BUILD THE TRUSS IN COMFORMANCE WITH TPI; OR FAREICALTING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AFAFA) AND TPI.

CONNECTOR PLATES ARE MADE OF 20/18/10GA (H.H/SS/K) ASTM A653 GRADE 40/60 (H. K/H.SS) GAV. STEEL, APPLY **WARNING** TRUSSES REDUIRE EXTREME CARE IN FABRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING.
RETER TO BCS3 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218
MORTH LEE STREE, SUITE 312, ALEXANDRAN, VA, 22314) AND TICA, (MOOD TRUSS COUNCIL OF AMERICA, 6300
ERREPARISE LAME, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERWISE HUDICAGED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGHD CEILING. DESIGN SHOWN. THE SUITABILITY AND USE OF THIS BUILDING DESIGNER PER ANSI/TPI I SEC. 2. DRAWING INDICATES $2X4(A1) \equiv$ R=565 U=170 W=4.95" 386-454-7298//397-8585wk . Design Crit: .83 (M.H/SS/K) ASIM ABDS GRANE WITHOUT THE PRESSION FREE BRANTINGS 160A-Z.
IS OTHERMISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z.
IS STALL BE PER ANKEX AS OF TPIL-2002 SEC.3.
IS SIGNAL ERSINEERING RESPONSIBILITY SOFTED THE TRUSS COMPONENT
FOR THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) -9-10-13 Over 3 Supports ** 1.5X4 Ⅲ HJ7) 3 \ 4 ≤ Wind reactions based on MWFRS pressures. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_{\rm W}=1.00$ GCpi(+/-)=0.55 OCUCENSE STONAL BAGGINE 0.66648 4 X 4 == 80' R=240 U=111 R=353 U=24 BC DL TC DL SPACING DUR.FAC. TC TOT.LD. FL/-/4/-/-/R/-40.0 20.0 24.0" 1.25 10.0 PSF 10.0 PSF 0.0 ה א הדיירשיזימנין יחחננדוודה מו וצחים נונצי PSF PSF PSF 8-0-0 DATE REF JREF -FROM SEQN-HC-ENG DAL/AP DRW HCUSR8228 08098005 Scale =.5"/Ft. R8228- 71166 1TGH8228Z01 71770 04/07/08

PLT Roof overhang supports 2.00 psf soffit load Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Haines City, FL 33844 FL Certificate of Authorization # 0 278 TW Building Components Group Inc. 8-020--OWNER BUILDER Mitchell Saad --TYP. ALPINE Wave **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, VEY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH FPI; OR FARRICATING, HANDLING, SHEPPIG, HISTALLING & BRACHES OF TRUSSES.

DESIGN COMPORES WITH APPLICABLE PROVISIONS OF DNDS (MATIONAL DESIGN SECG. W *ATRA) AND TPI. BCG CONNECTOR PLATES ARE MADE OF 20/18/160A (W H/SSK) ASKY ASKS AGADE 40/60 (W. K/M.SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWHOS 160A-Z. ANY INSPECTION OF FLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF FPI1-2002 SEC. 3. ASKAL ON THIS DESIGN SECONDAINT OF THE STATE OF THE STATE OF THE TRUSS COMPONENT FOR THE STATE OF THE TRUSS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE **WARNING** TRUSSES REDUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
RETER TO BOSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BRY TPI (TRUSS PLATE INSTITUTE, 218)
UNBTH LEE SIREE, SUITE 312, ALEXANDRA, VA, Z2314) AND HTCA (MODED TRUSS COUNCIL OF AMERICA, 6300
ERHERPLISE LAME, MADISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PEFFORM HG THESE FUNCTIONS. UNLESS
OTHERWISE HOLGANED TOR CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PARELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CEILING. BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. 386-454-7298//397-8585wk , 2X'4('A1) =Design Crit: R=332 U=64 W=3.5" 4 -5-0-0 Over 3 Supports TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0) * J5) Wind reactions based on MWFRS pressures 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART_ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 R=54 Rw=58 U=18 R=123 U=32 GOUGHAS FLES CENS 80 BC LL BC DL TC DL TC LL DUR.FAC SPACING TOT.LD. FL/-/4/-/-/R/-טיבה זוויטו (בעחעם ש שוחבושםועושם) סעפווווובע פו וועשם וודת. 40.0 24.0" 10.0 PSF 10.0 PSF 20.0 PSF 1.25 0.0 PSF PSF JREF -DATE REF FROM SEQN-HC-ENG DRW HCUSR8228 08098004 Scale = .5"/Ft. R8228- 71167 AH 1TGH8228Z01 DAL/AP 71766 04/07/08

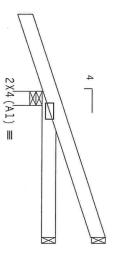
Top chord 2x4 SP Bot chord 2x4 SP #2 Dense #2 Dense

Roof overhang supports 2.00 psf soffit load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_{\rm W}=1.00$ GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures



R=58 U=16

 $R=23 Rw=37 U=\overline{13}$

R=265 U=58 W=3.5" 3-0-0 Over 3 Supports

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

TYP.

Wave

WARNING TRUSSES BEDUIRE EXTREME CARE IN FABRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST. (BUILDING COMPONENT SAFETY IMPORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 212 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND NICA (MOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LARE, MADISON, WI 53219) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING

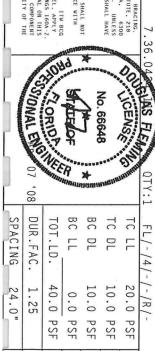
IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, VERY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH TPI: OR FARRECTHING, HANDLING, SHEPPING, INSTALLING A BRACTING OF TRUSSES, DESIGN COMPORES WITH APPLICABLE PROVISIONS OF THOS (MATIONAL DESIGN SPEC, BY AFREA) AND TPI. IT BCG CONNECTOR PLATES ARE MADE OF 20/18/19/66A (W.H/SYK), ASTH AGS JBRADE 40/60 (W. K./H.SY) GALY. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWHMS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER MEREA AS OF PPI1-2002 SEC. 3. SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL REGISHERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SULTABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

Haines City, FL 33844
FL Cortificate of Amborization #0 279

TW Building Components Group Inc.

ALPINE

BUILDING DESIGNER PER ANSI/TPI I SEC. 2.



JREF -FROM SEQN-HC-ENG

1TGH8228Z01

FL/-/4/-/-/R/-

Scale =.5"/Ft.

R8228- 71168

DATE REF

04/07/08

DRW HCUSR8228 08098001

DAL/AP

8-020--OWNER BUILDER Mitchell Saad --386-454-7298//397-8585wk . * J1)

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense

Roof overhang supports 2.00 psf soffit load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\mathrm{cm}$

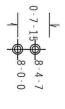
110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_W=1.00$ GCpi (+/-)=0.55

זייי טי (רבעורה מ הדוורויסדהווס) יהחנודוובה פו ועחפים וובעי

Wind reactions based on MWFRS pressures

R=-59 Rw=31 U=39

2X'4(A1) =



R=-21 Rw=21 U=19

1-0-0 Over 3 Supports R=264 U=80 W=3.5"

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

OTY:1

FL/-/4/-/-/R/-

Scale = .5"/Ft

R8228- 71169

10.0 PSF 20.0 PSF

DATE REF

04/07/08

10.0 PSF 0.0 PSF PSF

DRW HCUSR8228 08098002

DAL/AP 71757

PLT TYP.

Wave

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, BEFER TO BESS! (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312. ALEXANDRIA, VA, 22313) AND HICA (MODO TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS, UNLESS OTHERNISE INJECTION OF THE PROPERLY ATTACHED STRUCTURAL PAMELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGHD CELLING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. THE BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM HIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH FPI: OR FAREGAING, HANGLING, SHEPPING, INSTALLING A BRACHING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROPYISIONS OF HIS SEA, SHALLING A BRACHING SEG. A FAREA AND IPI. I'VE BCG CONNECTION PAIRS ARE MADE OF 20183 FAGEA (M.HISSYX) ASHY ASSO GRADE 40/50 (M. K/H.SS) GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWHINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX AS OF PILI-2002 SEC. 3. A SEA, ON THIS DRAWHING INDICATES ACCEPTANCE OF PROFESSIONAL REGIONER HIS RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT OR SIGNED SHOWN. THE SUITABLE TY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE SOLITOR FOR THE TRUSS CHAPPING AND SIGNED SHOWN. DESIGN SHOWN. THE SUITABILITY AND USE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

Haines City, FL 33844
FL Certificate of Authorization # 0 278

ITW Building Components Group Inc.

ALPINE

GOODS FLES SONAL ENGINE CENSE No. 66648 80 BC DL DUR.FAC. BC LL TC DL TC LL SPACING TOT.LD.

40.0

SEQN-HC-ENG

24.0" 1.25

JREF -FROM

1TGH8228Z01

PLT TYP. Roof overhang supports 2.00 psf soffit load Top chord 2x4 SP Bot chord 2x4 SP Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\mathrm{.}$ Haines City, FL 33844
FL Cortificate of Authorization # 0 278 ITW Building Components Group Inc. 8-020--OWNER BUILDER Mitchell Saad --ALPINE Wave #2 Dense #2 Dense **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, ING. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH FD: OR FABRICATHO. HANDLING, SHEPPING, INSTALLING AS BRACHE OF TRUSSES, DESIGN COMPORENT HITH APPLICABLE PROVISIONS OF BNDS (MATIONAL DESIGN SECE, BY AFREN) AND TPI. BCG CONNECTION PACES ARE MADE OF 20/18/166A (M.H/SS/K) ASTM A653 GRADE 40/50 (M.K/M.SS) GALV. STEEL APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DEMATHGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX 3.0 F FPI1-2002 SEC. 3. A SEAL ON THIS DESIGN SHOULD BE ASKED ON THIS DESIGN SHOULD BE SEAL ON THE SHOULD BE SEAL ON THIS DESIGN SHOULD BE SEAL ON THIS DESIGN SHOULD BE SEAL ON THE SHOULD BE SEAL DESIGN SHOWN. THE BUILDING DESIGNER PER A PROPERLY ATTACHED RIGID CEILING. 386-454-7298//397-8585wk . 2X'4('A1) =Design Crit: R = 407U=79 W=3.5" TPI-2002 (STD) /FBC Cq/RT=1.00(1.25) /0(0) 7-0-0 Over 3 Supports EJ7) Wind reactions based on MWFRS pressures. 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 7.36 GOUGNAS FLEA CENS R=82 Rw=82 U=26 R=182 U=46 80 BC DL TC DL TC LL SPACING DUR.FAC. TOT.LD. FL/-/4/-/-/R/-טיבא זחיטי (בעמטט פ טוובשפועחטן פטטוווובט טו ומטפט וודה. 40.0 10.0 PSF 20.0 PSF 24.0" 1.25 10.0 PSF 0.0 PSF PSF FROM JREF -DATE REF SEQN-HC-ENG DRW HCUSR8228 08098003 Scale = .5"/Ft. R8228- 71170 1TGH8228Z01 DAL/AP 71760 04/07/08

VALLEYTRUSS DETAIL

TOP CHORD CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 2X4 SP #3 OR BETTER. SPF #1/#2 OR BETTER

- 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: ASCE 7-02 OR ASCE 7-05 130 MPH. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF SBC 110 MPH, ASCE 7-93 110 MPH OR ASCE 7-98, (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9" OR CONTINUOUS LATERAL BRACING.

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY TRUSS

PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' INSTALLATION

SEALED DESIGN

ENGINEERS' SEALED DESIGN. BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED ON

*** ++ NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD.

CUT FROM 2X6 OR LARGER AS REQ'D

12 MAX.

W2X4

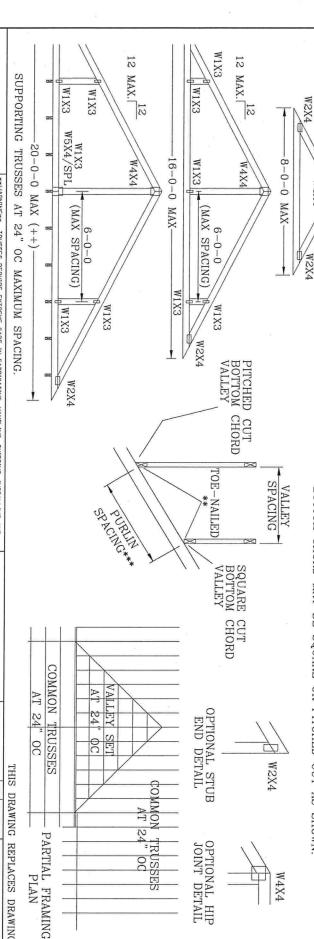
12

4-0-0

MAX

LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES NOT EXCEED 12'0"





THIS DRAWING REPLACES DRAWING A105

WALLER AND THE RESPONSIBILE FOR ANY DEVIATION FROM TIS DESIGN, MAY FAILURE IT BOILD TO BUILD THE FRISS IN CONFRENCIBLE. THE ANY DEVIATION FROM THIS DESIGN, MAY FAILURE IT BUILD THE FRISS IN DESIGNACING WITH THE DIR FARRICATING AND LING. SHIPPING, INSTALLINE IS BRACING OF TRUSSES.

DESIGN CONNECTING PLATES ARE MADE OF PROVISIONS OF ANIS ANALITIMAL DESIGN SEED. BY AREAS AND IT IN AREA CONNECTING PLATES ARE MADE OF PROVISIONS OF ANIS AND IN-LESS OFF AREAS CONNECTING PLATES TO FARRICATED IN THIS CON-POSITION FROM THE SEED OF ANIS TO THE SEED OF ANIS TO SHALL BE PER DESIGNACING PROVIDED BY TO SHALL BE PER CHINEREDIA OF ANIS TO ANIS TO THE STANDARD OF PROFESSIONAL OR SHALL BY THIS DESIGNACING PROVIDE THE SOURCE OF PROFESSIONAL OR SHALL BY THIS DEVIATIONAL DESIGNACING PROVIDED BY TO SHALL BE PER CHINEREDIA OR RESPONSIBILITY SULELY FOR THE TRUSS COMPONIANT DESIGN SHOWN. THE SULTABILITY ANIS THE PROPERTY OF THE BUILDING DESIGNACE, PER MANUSCA OF THE SULTABILITY AND THE BUILDING DESIGNACE, PER MANUSCA OF THE BUILDING DESIGNACE. **WARANING** TRUSSES REQUIRE EXTREME CARE IN FABRICATING, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BESS GUILDING COMPONENT SAFETY IN FORDAMITION, PUBLISHED BY TEI CIRUSS PLATE INSTITUTE, 218 NORTH LEE STR., SUITE 312, ALEXANIRIA, VA. 22314) AND WICK AVOIDD TRUSS COUNCIL, BACKICA, 6300 ENTERPRISE LN, MADISON, WI 53719) FOR SAFETY PRACTICES PRIDE TO PERFORMING THESE FUNCTIONS. UNIESS DITHEMISE INDICATED. TOP ORDER SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL ESSIONAL LL BE PER

ITW BUILDING COMPONENTS GROUP, INC POMPANO BEACH, FLORIDA

ALPINE

SIONAL ENGRAPE STATE OF 0.66648 7 SPACING

		1.15	1.15	5/1.33	DUR.FAC. 1.25/1.33 1.15 1.15
		55 57 PSF	55	60	TOT. LD.
MLH/KAR	-ENG	0 0 PSF -ENG	0	0	BC LL
10 10 PSF DRWG VALTRUSS0207	DRWG	10 PSF	10	10	BC DL
2/23/07	DATE	15 7 PSF DATE	15	20	TC DL
VALLEY DETAIL	KET	30 40 PSF KEF	ن	30	TC PF

Gulf Coast Supply & Mfg. Inc.

GULF-LOK PANEL STANDING SEAM

state of Florida Approved









The Gulf-Lok standing seam roofing system is the perfect choice in top-of-the-line roofing for residential and commercial applications and is our most popular, as well as our most cost-effective, standing seam roofing panel. Gulf-Lok features a 1" rib with slotted screw strip on the under-lap side for concealed fasteners, and comes in either Galvalume or any of over 20 colors of 24 gauge steel. Panels are available with either 12- or 16-inch coverage, with on-site manufacturing as the most popular option for delivery. For details and information about Gulf-Lok (including color availability), contact your Gulf Coast representative.

colors are representative of colors offered and are not intended for matching purposes.

Bronz

Regal

Matte

Patrici Bron

Sand

Evergre

Hartfe

Brand win

Sla

As Gr

Cha

Copp Pen

Medii Bron



GULF-LOK PANEL STANDING SEAM

Features:

Colors: 20+ Colors Available
Color Chart Available Upon

Request. Also Available in Mill Finished Galvalume.

Coverage: 12" & 16" Net Coverage

Gauge: 24 Gauge Steel

Substrate: AZ-50 Galvalume (Painted)

AZ-55 Galvalume (Mill

Finish)

Warranty: 40 years

Testing: UL580/UL1897 – Uplift Test

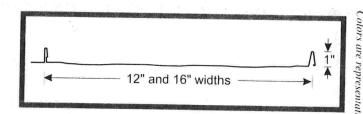
Florida State Approval

Minimum slope: 3:12

Substructure: 15/32" CDX (minimum)

Installation: Detail Manual /

Installation Guide – Available Upon Request







Reg Blu Color Rec Brit Rec Win

Copper Penny

ITW Building Components Group, Inc.

1950 Marley Drive Haines City, FL 33844
Florida Engineering Certificate of Authorization Number: 0 278
Florida Certificate of Product Approval # FL1999
Page 1 of 1 Document ID:1TE98228Z0521075904

Truss Fabricator: Anderson Truss Company

Job Identification: 8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , **

Truss Count: 18

Model Code: Florida Building Code 2004 and 2006 Supplement

Truss Criteria: ANSI/TPI-2002(STD)/FBC

Engineering Software: Alpine Software, Version 7.36.

Structural Engineer of Record: The identity of the structural EOR did not exist as of

Address: the seal date per section 61G15-31.003(5a) of the FAC

Minimum Design Loads: Roof - 40.0 PSF @ 1.25 Duration

Floor - N/A

Wind - 110 MPH ASCE 7-02 -Closed

Notes:

 Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1

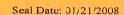
2. The drawing date shown on this index sheet must match the date shown on the individual truss component drawing.

As shown on attached drawings; the drawing number is preceded by: HCUSR8228

Details: VALTRUSS-

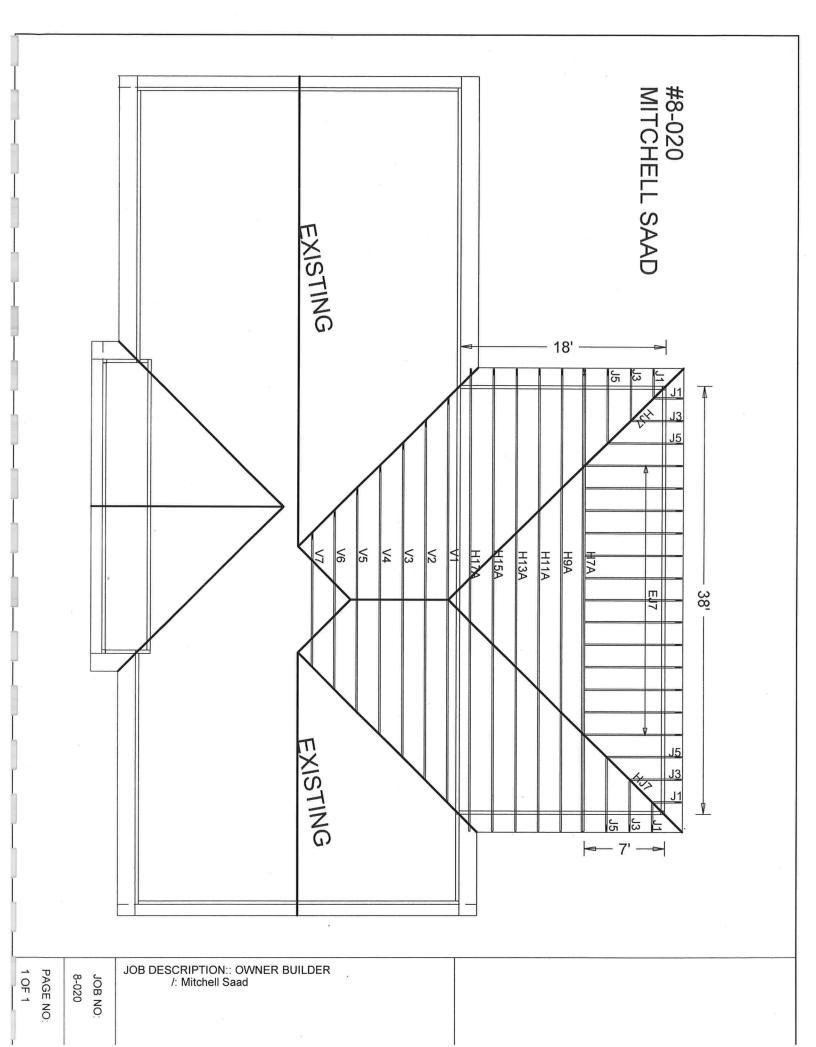
#	Ref Description	Drawing#	Date
1	26384 H7A	08021005	01/21/08
2	26385 H9A	08021006	01/21/08
3	26386H11A	08021007	01/21/08
4	26387H13A	08021008	01/21/08
5	26388H15A	08021009	01/21/08
6.	26389H17A	08021010	01/21/08
7	26390V1	08018044	01/18/08
8	26391V2	08018040	01/18/08
9	26392V3	08018041	01/18/08
10	26393 V4	08018042	01/18/08
11	26394V5	08018043	01/18/08
12	26395 V6	08021003	01/21/08
13	26396V7	08021004	01/21/08
14	26397 EJ7	08021011	01/21/08
15	26398 J5	08021012	01/21/08
16	26399HJ7	08021013	01/21/08
17	26400 J3	08021014	01/21/08
18	26401J1	08021015	01/21/08





-Truss Design Engineer-James F. Collins Jr. Florida License Number: 52212 1950 Marley Drive Haines City, FL 33844





110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, Iw=1.00 GCpi(+/-)=0.55 Top chord 2x4 SP #2 Dense :T2, T3 2x6 SP Bot chord 2x6 SP #2 :B2 2x6 SP #1 Dense: Webs 2x4 SP #3 #2:

Wind reactions based on MWFRS pressures

In lieu of structural panels use purlins to brace all flat $24\mbox{\ensuremath{^{\circ}}}\xspace$ oc. TC

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50.

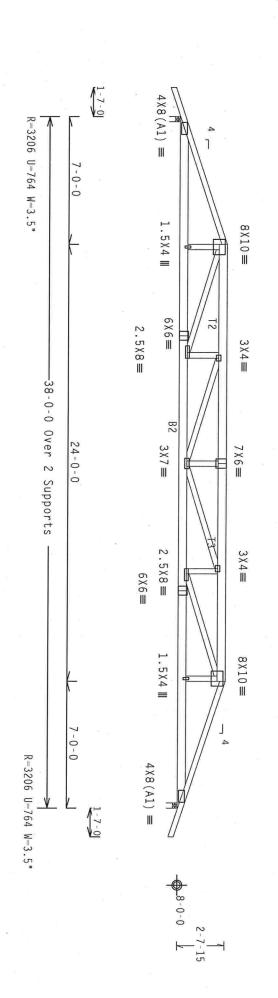
COMPLETE TRUSSES REQUIRED

Nailing Schedule: Top Chord: 1 Row (Bot Chord: 1 Row (Webs : 1 Row (Nailing Schedule: (12d_Common_(0.148"x3.25",_min.)_nails)
Top Chord: 1 Row @12.00" o.c.
Bot Chord: 1 Row @12.00" o.c.
Webs : 1 Row @ 4" o.c.
Use equal spacing between rows and stagger nails in each row to avoid spiitting.

Roof overhang supports 2.00 psf soffit load

#1 hip supports 7-0-0 jacks with no webs.

Calculated vertical deflection is 0.55" due to 0.84" due to dead load at $\rm X=19\text{-}0\text{-}0$. live load and



ITW Building Components Group, Inc. Haines City, FL 33844 FL Contificate of Authorization # 0 270

ALPINE

IMPORTANTGURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG. INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FALLURE TO BUILD THE TRUSS IN COMFORMANCE WITH TPI; OR FARELATING, MANDLING, SUMPTION, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NOS (MATIONAL DESIGN SPEC, BY AFAPA) AND TPI. ITW BCG CONNECTOR PLATES ARE MADE OF 20/18/16GA (M.H/SS/K) ASTH A653 GRADE 40/60 (M.K/H.SS) GALV. STEEL, APPLY

INDS (MATIONAL DESIGN SPEC, BY AFRA) AND PFI. ITH BEC SS/K) ASTH A653 GRADE 40/60 (M. K/H.SS) GALV. STEEL, APPLY ERHISE LOCATED ON THIS DESIGN, POSITION PER DRAHINGS 160A-L BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THI

SOLELY FOR THE TRUSS COMPONENT

THE RESPONSIBILITY OF

an

JANO/C CORIO

DUR.FAC.

1.25 40.0

FROM

SEQN-

71763

TOT.LD.

SPACING

24.0"

JRFF -

1TE98228Z05

BC LL BC DL TC DL

0.0 10.0 PSF

PSF PSF

HC-ENG DAL/AP

DRW HCUSR8228 08021005

TC LL

20.0 10.0 PSF

PSF

R8228- 26384

DATE REF

01/21/08

FL/-/4/-/-/R/-

Scale

=.1875"/Ft

WARNING TRUSSES REQUIRE EXTREME CARE IN FABRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BEST: (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREET, SUITE 312, ALEXANDRIA, VA., 22314) AND NTCA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LAME, MADISON, NT 33719) FOR SAFETY PRACTICES PRIOR TO PREFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE

Design Crit:

TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0)

PLT TYP.

Wave

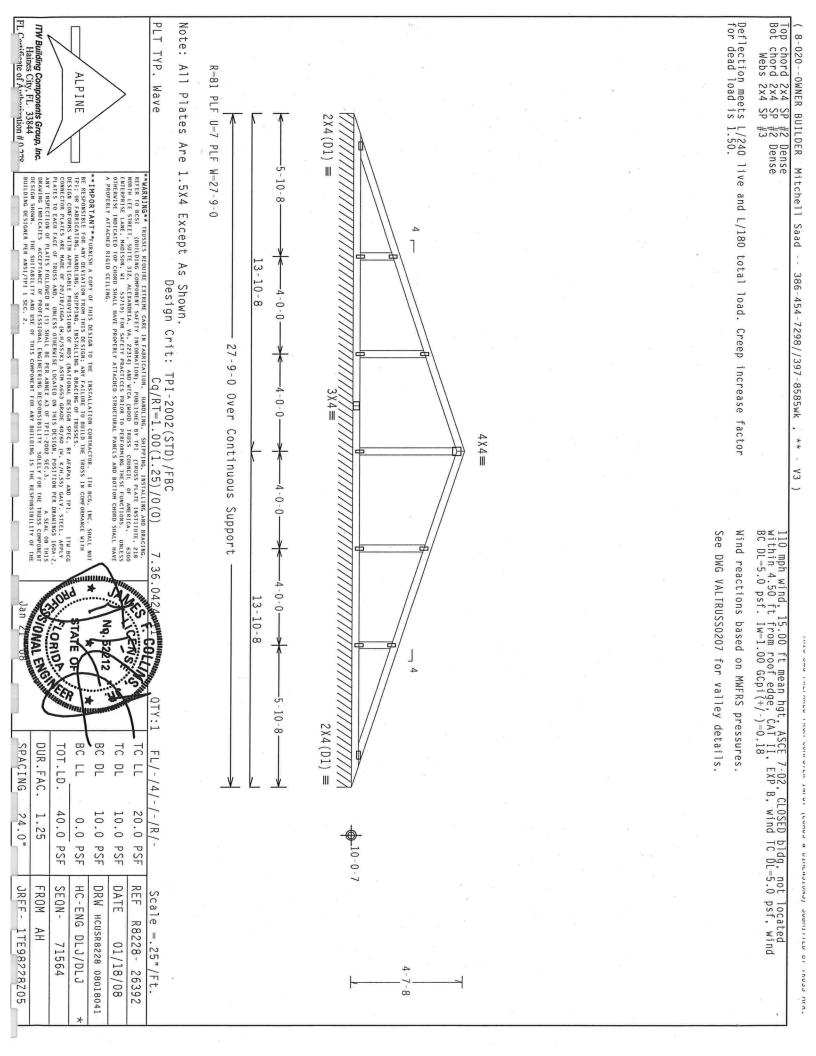
ITW Building Components Group, Inc. Haines City, FL 33844 FL Certificate of Authorization # 0.278 In lieu of structural panels use purlins to brace all flat TC $24\mbox{\tt "}0\text{\tt C}.$ Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 Roof overhang supports 2.00 psf soffit load PLT TYP. 8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , ALPINE 20 Gauge HS, Wave 1-7-0 3X8(A1) = R=1644 U=387 W=3.5' **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITM BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, VAY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH TPI: OR FARELOCATION, INAULING, SHEPPING, INSTALLING A BRACING OF TRUSSES, DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF ANDS (MATIONAL DESIGN SECE, AY AREAY) AND TPI. ITM BCG CONNECTOR PLATES ARE MADE OF 20/18/1666 (M.H/SS/K) ASTM A653 GRADE 40/160 (M.K/M.SS) GALV. SITEL APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERRISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX AS OF TPII: 2002 SEC. 3. ASAL ON THIS BRAHING INDICATES ACCEPTANCE OF ADDRESSIONAL TRUSHERERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SULTABLILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE **WARNING** TRUSSES REQUIRE EXTREME CARE IN FARRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCS1 (BUILDING COMPONENT SAFETY HEROMATICH), PUBLISHED BY TPT (TRUSS PLATE INSTITUTE, 218 MORTH LEE STREE, SUITE 312. ALEXANDRIA, VA, 22314) AND NTCA (MODO TRUSS COUNCIL OF AMERICA, 6300 ENTERPRESE LAME, MADISON, NI 55719) FOR SAFETY PRACTICES PRIOR TO PEFFORNING THESE FUNCTIONS. UNLESS OTHERHISE INDICATED TO ROBOD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE 1.5X4 ₩ 1 - 0 - 0Design Crit: 5 X 5 🗏 3X7= HS2512 ≡ TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0)38-0-0 Over 2 Supports 3 X 4 ≡ * H11A) 3 X 4 ≡ .6-0-0 3 X 4 ≡ Wind reactions based on MWFRS pressures 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART _ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. HS2512 ≡ 3 X 7 ≡ 5 X 5 = ATE O 11 - 0 - 0BC DL TC DL DUR.FAC. TC LL SPACING TOT.LD. FL/-/4/-/-/R/-R=1644 U=387 W=3.5" 3X8(A1) = 10.0 40.0 20.0 24.0" 10.0 PSF 1.25 0.0 1-7-0 PSF PSF PSF PSF DATE JREF -FROM SEQN-REF HC-ENG DRW HCUSR8228 08021007 Scale =.1875"/Ft 8-0-0_ R8228- 26386 1TE98228Z05 DAL/AP 71794 01/21/08

ITW Building Components Group, Inc. Haines City, FL 33844 FL Certificate of Authorization # 0.278 Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense Webs 2x4 SP #3 In lieu of structural panels use purlins to brace all flat TC $24\mbox{\ensuremath{^{\circ}}}\xspace$ 0C. Roof overhang supports 2.00 psf soffit load (8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , TYP. ALPINE 20 Gauge HS, $3X8(A1) \equiv$ R=1644 U=385 W=3.5" .Wave **IMPORTANT***URRHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH DCG. INC. SHALL NOT BE RESPONSIBLE FOR NAY DEVIATION FROM THIS DESIGN; NAY FALIURE TO BUILD THE TRUSS IN COMFORMANCE HITH IP: OR FABRICATIO. MANIPLIO. SHIPPING. INSTALLING & BRACHING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF HOS (MATHONAL DESIGN SPEC, BY AFADA) AND IPI. THE BOSION CONNECTOR PLATES RE MODE OF ZO/1891/GRA (M.H.SSY) ASTH MOS GRADE 40/50 (M.K.M.SS) GAU. SITEL, APPLY PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 150A-Z PLATES TO EACH FACE OF TRI ANY INSPECTION OF PLATES 1.5X4 .5-0-0 3 X 4 ≡ Design Crit: 5×5# HS2512 ≡ TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0) 5 X 5 = 3 X 7 ≡ 38-0-0 Over H15A) 8-0-0 3 X 4 ≡ 2 Supports Wind reactions based on MWFRS pressures 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.55 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 3 X 7 ≡ 5 X 5 ≡ HS2512 ≡ Jan CORIDE 5×5# 3×4≡ 15-0-0 1.5X4 / BC LL BC DL TC SPACING DUR.FAC. TOT.LD. וכ רר FL/-/4/-/-/R/-DL R-1644 U=385 W=3.5" 3X8(A1) =40.0 10.0 PSF 1.25 10.0 PSF 20.0 24.0" 0.0 1-7-0 PSF PSF PSF DATE REF JREF -FROM SEQN-HC-ENG DRW HCUSR8228 08021009 Scale = .1875"/Ft. R8228- 26388 1TE98228Z05 DAL/AP 71802 01/21/08

בווו טי (בטקטט מ מזויבוסזטווט) טטטווזוובט טו ומטטט וורה.

Top chord 2x4 SP / Bot chord 2x4 SP / Webs 2x4 SP / ITW Building Components Group, Inc. Haines City, FL 33844 FL Certificate of Authorization # 0.278 In lieu of structural panels use purlins to brace all flat TC @ $24\mbox{\,\sc m}$ OC. Roof overhang supports 2.00 psf soffit load (8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , TYP. ALPINE Wave 1-7-0 3X8(A1) = #2 Dense #2 Dense #3 R=1644 U=384 W=3.5" **IMPORTANT**FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR MAY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH IP: OR FAREIGATING, HANDLING. SHEPPING, INSTALLING & BRACING OF TRUSSES, AFFEND, AND TRIPLED OF THE STATE AFFENDE AND TRIPLED OF THE STATE AFFENDE AND THE STATE OF THE STATE AFFENDE AND THE STATE AND THE STATE AFFENDE AND THE STATE AND THE STATE AFFENDE AND THE STATE **WARNING** TRUSSES REQUIRE EXTREME CARE IN FARRICATION. HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BESS! GUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218
NORTH LEE STREET, SUITE 312. ALEXANDRIA, VA, 22314) AND NICA (MOOD TRUSS COUNCIL OF AMERICA, 6300
ENTERPRISE LANE, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PEFFORMING THESE FUNCTIONS. UNLESS
OTHERWISE HOLDCANED TOP CHORD SMALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SMALL HAVE
A PROPERLY ATTACHED RIGID CEILING. DESIGN SHOWN. THE SUITABILITY BUILDING DESIGNER PER ANSI/TPI 1 CONNECTOR PLATES ARE MADE OF 20/18/16GA (W.H/SS/K)
PLATES TO EACH FACE OF TRUSS AND. UNLESS OTHERWIS
ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE DRAWING INDICATES 1.5X4 // 17-0-0 Design Crit: 3X4≡ 3X4# 4 X 5 ≡ TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0)38-0-0 Over 2 Supports * 5 X 5 3 X 7 ≡ D2 SEC.3. A SEAL ON THIS
SOLELY FOR THE TRUSS COMPONENT IS THE RESPONSIBILITY OF H17A) 4-0-0 5 X 8 ≡ 3 X 4 ≡ Wind reactions based on MWFRS pressures 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi (+/-)=0.55 Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 4 X 5 ≡ 3×4≢ an 3X5₩ 3 X 4 ≡ 17-0-0 .5X4 / BC LL BC DL TC DL TC LL DUR.FAC. SPACING TOT.LD. FL/-/4/-/-/R/-יבה זוויטי (במעמי מ מזוורוויזוטווט) יומטווזווובט מו ועמיים וווער. R=1644 U=384 W=3.5" 3X8(A1) = 20.0 40.0 1.25 10.0 PSF 10.0 PSF 24.0" 0.0 1-7-0 PSF PSF PSF DATE FROM SEQN-HC-ENG REF JRFF-DRW HCUSR8228 08021010 Scale =.1875"/Ft. R8228- 26389 8-0-0 1TE98228Z05 DAL/AP 01/21/08 71805



ITW Building Components Group, Inc. Haines City, FL 33844 FL Conference of Authorization # 0 270 PLT TYP. Note: All Plates Top chord 2x4 SP Bot chord 2x4 SP Webs 2x4 SP Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 8-020--OWNER BUILDER Mitchell Saad --R-81 PLF U-7 PLF W-23-9-0 ALPINE Wave #2 Dense #2 Dense #3 Are 1.5X4 Except As Shown. 3 - 10 - 8***IMPORTANT*** THEN SEN A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG. INC. SHALL NOT BE RESONSIBLE FOR ANY DEPLATION FROM THIS DESIGN; AMY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH THIS DESIGN; AMY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH THIS DESIGN. DEPLATED THE RESONSIBLE FROM THE SENTIAL THE BCG. BEACHER OF TRUSSES. AND THIS THE BCG. CONNECTOR PLATES ARE NOTE OF EQUIPMENT OF MUS (ANATIONAL DESIGN SPEC, BY AFRA) AND THIS THE CONNECTOR PLATES ARE NOTE OF EQUIPMENT OF MUS (ANATIONAL DESIGN SPEC, BY AFRA) AND THIS THE CONNECTOR PLATES ARE NOTE OF EQUIPMENT OF MUS (ANATIONAL DESIGN SPEC, BY AFRA) AND THIS DESIGN CONNECTOR PLATES ARE NOTE OF EQUIPMENT OF THE SENTIAL THE BCG. A PROPERLY ATTACHED RIGID CEILING DRAWING INDICATES 386-454-7298//397-8585wk , 11-10-8 4-0-0-Design Crit: 23-9-0 Over Continuous Support TPI-2002(STD)/FBC-Cq/RT=1.00(1.25)/0(0) -4-0-0- $4 \times 4 \equiv$ 5 X 4 = ۷4) AMERICA. 6300 UNCTIONS. UNLESS M CHORD SHALL HAVE 110 mph wind, 15.00 ft mean hgt, ASCE / within 4.50 ft from roof edge, CAT II, BC DL=5.0 psf. Iw=1.00 GCpi(\pm /-)=0.18 See DWG VALTRUSS0207 for valley details. Wind reactions based on MWFRS pressures. 1-10-8 ATE OI -4-0-0-QTY:1BC LL BC DL TC DL TC LL DUR.FAC. TOT.LD. FL/-/4/-/-/R/--3-10-8 7-02, CLOSED bldg, not located EXP B, wind TC DL=5.0 psf, wind 40.0 20.0 1.25 10.0 PSF 10.0 PSF 0.0 PSF PSF PSF FROM DATE REF HC-ENG SEQN-DRW HCUSR8228 08018042 Scale R8228- 26393 =.3125"/Ft. DLJ/DLJ 71569 01/18/08

I AUTI CONTROLLA TREAT (COROS & OTHERSTORS) SOBRITHED BY IROSS PIER.

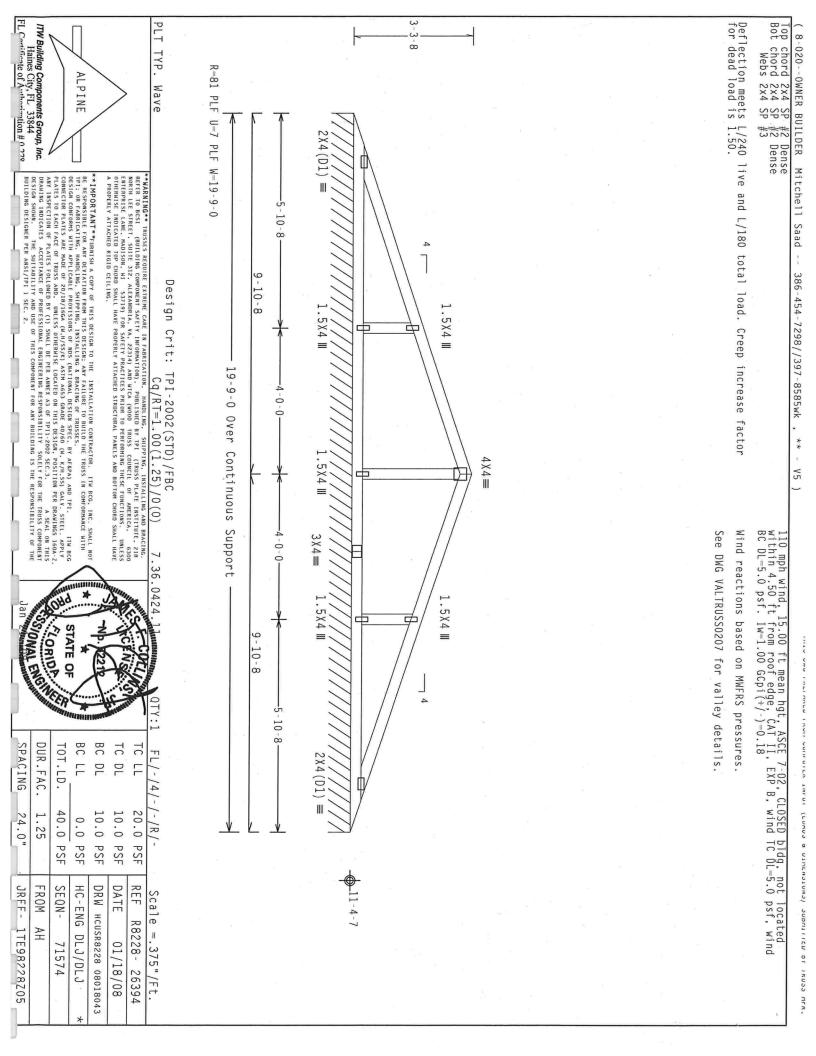
BUILDING DESIGNER PER

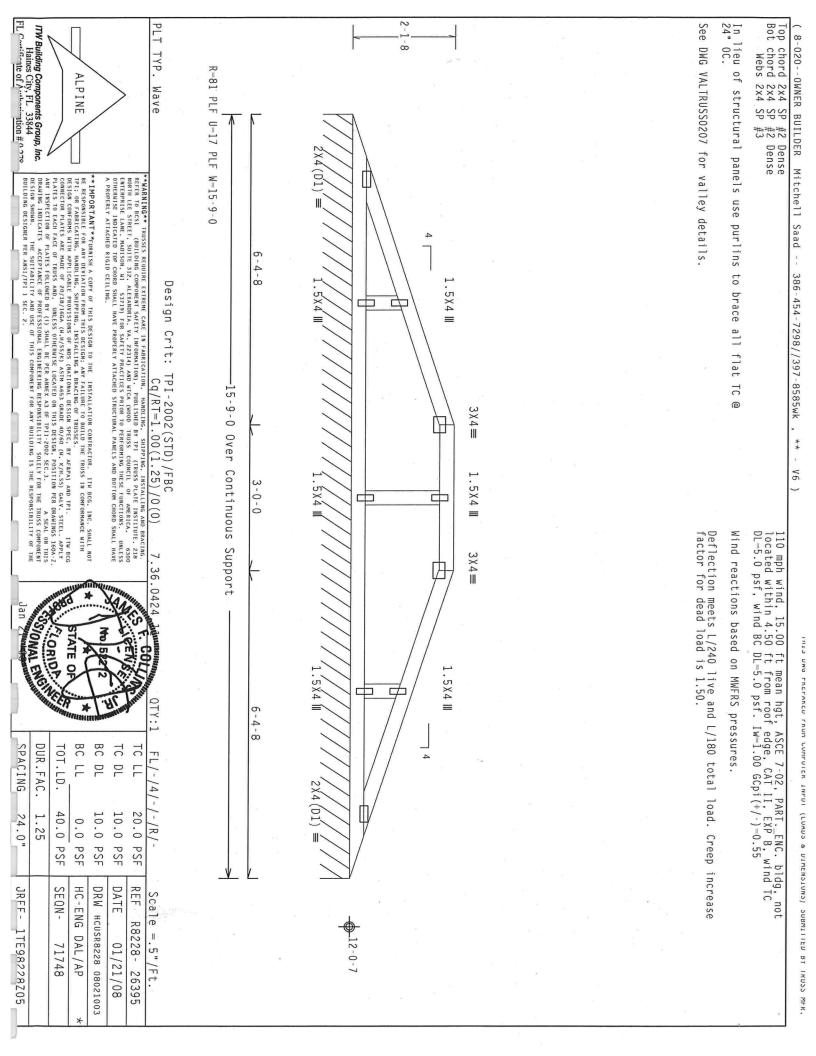
SPACING

24.0"

JRFF-

1TE98228Z05



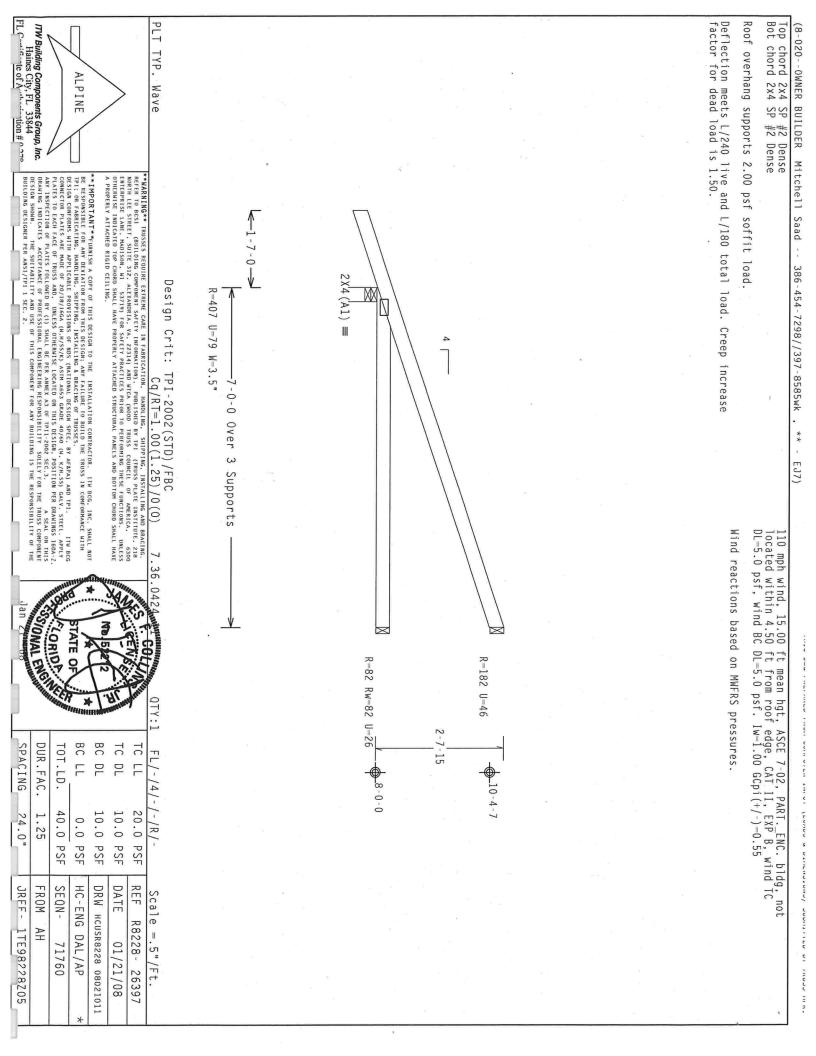


TW Building Components Group, Inc. Haines City, FL 33844 FL Continue of Authorization # 0 270 Top Bot In lieu of structural panels use purlins to brace all flat TC @ $24\mbox{\ensuremath{^{\circ}}}\xspace$ 0C. Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 1.50. 8-020--OWNER BUILDER Mitchell Saad -chord 2x4 SP #2 Dense chord 2x4 SP #2 Dense Webs 2x4 SP #3 TYP. R-81 PLF U-17 PLF W-11-9-0 ALPINE Wave 2-4-8 **IMPORTANT**GURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVLATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMFORMANCE WITH TPI; OR FARBICATING, HANDLING, SURPPING, INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (MATIONAL DESIGN SPEC, BY AFRA) AND TPI. CONNECTOR PLATES, ARE MADE OF 20/18/166A (M. 1/8/S/P) ASTM A653 GRADE 40/60 (M. K/M.SS) GALV. STEEL, APPLY PLATES TO EACH FACE OF TRUSS AND.

MINISPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2. *WARNING** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING, REFER TO BEST. (BUILDING COMPOMENT SAFETY IMFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WICA (MOOD TRUSS COUNCIL OF AMERICA, 6300 EMTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE DRAWING INDICATES PROPERLY ATTACHED RIGID CEILING 1.5 X 4 (**) Ⅲ 4 X 4 == 386-454-7298//397-8585wk . Design Crit: -11-9-0 Over Continuous Support 1.5X4(**) ■ TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0) -0-0 4 ** ٧7) 110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART_ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55 $\binom{**}{4}$ 4 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements. 5 X 4 (* * See DWG VALTRUSS0207 for valley details. Wind reactions based on MWFRS pressures. 4×4≡ 2X4(D1) =4 2-4-8 BC LL BC DL TC DL TC LL DUR.FAC. SPACING TOT.LD. FL/-/4/-/-/R/-40.0 24.0" 20.0 PSF 1.25 10.0 PSF 10.0 PSF 0.0 PSF PSF DATE REF JREF -SEQN-HC-ENG DRW HCUSR8228 08021004 Scale =.5"/Ft. R8228- 26396 1TE98228Z05 DAL/AP 71752 01/21/08

טובת זייו טו (בטחטם מ טוויבתסונתם) טעמויווויבט פו ומטפט וודת.



(8-020--OWNER BUILDER Mitchell Saad --386-454-7298//397-8585wk , * J5)

ווודא השה בערבעערה בעהנו רחנובהובע זועבה! לרחשה פ הזונבשפוחוגא) פתפעזוובה פו ואחפט עבצי

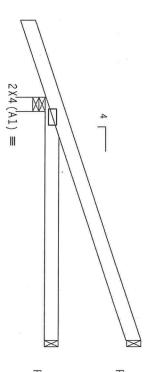
Top chord 2x4 SP Bot chord 2x4 SP #2 Dense #2 Dense

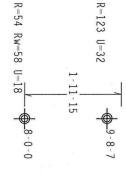
Roof overhang supports 2.00 psf soffit load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART. ENC. bldg, not located within 4.50 ft from roof edge, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures







Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00 (1.25) /0(0)

QTY:1

FL/-/4/-/-/R/-

Scale =.5"/Ft. R8228-

20.0

PSF

DATE REF

01/21/08 26398 PLT TYP.

Wave

WARNING RUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING.
REFER TO BESI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, Z18
MORTH LEE STREET, SUITE 31Z, ALEXANDRIA, VA, Z231J) AND HTCA (MODD TRUSS COUNCILS FOR AMERICA, 6300
ENTERPISE LONE, MOISON, HI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS
OTHERNISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE
A PROPERLY ATTACHED RIGID CELLING.

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR AWY DEVIATION FROM HIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORMANCE WITH IP: OR FABRICATING, HANDLING, SHIPPING, INSTALLING A BRACHIG OF TRUSSES, DESIGN CONTROLLING, SHEPPING, INSTALLING A BRACHIG OF TRUSSES, DESIGN CONTROLLING AND FULL REPOYLSIONS OF HIS CONTROLLING SPEC, BY AFRAY AND IPI. ITH BCG CONNECTION FURTHER ARE ASSOCIATED ON THIS DESIGN, POSITION OF REMAINES 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER MANEX, AS OF PDI-2002 SEC. 3. A SEAL ON THIS DRAHMS INDICATES ACCEPTANCE OF PROFESSIONAL REGISHER HANEX, AS OF PDI-2002 SEC. 3. A SEAL ON THIS DRAHMS INDICATES ACCEPTANCE OF PROFESSIONAL REGISHER HANEX, AS OF PDI-2002 SEC. 3. THE SUSTINGLIFY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE

Haines City, FL 33844
FL Continue of Ambanism tion # 0 270

DESIGN SHOWN. THE SUITABILITY AND USE OF BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

ALPINE

CORIDA TATE OF

> BC LL BC DL TC DL TC LL

0.0 PSF 10.0 PSF 10.0 PSF

DRW HCUSR8228 08021012

DAL/AP 71766

DUR.FAC. SPACING

FROM

AH

24.0" 1.25

JREF-

1TE98228Z05

TOT.LD.

40.0

PSF

SEQN-HC-ENG

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk , *

וחום טאש דאבדאאבט דאטח גטחדטובא ואיטו (בטאטס 6 טואבאסוטאס) סטטאווובט פי ואטסס ארא.

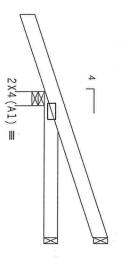
Top chord 2x4 SP Bot chord 2x4 SP #2 Dense #2 Dense

Roof overhang supports 2.00 psf soffit load

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,.$

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf. Iw=1.00 GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures



R=23 Rw=37 U= $\overline{13}$ + 8-0-0 R=58 U=16

R=265 U=58 W=3.5 3-0-0 Over 3 Supports

Design Crit: TPI-2002 (STD) /FBC Cq/RT=1.00(1.25)/0(0)

PLT

TYP.

Wave

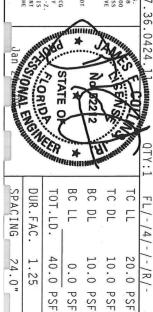
MARNING RUSSES REQUIRE EXTREME CARE IN FABRICATION. WANDLING. SHIPPING, INSTALLING AND BRACING. REFER TO BESS! (QUILDING COMPONENT SAFETY INFORMATION). PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 WORTH LEE STREET, SUITE 312, ALEXANDRIA, VA. Z2314) AND WICA (MODD TRUSS COUNCIL OF AMERICA, 6300 ENTERERISE LAME, MADISON, MI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TO PERFORMING THESE FUNCTIONS. UNLESS A PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE

IMPORTANTFURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITH BCG, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN COMPORNANCE WITH FP: OR FARBECTAING. HANDLING. SHEPPING. HISTALLING & BRACHING OF TRUSSES, BY AFRAD, AND TP: ITH BCG DESIGN CONFORMS HITH APPLICABLE PROVISIONS OF BUS (INATIONAL DESIGN SPEC, BY AFRAD, AND TP: CONNECTOR PLAIRS ARE MADE OF 20/18/18/GA. (HAITSKYK) ASTH AGES GRADE 40/50 (H. K/H.SS) GALY. STEEL. APPLY PLAIRS TO EACH FACE OF TRUSS AND. UNLESS OTHERMISE LOCATED ON HITS DESIGN. POSITION PER BRAHINGS 160A-2.

DRAWING INDICATES SUITABILITY AND USE OF THIS COMPONENT FOR ANSI/TPI 1 SEC. 2. TPI1-2002 SEC.3. A SEAL ON THIS BILLITY SOLELY FOR THE TRUSS COMPONENT BUILDING IS THE RESPONSIBILITY OF THE

Haines City, FL 33844
FL Carrier ne of A. A. L. Titon # 0 270

ALPINE



PSF

HC-ENG DAL/AP

DRW HCUSR8228 08021014

FROM SEQN-

JREF- 1TE98228Z05

PSF

REF

R8228- 26400 01/21/08

Scale = .5"/Ft.

DATE

(8-020--OWNER BUILDER Mitchell Saad -- 386-454-7298//397-8585wk .

has one incinned inch community through those a principality and incidential of those first.

Top chord 2x4 SP #2 Dense Bot chord 2x4 SP #2 Dense

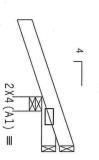
Roof overhang supports 2.00 psf soffit load

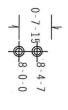
Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is $1.50\,\mathrm{cm}$

110 mph wind, 15.00 ft mean hgt, ASCE 7-02, PART._ENC. bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=5.0 psf, wind BC DL=5.0 psf, $I_{v}=1.00$ GCpi(+/-)=0.55

Wind reactions based on MWFRS pressures

R=-59 Rw=31 U=39





R=-21 Rw=21 U=19

1-0-0 Over 3 Supports

R=264 U=80 W=3.5"

Design Crit: TPI-2002(STD)/FBC Cq/RT=1.00(1.25)/0(0)

QTY:1

FL/-/4/-/-/R/-

Scale =.5"/Ft.

R8228- 26401

DATE REF

01/21/08

FROM JREF -

1TE98228Z05

SEQN-

71757

HC-ENG DAL/AP

DRW HCUSR8228 08021015

PLT TYP.

Wave

IMPORTANT*UNRHISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BCG, LHC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN. ANY FAILURE TO BUILD THE TRUSS IN COMFORMANCE WITH PP: OR FAREICALING. INSTALLING. INSTALLING & BRACING OF TRUSSES.

DESIGN CONFORMS HITH APPLICABLE PROVISIONS OF NOS (NATIONAL DESIGN SPEC, BY AERA) AND FPI. THE BCG CONNECTOR PLATES ARE MADE OF 20/18/166A (M.H/SS), VAIN ASS GRADE 40/50 (M.K.M.H.SS) GANV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS. AND. UNLESS OTHERWISE LOCATED ON THIS DESIGN POSITION PER DRAWNINGS. 160A—2. ANY INSPECTION OF PARTES FOLLOWED BY (1) SHALL BE DER NAWEA A3 OF TPIL-2002 SEC. 3.

DRAWLING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT

Haines City, FL 33844
FL Contents of Anthonymous Property of the Office of Anthonymous Property of the Propert

ALPINE



VALLEY TRUSS DETAIL

TOP CHORD BOT CHORD 2X4 SP #2 OR SPF #1/#2 OR BETTER. 2X3(*) OR 2X4 SP #2N OR SPF #1/#2 OR BETTER. 2X4 SP #3 OR BETTER.

- 2X3 MAY BE RIPPED FROM A 2X6 (PITCHED OR SQUARE).
- * ATTACH EACH VALLEY TO EVERY SUPPORTING TRUSS WITH: SBC 110 MPH, ASCE 7-93 110 MPH OR ASCE 7-98, ASCE 7-02 OR ASCE 7-05 130 MPH. 15' MEAN HEIGHT, ENCLOSED BUILDING, EXP. C, RESIDENTIAL, WIND TC DL=5 PSF (2) 16d BOX (0.135" X 3.5") NAILS TOE-NAILED FOR

UNLESS SPECIFIED ON ENGINEER'S SEALED DESIGN, APPLY 1X4 "T"-BRACE, 80% LENGTH OF WEB, VALLEY WEB, SAME SPECIES AND GRADE OR BETTER, ATTACHED WITH 8d BOX (0.113" X 2.5") NAILS AT 6" OC, OR CONTINUOUS LATERAL BRACING, EQUALLY SPACED, FOR VERTICAL VALLEY WEBS GREATER THAN 7'9".

MAXIMUM VALLEY VERTICAL HEIGHT MAY NOT EXCEED 12'0"

TOP CHORD OF TRUSS BENEATH VALLEY SET MUST BE BRACED WITH: PROPERLY ATTACHED, RATED SHEATHING APPLIED PRIOR TO VALLEY INSTALLATION

BY VALLEY TRUSSES USED IN LIEU OF PURLIN SPACING AS SPECIFIED PURLINS AT 24" OC OR AS OTHERWISE SPECIFIED ON ENGINEERS' SEALED DESIGN

ENGINEERS' SEALED DESIGN.

+ NOTE THAT THE PURLIN SPACING FOR BRACING THE TOP CHORD OF THE TRUSS BENEATH THE VALLEY IS MEASURED ALONG THE SLOPE OF THE TOP CHORD. LARGER SPANS MAY BE BUILT AS LONG AS THE VERTICAL HEIGHT DOES

CUT FROM 2X6 OR LARGER AS REQ'D

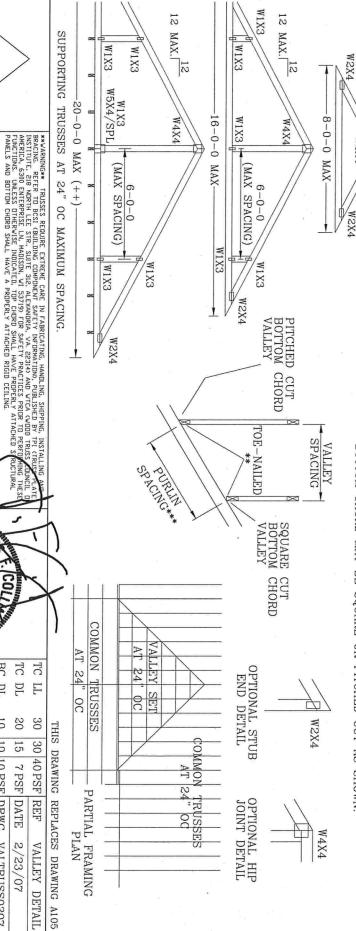
12 MAX.

W2X4

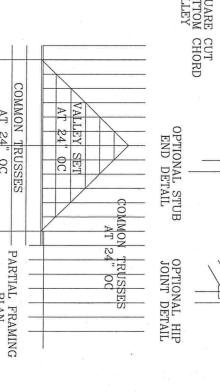
2

4-0-0

MAX







No. 529 STATE OF * BC DL BC LL TCTC SPACING DUR.FAC. 1.25/1.33 1.15 1.15 TOT. LD DL Π 60 10 20 30 0 10 15 55 57 PSF 10 PSF DRWG 40 PSF 0 PSF 7 PSF DATE -ENG

> MLH/KAR VALTRUSS0207 2/23/07

MEMBERANITAM FURNISH CORY DIT THIS DESIGN TO INSTALLATION CONTRACTOR. ITV BCG, IND NOT BE RESPONSIBLE FUR ANY DEVIATION FROM THIS DESIGN ANY FALLURE TO BUILD THE TRUCCHORDRAMACE WITH FIFTO FABRICATION, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUCE CONTORNAMIC WITH FIFTO FABRICATION, HANDLING, SHIPPING, INSTALLING & SPEC, BY AFREND DESIGN CONTORNS WITH APPLICABLE PROVISIONS OF NOS CHATIONAL DESIGN SPEC, BY AFREND FOR THE TRUCE WITH APPLICABLE DESIGN CHAIRD STANDARD STANDAR PER

ITW BUILDING COMPONENTS GROUP, INC. POMPANO BEACH, FLORIDA

ALPINE

SIONAL ENGINE





ENGINEERING & TESTING LABORATORY

P.O. Box 1625, Lake City, FL 32056-1625 4784 Rosselle St. • Jacksonville, FL 32254 2230 Greensboro Hwy., Quincy, FL 32351

Lake City • (386) 755-3633

Fax • (386) 752-5456

Jacksonville • (904) 381-8901

Fax • (904) 381-8902

Quincy • (850) 442-3495

Fax • (850) 442-4008

JOB NO .: 08 205 DATE TESTED: 04-08-0

REPORT OF IN-PLACE DENSITY TEST

#26816

ASTM METHOD: (D-2922) Nucl	ear	(D	-2937) Driv	e Cylinder		Other
PROJECT: MITCHELL SAAD						
CLIENT: MITCHELL SAAD		illy 3				
GENERAL CONTRACTOR: 5AC	EARTHW	ORK CON	ITRACTOR:	SAC	2	
SOIL USE (SEE NOTE):	SPECIFIC	ATION RI	EQUIREMEN	ITS:	95%	
TECHNICIAN: S. OSTEEN			*		- Ŧ.,	
MODIFIED (ASTM D-1557):	STANDAR	STANDARD (ASTM D-698):				
TEST TEST LOCATION	TEST:DEPTHELEVLIFT	PROCTOR NO.	WET DENS. LBS.CU.FT.	DRY DENS. LBS.CU.FT.	MOIST PERCENT	% MAX. DENS.
4. 12 FROM SE CORNER	12"		107.6		4.0	100.4
5 CENTER OF PORCH PAD	12	1	113.8	108.1	5.3	104.8
REMARKS:						
PROCTOR NO. SOIL DESCRIPTION			PROCTOF	R VALUE	OPT	MOIST.
1 RICHARDSON'S FORT WHITE PI	T		103.1		10.	0
NOTE: 1 Building Fill 2 Trench Backfill 3 Base Course 4 Subbase/St	abilized Subsec	do 6 Ember	nkmont 6 Cult	reado/Nistrasi (Coil 7 Oth-	

The test results presented in this report are specific only to the samples tested at the time of testing. The tests were performed in accordance with generally accepted methods and standards. Since material conditions can vary between test location and change with time, sound judgement should be exercised with regard to the use and interpretation of the data.



13 MAY 2008

JOHNNY KEARSE, BUILDING OFFICIAL COLUMBIA COUNTY, BUILDING DEPT. COLUMBIA COUNTY COURTHOUSE ANNEX LAKE CITY, FLORIDA 32055

RE: SAAD RESIDENCE PERMIT Nr.: \$268/6

DEAR SIR:

PLEASE BE ADVISED OF THE FOLLOWING CHANGE TO THE CONSTRUCTION DOCUMENTS FOR THE ABOVE REFERENCED PROJECT:

IN LIEU OF THE TRUSS ANCHORS SPECIFIED IN THE PLANS, IT IS PERMISSIBLE TO USE "SIMPSON" HIG ANCHORS OR OTHER "SIMPSON" ANCHORS, EQUAL OR EXCEEDING THE DESIGN LOADS AS INDICATED IN THE ENGINEERED TRUSS SHOP DRAWINGS.

SHOULD YOU HAVE ANY FURTHER QUESTIONS WITH THIS, PLEASE CALL FOR ASSISTANCE.

YOURS TRULY, NICHOLAS PAUL GEISLER, ARCHITECT AROOOTOOS

NOTICE OF TREATMENT

	Moral	1 Ser	vid
Applicator Name			
11001000	The state of the s		54 Suite
City			
Time	10:46	Date _	9-29-08
			26816
	SITE LO	CATION	
Lot #	Block #	Perr	nit # <u>0060 268 1</u> 0
Subdivision			
Address	349 51	w Thorn	LN
Name of Obomics	Pro va	- Ded	Used <u>-05</u> %
Area Treated	1288 9	ig ft	136 LN F+
	213 go		
Remarks	Soil Trea	tunt o	n Patio Pool
Addin	tion		

Permit File - Canary

Permit Holder - Pink

CAOL MA

Applicator - White

NOTICE OF TREATMENT

Applicator Name	M'c Cal	IL Se	rvice	
Address	415 NW	250th	57 5	svite!
City	Newbe	rry		
Time	7:48 /	m_ Date	44.8	-08
				0
	SITE LOC	CATION	*26	816
Lot #	Block #	P	ermit # 👓	0026816
Subdivision				
Address 349	9 SW Th	orne	Lane	
Name of Chemica	I AppliedP	emise.	Used	05%
Area Treated	oil Pretrea	t on	Patio	Addition
Gallons Used	50			
Remarks 68	34 sq ft	110 LN	A	
St	em Wall			
Mit	chell Saa	d		
	Autoria de la compania del compania del compania de la compania del compania de la compania de la compania del compania de la compania de la compania de la compania de la compania del compania del compania del compania del compania del la compania del		The second	

Applicator - White Permit File - Canary Permit Holder - Pink

MA HOAD